

# Drainage Narrative

**607 Calef Highway  
Barrington, NH  
Tax Map 238, Lot 44**

Prepared for

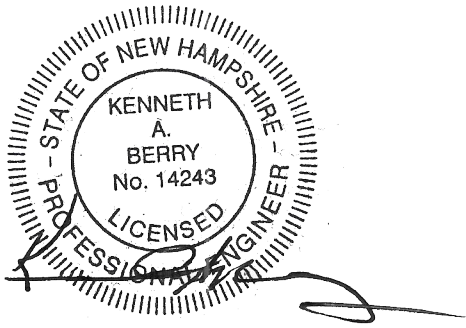
TURBOCAM, INC.  
607 Calef Highway Suite 200  
Barrington, NH 03825

Land of

Virtuous Realty, LLC  
607 Calef Highway Suite 200  
Barrington, NH 03825

Prepared By

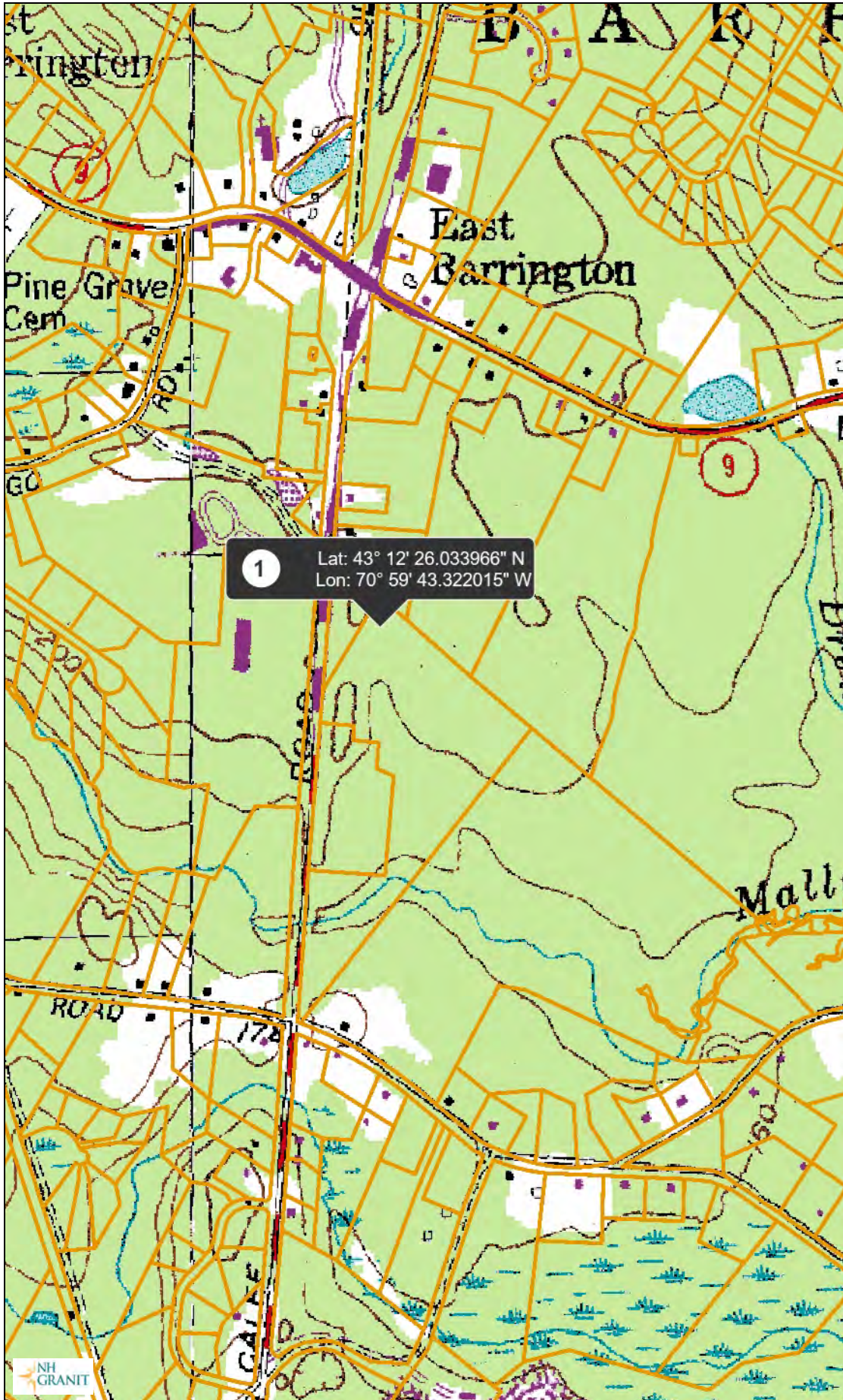
Berry Surveying & Engineering  
335 Second Crown Point Road  
Barrington, NH 03825  
603-332-2863



File Number  
DB2023-017

February 5, 2024  
Revised: April 17, 2024

# Map by NH GRANIT



## Legend

- Parcels
- State
- County
- City/Town

1 Lat: 43° 12' 26.033966" N  
Lon: 70° 59' 43.322015" W

Map Scale

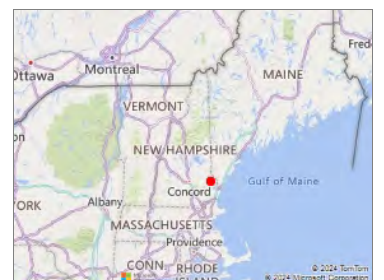
1: 10,000



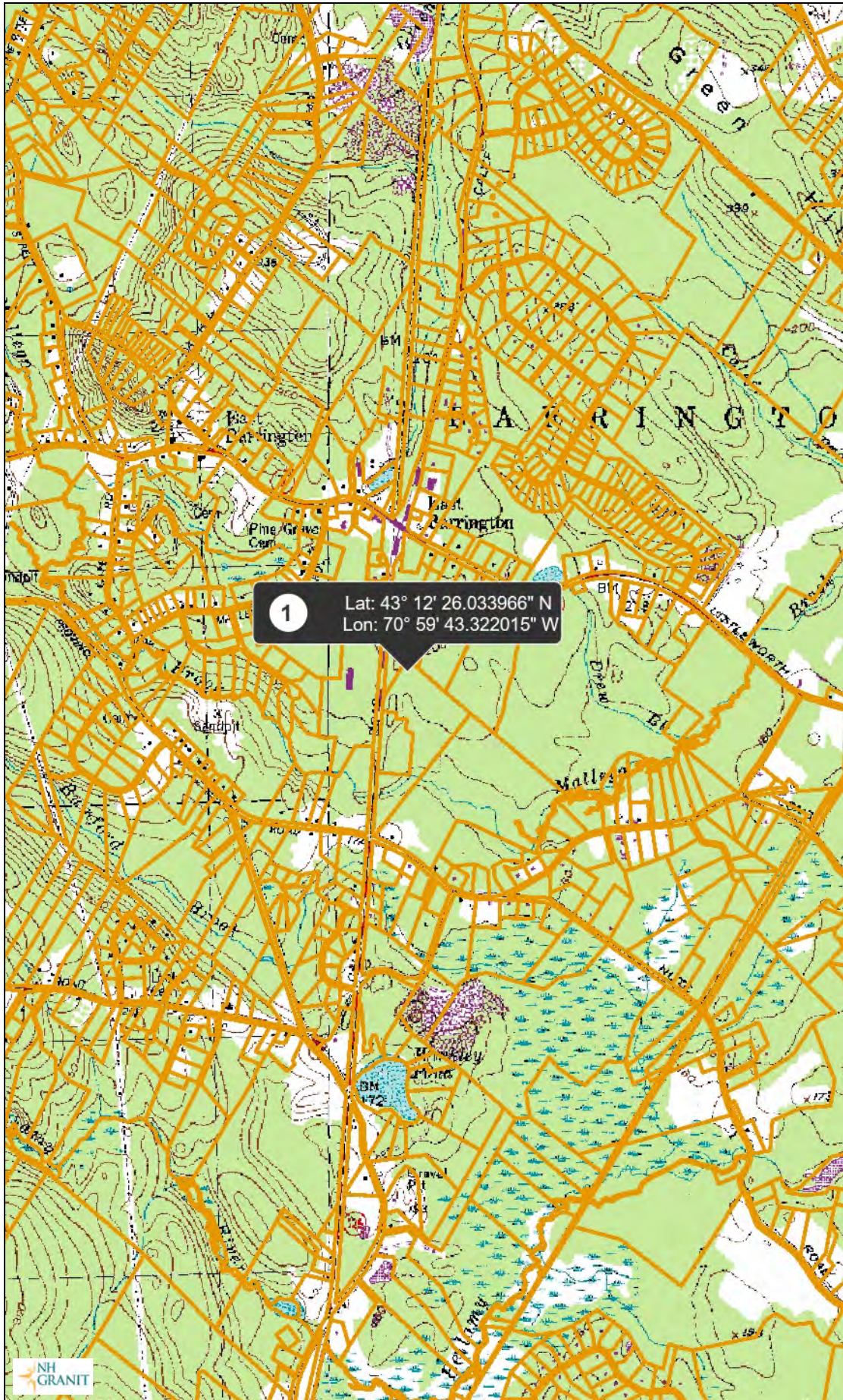
© NH GRANIT, [www.granit.unh.edu](http://www.granit.unh.edu)

Map Generated: 4/17/2024

## Notes



# Map by NH GRANIT



## Legend

- Parcels
- State
- County
- City/Town

Map Scale

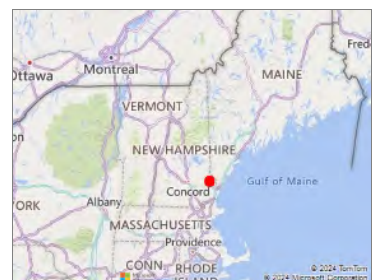
1: 24,000

© NH GRANIT, [www.granit.unh.edu](http://www.granit.unh.edu)

Map Generated: 4/17/2024



## Notes



## Table of Contents

USGS Quadrangle Location Map	
Design Method Objectives	Page 2
1.0 Existing Conditions Analysis	Page 3
2.0 Proposed Site Plan Analysis	Page 5
3.1 Full Comparative Analysis	Page 8
3.2 Swale Capacity Analysis	Page 8
4.0 Erosion and Sediment Control, BMP's	Page 9
5.0 Conclusion	Page 21

### Appendix I - Existing Conditions Analysis

- 25 Yr-24 Hr. Full Summary
- 2 Yr-24 Hr. Node Listing
- 10 Yr-24 Hr. Node Listing
- 25 Yr-24 Hr. Node Listing
- 50 Yr-24 Hr. Node Listing

### Appendix II - Proposed Conditions Analysis

- 25 Yr-24 Hr. Full Summary
- 2 Yr-24 Hr. Node Listing
- 10 Yr-24 Hr. Node Listing
- 25 Yr-24 Hr. Node Listing
- 50 Yr-24 Hr. Node Listing
- 50 YR-24-Hr. Swale Capacity Analysis

### Appendix III - Calculations, Charts, & Graphs

- Extreme Precipitation Tables
- Rip Rap Calculations
- AoT Stormwater Treatment Spreadsheets
- NCRS USDA Web-soil Map
- Site Specific Soil Survey Report & Plan
- Stormwater System Management: Inspection & Maintenance Manual, Plan, Invasive Species & NHDES
- Green SnoPro Utilization Chart
- Infiltration Feasibility Study & Report
- Ksat Values for New Hampshire Soils, SSSNNE Special Publication #5, 2009
- UNH Stormwater Center Hybrid Bioretention Template
- Filtrexx Specifications Sheets

Enclosed:      W-1 Sheet                      Existing Conditions Watershed Plan    4 Sheets  
                    W-2 Sheet                      Post Construction Watershed Plan    4 Sheets  
                    Erosion & Sediment Control Plan

## DESIGN METHOD OBJECTIVES

The owner / developer of Tax Map 238, Lot 44, TURBOCAM, INC. is proposing to develop the property at 607 Calef Highway. The site is currently vacant land. TURBOCAM, INC. is proposing two parking areas with a total of 113 parking spaces and outdoor recreation improvements.

An on-site topography survey was completed by field crews of Berry Surveying & Engineering in April of 2023 and a Site Specific Soil Survey was completed by John P. Hayes with a report generated on May 5, 2023. Soils on site are included in all four hydrologic soil groups: HSG A, HSG B, HSG C, and HSG D (No HSG D in analysis). (See attached report). A wetland delineation was completed as part of the existing conditions package. The off-site land which drains onto the locus parcel has been delineated by USDA / NRCS soils in Websoil and USGS Equivalent contours from public sources. (Google Tin & NH Lidar)

An Existing and Proposed Conditions analysis was conducted for the purpose of calculating the peak rate of stormwater run-off and to subsequently design adequate mitigation of drainage. There are three existing drainage discharge point which was identified in the existing analysis and duplicated in the proposed conditions analysis. This Discharge Point, or Point of Analysis, is considered the area contributing runoff to the westerly side of the wetlands, south of the existing Turbocam driveway, located on Lot 44-1 and Lot 44. Designing two watershed models we have compared the differences in these rates of peak run-off and surface water volume. Sheet W1, Existing Conditions Watershed Plan, outlines the characteristics of the site in its existing or pre-construction conditions. The second analysis displays the proposed (post-construction) conditions (See Sheet W2). HydroCAD uses a series of node suffixes for numbering purposes (S = Subcatchment, P = Pond Device, R = Reach), to simplify annotation these suffixes are left off the watershed plans and node type is denoted by the symbol shape according to the displayed legend which coincides with HydroCAD graphics. The analysis was conducted using data for; 2 Yr - 24 Hr (3.08"), 10 Yr - 24 Hr (4.65"), 25 Yr - 24 Hr (5.87"), 50 Yr - 24 Hr (7.02"), and 100 YR - 24 Hr (8.39") storm events. Storm event analysis was accomplished using the USDA SCS TR-20 method within the HydroCAD Stormwater Modeling System environment. Rainfall quantities are based on the Extreme Precipitation Table for this location from the Northeast Regional Climate Center / Cornell University (<http://precip.eas.cornell.edu>).

## 1.0 Existing Conditions Analysis:

Reference: Sheet W1 - Existing Conditions Watershed Plan (Enclosed)  
Existing Conditions Plan

The existing parcel is currently an operating light manufacturing facility. The analyzed soils within the locus parcel are made up of multiple soil types, containing Hydrologic Soil Group (HSG) A, B, C, & D. See Site Specific Soils Map and report for more information. The land cover types involved are grassed land, woods, roofs, gravel, and pavement. Off-site soils are likewise HSG A and based on USDA / NRCS Websoil.

The land area analyzed consists of 11.40 acres of the 27.94 acre parcel as well as offsite land. The total area of analysis for the Existing Conditions Analysis is 16.92 acres. The land analyzed is made up of nine subcatchments analyzed at three individual non-point final reaches. These reaches include the westerly sideline of the jurisdictional wetland, to the south of the existing Turbocam driveway, the southeastern property line, and the existing drainage practice in front of the Turbocam building. There is a discrepancy in the area between the two models, due to the addition of 0.006 Ac in subcatchment #70 from the grading of the swale line adjacent to the parking area.

Receiving Waters and Impairments: The Mallego Brook (NHRIV600030903-02) watershed will receive all of the runoff from the site directly. The impairment of the watershed are as follows:

- Mercury, NE Regional Mercury TMDL, December 20, 2008, TMDL #33883
- pH, Low Priority TMDL
- Dissolved Oxygen Saturation, Low Priority TMDL
- Oxygen, Dissolved, Low Priority TMDL

### **Final Reach #400**

**Subcatchment #72** is made up exclusively of offsite land along the eastern side of Calef Highway north of the locus parcel. The subcatchment extends back to the roof lines of the businesses along the highway, following the existing topography of the land. Runoff generally flows south down the side of Calef Highway and into an existing roadside depression (**Pond #72**) where it is infiltrated into the soil. Excess runoff flows over an offsite driveway and along a roadside swale (**Reach #72**) into a catch basin adjacent to the locus parcel (**Pond #71**) where it is directed onto the parcel through an outlet pipe and a swale (**Reaches #71a & #71b**).

**Subcatchment #71** encompasses offsite land spanning Calef Highway and extending up the driveway of the plaza across Calef Highway northwest of the parcel. This subcatchment encompasses a closed drainage system being analyzed as a single pond at the outlet of the final catch basin (**Pond #71**) before it directs runoff onto the parcel. This is done to analyze the runoff flowing onto the parcel and not to analyze the performance of the offsite drainage system.

**Subcatchment #70** consists of partially wooded land at the front of the parcel along Calef Highway. The subcatchment is defined by the crown of Calef Highway and the crown of the Turbocam driveway. Runoff generally flows east and southeast to a catch basin (**Pond #70**) which directs runoff through a driveway culvert to an existing detention pond being analyzed as **Final Reach #100**.

### **Final Reach #200**

**Subcatchment #2** is made up of land southwest of the Turbocam building extending from the crown of the main Turbocam driveway downhill through a recreation area and into undisturbed wooded land generally following the natural topography of the parcel. Runoff flows southwest to the eastern edge of the delineated wetland. The eastern wetland line is being evaluated as **Final Reach #200**.

### **Final Reach #300**

**Subcatchment #3** is land area beginning at the high point of the open field to the east of the existing Turbocam building extending east to the property line. Runoff flows generally in a southeasterly direction to the property line being analyzed at **Final Reach #300**, which flows offsite.

**Subcatchment #30** is made up of a large portion of mostly grassed land south of the Turbocam building. Runoff flows to the stone trench toward the middle of the subcatchment (**Pond #30**) where a portion of runoff is infiltrated before outletting to a catch basin (**Pond #E03**) which is part of the closed drainage system eventually draining to the perforated HDPE infiltration pipe and the final catch basin in the system (**Pond #E04**) draining to **Final Reach #300** through the overland reaches (**Reaches #34a-#34d**). In some cases, the stone trench may flood and contribute runoff to **Final Reach #200** through a series of overland reaches (**Reaches #30a-#30d**). Catch Basin #3 (**Pond #E03**) also overflows in some cases contributing runoff to **Final Reach #300** through a separate series of overland reaches (**Reaches #33a & #34b-#34d**).

**Subcatchment #31** consists of a small portion of mostly paved parking area at the southeastern corner of the Turbocam building. Runoff flows in a northeast direction to a catch basin (**Pond #E01**) which is the first in a closed drainage system eventually draining to a perforated HDPE pipe which infiltrates a portion of runoff into the soils. Runoff that is not infiltrated will eventually exceed the storage of the system overflowing the final catch basin in the system (**Pond #E04**) and drain to **Final Reach #300** through a series of overland reaches (**Reaches #34a-#34d**).

**Subcatchment #32** encompasses a portion of mostly paved parking area at the southern corner of the Turbocam building. Runoff flows in a southeast direction to a catch basin (**Pond #E02**) which is part of the closed drainage system eventually draining to the perforated HDPE infiltration pipe and the final catch basin in the system

(**Pond #E04**) draining to **Final Reach #300** through the overland reaches (**Reaches #34a-#34d**).

**Subcatchment #34** is a small area of land surrounding a catch basin (Pond #E04) which collects runoff as part of the closed drainage system eventually draining to the perforated HDPE infiltration pipe and the final catch basin in the system (**Pond #E04**) draining to **Final Reach #300** through the overland reaches (**Reaches #34a-#34d**).

## 2.0 Proposed Conditions Analysis:

Reference: Sheet W2 - Proposed Conditions Watershed Plan (Enclosed)  
Proposed Grading & Drainage Plan

The applicant is proposing to improve the parcel with multiple parking areas and an outdoor function area. The proposal is supported by two Bioretention w/ ISR Systems, an Infiltration Basin, a Detention Pond, and a closed drainage system of catch basins and drain manholes to direct runoff to the practices.

### Final Reach #400

**Subcatchments #72 & #71** are exclusively offsite and remain unchanged.

**Subcatchment #50** consists of a small portion of partially offsite land between the parcel and Calef Highway. The purpose of this subcatchment is the proposed inlet sump (**Pond #C50**) that intercepts the offsite runoff from **Subcatchments #72 & #71** and directs it to the existing detention and fire pond being analyzed as **Final Reach #400**.

**Subcatchment #70** is moderately decreased in size due to the construction of the inlet sump (**Pond #C50**). Land area in this subcatchment includes wooded, grassed, and paved areas. Runoff generally flows south and southeast to **Bioretention Pond w/ISR #201** directed to **Final Reach #400** through Drain Manhole #51 (**Pond #D51**) which is an upgraded catch basin structure (formerly **Pond #70**).

### Final Reach #200

**Subcatchment #2** is mostly unchanged in area from existing to proposed. Runoff flows southwest to the eastern edge of the delineated wetland. This wetland line is being evaluated as **Final Reach #200**.

### Final Reach #300

**Subcatchment #3** is greatly reduced in size due to the proposed development of the parcel and the construction of two drainage practices near the property line. The remaining area of **Subcatchment #3** consists of onsite, largely undisturbed land. Runoff still flows to the property line analyzed as **Final Reach #300**.



**Subcatchment #4** is made up of the remaining portion of the existing **Subcatchment #3** which crosses the northeast property line consisting of largely offsite land. Runoff still flows generally toward the southeast property line being analyzed as **Final Reach #300**.

**Subcatchments #43 & #44** encompass the majority of the western portion of the proposed paved parking area including the grassed island in the middle of the parking area. Runoff from these subcatchments flows southeast to their respective catch basins (**Ponds #C43 & #C44**) and into the closed drainage system that outlets to **Infiltration Pond #203** through **Bioretention Pond w/ISR #202**.

**Subcatchments #45 & #46** consist of the eastern portion of the proposed paved parking area extending north to the rear of the existing storage building and east to the curbed edge of the pavement and the roof line of two proposed storage sheds. Runoff from these subcatchments flows southeast to the respective catch basins (**Ponds #C45 & #C46**) and into the closed drainage system directed to **Infiltration Pond #203** through **Bioretention Pond w/ISR #202**.

**Subcatchment #47** includes a small portion of pavement at the southernmost edge of the paved parking area. Runoff flows generally south to a catch basin at the low point of the paved parking (**Pond #C47**) where it enters the closed drainage system combining with runoff from **Ponds #C44 & #C46** and is directed to **Infiltration Pond #203** through **Bioretention Pond w/ISR #202**.

**Subcatchment #62** is made up of the land area contributing runoff directly to **Bioretention Pond w/ISR #202**. The limits of this subcatchment extend north to the edge of the proposed paved parking area and west to the high point of the existing grassed field south of the Turbocam building. Runoff is treated in the ISR of the practice and outlets through an outlet structure and an emergency spillway to **Infiltration Pond #203** where it is infiltrated into the soil.

**Subcatchment #63** encompasses the land area contributing runoff directly to **Infiltration Pond #203**. The limits of the subcatchment extend west from the drainage practice to the high point of the field. Runoff flows east into the practice where it is infiltrated into the soil with any excess runoff flowing to **Final Reach #300** through an emergency spillway.

**Subcatchment #30** is slightly reduced in size due to the construction of the outdoor function area and the related gabion basket seating. Runoff still flows inward to the infiltration trench (**Pond #30**) near the middle of the subcatchment where it infiltrates and excess runoff flows to a six-inch drain pipe which outlets into the closed drainage system through a catch basin structure (**Pond #C42**) and subsequently proposed Drain Manhole #52 (**Pond #D52**) which is an upgraded catch basin structure (formerly **Pond #E04**). The closed drainage system directs runoff through a series of catch basins and drain manholes to **Detention Pond #204** and subsequently **Final Reach**

**#300**. In some cases, excess runoff may also flood the trench and contribute to **Final Reach #200** through a series of overland reaches (**Reaches #30a-#30c**). The conveyance swale around the proposed EDA for lot 44-1 causes both the Tc. and the overflow reach to be longer in the proposed conditions than in the existing conditions.

**Subcatchment #41** is made up of the outer edge of the existing parking area and driveway along the southern end of the existing Turbocam building and extends northeast along the edge of pavement around the southeast corner of the building. Runoff flows generally northeast to a catch basin (**Pond #C41**) where it enters the closed drainage system which eventually outlets to Detention Pond #204 (**Pond #204**).

**Subcatchments #31 & #32** are both undisturbed subcatchments. Runoff in each subcatchment still flows to each catch basin (**Ponds #E01 & #E02** respectively) and into the closed drainage system flowing to **Detention Pond #204**.

**Subcatchment #64** consists of the land area directly contributing runoff to **Detention Pond #204**. This area is mostly the disturbed grassed land in and around the pond with the exception of a small portion of wooded land. Runoff is directed to **Final Reach #300** through an outlet structure and an emergency spillway, both mitigated by a stone level spreader.

### **3.0a Stormwater Treatment:**

Treatment takes place within the two Bioretention W/ ISRs designed to support the development on site. Pre-treatment will be provided in the sediment forebays of Bioretention W/ ISR #201 & #202. The water quality volume is treated within provided treatment area of the practices.

### **3.0b Stormwater Infiltration:**

Groundwater recharge volume requirements are satisfied by Infiltration Basin #203 (Pond #203) (Sheet P-203). See Infiltration Feasibility Study also prepared by Berry Surveying & Engineering and published on the same day.

### 3.1 FULL COMPARATIVE ANALYSIS

ANALYSIS      COMPONENT: PEAK RATE DISCHARGE (Cubic Feet / Second)

		2 Yr	10 Yr	25 Yr	50 Yr
Final Reach #200	Existing	0.78	3.31	5.85	8.53
	Proposed	0.77	3.28	5.80	8.45
Final Reach #300	Existing	2.33	4.69	6.68	9.01
	Proposed	1.55	3.49	4.82	6.43
Final Reach #400	Existing	0.40	2.18	4.22	6.45
	Proposed	0.36	1.84	4.05	6.44

ANALYSIS                      COMPONENT: VOLUME (Acre Feet)

		2 Yr	10 Yr	25 Yr	50 Yr
Final Reach #200	Existing	0.123	0.362	0.598	0.857
	Proposed	0.124	0.365	0.602	0.852
Final Reach #300	Existing	0.276	0.696	1.117	1.561
	Proposed	0.217	0.599	1.034	1.461
Final Reach #400	Existing	0.095	0.373	0.663	0.978
	Proposed	0.089	0.367	0.668	0.991

### 3.2 SWALE CAPACITY ANALYSIS

ANALYSIS                      COMPONENT: PEAK RATE DISCHARGE (Cubic Feet / Second)

50YR 24-HR Storm Event Used	Area (Ac.)	Swale Depth (ft.)	Bottom Width (Ft.)	Lt. Slope (X:1)	Rt. Slope (X:1)	Peak Rate (CFS)	50Yr Avg. Depth (Ft.)	Manning's "n"
Reach #70a	0.219	2	2	4	3	0.51	0.11	0.022

#### **4.0 EROSION and SEDIMENT CONTROL PLAN & BEST MANAGEMENT PRACTICES (BMP's)**

Reference: Proposed Site Plan and Grading Plan  
Erosion & Sediment Control Plan  
Erosion & Sediment Control Details

The proposed site development is protected from erosion and the abutting easements and properties are protected from sediment by the use of Best Management Practices as outlined in the New Hampshire Stormwater Manual, Volume 2, Post-Construction Best Management Practices Selection & Design (December 2008, NHDES & US EPA). Any area disturbed by construction will be temporarily or permanently re-stabilized within 30 days and abutting easements and properties will not be adversely affected by this development. All swales and drainage structures will be constructed and stabilized prior to having run-off directed to them. Reference is also made to the Stormwater System Management: Inspection & Maintenance Manual and Stormwater Operations, Inspection & Maintenance Plan which has been developed specifically for this project and available to the owner.

#### **Perimeter Control (Silt Fence / SiltSoxx / Erosion Control Mix Berm)**

The plan set demonstrates the location of perimeter sediment control. The Erosion and Sediment Control Details, Sheet E-101, has the specifications for installation and maintenance of the silt fence, Filtrexx mulch filled SiltSoxx (or approved equal), and Erosion Control Mix Berm. There are locations on the site, for example bio-media rain garden protection, where SiltSoxx protection is specified. An area of permanent perimeter control is shown by the well house for wetland buffer protection from steeper slopes.

#### **Catch Basins (Without Sumps) & Drain Manholes**

Description: Catch Basins are used throughout the site to capture and, along with culvert pipes and manhole, route surface water runoff to stormwater treatment and detention infrastructure. During construction the catch basins will be protected by inlet protection per the approved construction plans. The practice of street sweeping on a bi-annual basis will help reduce maintenance of these catch basins and culvert pipes.

Note: Deep sump catch basins are not allowed to be used on this proposed development due to wildlife concerns and any manufacturer sump resulting in a catch basin must be filled with washed crushed stone. Sediment should be trapped in the sediment forebays but is also a concern in earlier structures. See construction details for specifications of these conveyance practices.

Maintenance Considerations: Sediment must be removed from Catch Basins and Manholes on a regular basis, at least twice a year and more often if post-winter maintenance and street sweeping is not conducted. Inspections should be conducted

periodically. At a minimum they should be cleaned after snow-melt and after leaf-drop. Disposal of all material, sediment, and debris must be done in accordance with state and federal regulations. Culvert pipes will be inspected to ensure that surface water runoff is capable of leaving the structures. Drain manholes will be inspected to make sure there is not sediment build-up or blockages.

### **Conveyance Swale**

Description: Conveyance swales are stabilized channels designed to convey runoff at non-erosive velocities. They may be stabilized using vegetation, riprap, or a combination, or with an alternative lining designed to accommodate design flows while protecting the integrity of the sides and bottom of the channel. Conveyance channels may provide incidental water quality benefits, but are not specifically designed to provide treatment. Conveyance swales are not considered a Treatment or Pretreatment Practice under the AoT regulations, unless they are also designed to meet the requirements of an acceptable Treatment/Pretreatment Practice as described elsewhere in this Chapter. See SWM Volume 2, 4-6.3 Conveyance Practices, Conveyance Swale, page 166.

Maintenance Considerations: Grassed channels should be inspected periodically (at least annually) for sediment accumulation, erosion, and condition of surface lining (vegetation or riprap). Repairs, including stone or vegetation replacement, should be made based on this inspection. Remove sediment and debris annually, or more frequently as warranted by inspection. Mow vegetated channels based on frequency specified by design. Mowing at least once per year is required to control establishment of woody vegetation. It is recommended to cut grass no shorter than 4 inches.

### **Sediment Forebay**

Description: A sediment forebay is an impoundment, basin, or other storage structure designed to dissipate the energy of incoming runoff and allow for initial settling of coarse sediments. Forebays are used for pretreatment of runoff prior to discharge into the primary water quality treatment BMP. In some cases, forebays may be constructed as separate structures but often, they are integrated into the design of larger stormwater management structures. See SWM Volume 2, 4-4.1 Pre-treatment Practices, Sediment Forebay, page 140.

Maintenance Considerations: Forebays help reduce the sediment load to downstream BMPs, and will therefore require more frequent cleaning. Inspect at least annually; Conduct periodic mowing of embankments (generally two times per year) to control growth of woody vegetation on embankments; Remove debris from outlet structures at least once annually; Remove and dispose of accumulated sediment based on inspection; Install and maintain a staff gage or other measuring device, to indicate depth of sediment accumulation and level at which clean-out is required. Preserving the drainage between the Sediment Forebay and the stormwater BMP by inspecting and maintaining the connecting drainage pipes and perforations should be completed semi annually or as required to ensure the forebay is dry.

## **Bioretention W/ Internal Storage Reservoir (ISR)**

Description: A practice that provides temporary storage of runoff for filtering through an engineered soil media, augmented for enhanced phosphorus removal, followed by detention and denitrification in a subsurface internal storage reservoir (ISR) comprised of gravel. Runoff flows are routed through filter media and directed to the underlying ISR via an impermeable membrane for temporary storage. An elevated outlet control at the top of the ISR is designed to provide a retention time of at least 24 hours in the system to allow for sufficient time for denitrification and nitrogen reduction to occur prior to discharge. The design storage capacity for using the cumulative performance curves is comprised of void spaces in the filter media, temporary ponding at the surface of the practice and the void spaces in the gravel ISR. The volume of the ISR will exceed 26% of the Water Quality Volume (WQV). Reference: 2017 NH Small MS4 General Permit, Appendix F Attachment 3, and UNH Stormwater Center, "UNH Stormwater Center Hybrid Bioretention Template" (2020). *UNH Stormwater Center*. 73. <https://scholars.unh.edu/stormwater/73>

Maintenance Considerations: The outlet to the Internal Storage Reservoir consists of a 1.25" or 1.5" orifice in a threaded end-cap after the goose-neck pipe within the concrete outlet structure. The inlet manifold and threaded pipe outlet manifold system is designed so that the ISR, or anaerobic reservoir can be completely drained and the sump of the outlet structure pumped dry. The orifice requires periodic inspection, initially on a semi-annual basis. This time increment may need to be adjusted based on the experience on the maintenance of the device. The draining of the ISR would only be accomplished if issues developed.

The enhanced bio-media will require additional material rototilled into the top 10-inches to foot of the rain garden after a period of approximately 20 years. The timing of this maintenance period is a factor of the methodology applied during construction and will need to be evaluated as the rain gardens age.

Rain Gardens should be inspected at least twice annually and following any rainfall event exceeding 2.5 inches in a twenty-four hour period. Maintenance rehabilitation will be conducted as warranted by each inspection. Trash and debris will be removed at each inspection.

On an annual basis the infiltration capabilities need to be confirmed by evaluation the drawdown time. If the bioretention system does not drain within 72-hours following a rainfall event, a qualified professional will assess the condition of the rain garden to determine measures required to restore the infiltration function. This is normally the direct result of sediment accumulation which will be removed to restore the filter media ratio.

Proposed side slopes of 2:1 will be maintained with a weedwhacker, with vegetation being removed from the BMP with each maintenance application.

## **Detention Basins**

Description: A detention basin is an impoundment designed to temporarily store runoff and release it at a controlled rate, reducing the intensity of peak flows during storm events. Conventional detention basins are typically designed to control peak runoff rates under a range of storm conditions, and can be used to control discharges as required under the AoT Regulations and other requirements, including, but not necessarily limited to: Storage and peak rate control to meet Channel Protection Requirements (see Section 2-17); Storage and peak rate control to meet Peak Runoff Control Requirements (see Section 2-18) (10-year and 50-year frequency, 24-hour storm events); Storage and peak rate control to prevent flood impacts within the 100-year flood plain; Storage and peak rate control to meet other regulatory requirements, including local permitting standards.

Detention basins may consist of surface basins (pond-type structures) or subsurface basins (enclosed structures located below ground. Surface basins should be designed with an emergency spillway or bypass meeting applicable dam safety standards (Env-Wr 100 - 700: Dam Safety Rules). Subsurface basins should also be designed to safely bypass flows exceeding the engineered capacity of the structure. Detention basins may be combined with treatment BMPs discussed in this guidance document, to provide for other stormwater management objectives. For example, a stormwater pond may be designed to provide treatment as well as detention. However, a detention basin is not by itself considered a "Treatment Practice" under the AoT Regulations. See SWM Volume 2, 4-6.1 Conveyance Practices, Detention Basins, page 156.

Maintenance Considerations: The bottoms, interior and exterior side slopes, and crest of earthen detention basins should be mowed, and the vegetation maintained in healthy condition, as appropriate to the function of the facility and type of vegetation. Vegetated embankments that serve as "berms" or "dams" that impound water should be mowed at least once annually to prevent the establishment of woody vegetation.

## **In-Ground Infiltration Basin**

Description: Infiltration basins are impoundments designed to temporarily store runoff, allowing all or a portion of the water to infiltrate into the ground. An infiltration basin is designed to completely drain between storm events. An infiltration basin is specifically designed to retain and infiltrate the entire Water Quality Volume. Some infiltration basins may infiltrate additional volumes during larger storm events, but many will be designed to release stormwater exceeding the water quality volume from the larger storms. In a properly sited and designed infiltration basin, water quality treatment is provided by runoff pollutants binding to soil particles beneath the basin as water percolates into the subsurface. Biological and chemical processes occurring in the soil also contribute to the breakdown of pollutants. Infiltrated water is used by plants to support growth or it is recharged to the underlying groundwater. As with all impoundment BMPs, surface infiltration basins should be designed with an outlet structure to pass peak flows during a range of storm events, as well as with an

emergency spillway to pass peak flows around the embankment during extreme storm events that exceed the combined infiltration capacity and outlet structure capacity of the facility. See SWM Volume 2, 4-3.3b, Treatment Practices, In-Ground Infiltration Basin, page 88.

Maintenance Considerations: Removal of debris from inlet and outlet structures. Removal of accumulated sediment. Inspection and repair of outlet structures and appurtenances. Inspection of infiltration components at least twice annually, and following any rainfall event exceeding 2.5 inches in a 24 hour period, with maintenance or rehabilitation conducted as warranted by such inspection. Inspection of pretreatment measures at least twice annually, and removal of accumulated sediment as warranted by inspection, but no less than once annually. If an infiltration system does not drain within 72-hours following a rainfall event, then a qualified professional should assess the condition of the facility to determine measures required to restore infiltration function, including but not limited to removal of accumulated sediments or reconstruction of the infiltration trench.

### **Stone Berm Level Spreader**

Description: A stone berm level spreader is an outlet structure constructed at zero percent grade across a slope used to convert concentrated flow to "sheet flow." It disperses or "spreads" flow thinly over a receiving area, usually consisting of undisturbed, vegetated ground. The conversion of concentrated flow to shallow, sheet flow allows runoff to be discharged at non-erosive velocities onto natural ground. To stabilize the spreader outlet, a stone berm is provided to dissipate flow energy, and help disperse flows along the length of the spreader. Level spreaders are not designed to remove pollutants from stormwater; however, some suspended sediment and associated phosphorous, nitrogen, metals and hydrocarbons will settle out of the runoff through settlement, filtration, infiltration, absorption, decomposition and volatilization. See SWM Volume 2, 4-6.6 Conveyance Practices, Stone Berm Level Spreader, page 162.

Maintenance Considerations: Inspect at least once annually for accumulation of sediment and debris and for signs of erosion within approach channel, spreader channel or down-slope of the spreader. Remove debris whenever observed during inspection. Remove sediment when accumulation exceeds 25% of spreader channel depth. Mow as required by landscaping design. At a minimum, mow annually to control woody vegetation within the spreader. Snow should not be stored within or down-slope of the level spreader or its approach channel. Repair any erosion and re-grade or replace stone berm material, as warranted by inspection. Reconstruct the spreader if down-slope channelization indicates that the spreader is not level or that discharge has become concentrated, and corrections cannot be made through minor re-grading.



## **Stockpiled Sediment or Soil**

Stockpiled materials including topsoil, excavated materials, borrow materials imported onto the site, construction aggregates, and sediment removed from temporary sediment traps will be located in designated areas at least 50 feet away from concentrated flows. All stockpiles will have erosion protection in the form of silt fence and diversion swales will be applied to protect the material and surrounding areas. Inactive stockpiles will be seeded for temporary stabilization. Erosion control measures will be inspected in accordance with the schedule for all other activities on site.

At a minimum, you must comply with following (EPA 2012 CGP Part 2.1.2.4d) "Do not hose down or sweep soil or sediment accumulated on pavement or other impervious surfaces into any stormwater conveyance (unless connected to a sediment basin, sediment trap, or similar effective control,) storm drain inlet, or surface water."

## **Dewatering Practices**

Dewatering practices are not known to be required on this site. If during construction this becomes required, an addendum will be published specific for the requirements. As a general rule, ground water that needs to be removed from an excavation will be pumped to a sediment basin or a storm drain inlet prior to discharge from the site.

At a minimum, you must comply with following (EPA CGP Part 2.1.3.4) "With backwash water, either haul it away for disposal or return it to the beginning of the treatment process; and replace and clean the filter media used in dewatering devices when the pressure differential equals or exceeds the manufacturer's specifications."

Regarding dewatering practices in the State of New Hampshire, specifically see Construction General Permit Section 9.1.2 NHR12000 State of New Hampshire and "Clarification of Section 9.1.2 ... and other New Hampshire specific information for the U.S. EPA 2012 NPDES Construction General Permit (CGP), May 3, 2012"

Please be advised that should dewatering become required, the EPA CGP 2022 requires daily inspections, monitoring, and reporting quarterly to the agency.

### Stabilization for Long Term Cover

#### Vegetated Stabilization – Original Planting

All areas that are disturbed during construction will be stabilized with vegetated material within 30 days of breaking ground. Construction will be managed in such a manner that erosion is prevented and that no abutter’s property will be subjected to any siltation, unless otherwise permitted. All areas to be planted with grass for long-term cover will follow the specification and on Sheet E-102 using seeding mixture C, as follows:

Mixture	Pounds per Acre	Pounds per 1,000 Sq. Ft.
Tall Fescue	24	0.55
Creeping Red Fescue	24	0.55
<b>Total</b>	<b>48</b>	<b>1.10</b>

#### Conservation Mix

Virginia Wild Rye	Native	FACW-
Little Bluestem	Native	FACU
Big Bluestem	Native	FAC
Red Fescue	Native	FACU
Switch Grass	Native	FAC
Partridge Pea	Native	FACU
Showy Tick Trefoil	Native	FAC
Butterfly Milkweed	Native	NI
Beggar Ticks	Native	FACW
Purple Joe Pye Weed	Native	FAC
Black Eyed Susan	Native	FACU-
<b>Total</b>	<b>25</b>	<b>0.57</b>

Conservation Mix to be provided by New England Wetland Plants, Inc., Amherst, MA as outline in their New England Conservation / Wildlife Mix or approved equal. Mix to be applied at a rate of 25 lbs. per acre or one-lb. per 1750 square feet. Ratio of seed is proprietary and substitutions are not allowed.

Conservation Mix will used to stabilize all 2:1 slopes and all land area disturbed within the wetland buffer.

#### Stormwater BMP Mix:

The grass that is planted within a stormwater BMP will be a mix designed for both inundation and dry conditions such as Ernst Seeds, Retention Basin Floor Mix ERNMX-126.

Maintenance Considerations: Permanent seeded areas for long-term cover will be inspected on a periodic basis looking for signs of growth loss or erosion. Any areas found to be damaged will be repaired and replanted to reestablish the growth. The grass should be mowed at least twice per year and any dead material removed. Any woody growth that becomes established will need to be cut and removed.

Long-term maintenance of the land cover is critical and must be maintained at least 85% grass / vegetation coverage, must be inspected for concentrated flow, rills, and channels; and must be repaired as necessary to prevent erosion.

### **Rolled Erosion Control Blanket**

Description: Rolled Erosion Control Blankets, such as American Excelsior Company Curlex III, (or equal), North American Green BioNet series, consist of interlocking fiber mesh which is bio-degradable, used to stabilize sloping earth while vegetation is being established. The product comes in rolls that are laid out over the earth, normally overlapped, and secured to the soil by the use of anchors or staples. The RECB may be anchored in the earth at the top of the slope to prevent wash-out. Construction specifications are included in the plan set and New Hampshire Stormwater Manual, Volume 3, 4-1 Erosion Control Practices, Temporary Erosion Control Blanket. See the chart on E-102 for compatible products with given slopes.

Construction Considerations: It is recommended that the blanket be installed in the same direction as the water flow or perpendicular to the slope. The manufacturer will recommend the amount of over-lap from one row to the next and on longer slopes between sections. Care must be taken that the RECB is laid directly on the earth / topsoil and that any existing vegetation not cause tenting as this will cause an issue with the blanket not staying in place. The staples or stakes are to be placed according to the manufacturer based on the slope of the receiving soil and forces that may be encountered. Care must be taken to utilize the correct product as specified. The choice of product are all different and in most cases are not interchangeable. NHDES or NH F&G may specify that some RECBs not be used in some applications.

Maintenance Considerations: RECBs will be inspected during the regular inspection schedule and any construction corrections made if the blanket is compromised.

### **Inlet Protection / Storm Drain Inlets**

Storm drain inlet protection will be installed per the Erosion & Sediment Control Details as a sediment barrier installed around a storm drain inlet, catch basin, or curb inlet to reduce sediment intrusion into a system after it has been constructed and existing catch basins. These are to be constructed in accordance with the Erosion & Sediment Control Details, Sheet E-101 and maintained after every rain event.

At a minimum, you must comply with following (EPA CGP Part 2.1.2.9.b) "Clean, or remove and replace, the protection measures as sediment accumulates, the filter

becomes clogged, and/or performance is compromised. Where there is evidence of sediment accumulation adjacent to the inlet protection measure, you must remove the deposited sediment by the end of the same work day in which it is found or by the end of the following work day if removal by the same work day is not feasible.”

### **Stabilized Construction Entrance**

Description: A temporary gravel construction entrance provides an area where mud can be dislodged from tires before the vehicle leaves the construction site to reduce the amount of mud and sediment transported onto paved municipal and state roads. The stone size for the pad should be 3” angular aggregate, and the pad itself constructed to a minimum length of 75’ for the full width of the access road. The aggregate should be placed at least six inches thick applied over a non-woven engineered fabric such as Mirafi 140N. A plan view and profile are shown on Sheet E-101- Erosion & Sediment Control Detail Plan.

Maintenance Considerations: The stone must be refreshed and kept clean in order for the practice to prevent tracking on the abutting roadway. If vehicle traffic by-pass the practice, it should either be channelized or the practice widened to be properly utilized. Tracking on the abutting roadway is not allowed and materials that are deposited on the abutting highway or any internal roadway must be swept daily.

### **Environmental Dust Control**

Dust will be controlled on the site by the use of multiple Best Management Practices. Mulching and temporary seeding will be the first line of protection to be utilized where problems occur. If dust problems are not solved by these applications, the use of water and calcium chloride can be applied. Calcium chloride will be applied at a rate that will keep the surface moist but not cause pollution.

## Construction Sequence

1. Cut and remove trees in construction area only as required.
2. Construct and/or install temporary and permanent sediment erosion and detention control facilities as specified. Erosion and sediment control measures shall be installed prior to any soil land disturbance.
3. Erosion, sediment and detention control facility shall be installed & stabilized prior to directing runoff to them, temporary diversions may be required. Post construction storm water management practices must be initiated and stabilized early in the process.
4. Clear, cut and dispose of debris in approved facility. Grubbing and stockpiling shall not occur until after erosion & sediment control measures are installed.
5. Construct temporary water diversions (swales, basins, etc.) as needed until site is stabilized.
6. All swales are to be installed prior to rough grading of the site. Temporary water diversion (swales, etc.) must be used as necessary until areas are stabilized.
7. Construct roadways for access to desired construction areas. All roads shall be stabilized immediately.
8. Install pipe and construction associated appurtenances as required or directed. Install Bioretention W/ ISRs, Infiltration Pond, and stormtech system. All disturbed areas shall be stabilized immediately after grading.
9. Begin permanent and temporary seeding and mulching. All cut and fill slopes and disturbed areas shall be seeded or mulched as required, or directed. Any area disturbed by construction will be re-stabilized within 45 days (Env-Wq 1504.16) and abutting properties will not be adversely affected by this development. All swales and drainage structures will be constructed and stabilized prior to having run-off directed to them. IAW EPA 2022 CGP 2.2.14, site stabilization will be initiated immediately in any areas of exposed soil where construction activities have permanently ceased or will be temporarily inactive for 14 or more calendar days. The installation of stabilization will be completed as soon as practicable but no later than 14 calendar days. All roadways and parking areas shall be stabilized within 72 hours of achieving finished grades. All cut and fill slopes shall be stabilized within 72 hours of achieving finished grades.

10. Construct temporary berms, drains ditches, silt fences, sediment traps, etc. Mulch and seed as required.
11. Inspect and maintain all erosion and sediment control measures during construction. All SWPPP inspections must be conducted by a qualified professional such as a professional engineer (PE), a certified professional in erosion and sediment control (CPESC), a certified erosion sediment and storm water inspector (CESSWI), or a certified professional in storm water quality (CPSWQ). Inspection reports shall be submitted to the Planning Department. Inspections shall be conducted weekly and within 24 hours of a 0.25 inch rain event.
12. Complete permanent seeding and landscaping.
13. Remove temporary erosion control measures after seeding areas have established themselves and site improvements are complete.
14. Smooth and revegetate all disturbed areas. Stabilization should occur within 14 days of removing temporary measures.
15. Finish paving all roadways.

### **Temporary Erosion Control Measures**

1. The smallest practical area of land shall be exposed at any one time.
2. Erosion, sediment and detention measures shall be installed as shown on the plans and at locations as required, directed by the engineer.
3. All disturbed areas shall be returned to original grades and elevations. Disturbed areas shall be loamed with a minimum of 4" of loam and seeded with not less than one pound of seed per 50 square yards of area. Apply hay or straw mulch or straw mulch with rye grass seed to temporarily stabilize the area until final grade is achieved.
4. All disturbed areas will be restabilized within 45 days. At any one time, no more than 5 acres, (217,800 sq. Ft.) Will be disturbed.
5. Silt fences and perimeter barriers shall be inspected periodically and after every rain during the life of the project. All damaged areas shall be repaired, sediment deposits shall periodically be removed and disposed of.
6. After all disturbed areas have been stabilized, the temporary erosion and sediment control measures are to be removed and the area disturbed by the removal smoothed and re-vegetated.

7. Per the EPA CGP requirements there will be reports of the erosion control inspections IAW SWPPP prepared by BS&E. All erosion controls shall be inspected weekly and within 24 hours after 0.25" or greater rain event.
8. Ditches, swales, and basins shall be stabilized prior to directing runoff to them.
9. Do not traffic exposed soil surfaces with construction equipment. If feasible, perform excavations with equipment positioned outside the limits of the infiltration system.
10. Roadways, driveways and cut and fill slopes must be stabilized within 72 hours of achieving final grade.
11. Stabilization means:
  - 11.1. A minimum of 85% of vegetative cover has been established.
  - 11.2. A minimum of 3 inches of non-erosive material such as stone or rip rap has been installed, or
  - 11.3. Erosion control blankets have been installed.
12. This project is to be managed in a manner that meets the requirements and intent of RSA 430:53 and chapter AGR 3800 relative to invasive species.
13. The NHDES stormwater manual, in three volumes, dated December 2008, is a part of this plan set and the more restrictive will govern. (NH SWM)

### **Inspection and Maintenance Schedule**

Perimeter control and catch basin inlet protection will be inspected during and after storm events of 0.25" or greater to ensure that the BMP still has integrity and is not allowing sediment to pass. Depending on SWPPP criteria, all BMP controls will be inspected once every 7 days and after storm events. Inspection reports must be submitted to Town of Barrington Planning Department. See also Stormwater System Management: Inspection and Maintenance Manual with accompanying plan published separately also by Berry Surveying & Engineering. See also Storm Water Pollution Prevention Plan (SWPPP) developed in accordance with EPA NPDES requirements & the Town of Barrington Stormwater regulations.

Corrective Action measures will be made in accordance with SWPPP requirements and records maintained on site by the Contractor.


## 5.0 CONCLUSION

Peak rates of runoff flow are modeled to be reduced in the post-construction analysis, as compared to the pre-construction analysis. This reduction occurs at all storm events due to the installation of the low impact development stormwater devices.


The volume of stormwater discharge from the site at the final reaches is reduced at the 2Yr.-24Hr. storm event for channel protection purposes.

A Site Specific, Terrain Alteration Permit (RSA 485: A-17) is required for this site plan due to the area of disturbance being greater than 100,000 SF (162,000SF). This will be an amendment to the existing AoT Permit.


Respectfully Submitted,  
BERRY SURVEYING & ENGINEERING



Kevin R. Poulin, PE  
Design Engineer



Christopher R. Berry, SIT 567  
Principal, President



Kenneth A. Berry PE, LLS,  
CPSWQ, CPESC, CESSWI  
Principal, VP - Technical Operations



## **Appendix I –Existing Conditions Analysis**

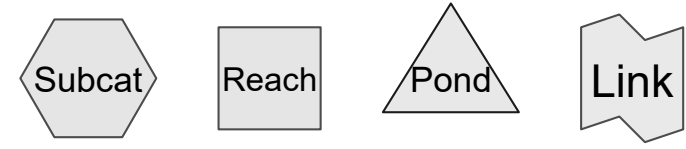
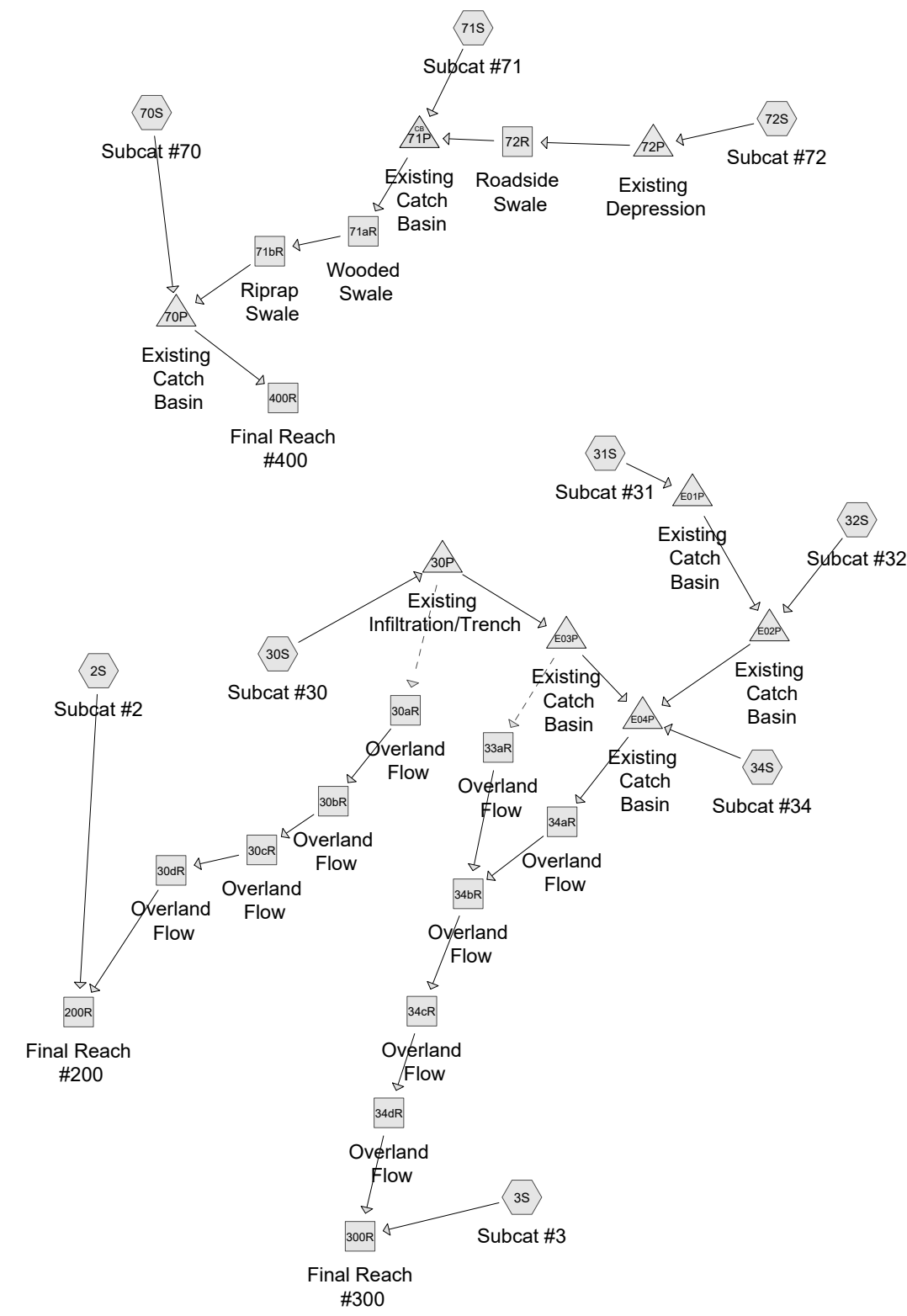
25 Yr - 24 Hr. Full Summary

2 Yr - 24 Hr. Node Listing

10 Yr -24 Hr. Node Listing

25 Yr -24 Hr. Node Listing

50 Yr - 24 Hr. Node Listing



**Routing Diagram for 23-017 Existing Analysis Ex TCAM**  
 Prepared by Berry Surveying & Engineering, Printed 4/17/2024  
 HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

## 23-017 Existing Analysis Ex TCAM

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 1

### Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2.193	39	>75% Grass cover, Good, HSG A (3S, 70S, 71S, 72S)
5.331	61	>75% Grass cover, Good, HSG B (2S, 3S, 30S, 31S, 32S, 34S, 70S)
0.867	96	Gravel surface, HSG B (2S, 3S, 30S, 34S)
1.541	98	Paved parking, HSG A (70S, 71S, 72S)
1.228	98	Paved parking, HSG B (3S, 30S, 31S, 32S, 70S)
0.044	98	Roofs, HSG B (3S, 31S, 32S)
0.086	98	Unconnected pavement, HSG B (2S)
0.001	98	Unconnected roofs, HSG B (2S)
2.953	30	Woods, Good, HSG A (3S, 70S, 71S, 72S)
2.697	55	Woods, Good, HSG B (2S, 3S, 30S, 32S, 34S)
<b>16.942</b>	<b>60</b>	<b>TOTAL AREA</b>

## 23-017 Existing Analysis Ex TCAM

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 2

### Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
6.688	HSG A	3S, 70S, 71S, 72S
10.254	HSG B	2S, 3S, 30S, 31S, 32S, 34S, 70S
0.000	HSG C	
0.000	HSG D	
0.000	Other	
<b>16.942</b>		<b>TOTAL AREA</b>

## 23-017 Existing Analysis Ex TCAM

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 3

### Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
2.193	5.331	0.000	0.000	0.000	7.524	>75% Grass cover, Good	2S, 3S, 30S, 31S, 32S, 34S, 70S, 71S, 72S
0.000	0.867	0.000	0.000	0.000	0.867	Gravel surface	2S, 3S, 30S, 34S
1.541	1.228	0.000	0.000	0.000	2.769	Paved parking	3S, 30S, 31S, 32S, 70S, 71S, 72S
0.000	0.044	0.000	0.000	0.000	0.044	Roofs	3S, 31S, 32S
0.000	0.086	0.000	0.000	0.000	0.086	Unconnected pavement	2S
0.000	0.001	0.000	0.000	0.000	0.001	Unconnected roofs	2S
2.953	2.697	0.000	0.000	0.000	5.651	Woods, Good	2S, 3S, 30S, 32S, 34S, 70S, 71S, 72S
<b>6.688</b>	<b>10.254</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>16.942</b>	<b>TOTAL AREA</b>	

## 23-017 Existing Analysis Ex TCAM

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 4

### Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	30P	183.15	183.15	1.0	0.0000	0.012	6.0	0.0	0.0
2	70P	180.14	179.01	62.8	0.0180	0.012	18.0	0.0	0.0
3	71P	187.90	187.80	10.2	0.0098	0.012	18.0	0.0	0.0
4	E01P	183.50	183.22	57.0	0.0049	0.012	15.0	0.0	0.0
5	E02P	183.02	179.71	122.2	0.0271	0.012	24.0	0.0	0.0
6	E03P	179.56	179.56	36.8	0.0000	0.012	15.0	0.0	0.0

**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 5

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 2S: Subcat #2</b>	Runoff Area=163,452 sf 2.33% Impervious Runoff Depth>1.91" Flow Length=301' Tc=15.8 min CN=61 Runoff=5.85 cfs 0.598 af
<b>Subcatchment 3S: Subcat #3</b>	Runoff Area=222,064 sf 1.16% Impervious Runoff Depth>1.42" Flow Length=682' Tc=43.7 min CN=55 Runoff=3.57 cfs 0.605 af
<b>Subcatchment 30S: Subcat #30</b>	Runoff Area=58,317 sf 10.64% Impervious Runoff Depth>2.25" Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=65 Runoff=2.87 cfs 0.251 af
<b>Subcatchment 31S: Subcat #31</b>	Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>3.86" Tc=6.0 min CN=82 Runoff=1.99 cfs 0.145 af
<b>Subcatchment 32S: Subcat #32</b>	Runoff Area=40,270 sf 63.40% Impervious Runoff Depth>4.07" Tc=6.0 min CN=84 Runoff=4.26 cfs 0.314 af
<b>Subcatchment 34S: Subcat #34</b>	Runoff Area=1,936 sf 0.00% Impervious Runoff Depth>2.52" Tc=6.0 min CN=68 Runoff=0.13 cfs 0.009 af
<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=62,561 sf 28.86% Impervious Runoff Depth>1.44" Flow Length=380' Tc=15.2 min CN=55 Runoff=1.58 cfs 0.172 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>1.28" Flow Length=563' Tc=39.5 min CN=53 Runoff=1.48 cfs 0.246 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>2.07" Flow Length=478' Tc=32.0 min CN=63 Runoff=2.04 cfs 0.273 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.01' Max Vel=0.28 fps Inflow=0.01 cfs 0.000 af n=0.022 L=43.0' S=0.0105 '/' Capacity=16.58 cfs Outflow=0.01 cfs 0.000 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.01' Max Vel=0.56 fps Inflow=0.01 cfs 0.000 af n=0.030 L=63.5' S=0.1339 '/' Capacity=43.48 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.01' Max Vel=0.18 fps Inflow=0.00 cfs 0.000 af n=0.030 L=230.5' S=0.0130 '/' Capacity=13.56 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30dR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.41 fps Inflow=0.00 cfs 0.000 af n=0.030 L=11.0' S=0.1364 '/' Capacity=43.88 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 33aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.016 L=50.0' S=0.0198 '/' Capacity=31.35 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 34aR: Overland Flow</b>	Avg. Flow Depth=0.25' Max Vel=3.36 fps Inflow=6.40 cfs 0.513 af n=0.016 L=35.0' S=0.0140 '/' Capacity=26.36 cfs Outflow=6.08 cfs 0.513 af
<b>Reach 34bR: Overland Flow</b>	Avg. Flow Depth=0.27' Max Vel=3.03 fps Inflow=6.08 cfs 0.513 af n=0.016 L=194.0' S=0.0103 '/' Capacity=22.62 cfs Outflow=6.18 cfs 0.512 af

**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 6

<b>Reach 34cR: Overland Flow</b>	Avg. Flow Depth=0.20' Max Vel=5.07 fps Inflow=6.18 cfs 0.512 af n=0.030 L=36.0' S=0.1597 '/ Capacity=47.49 cfs Outflow=6.19 cfs 0.512 af
<b>Reach 34dR: Overland Flow</b>	Avg. Flow Depth=0.33' Max Vel=2.35 fps Inflow=6.19 cfs 0.512 af n=0.030 L=43.0' S=0.0174 '/ Capacity=15.69 cfs Outflow=6.20 cfs 0.512 af
<b>Reach 71aR: Wooded Swale</b>	Avg. Flow Depth=0.46' Max Vel=1.53 fps Inflow=3.40 cfs 0.492 af n=0.035 L=125.0' S=0.0064 '/ Capacity=79.88 cfs Outflow=3.39 cfs 0.492 af
<b>Reach 71bR: Riprap Swale</b>	Avg. Flow Depth=0.35' Max Vel=1.61 fps Inflow=3.39 cfs 0.492 af n=0.041 L=147.7' S=0.0135 '/ Capacity=31.94 cfs Outflow=3.39 cfs 0.491 af
<b>Reach 72R: Roadside Swale</b>	Avg. Flow Depth=0.25' Max Vel=1.71 fps Inflow=2.00 cfs 0.247 af n=0.022 L=495.6' S=0.0060 '/ Capacity=33.12 cfs Outflow=1.95 cfs 0.246 af
<b>Reach 200R: Final Reach #200</b>	Inflow=5.85 cfs 0.598 af Outflow=5.85 cfs 0.598 af
<b>Reach 300R: Final Reach #300</b>	Inflow=6.68 cfs 1.117 af Outflow=6.68 cfs 1.117 af
<b>Reach 400R: Final Reach #400</b>	Inflow=4.22 cfs 0.663 af Outflow=4.22 cfs 0.663 af
<b>Pond 30P: Existing Infiltration/Trench</b>	Peak Elev=183.96' Storage=3,170 cf Inflow=2.87 cfs 0.251 af Discarded=0.93 cfs 0.189 af Primary=0.65 cfs 0.053 af Secondary=0.01 cfs 0.000 af Outflow=1.47 cfs 0.242 af
<b>Pond 70P: Existing Catch Basin</b>	Peak Elev=181.13' Storage=0.000 af Inflow=4.22 cfs 0.663 af 18.0" Round Culvert n=0.012 L=62.8' S=0.0180 '/ Outflow=4.22 cfs 0.663 af
<b>Pond 71P: Existing Catch Basin</b>	Peak Elev=188.93' Inflow=3.40 cfs 0.492 af 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/ Outflow=3.40 cfs 0.492 af
<b>Pond 72P: Existing Depression</b>	Peak Elev=196.21' Storage=160 cf Inflow=2.04 cfs 0.273 af Discarded=0.02 cfs 0.024 af Primary=2.00 cfs 0.247 af Outflow=2.02 cfs 0.271 af
<b>Pond E01P: Existing Catch Basin</b>	Peak Elev=185.04' Storage=19 cf Inflow=1.99 cfs 0.145 af 15.0" Round Culvert n=0.012 L=57.0' S=0.0049 '/ Outflow=1.92 cfs 0.145 af
<b>Pond E02P: Existing Catch Basin</b>	Peak Elev=184.97' Storage=135 cf Inflow=6.18 cfs 0.459 af Discarded=0.01 cfs 0.006 af Primary=6.73 cfs 0.453 af Outflow=6.73 cfs 0.459 af
<b>Pond E03P: Existing Catch Basin</b>	Peak Elev=184.31' Storage=60 cf Inflow=0.65 cfs 0.053 af Primary=0.42 cfs 0.052 af Secondary=0.00 cfs 0.000 af Outflow=0.42 cfs 0.052 af
<b>Pond E04P: Existing Catch Basin</b>	Peak Elev=184.75' Storage=75 cf Inflow=6.85 cfs 0.514 af Outflow=6.40 cfs 0.513 af

**Total Runoff Area = 16.942 ac Runoff Volume = 2.614 af Average Runoff Depth = 1.85"**  
**82.88% Pervious = 14.041 ac 17.12% Impervious = 2.901 ac**



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 7

**Summary for Subcatchment 2S: Subcat #2**

Runoff = 5.85 cfs @ 12.24 hrs, Volume= 0.598 af, Depth> 1.91"

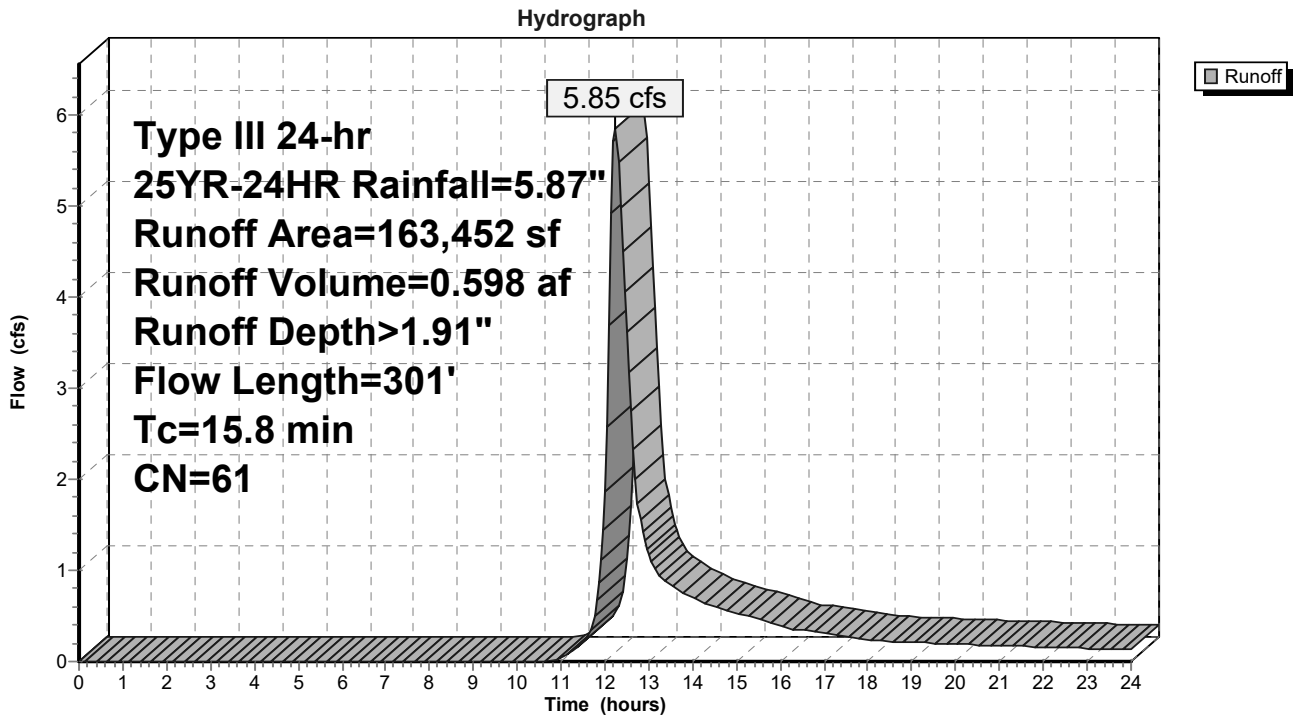
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
86,449	61	>75% Grass cover, Good, HSG B
3,761	98	Unconnected pavement, HSG B
48	98	Unconnected roofs, HSG B
7,782	96	Gravel surface, HSG B
65,412	55	Woods, Good, HSG B

163,452	61	Weighted Average
159,643		97.67% Pervious Area
3,809		2.33% Impervious Area
3,809		100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.2	100	0.0100	0.13		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
1.6	78	0.0128	0.79		<b>Shallow Concentrated Flow, Segment #2</b> Short Grass Pasture Kv= 7.0 fps
0.3	45	0.2000	2.24		<b>Shallow Concentrated Flow, Segment #3</b> Woodland Kv= 5.0 fps
0.4	49	0.1071	2.29		<b>Shallow Concentrated Flow, Segment #4</b> Short Grass Pasture Kv= 7.0 fps
0.3	29	0.0690	1.84		<b>Shallow Concentrated Flow, Segment #5</b> Short Grass Pasture Kv= 7.0 fps
15.8	301	Total			

Subcatchment 2S: Subcat #2



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 9

**Summary for Subcatchment 3S: Subcat #3**

Runoff = 3.57 cfs @ 12.68 hrs, Volume= 0.605 af, Depth&gt; 1.42"

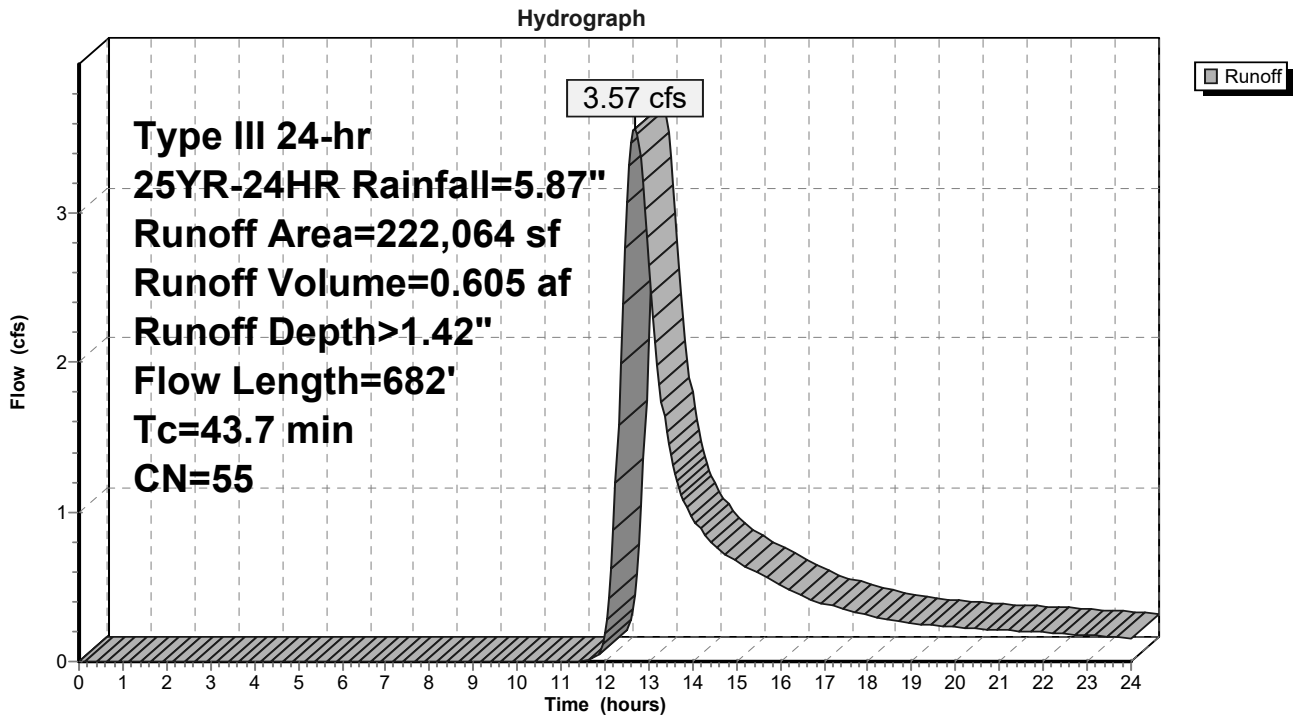
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
10,402	39	>75% Grass cover, Good, HSG A
65,404	30	Woods, Good, HSG A
1,526	98	Roofs, HSG B
74,474	61	>75% Grass cover, Good, HSG B
1,060	98	Paved parking, HSG B
40,852	55	Woods, Good, HSG B
28,346	96	Gravel surface, HSG B
222,064	55	Weighted Average
219,478		98.84% Pervious Area
2,586		1.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.9	100	0.0100	0.06		<b>Sheet Flow, Segment #1</b> Woods: Light underbrush n= 0.400 P2= 3.08"
10.0	252	0.0070	0.42		<b>Shallow Concentrated Flow, Segment #2</b> Woodland Kv= 5.0 fps
1.5	74	0.0270	0.82		<b>Shallow Concentrated Flow, Segment #3</b> Woodland Kv= 5.0 fps
1.8	157	0.0828	1.44		<b>Shallow Concentrated Flow, Segment #4</b> Woodland Kv= 5.0 fps
1.5	99	0.0505	1.12		<b>Shallow Concentrated Flow, Segment #5</b> Woodland Kv= 5.0 fps
43.7	682	Total			

Subcatchment 3S: Subcat #3



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 11

**Summary for Subcatchment 30S: Subcat #30**

Runoff = 2.87 cfs @ 12.17 hrs, Volume= 0.251 af, Depth> 2.25"

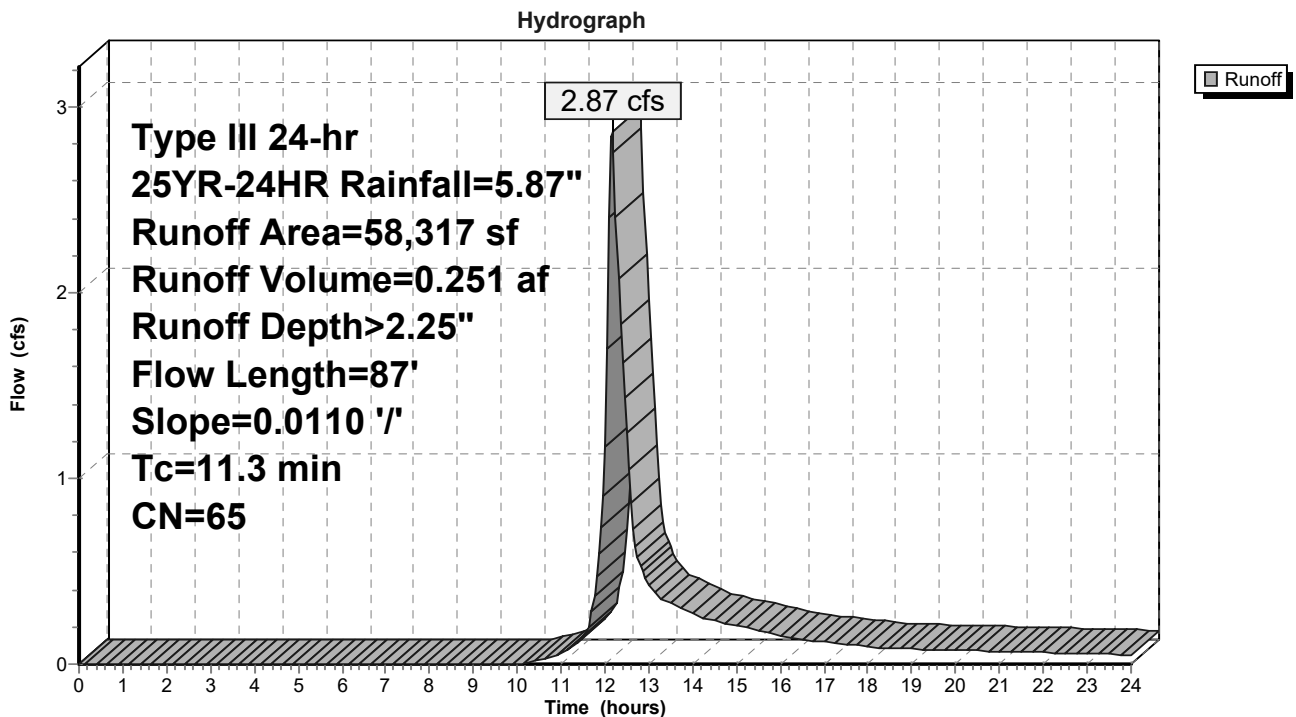
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
45,589	61	>75% Grass cover, Good, HSG B
6,207	98	Paved parking, HSG B
1,222	96	Gravel surface, HSG B
5,299	55	Woods, Good, HSG B
58,317	65	Weighted Average
52,110		89.36% Pervious Area
6,207		10.64% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.3	87	0.0110	0.13		

**Sheet Flow, Segment #1**  
Grass: Short n= 0.150 P2= 3.08"

**Subcatchment 30S: Subcat #30**



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 12

**Summary for Subcatchment 31S: Subcat #31**

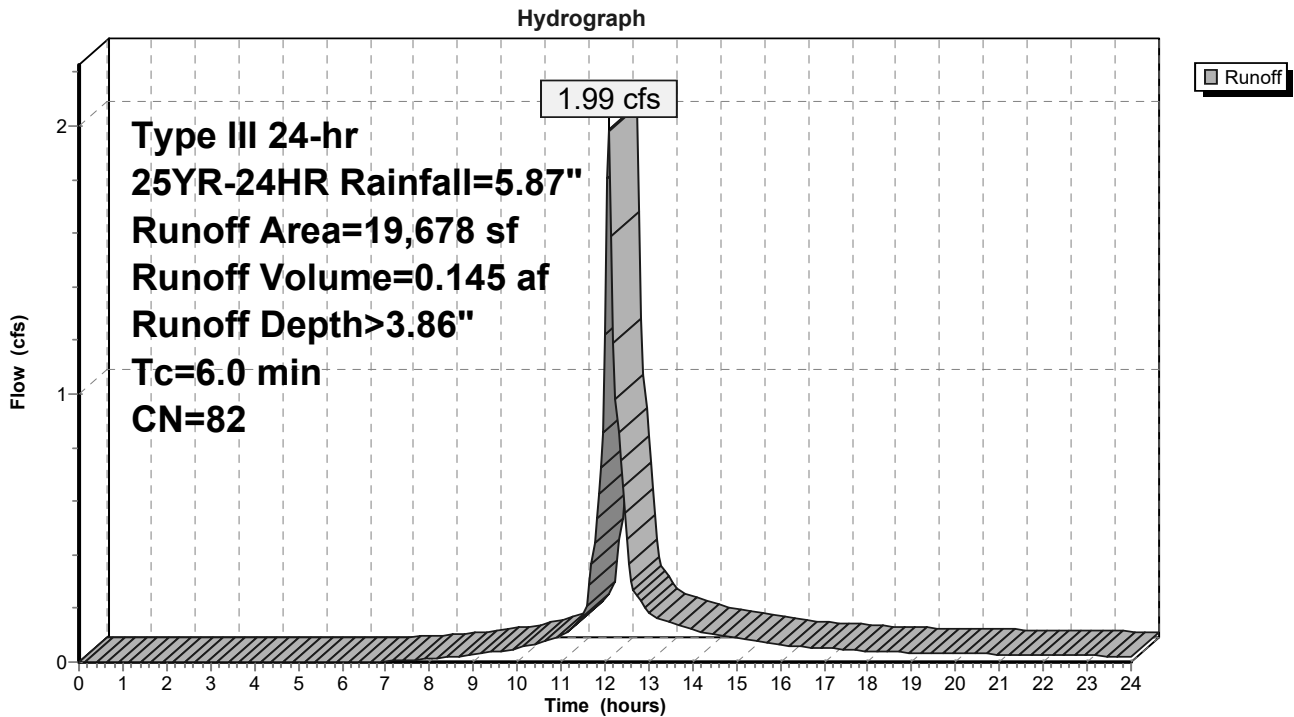
Runoff = 1.99 cfs @ 12.09 hrs, Volume= 0.145 af, Depth> 3.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
57	98	Roofs, HSG B
8,646	61	>75% Grass cover, Good, HSG B
10,975	98	Paved parking, HSG B
19,678	82	Weighted Average
8,646		43.94% Pervious Area
11,032		56.06% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Entry

**Subcatchment 31S: Subcat #31**



**23-017 Existing Analysis Ex TCAM**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 13

**Summary for Subcatchment 32S: Subcat #32**

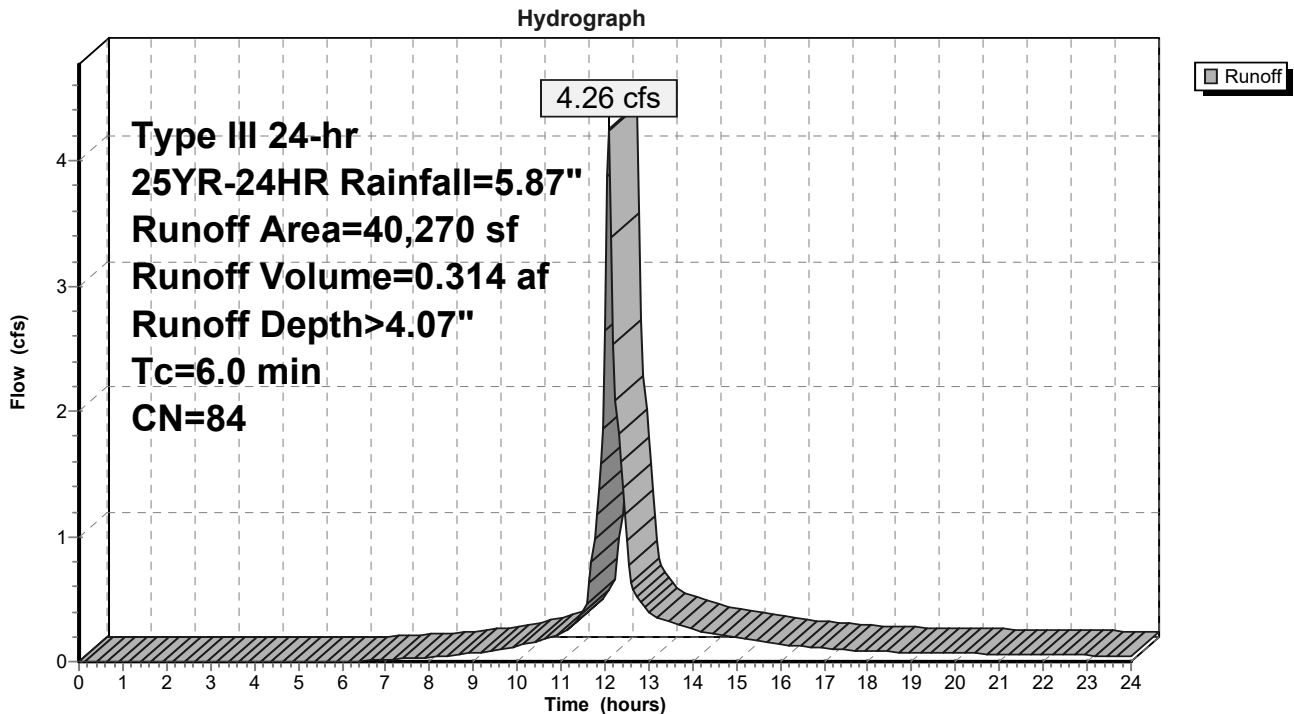
Runoff = 4.26 cfs @ 12.09 hrs, Volume= 0.314 af, Depth> 4.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
341	98	Roofs, HSG B
9,068	61	>75% Grass cover, Good, HSG B
25,189	98	Paved parking, HSG B
5,672	55	Woods, Good, HSG B
40,270	84	Weighted Average
14,740		36.60% Pervious Area
25,530		63.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Entry

**Subcatchment 32S: Subcat #32**



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 14

**Summary for Subcatchment 34S: Subcat #34**

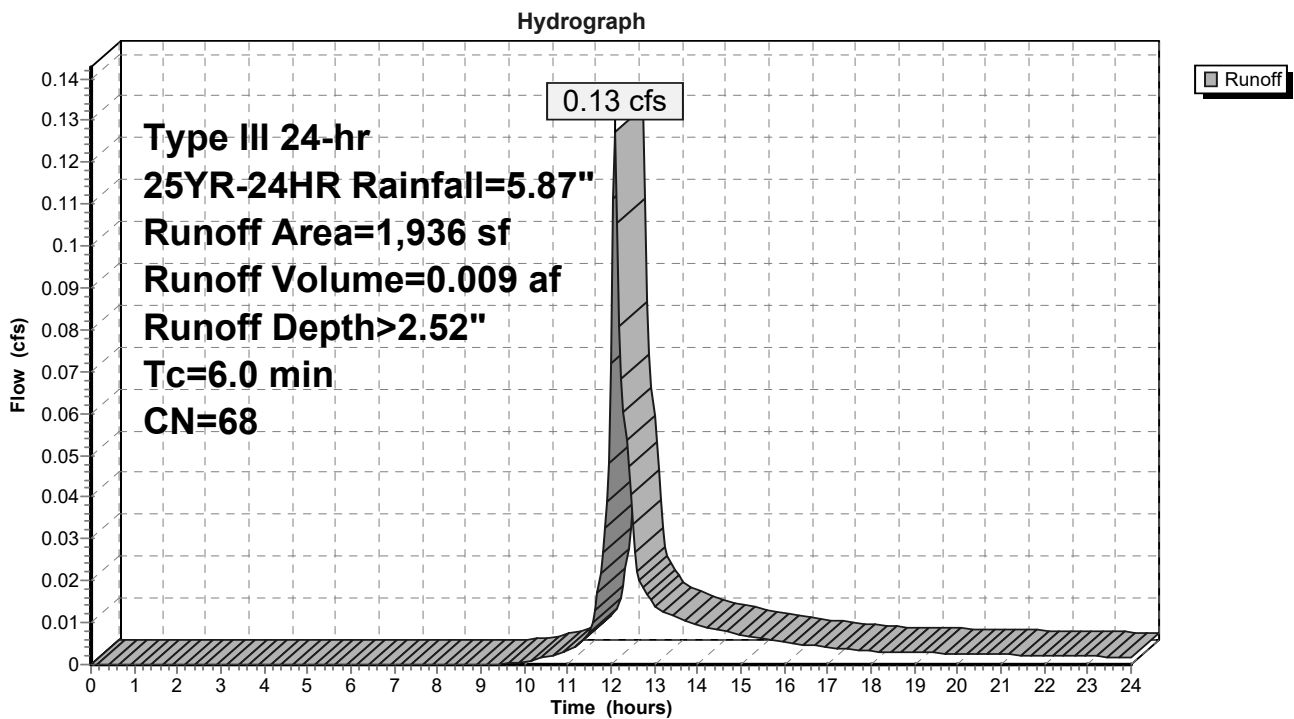
Runoff = 0.13 cfs @ 12.10 hrs, Volume= 0.009 af, Depth> 2.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
1,270	61	>75% Grass cover, Good, HSG B
260	55	Woods, Good, HSG B
406	96	Gravel surface, HSG B
1,936	68	Weighted Average
1,936		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Entry

**Subcatchment 34S: Subcat #34**





**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 15

**Summary for Subcatchment 70S: Subcat #70**

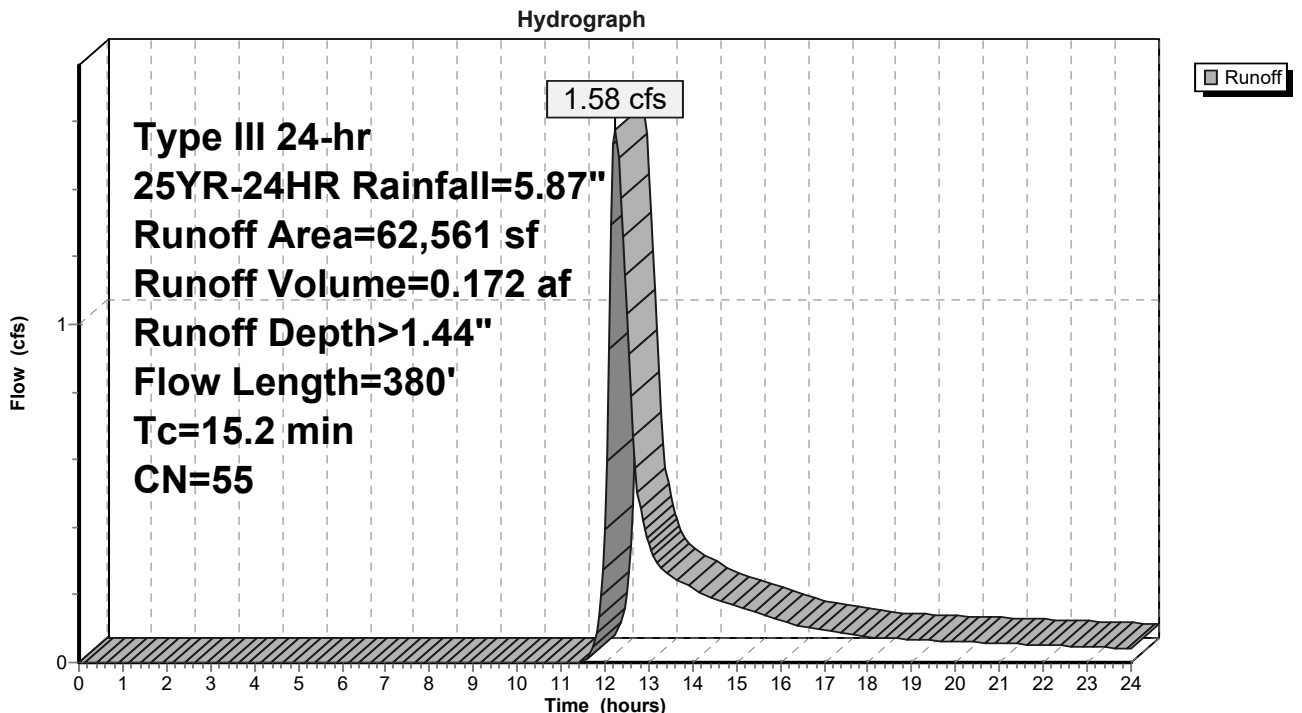
Runoff = 1.58 cfs @ 12.24 hrs, Volume= 0.172 af, Depth> 1.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
17,367	39	>75% Grass cover, Good, HSG A
8,001	98	Paved parking, HSG A
20,432	30	Woods, Good, HSG A
6,708	61	>75% Grass cover, Good, HSG B
10,053	98	Paved parking, HSG B
62,561	55	Weighted Average
44,507		71.14% Pervious Area
18,054		28.86% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	94	0.0319	0.20		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
5.9	136	0.0060	0.39		<b>Shallow Concentrated Flow, Segment #2</b> Woodland Kv= 5.0 fps
1.4	150	0.0135	1.74		<b>Shallow Concentrated Flow, Segment #3</b> Grassed Waterway Kv= 15.0 fps
15.2	380	Total			

**Subcatchment 70S: Subcat #70**



**23-017 Existing Analysis Ex TCAM**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 16

**Summary for Subcatchment 71S: Subcat #71**

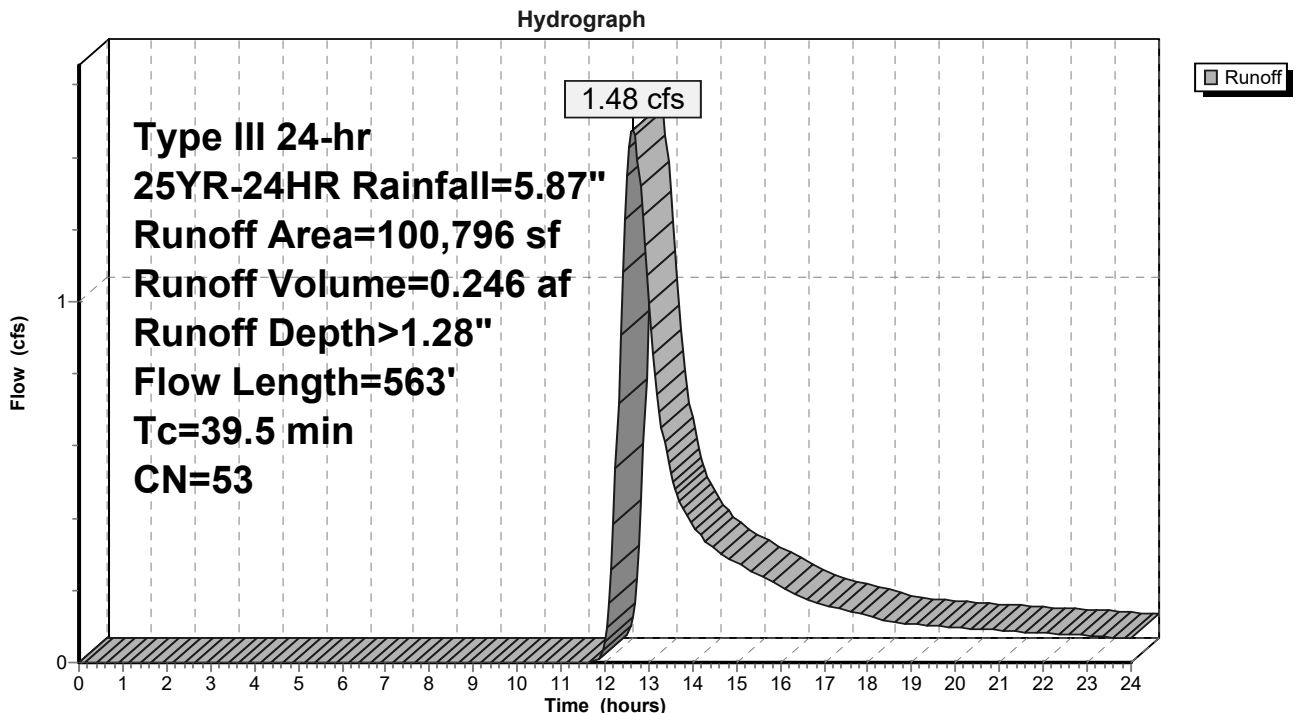
Runoff = 1.48 cfs @ 12.63 hrs, Volume= 0.246 af, Depth> 1.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
35,048	39	>75% Grass cover, Good, HSG A
29,681	98	Paved parking, HSG A
36,067	30	Woods, Good, HSG A
100,796	53	Weighted Average
71,115		70.55% Pervious Area
29,681		29.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.6	100	0.0150	0.07		<b>Sheet Flow, Segment #1</b> Woods: Light underbrush n= 0.400 P2= 3.08"
11.4	285	0.0070	0.42		<b>Shallow Concentrated Flow, Segment #2</b> Woodland Kv= 5.0 fps
0.6	65	0.0615	1.74		<b>Shallow Concentrated Flow, Segment #3</b> Short Grass Pasture Kv= 7.0 fps
2.9	113	0.0088	0.66		<b>Shallow Concentrated Flow, Segment #4</b> Short Grass Pasture Kv= 7.0 fps
39.5	563	Total			

**Subcatchment 71S: Subcat #71**



**23-017 Existing Analysis Ex TCAM**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 17

**Summary for Subcatchment 72S: Subcat #72**

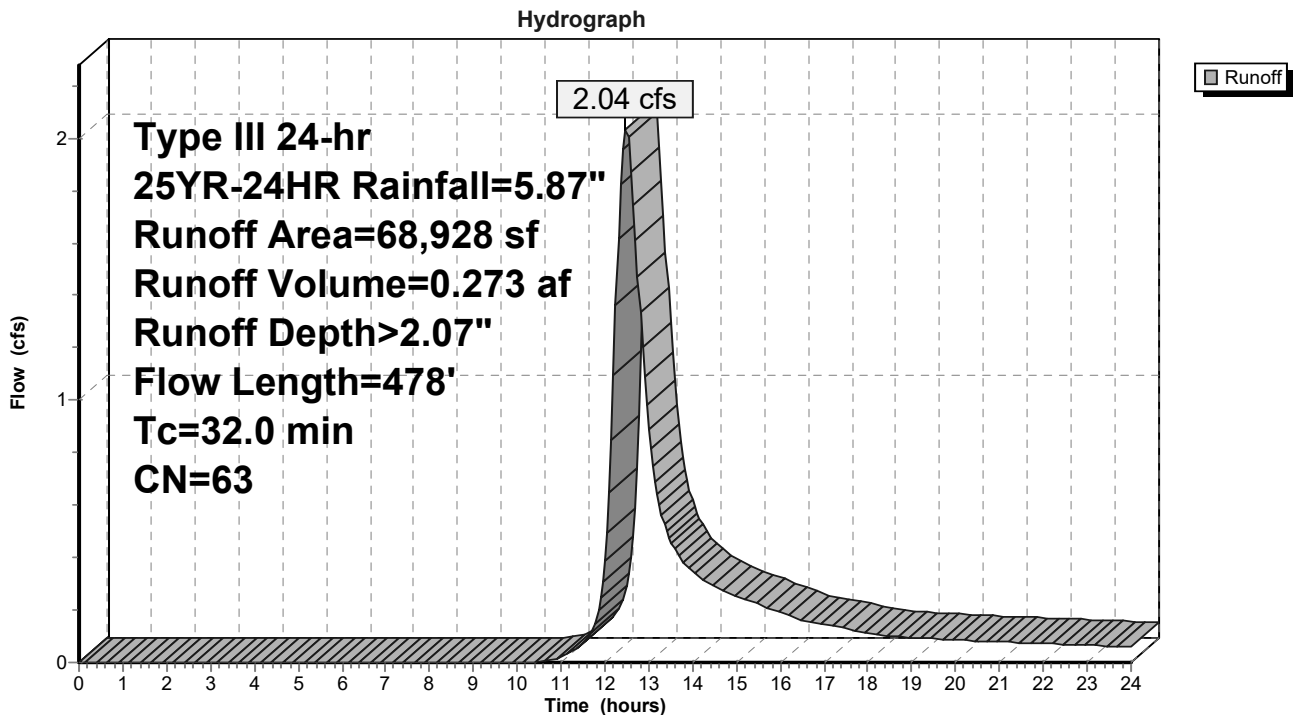
Runoff = 2.04 cfs @ 12.48 hrs, Volume= 0.273 af, Depth> 2.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
32,729	39	>75% Grass cover, Good, HSG A
29,456	98	Paved parking, HSG A
6,743	30	Woods, Good, HSG A
68,928	63	Weighted Average
39,472		57.27% Pervious Area
29,456		42.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.6	100	0.0150	0.07		<b>Sheet Flow, Segment #1</b> Woods: Light underbrush n= 0.400 P2= 3.08"
0.7	27	0.0150	0.61		<b>Shallow Concentrated Flow, Segment #2</b> Woodland Kv= 5.0 fps
6.7	351	0.0157	0.88		<b>Shallow Concentrated Flow, Segment #3</b> Short Grass Pasture Kv= 7.0 fps
32.0	478	Total			

**Subcatchment 72S: Subcat #72**



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 18

**Summary for Reach 30aR: Overland Flow**

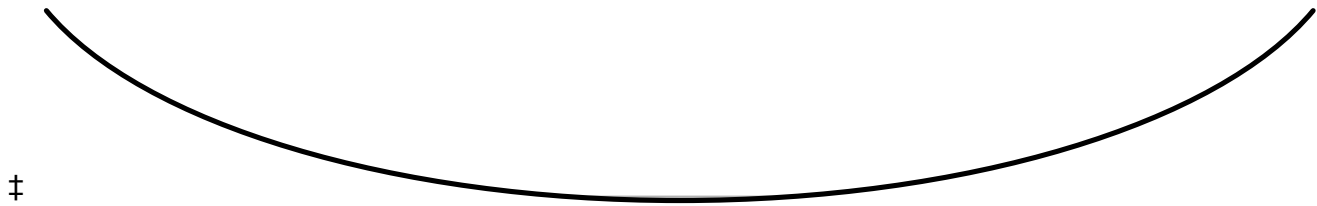
[80] Warning: Exceeded Pond 30P by 0.33' @ 14.70 hrs (0.00 cfs 0.000 af)

Inflow	=	0.01 cfs @ 12.65 hrs,	Volume=	0.000 af
Outflow	=	0.01 cfs @ 12.68 hrs,	Volume=	0.000 af, Atten= 48%, Lag= 2.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 0.28 fps, Min. Travel Time= 2.5 min  
 Avg. Velocity = 0.20 fps, Avg. Travel Time= 3.6 min

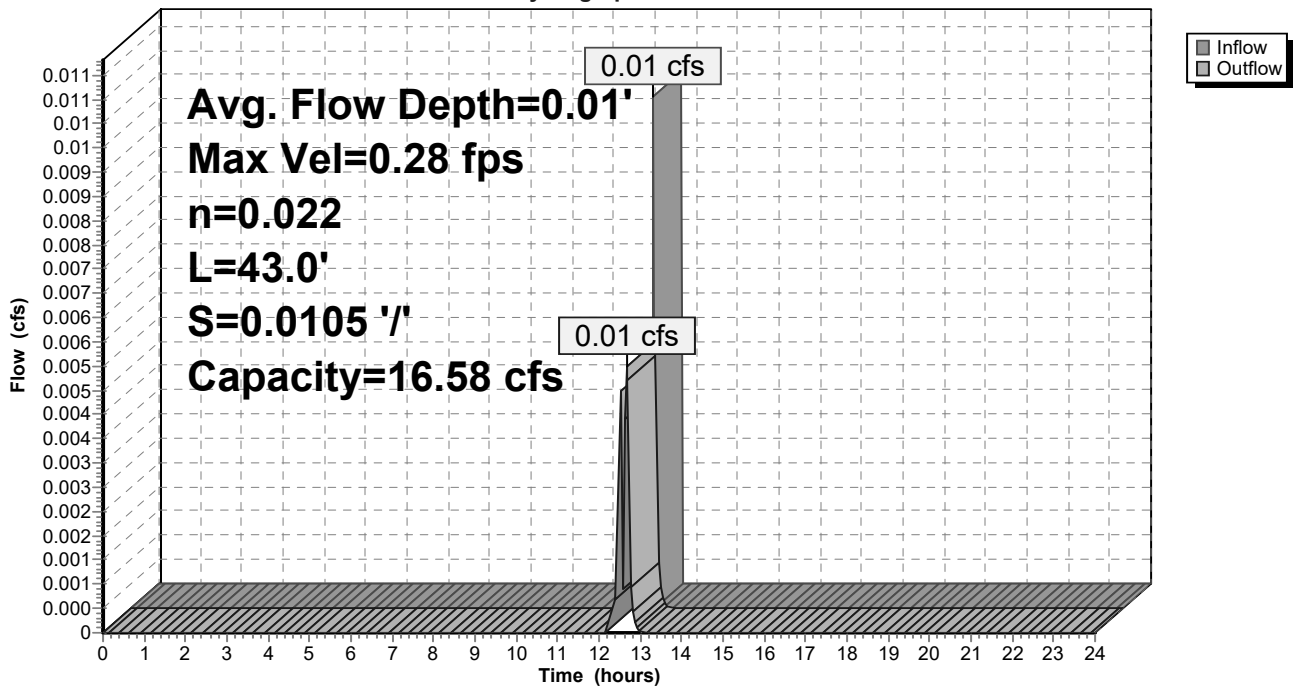
Peak Storage= 1 cf @ 12.68 hrs  
 Average Depth at Peak Storage= 0.01'  
 Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 16.58 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight  
 Length= 43.0' Slope= 0.0105 '/  
 Inlet Invert= 183.95', Outlet Invert= 183.50'



**Reach 30aR: Overland Flow**

Hydrograph



# 23-017 Existing Analysis Ex TCAM

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 19

## Summary for Reach 30bR: Overland Flow

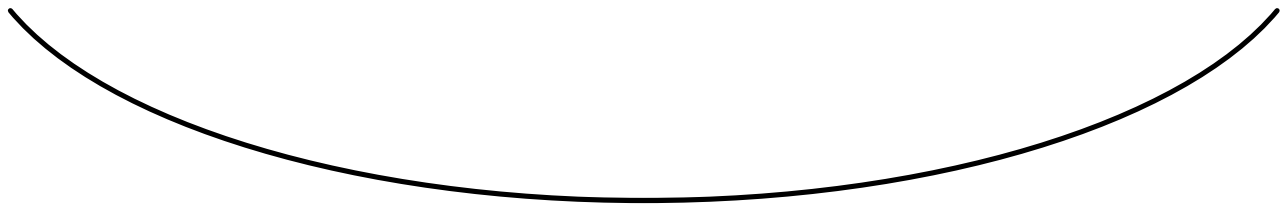
[61] Hint: Exceeded Reach 30aR outlet invert by 0.01' @ 12.70 hrs

Inflow = 0.01 cfs @ 12.68 hrs, Volume= 0.000 af  
Outflow = 0.00 cfs @ 12.70 hrs, Volume= 0.000 af, Atten= 11%, Lag= 1.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Max. Velocity= 0.56 fps, Min. Travel Time= 1.9 min  
Avg. Velocity = 0.45 fps, Avg. Travel Time= 2.4 min

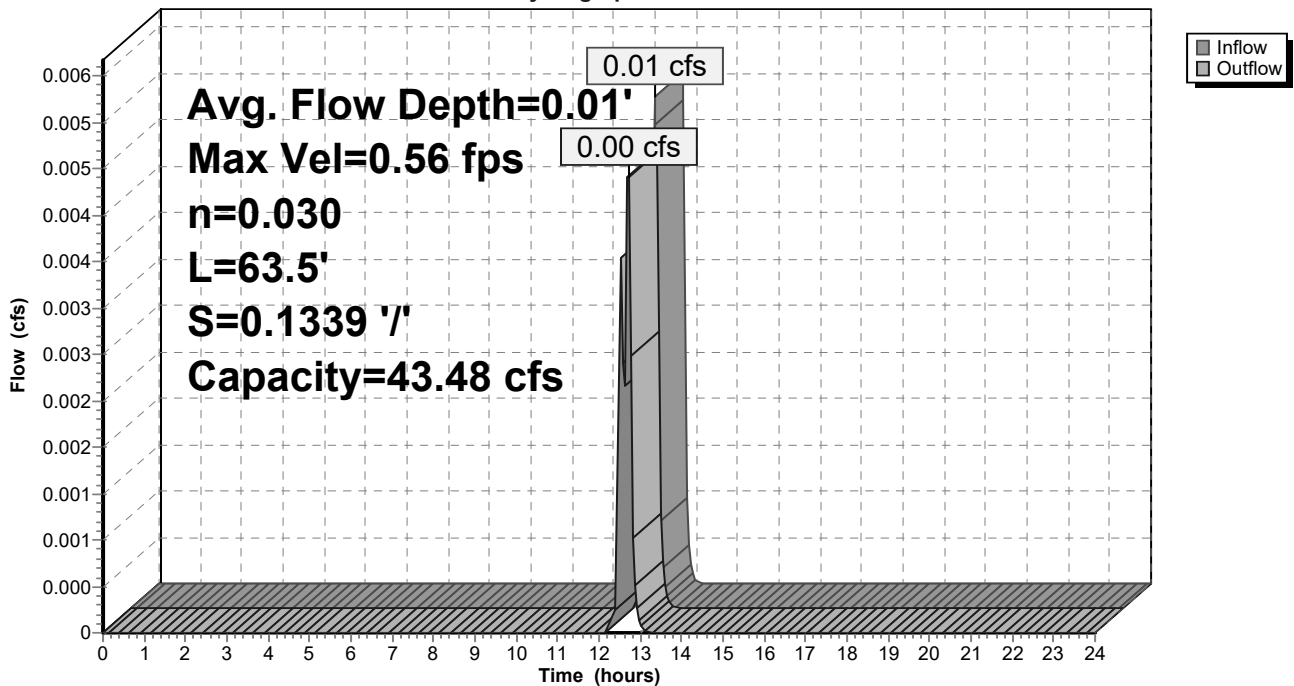
Peak Storage= 1 cf @ 12.70 hrs  
Average Depth at Peak Storage= 0.01'  
Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 43.48 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.030 Earth, grassed & winding  
Length= 63.5' Slope= 0.1339 '/  
Inlet Invert= 183.50', Outlet Invert= 175.00'



## Reach 30bR: Overland Flow

Hydrograph



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 20

**Summary for Reach 30cR: Overland Flow**

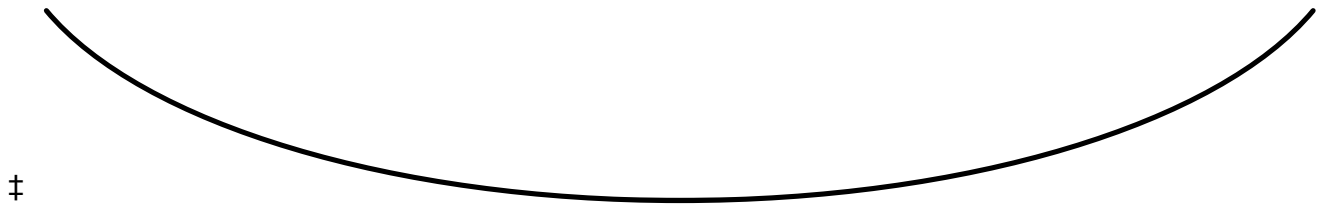
[62] Hint: Exceeded Reach 30bR OUTLET depth by 0.01' @ 12.90 hrs

Inflow	=	0.00 cfs @ 12.70 hrs,	Volume=	0.000 af
Outflow	=	0.00 cfs @ 12.78 hrs,	Volume=	0.000 af, Atten= 61%, Lag= 4.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 0.18 fps, Min. Travel Time= 20.8 min  
 Avg. Velocity = 0.13 fps, Avg. Travel Time= 28.8 min

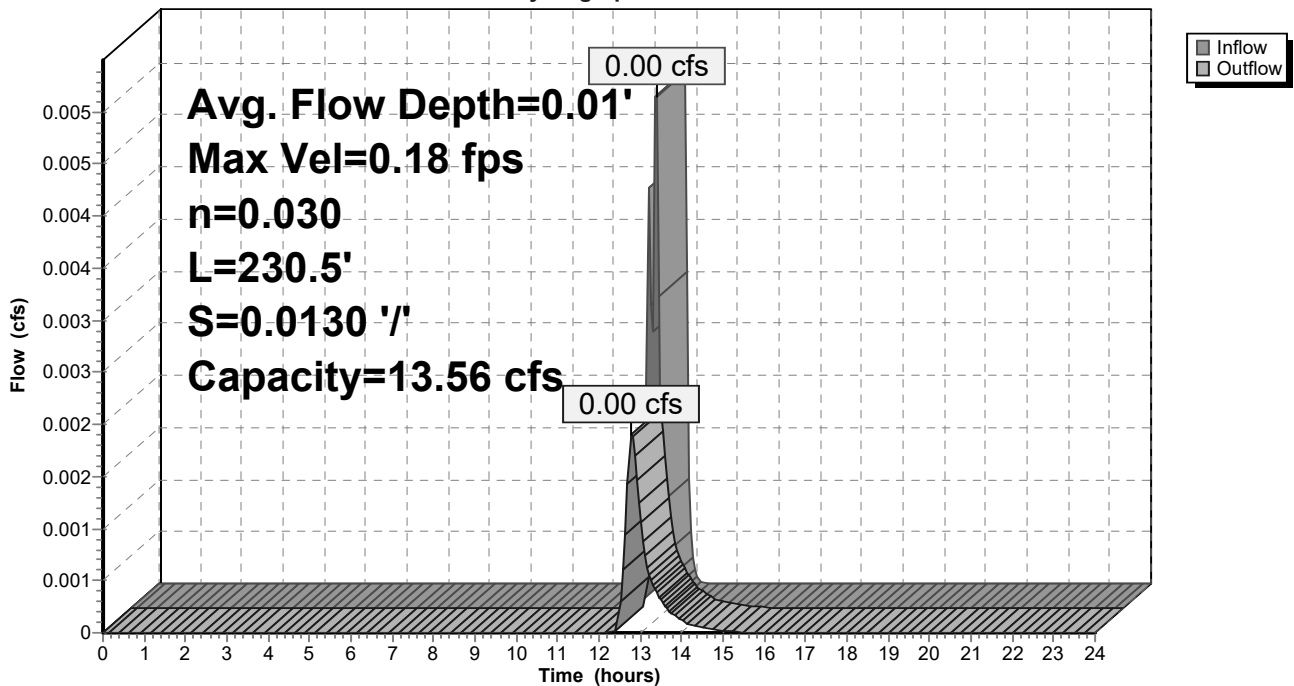
Peak Storage= 2 cf @ 12.78 hrs  
 Average Depth at Peak Storage= 0.01'  
 Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 13.56 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.030 Earth, grassed & winding  
 Length= 230.5' Slope= 0.0130 '/'  
 Inlet Invert= 175.00', Outlet Invert= 172.00'



**Reach 30cR: Overland Flow**

Hydrograph



# 23-017 Existing Analysis Ex TCAM

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 21

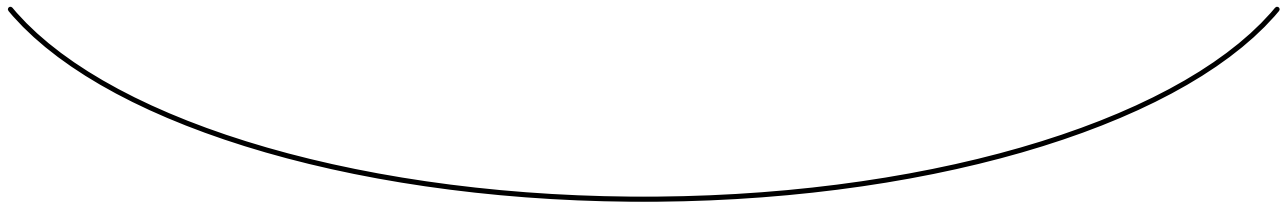
## Summary for Reach 30dR: Overland Flow

Inflow = 0.00 cfs @ 12.78 hrs, Volume= 0.000 af  
Outflow = 0.00 cfs @ 12.79 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Max. Velocity= 0.41 fps, Min. Travel Time= 0.4 min  
Avg. Velocity = 0.41 fps, Avg. Travel Time= 0.4 min

Peak Storage= 0 cf @ 12.79 hrs  
Average Depth at Peak Storage= 0.00'  
Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 43.88 cfs

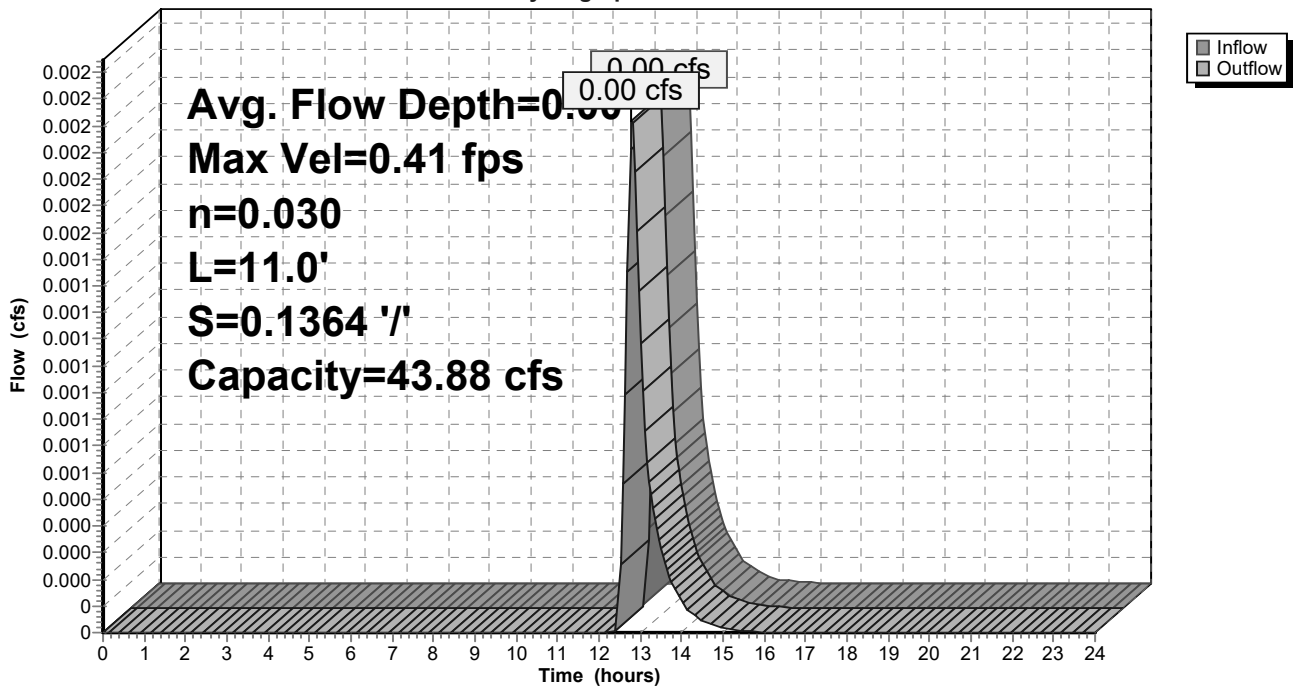
15.00' x 0.50' deep Parabolic Channel, n= 0.030 Earth, grassed & winding  
Length= 11.0' Slope= 0.1364 '/'  
Inlet Invert= 172.00', Outlet Invert= 170.50'



‡

## Reach 30dR: Overland Flow

Hydrograph



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 22

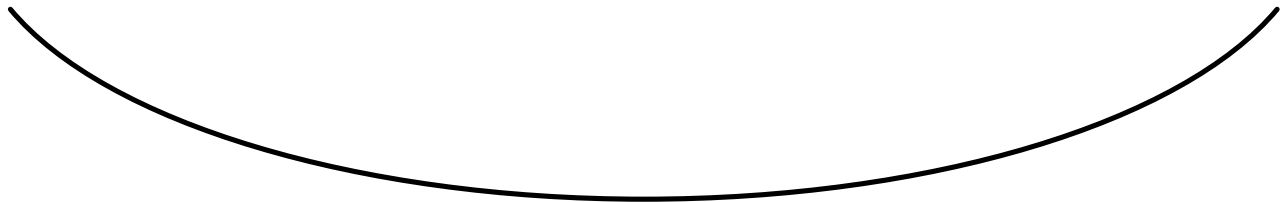
**Summary for Reach 33aR: Overland Flow**

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af  
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min  
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

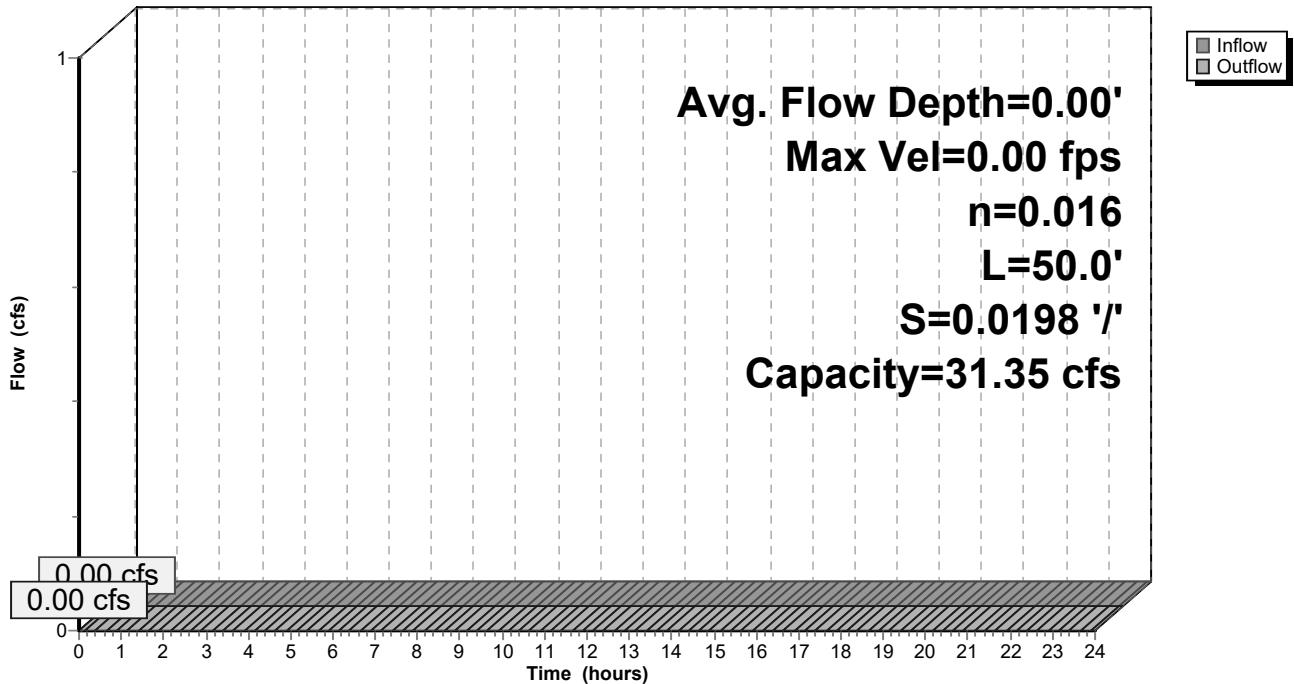
Peak Storage= 0 cf @ 0.00 hrs  
Average Depth at Peak Storage= 0.00'  
Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 31.35 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.016 Asphalt, rough  
Length= 50.0' Slope= 0.0198 '/  
Inlet Invert= 184.49', Outlet Invert= 183.50'



**Reach 33aR: Overland Flow**

Hydrograph





**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 23

**Summary for Reach 34aR: Overland Flow**

[80] Warning: Exceeded Pond E04P by 0.75' @ 15.85 hrs (0.14 cfs 0.003 af)

Inflow Area =	2.759 ac, 35.58% Impervious, Inflow Depth > 2.23"	for 25YR-24HR event
Inflow =	6.40 cfs @ 12.10 hrs, Volume=	0.513 af
Outflow =	6.08 cfs @ 12.10 hrs, Volume=	0.513 af, Atten= 5%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 3.36 fps, Min. Travel Time= 0.2 min  
 Avg. Velocity = 1.23 fps, Avg. Travel Time= 0.5 min

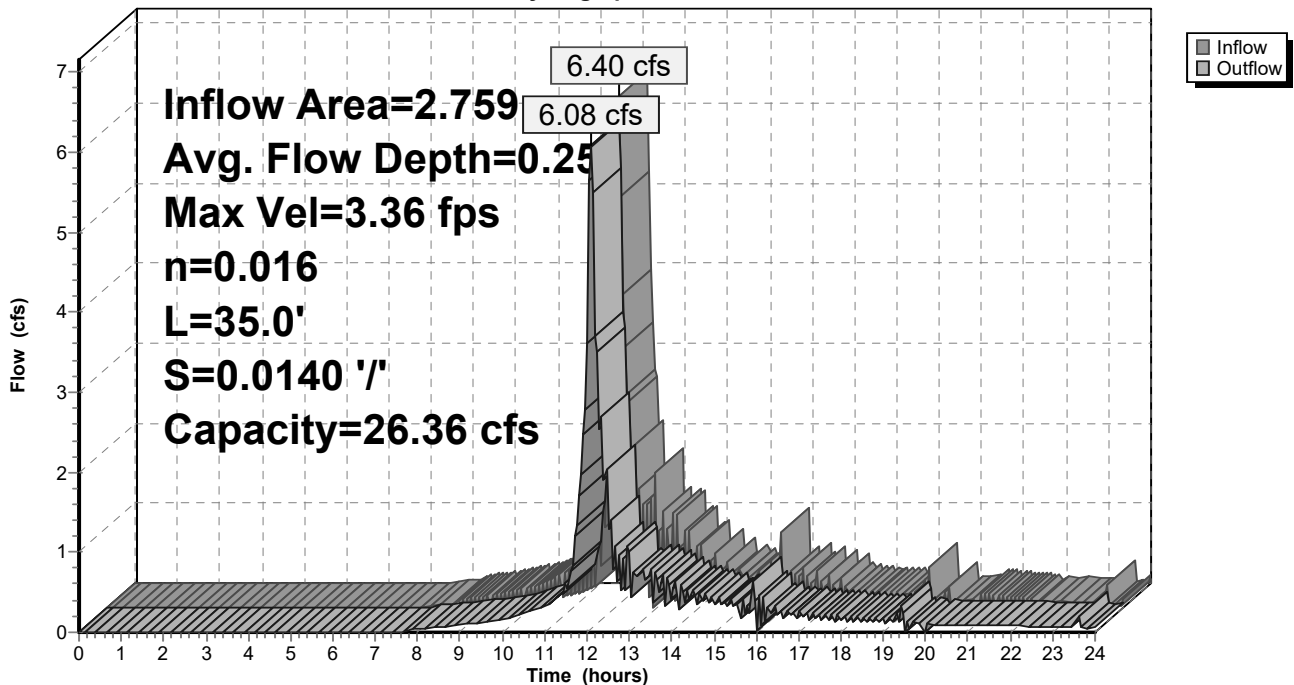
Peak Storage= 63 cf @ 12.10 hrs  
 Average Depth at Peak Storage= 0.25'  
 Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 26.36 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.016 Asphalt, rough  
 Length= 35.0' Slope= 0.0140 '/'  
 Inlet Invert= 183.99', Outlet Invert= 183.50'



**Reach 34aR: Overland Flow**

Hydrograph



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 24

**Summary for Reach 34bR: Overland Flow**

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

[62] Hint: Exceeded Reach 33aR OUTLET depth by 0.27' @ 12.10 hrs

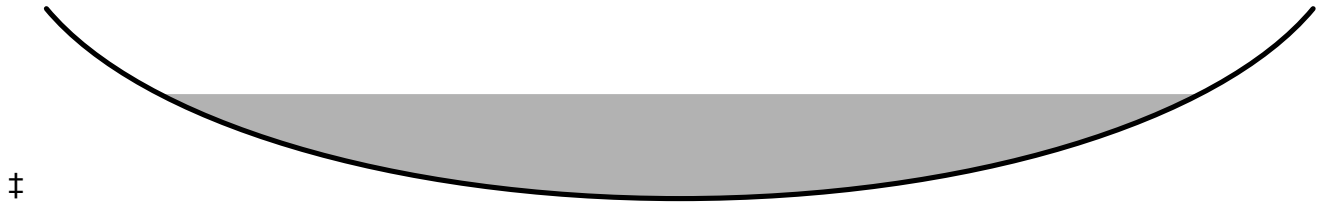
[62] Hint: Exceeded Reach 34aR OUTLET depth by 0.05' @ 19.50 hrs

Inflow Area =	2.759 ac, 35.58% Impervious, Inflow Depth > 2.23"	for 25YR-24HR event
Inflow =	6.08 cfs @ 12.10 hrs, Volume=	0.513 af
Outflow =	6.18 cfs @ 12.11 hrs, Volume=	0.512 af, Atten= 0%, Lag= 0.8 min

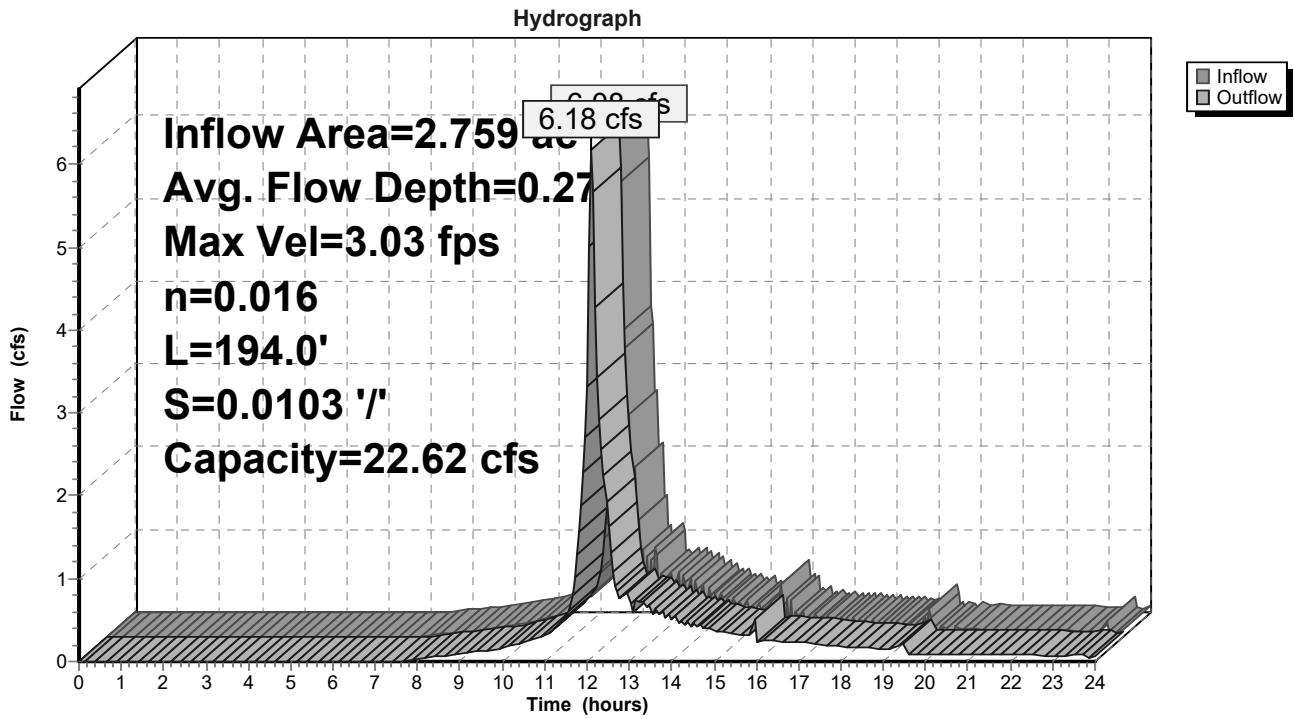
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 3.03 fps, Min. Travel Time= 1.1 min  
 Avg. Velocity = 1.10 fps, Avg. Travel Time= 2.9 min

Peak Storage= 395 cf @ 12.11 hrs  
 Average Depth at Peak Storage= 0.27'  
 Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 22.62 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.016 Asphalt, rough  
 Length= 194.0' Slope= 0.0103 '/'  
 Inlet Invert= 183.50', Outlet Invert= 181.50'



### Reach 34bR: Overland Flow



**Summary for Reach 34cR: Overland Flow**

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=2)

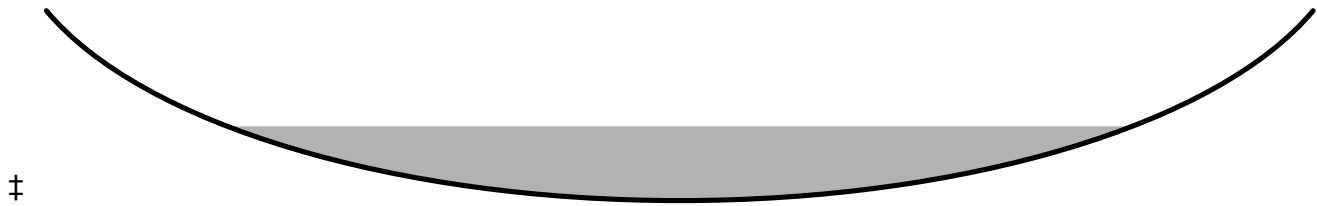
[61] Hint: Exceeded Reach 34bR outlet invert by 0.19' @ 12.10 hrs

Inflow Area =	2.759 ac, 35.58% Impervious, Inflow Depth > 2.23"	for 25YR-24HR event
Inflow =	6.18 cfs @ 12.11 hrs, Volume=	0.512 af
Outflow =	6.19 cfs @ 12.11 hrs, Volume=	0.512 af, Atten= 0%, Lag= 0.1 min

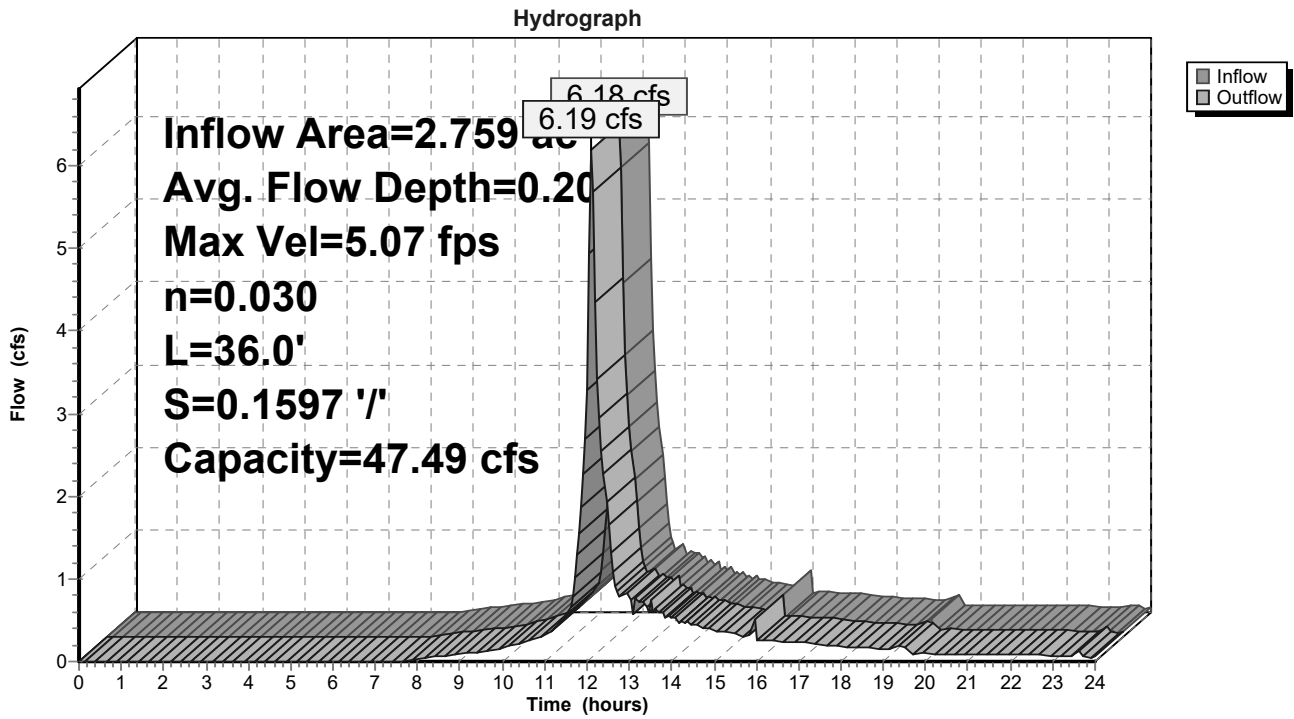
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 5.07 fps, Min. Travel Time= 0.1 min  
 Avg. Velocity = 1.85 fps, Avg. Travel Time= 0.3 min

Peak Storage= 44 cf @ 12.11 hrs  
 Average Depth at Peak Storage= 0.20'  
 Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 47.49 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.030 Earth, grassed & winding  
 Length= 36.0' Slope= 0.1597 '/  
 Inlet Invert= 181.50', Outlet Invert= 175.75'



### Reach 34cR: Overland Flow



# 23-017 Existing Analysis Ex TCAM

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 28

## Summary for Reach 34dR: Overland Flow

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

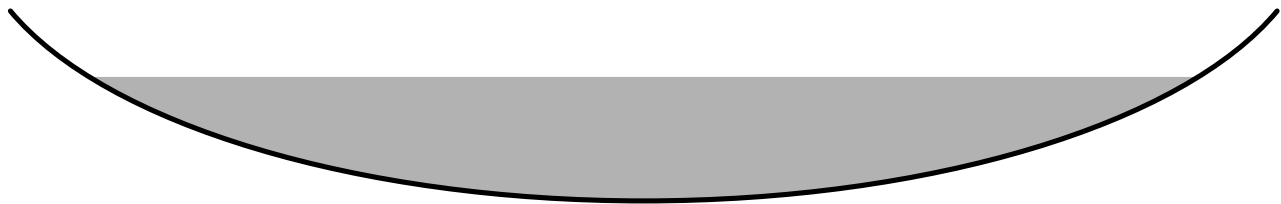
[62] Hint: Exceeded Reach 34cR OUTLET depth by 0.13' @ 12.10 hrs

Inflow Area = 2.759 ac, 35.58% Impervious, Inflow Depth > 2.23" for 25YR-24HR event  
Inflow = 6.19 cfs @ 12.11 hrs, Volume= 0.512 af  
Outflow = 6.20 cfs @ 12.11 hrs, Volume= 0.512 af, Atten= 0%, Lag= 0.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Max. Velocity= 2.35 fps, Min. Travel Time= 0.3 min  
Avg. Velocity = 0.86 fps, Avg. Travel Time= 0.8 min

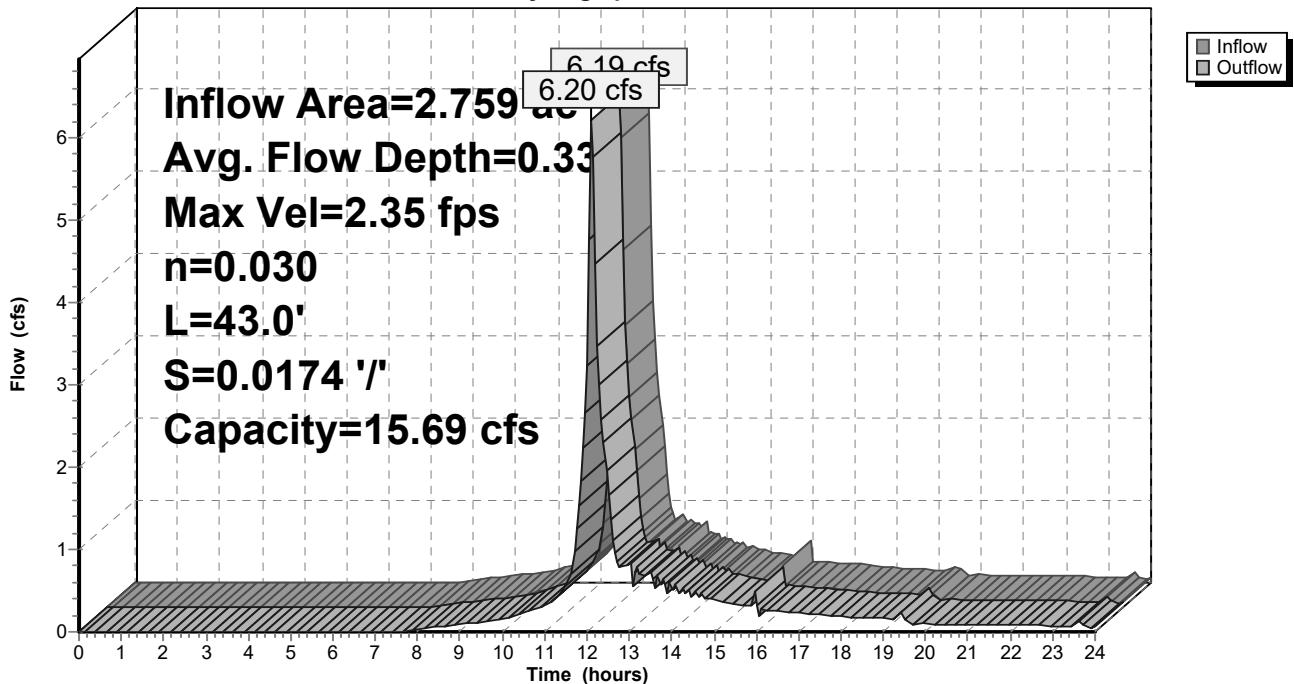
Peak Storage= 113 cf @ 12.11 hrs  
Average Depth at Peak Storage= 0.33'  
Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 15.69 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.030 Earth, grassed & winding  
Length= 43.0' Slope= 0.0174 '/  
Inlet Invert= 175.75', Outlet Invert= 175.00'



## Reach 34dR: Overland Flow

Hydrograph



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 29

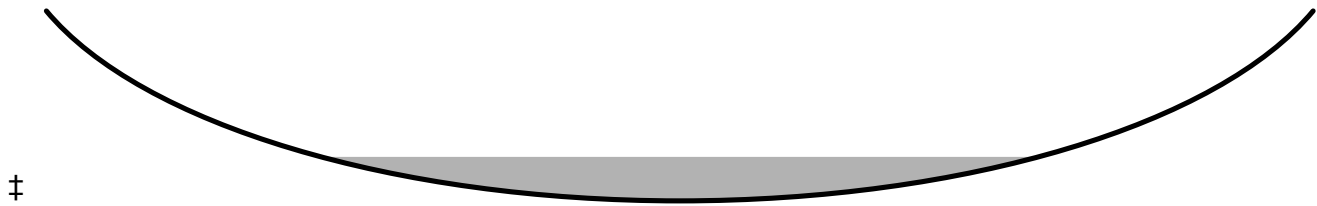
**Summary for Reach 71aR: Wooded Swale**

Inflow Area = 3.896 ac, 34.84% Impervious, Inflow Depth > 1.52" for 25YR-24HR event  
 Inflow = 3.40 cfs @ 12.58 hrs, Volume= 0.492 af  
 Outflow = 3.39 cfs @ 12.60 hrs, Volume= 0.492 af, Atten= 0%, Lag= 1.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 1.53 fps, Min. Travel Time= 1.4 min  
 Avg. Velocity = 0.75 fps, Avg. Travel Time= 2.8 min

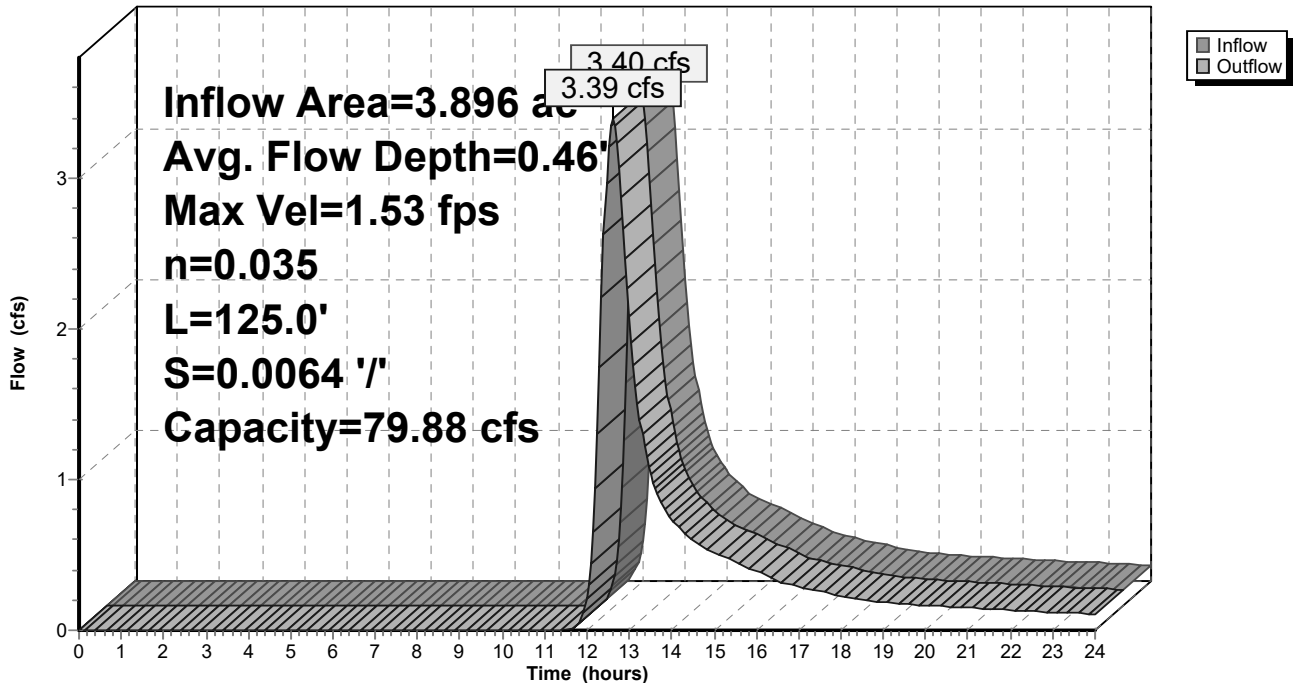
Peak Storage= 276 cf @ 12.60 hrs  
 Average Depth at Peak Storage= 0.46'  
 Bank-Full Depth= 2.00' Flow Area= 20.0 sf, Capacity= 79.88 cfs

15.00' x 2.00' deep Parabolic Channel, n= 0.035 Earth, dense weeds  
 Length= 125.0' Slope= 0.0064 '/'  
 Inlet Invert= 187.80', Outlet Invert= 187.00'



**Reach 71aR: Wooded Swale**

Hydrograph



# 23-017 Existing Analysis Ex TCAM

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 30

## Summary for Reach 71bR: Riprap Swale

[61] Hint: Exceeded Reach 71aR outlet invert by 0.35' @ 12.60 hrs

Inflow Area = 3.896 ac, 34.84% Impervious, Inflow Depth > 1.51" for 25YR-24HR event  
Inflow = 3.39 cfs @ 12.60 hrs, Volume= 0.492 af  
Outflow = 3.39 cfs @ 12.62 hrs, Volume= 0.491 af, Atten= 0%, Lag= 1.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Max. Velocity= 1.61 fps, Min. Travel Time= 1.5 min  
Avg. Velocity = 0.78 fps, Avg. Travel Time= 3.2 min

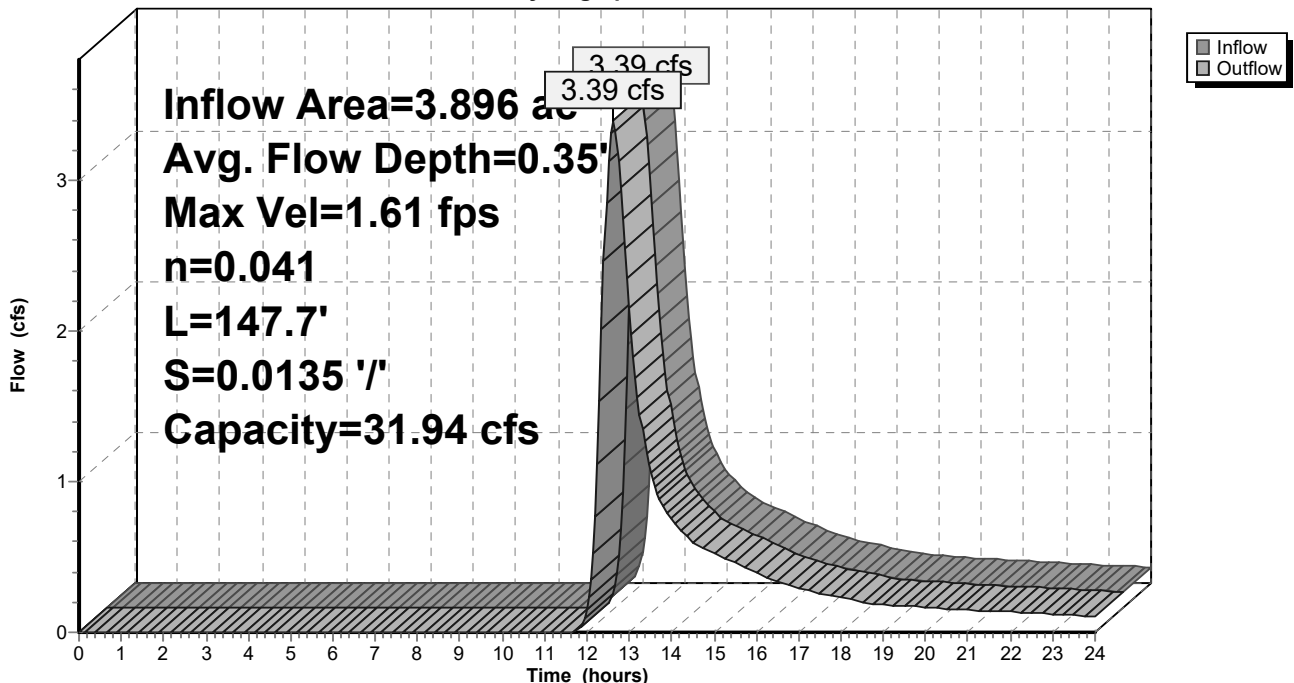
Peak Storage= 311 cf @ 12.62 hrs  
Average Depth at Peak Storage= 0.35'  
Bank-Full Depth= 1.00' Flow Area= 10.0 sf, Capacity= 31.94 cfs

15.00' x 1.00' deep Parabolic Channel, n= 0.041 Riprap, 2-inch  
Length= 147.7' Slope= 0.0135 '/'  
Inlet Invert= 187.00', Outlet Invert= 185.00'



## Reach 71bR: Riprap Swale

Hydrograph





**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 31

**Summary for Reach 72R: Roadside Swale**

[80] Warning: Exceeded Pond 72P by 0.02' @ 12.25 hrs (1.74 cfs 0.063 af)

Inflow Area =	1.582 ac, 42.73% Impervious, Inflow Depth > 1.87"	for 25YR-24HR event
Inflow =	2.00 cfs @ 12.49 hrs, Volume=	0.247 af
Outflow =	1.95 cfs @ 12.55 hrs, Volume=	0.246 af, Atten= 2%, Lag= 3.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 1.71 fps, Min. Travel Time= 4.8 min  
 Avg. Velocity = 0.74 fps, Avg. Travel Time= 11.2 min

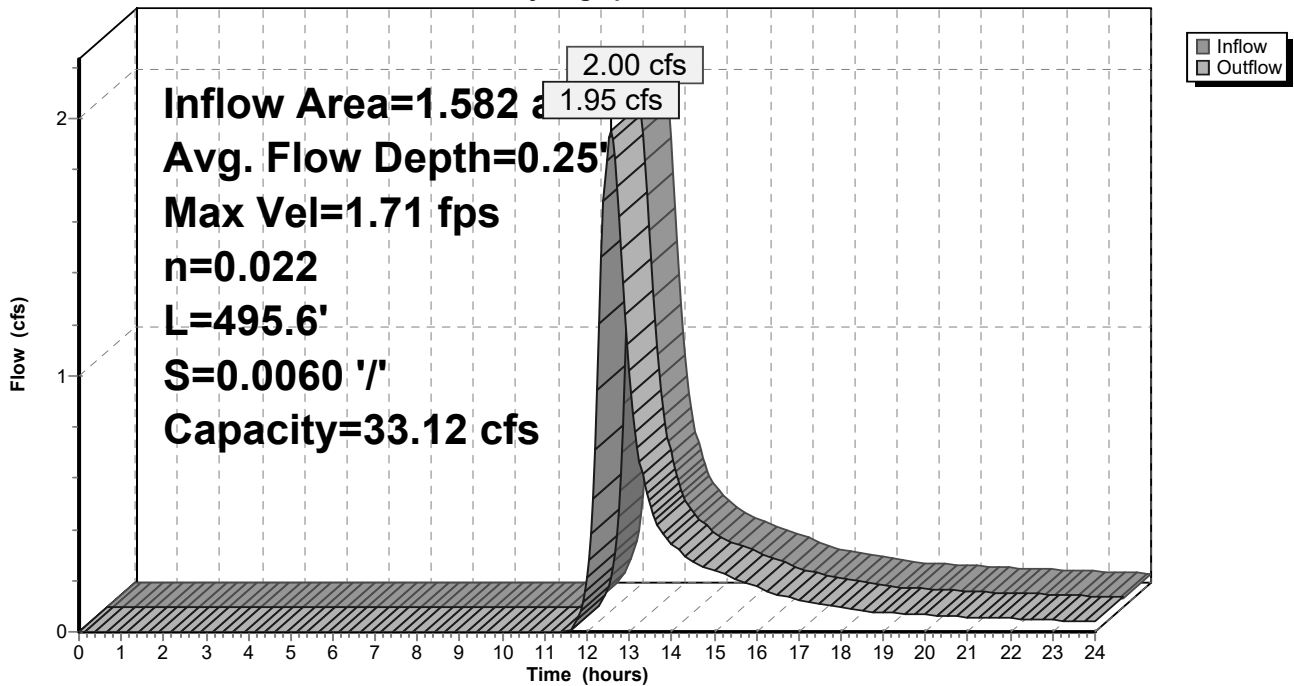
Peak Storage= 566 cf @ 12.55 hrs  
 Average Depth at Peak Storage= 0.25'  
 Bank-Full Depth= 1.00' Flow Area= 9.0 sf, Capacity= 33.12 cfs

3.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight  
 Side Slope Z-value= 6.0 '/' Top Width= 15.00'  
 Length= 495.6' Slope= 0.0060 '/'  
 Inlet Invert= 195.95', Outlet Invert= 193.00'



**Reach 72R: Roadside Swale**

Hydrograph



**23-017 Existing Analysis Ex TCAM**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 32

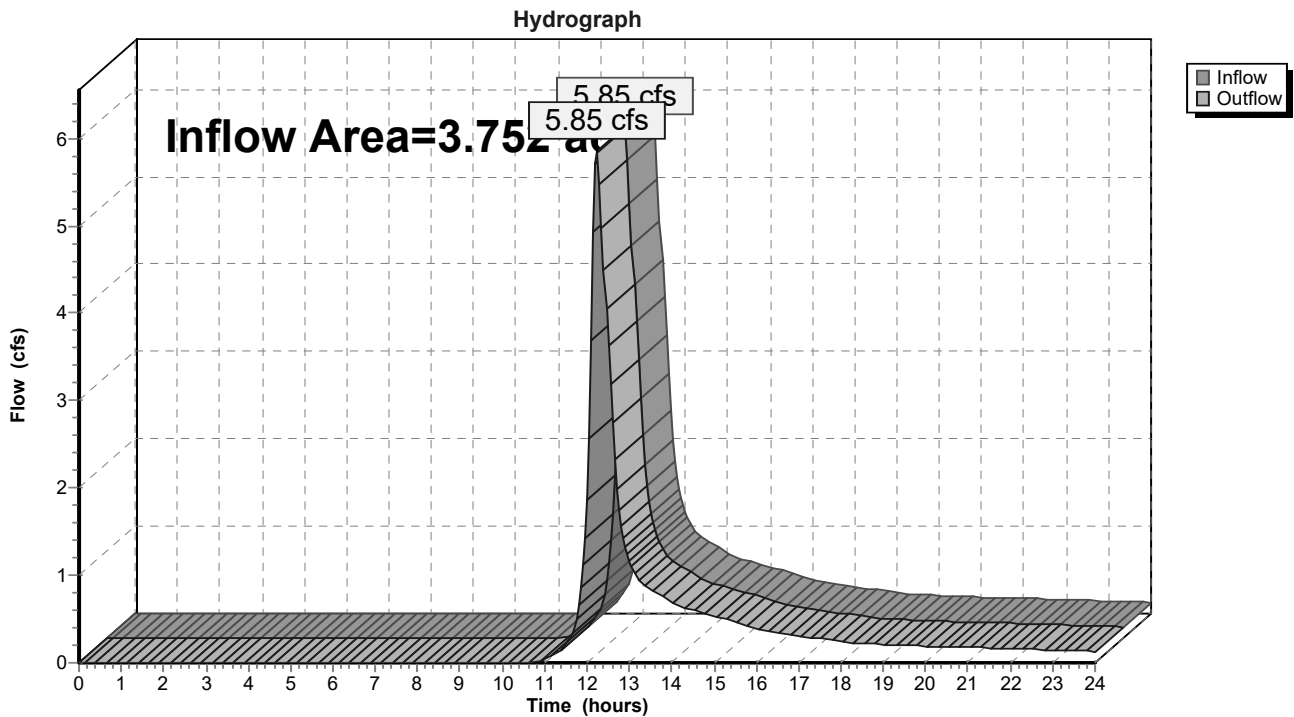
**Summary for Reach 200R: Final Reach #200**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.752 ac, 2.33% Impervious, Inflow Depth > 1.91" for 25YR-24HR event  
Inflow = 5.85 cfs @ 12.24 hrs, Volume= 0.598 af  
Outflow = 5.85 cfs @ 12.24 hrs, Volume= 0.598 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

**Reach 200R: Final Reach #200**



**23-017 Existing Analysis Ex TCAM**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 33

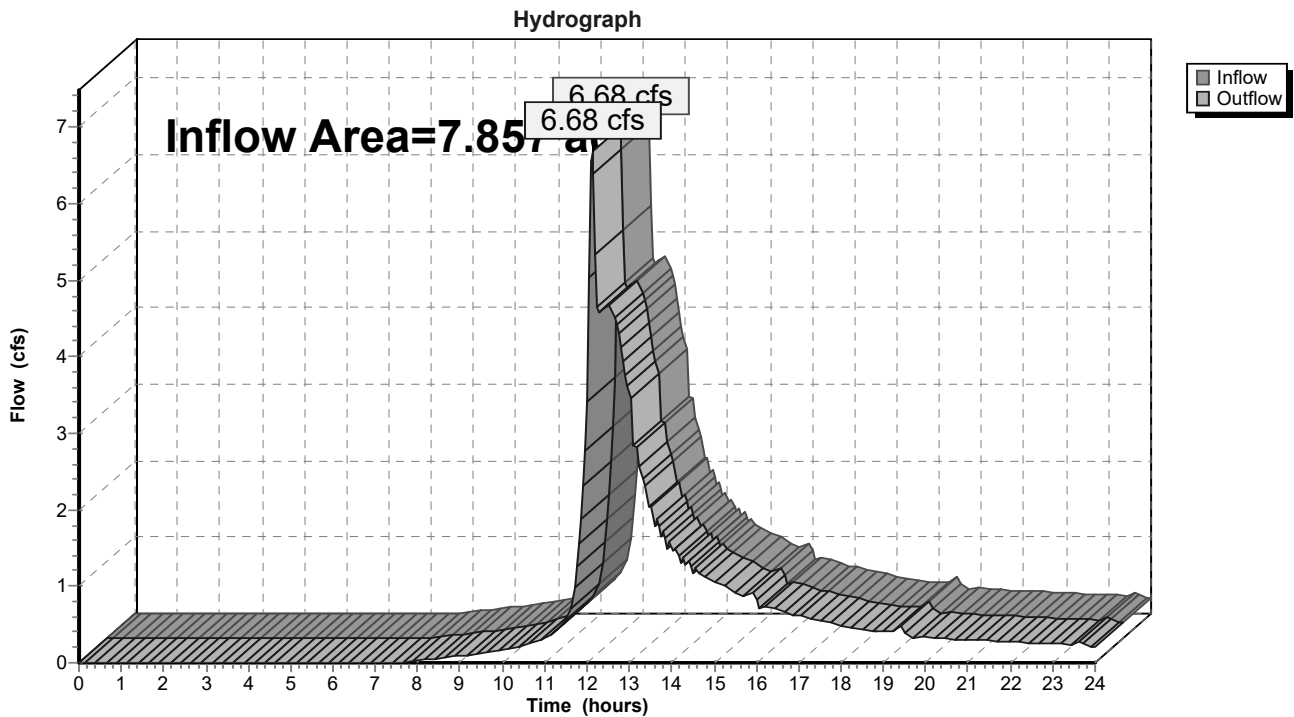
**Summary for Reach 300R: Final Reach #300**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 7.857 ac, 13.25% Impervious, Inflow Depth > 1.71" for 25YR-24HR event  
Inflow = 6.68 cfs @ 12.12 hrs, Volume= 1.117 af  
Outflow = 6.68 cfs @ 12.12 hrs, Volume= 1.117 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

**Reach 300R: Final Reach #300**



**23-017 Existing Analysis Ex TCAM**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 34

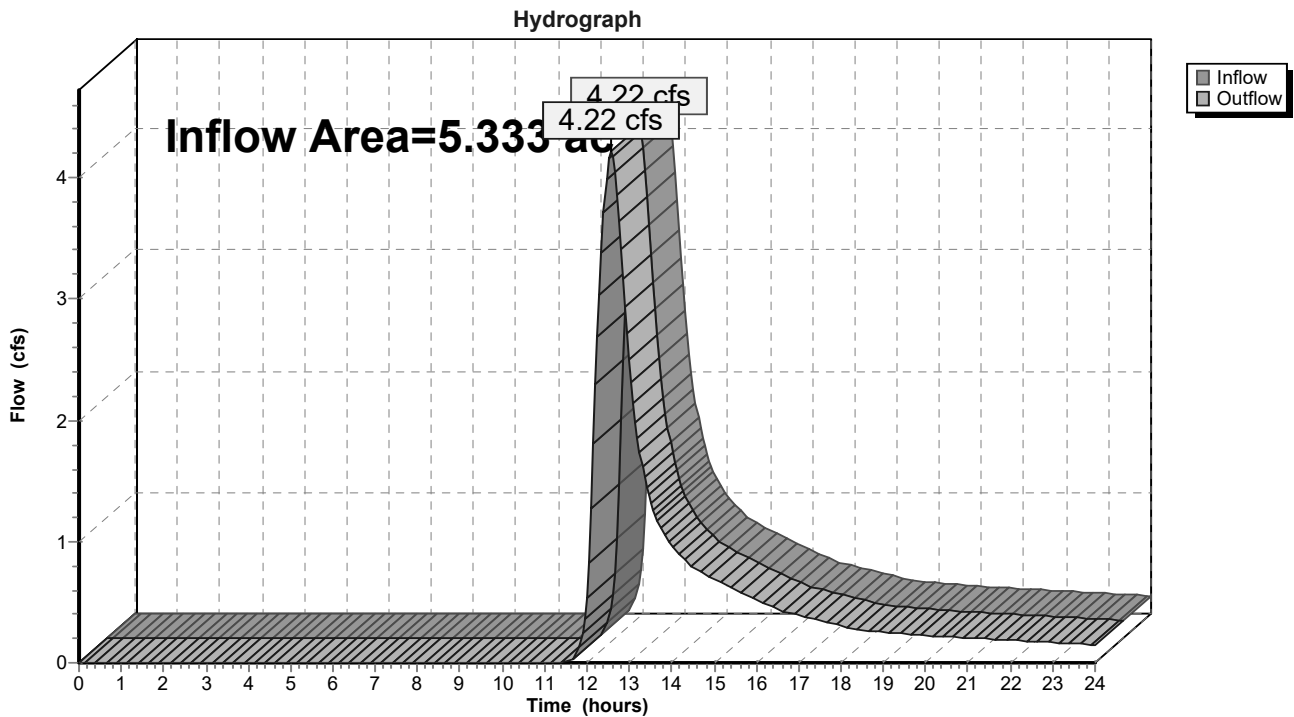
**Summary for Reach 400R: Final Reach #400**

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 5.333 ac, 33.23% Impervious, Inflow Depth > 1.49" for 25YR-24HR event  
Inflow = 4.22 cfs @ 12.55 hrs, Volume= 0.663 af  
Outflow = 4.22 cfs @ 12.55 hrs, Volume= 0.663 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

**Reach 400R: Final Reach #400**



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 35

**Summary for Pond 30P: Existing Infiltration/Trench**

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=43)

Inflow Area = 1.339 ac, 10.64% Impervious, Inflow Depth > 2.25" for 25YR-24HR event  
 Inflow = 2.87 cfs @ 12.17 hrs, Volume= 0.251 af  
 Outflow = 1.47 cfs @ 13.00 hrs, Volume= 0.242 af, Atten= 49%, Lag= 49.9 min  
 Discarded = 0.93 cfs @ 12.59 hrs, Volume= 0.189 af  
 Primary = 0.65 cfs @ 13.00 hrs, Volume= 0.053 af  
 Secondary = 0.01 cfs @ 12.65 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 183.96' @ 12.59 hrs Surf.Area= 13,324 sf Storage= 3,170 cf  
 Flood Elev= 184.10' Surf.Area= 14,890 sf Storage= 5,159 cf

Plug-Flow detention time= 47.6 min calculated for 0.242 af (96% of inflow)  
 Center-of-Mass det. time= 28.2 min ( 883.1 - 854.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	183.50'	6,797 cf	<b>Ponding Area (Irregular)</b> Listed below (Recalc)
#2	182.75'	468 cf	<b>Stone Trench (Irregular)</b> Listed below (Recalc) 533 cf Overall - 65 cf Embedded = 468 cf
#3	183.15'	48 cf	<b>6.0" Round Pipe Storage</b> Inside #2 L= 244.0' 65 cf Overall - 0.5" Wall Thickness = 48 cf
		7,313 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
183.50	538	154.3	0	0	538
183.75	6,179	527.1	712	712	20,753
184.00	14,357	677.3	2,496	3,208	35,149
184.25	14,357	677.3	3,589	6,797	35,319

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
182.75	533	495.9	0	0	533
183.75	533	495.9	533	533	1,029

Device	Routing	Invert	Outlet Devices
#1	Discarded	182.75'	<b>3.000 in/hr Infiltration over Surface area</b>
#2	Primary	183.15'	<b>6.0" Round 6" HDPE N-12</b> L= 1.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 183.15' / 183.15' S= 0.0000 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#3	Secondary	183.95'	<b>10.0' long x 10.0' breadth Overflow to Wetland</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

**23-017 Existing Analysis Ex TCAM**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 36

**Discarded OutFlow** Max=0.92 cfs @ 12.59 hrs HW=183.96' (Free Discharge)

↳ **1=Infiltration** (Exfiltration Controls 0.92 cfs)

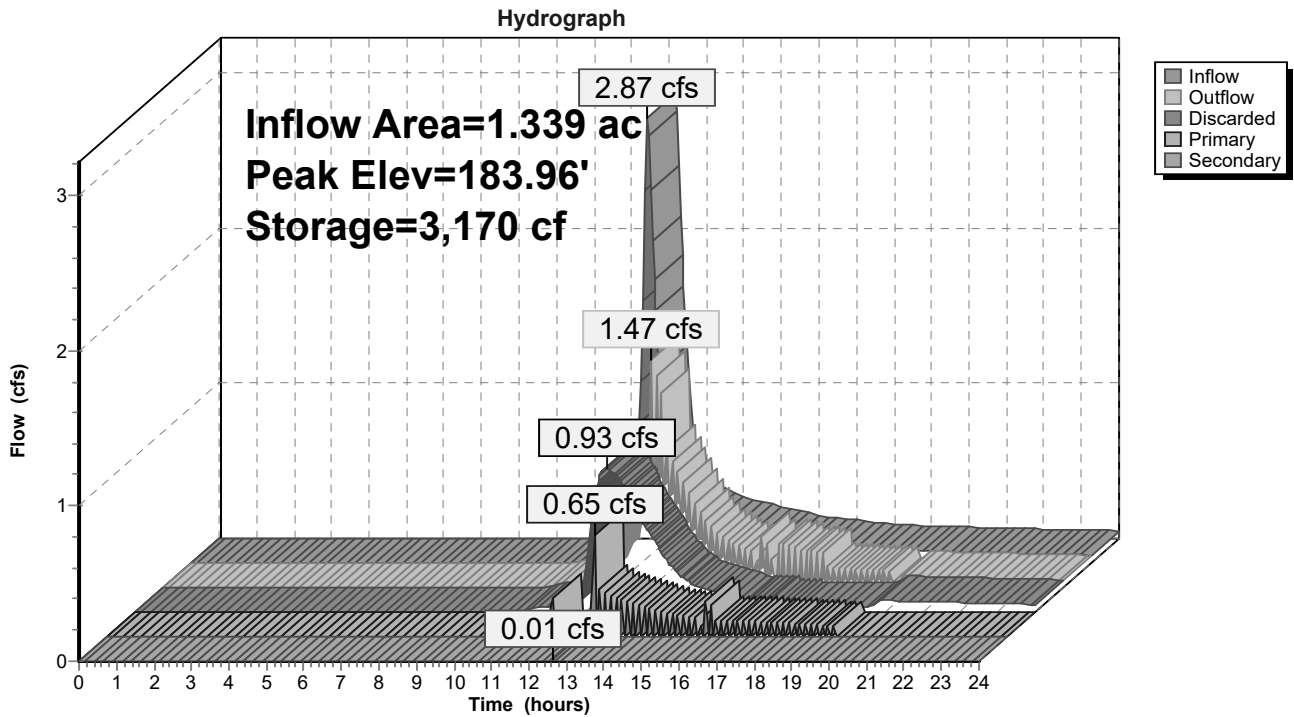
**Primary OutFlow** Max=0.00 cfs @ 13.00 hrs HW=183.92' TW=184.30' (Dynamic Tailwater)

↳ **2=6" HDPE N-12** ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 12.65 hrs HW=183.96' TW=183.96' (Dynamic Tailwater)

↳ **3=Overflow to Wetland** ( Controls 0.00 cfs)

**Pond 30P: Existing Infiltration/Trench**



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 37

**Summary for Pond 70P: Existing Catch Basin**

Inflow Area = 5.333 ac, 33.23% Impervious, Inflow Depth > 1.49" for 25YR-24HR event  
 Inflow = 4.22 cfs @ 12.55 hrs, Volume= 0.663 af  
 Outflow = 4.22 cfs @ 12.55 hrs, Volume= 0.663 af, Atten= 0%, Lag= 0.0 min  
 Primary = 4.22 cfs @ 12.55 hrs, Volume= 0.663 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 181.13' @ 12.55 hrs Surf.Area= 0.000 ac Storage= 0.000 af  
 Flood Elev= 185.00' Surf.Area= 0.000 ac Storage= 0.001 af

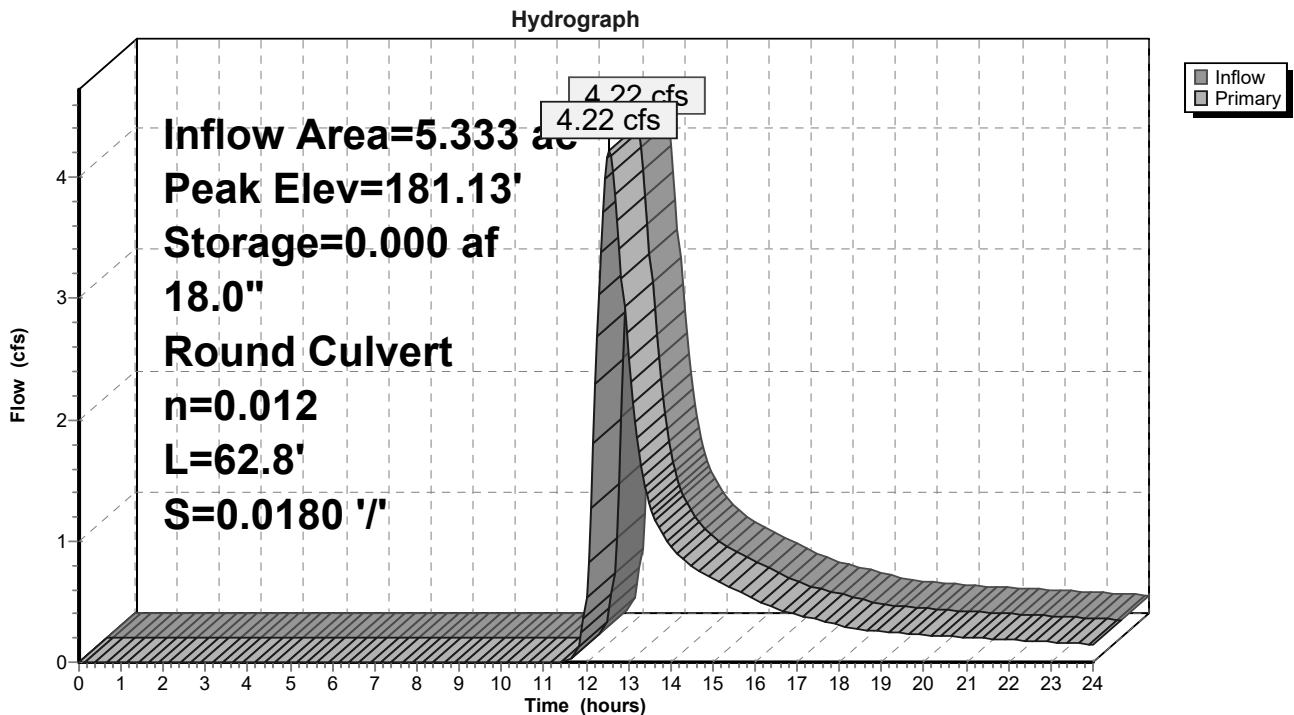
Plug-Flow detention time= 0.1 min calculated for 0.663 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 889.9 - 889.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	180.14'	0.001 af	<b>4.00'D x 3.60'H Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	180.14'	<b>18.0" Round 18" RCP</b> L= 62.8' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 180.14' / 179.01' S= 0.0180 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

**Primary OutFlow** Max=4.22 cfs @ 12.55 hrs HW=181.13' TW=0.00' (Dynamic Tailwater)  
 ↳ 1=18" RCP (Inlet Controls 4.22 cfs @ 3.39 fps)

**Pond 70P: Existing Catch Basin**



# 23-017 Existing Analysis Ex TCAM

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Type III 24-hr 25YR-24HR Rainfall=5.87"

Printed 4/17/2024

Page 38

## Summary for Pond 71P: Existing Catch Basin

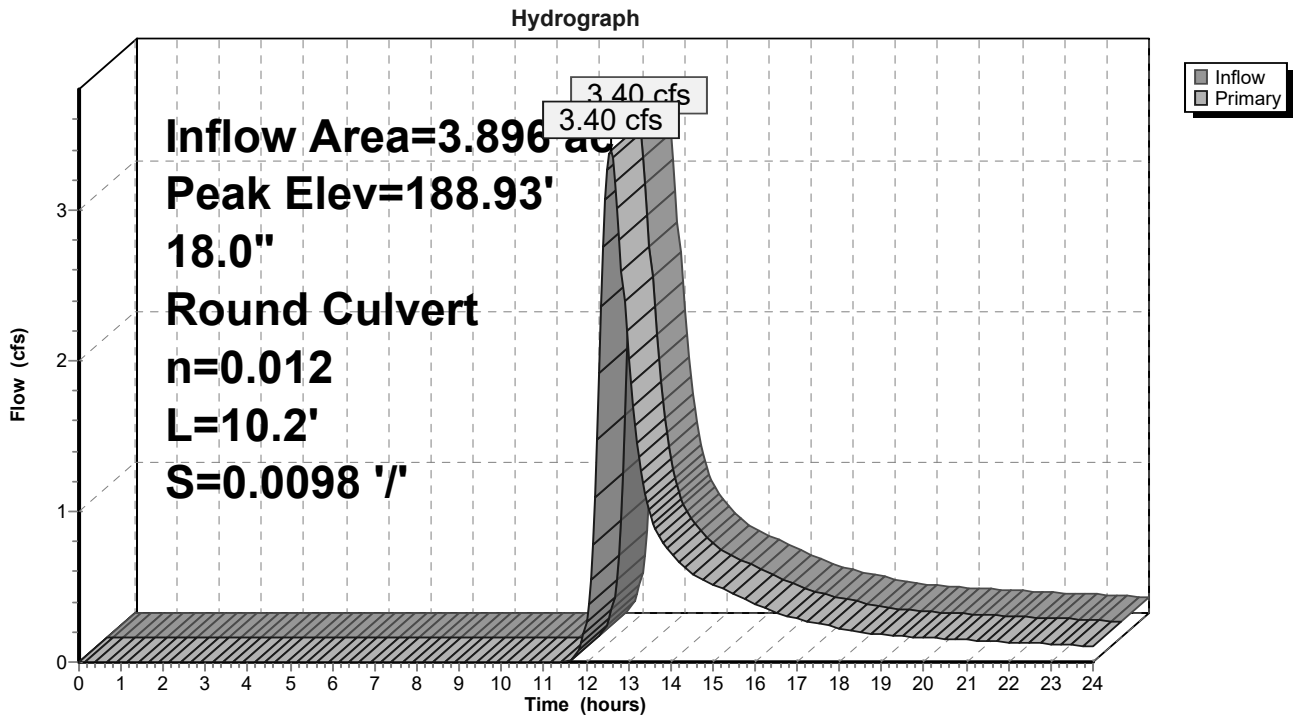
Inflow Area = 3.896 ac, 34.84% Impervious, Inflow Depth > 1.52" for 25YR-24HR event  
Inflow = 3.40 cfs @ 12.58 hrs, Volume= 0.492 af  
Outflow = 3.40 cfs @ 12.58 hrs, Volume= 0.492 af, Atten= 0%, Lag= 0.0 min  
Primary = 3.40 cfs @ 12.58 hrs, Volume= 0.492 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Peak Elev= 188.93' @ 12.58 hrs  
Flood Elev= 192.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	187.90'	<b>18.0" Round 18" RCP</b> L= 10.2' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 187.90' / 187.80' S= 0.0098 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.39 cfs @ 12.58 hrs HW=188.92' TW=188.26' (Dynamic Tailwater)  
↑1=18" RCP (Barrel Controls 3.39 cfs @ 3.72 fps)

## Pond 71P: Existing Catch Basin





**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 39

**Summary for Pond 72P: Existing Depression**

[58] Hint: Peaked 0.21' above defined flood level

Inflow Area = 1.582 ac, 42.73% Impervious, Inflow Depth > 2.07" for 25YR-24HR event  
 Inflow = 2.04 cfs @ 12.48 hrs, Volume= 0.273 af  
 Outflow = 2.02 cfs @ 12.49 hrs, Volume= 0.271 af, Atten= 1%, Lag= 0.5 min  
 Discarded = 0.02 cfs @ 11.95 hrs, Volume= 0.024 af  
 Primary = 2.00 cfs @ 12.49 hrs, Volume= 0.247 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 196.21' @ 12.59 hrs Surf.Area= 333 sf Storage= 160 cf  
 Flood Elev= 196.00' Surf.Area= 333 sf Storage= 91 cf

Plug-Flow detention time= 6.1 min calculated for 0.270 af (99% of inflow)  
 Center-of-Mass det. time= 2.2 min ( 878.0 - 875.8 )

Volume	Invert	Avail.Storage	Storage Description			
#1	195.50'	257 cf	<b>Ponding Area (Irregular)</b> Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
195.50	55	32.6	0	0	55	
195.75	179	63.6	28	28	293	
196.00	333	92.1	63	91	646	
196.50	333	92.1	167	257	692	

Device	Routing	Invert	Outlet Devices									
#1	Discarded	195.50'	<b>3.000 in/hr Infiltration over Surface area</b>									
#2	Primary	195.95'	<b>20.0' long x 50.0' breadth Overflow over DW</b>									
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60									
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63									

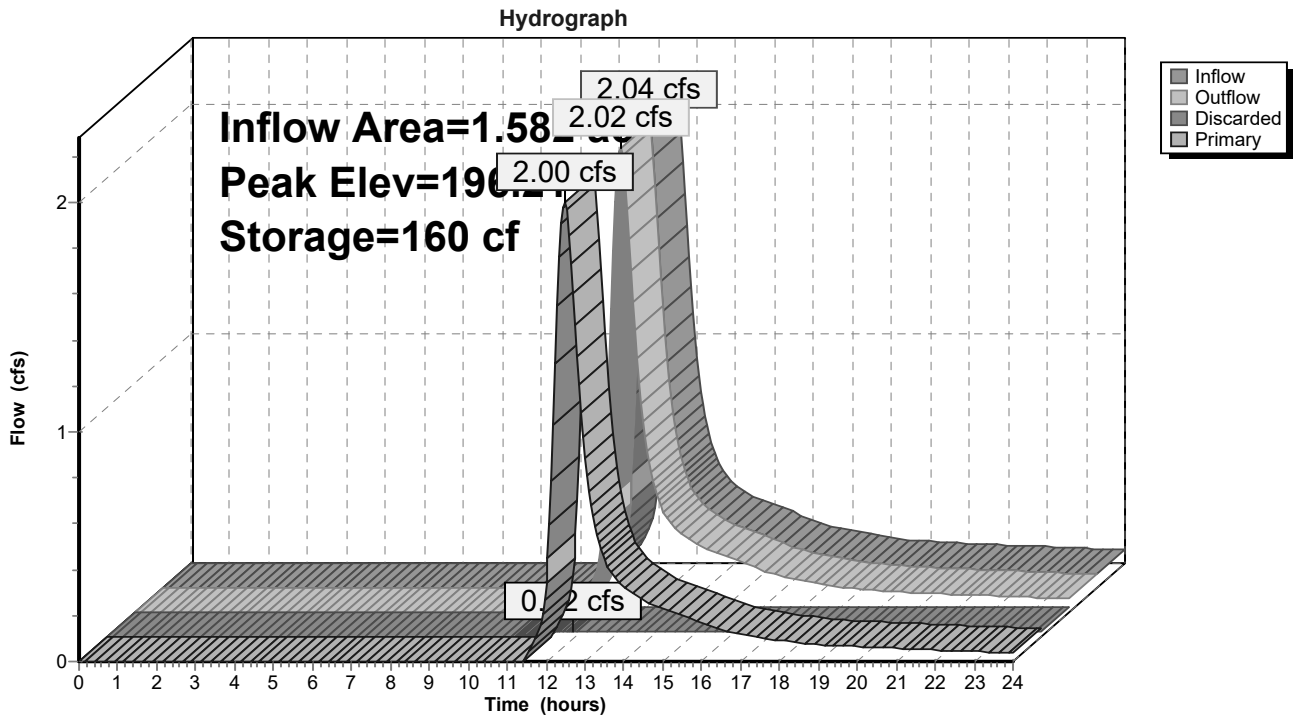
**Discarded OutFlow** Max=0.02 cfs @ 11.95 hrs HW=196.00' (Free Discharge)

↳ **1=Infiltration** (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=0.00 cfs @ 12.49 hrs HW=196.20' TW=196.20' (Dynamic Tailwater)

↳ **2=Overflow over DW** ( Controls 0.00 cfs)

### Pond 72P: Existing Depression



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 41

**Summary for Pond E01P: Existing Catch Basin**

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=126)

Inflow Area = 0.452 ac, 56.06% Impervious, Inflow Depth > 3.86" for 25YR-24HR event  
 Inflow = 1.99 cfs @ 12.09 hrs, Volume= 0.145 af  
 Outflow = 1.92 cfs @ 12.08 hrs, Volume= 0.145 af, Atten= 3%, Lag= 0.0 min  
 Primary = 1.92 cfs @ 12.08 hrs, Volume= 0.145 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 185.04' @ 12.16 hrs Surf.Area= 13 sf Storage= 19 cf  
 Flood Elev= 190.33' Surf.Area= 13 sf Storage= 86 cf

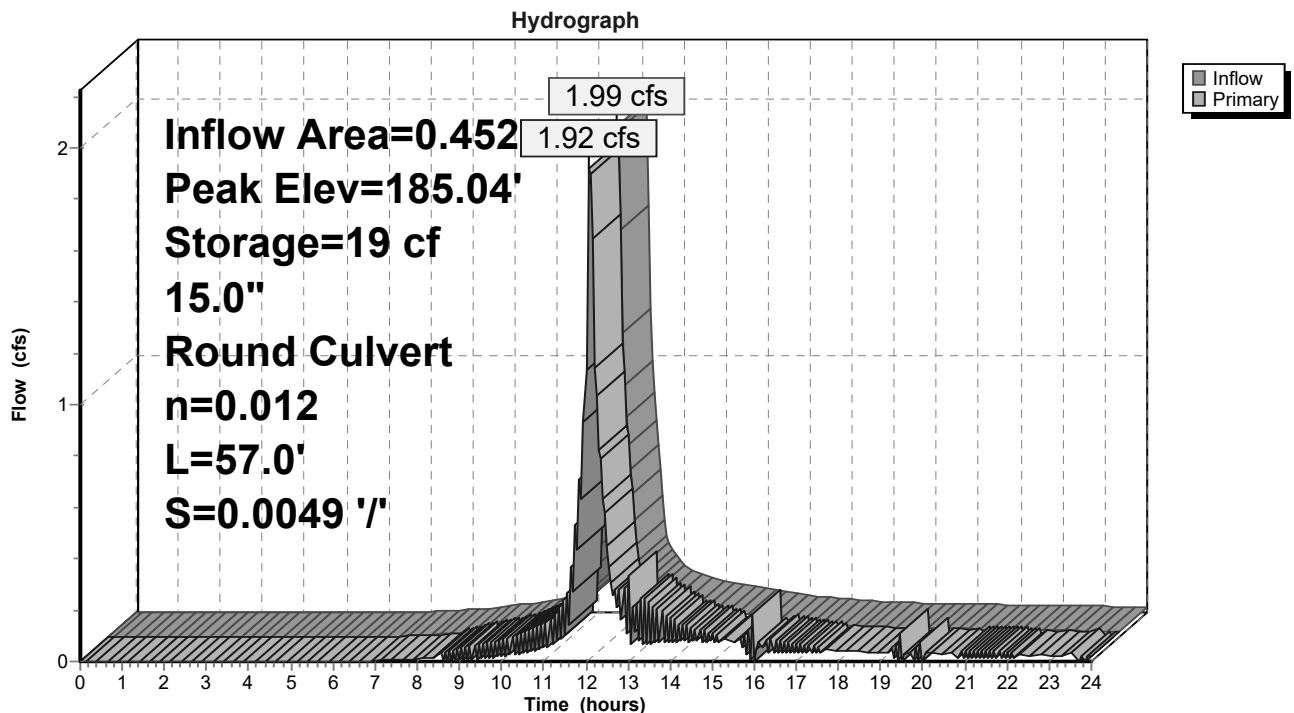
Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.6 min ( 809.8 - 809.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	183.50'	86 cf	<b>4.00'D x 6.83'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	183.50'	<b>15.0" Round 15" HDPE N-12</b> L= 57.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 183.50' / 183.22' S= 0.0049 1/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

**Primary OutFlow** Max=0.00 cfs @ 12.08 hrs HW=184.82' TW=184.88' (Dynamic Tailwater)  
 ←1=15" HDPE N-12 ( Controls 0.00 cfs)

**Pond E01P: Existing Catch Basin**



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 42

**Summary for Pond E02P: Existing Catch Basin**

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=73)

[80] Warning: Exceeded Pond E01P by 0.57' @ 16.00 hrs (1.08 cfs 0.160 af)

Inflow Area = 1.376 ac, 60.99% Impervious, Inflow Depth > 4.00" for 25YR-24HR event  
 Inflow = 6.18 cfs @ 12.09 hrs, Volume= 0.459 af  
 Outflow = 6.73 cfs @ 12.10 hrs, Volume= 0.459 af, Atten= 0%, Lag= 0.6 min  
 Discarded = 0.01 cfs @ 12.12 hrs, Volume= 0.006 af  
 Primary = 6.73 cfs @ 12.10 hrs, Volume= 0.453 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 184.97' @ 12.11 hrs Surf.Area= 116 sf Storage= 135 cf  
 Flood Elev= 189.42' Surf.Area= 0 sf Storage= 464 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 1.2 min ( 806.9 - 805.7 )

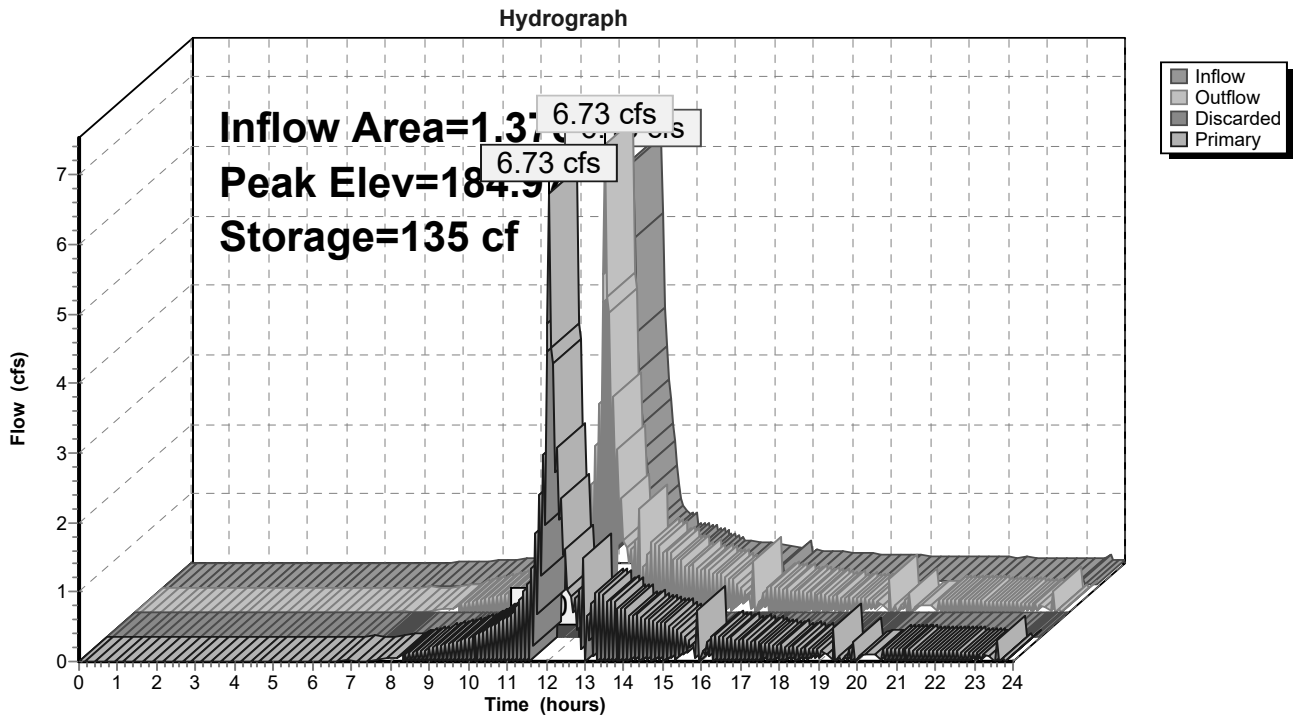
Volume	Invert	Avail.Storage	Storage Description
#1	183.02'	80 cf	<b>4.00'D x 6.40'H 4' Structure-Impervious</b>
#2	183.02'	384 cf	<b>24.0" Round 24" HDPE N-12 Perf</b> L= 122.2' S= 0.0270 '/'
		464 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	183.02'	<b>24.0" Round 24" HDPE N-12</b> L= 122.2' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 183.02' / 179.71' S= 0.0271 '/' Cc= 0.900 n= 0.012, Flow Area= 3.14 sf
#2	Discarded	183.02'	<b>3.000 in/hr Infiltration over Surface area</b>

**Discarded OutFlow** Max=0.01 cfs @ 12.12 hrs HW=184.94' (Free Discharge)  
 ↳ **2=Infiltration** (Exfiltration Controls 0.01 cfs)

**Primary OutFlow** Max=5.53 cfs @ 12.10 hrs HW=184.95' TW=184.75' (Dynamic Tailwater)  
 ↳ **1=24" HDPE N-12** (Outlet Controls 5.53 cfs @ 2.27 fps)

### Pond E02P: Existing Catch Basin



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 44

**Summary for Pond E03P: Existing Catch Basin**

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=2)

[80] Warning: Exceeded Pond 30P by 0.76' @ 11.90 hrs (0.83 cfs 0.353 af)

Inflow Area = 1.339 ac, 10.64% Impervious, Inflow Depth = 0.47" for 25YR-24HR event  
 Inflow = 0.65 cfs @ 13.00 hrs, Volume= 0.053 af  
 Outflow = 0.42 cfs @ 13.01 hrs, Volume= 0.052 af, Atten= 35%, Lag= 0.5 min  
 Primary = 0.42 cfs @ 13.01 hrs, Volume= 0.052 af  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 184.31' @ 11.90 hrs Surf.Area= 13 sf Storage= 60 cf  
 Flood Elev= 184.49' Surf.Area= 17 sf Storage= 62 cf

Plug-Flow detention time= 10.7 min calculated for 0.052 af (98% of inflow)  
 Center-of-Mass det. time= 5.2 min ( 917.4 - 912.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	179.56'	62 cf	<b>4.00'D x 4.93'H 4' Structure</b>
#2	184.49'	21 cf	<b>Ponding Area (Irregular)</b> Listed below (Recalc)
		83 cf	Total Available Storage

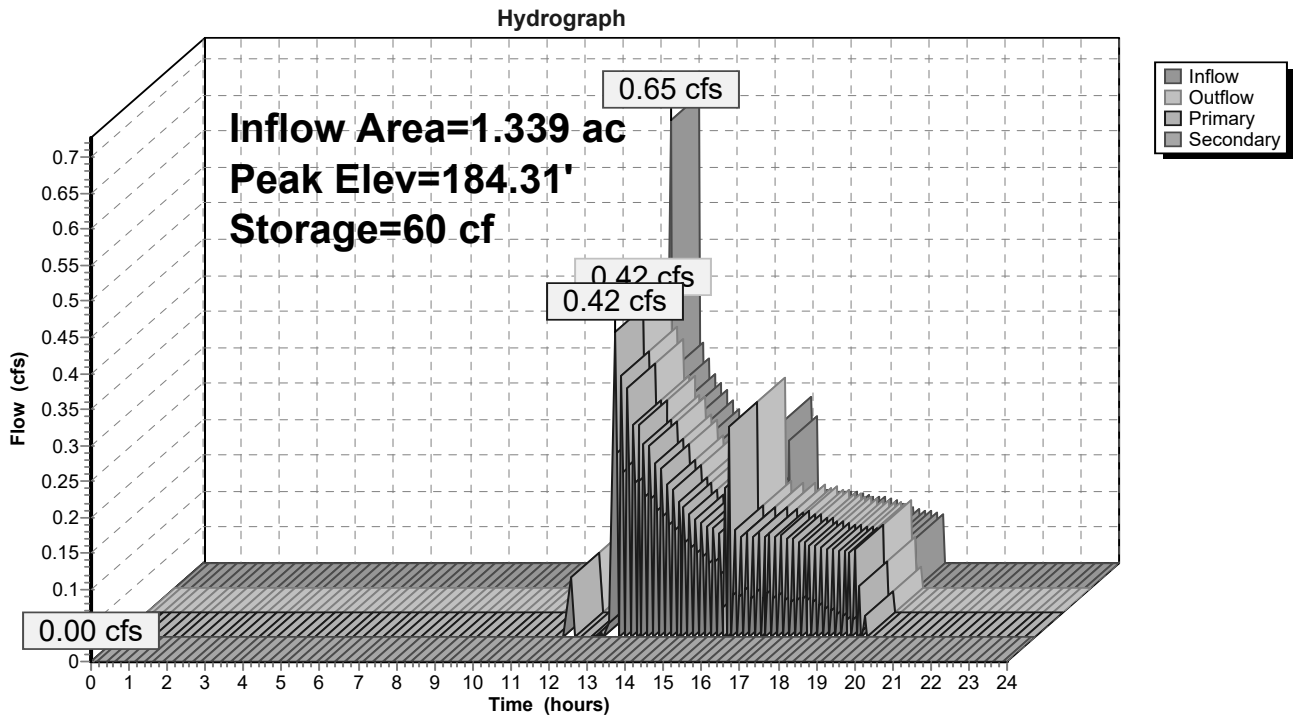
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
184.49	4	4.0	0	0	4
185.00	100	20.0	21	21	35

Device	Routing	Invert	Outlet Devices
#1	Primary	179.56'	<b>15.0" Round 15" HDPE N-12</b> L= 36.8' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 179.56' / 179.56' S= 0.0000 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf
#2	Secondary	184.49'	<b>4.0' long x 2.0' breadth Overflow</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

**Primary OutFlow** Max=2.10 cfs @ 13.01 hrs HW=184.28' TW=184.15' (Dynamic Tailwater)  
 ↖1=15" HDPE N-12 (Inlet Controls 2.10 cfs @ 1.71 fps)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=179.56' TW=184.49' (Dynamic Tailwater)  
 ↖2=Overflow ( Controls 0.00 cfs)

### Pond E03P: Existing Catch Basin



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 46

**Summary for Pond E04P: Existing Catch Basin**

[58] Hint: Peaked 0.76' above defined flood level

[80] Warning: Exceeded Pond E02P by 1.12' @ 13.00 hrs (6.11 cfs 0.885 af)

[80] Warning: Exceeded Pond E03P by 4.72' @ 11.70 hrs (11.97 cfs 7.920 af)

Inflow Area = 2.759 ac, 35.58% Impervious, Inflow Depth > 2.23" for 25YR-24HR event  
 Inflow = 6.85 cfs @ 12.10 hrs, Volume= 0.514 af  
 Outflow = 6.40 cfs @ 12.10 hrs, Volume= 0.513 af, Atten= 7%, Lag= 0.0 min  
 Primary = 6.40 cfs @ 12.10 hrs, Volume= 0.513 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 184.75' @ 12.10 hrs Surf.Area= 76 sf Storage= 75 cf  
 Flood Elev= 183.99' Surf.Area= 17 sf Storage= 54 cf

Plug-Flow detention time= 2.5 min calculated for 0.513 af (100% of inflow)  
 Center-of-Mass det. time= 1.1 min ( 818.3 - 817.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	179.71'	54 cf	<b>4.00'D x 4.28'H 4' Structure</b>
#2	183.99'	41 cf	<b>Flood Storage (Irregular)</b> Listed below (Recalc)
		95 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
183.99	4	8.0	0	0	4
185.00	98	45.2	41	41	164

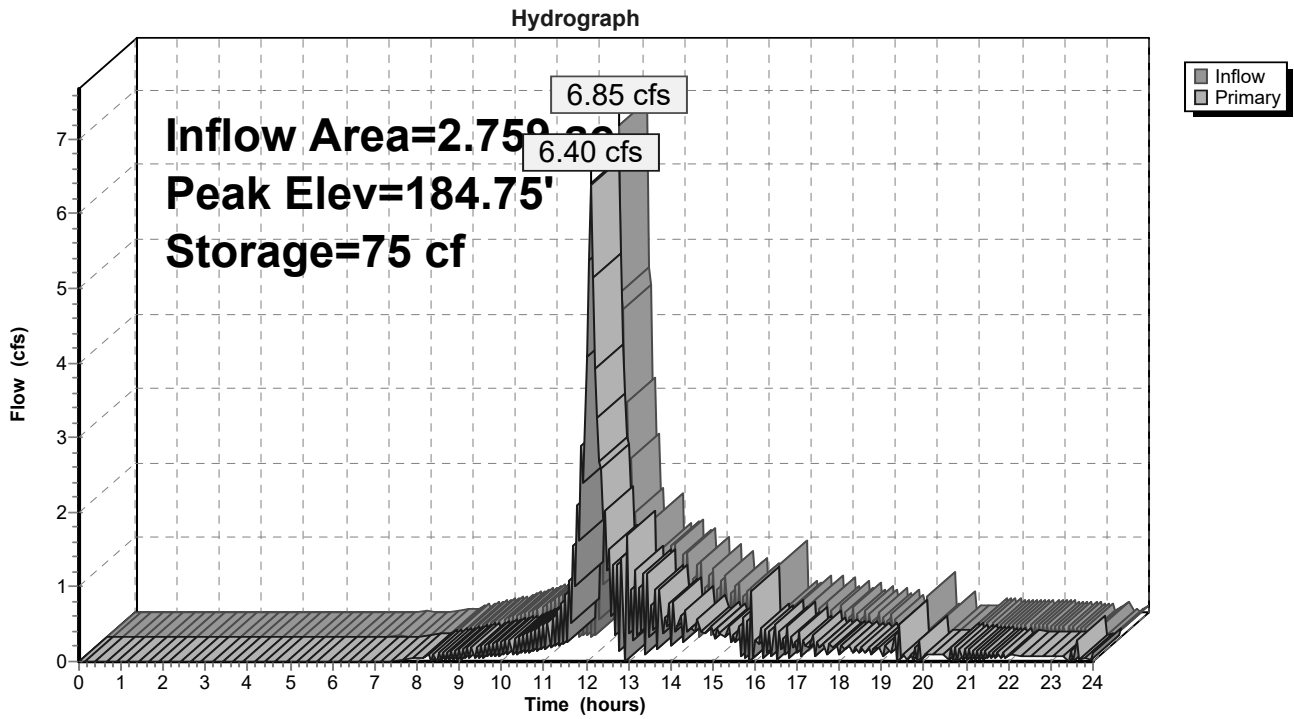
Device	Routing	Invert	Outlet Devices
#1	Primary	183.99'	<b>4.0' long x 2.0' breadth Overflow</b>
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.20 3.32

**Primary OutFlow** Max=6.31 cfs @ 12.10 hrs HW=184.75' TW=184.24' (Dynamic Tailwater)

←1=Overflow (Weir Controls 6.31 cfs @ 2.08 fps)



### Pond E04P: Existing Catch Basin



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 2YR-24HR Rainfall=3.08"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 1

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 2S: Subcat #2</b>	Runoff Area=163,452 sf 2.33% Impervious Runoff Depth>0.39" Flow Length=301' Tc=15.8 min CN=61 Runoff=0.78 cfs 0.123 af
<b>Subcatchment 3S: Subcat #3</b>	Runoff Area=222,064 sf 1.16% Impervious Runoff Depth>0.21" Flow Length=682' Tc=43.7 min CN=55 Runoff=0.26 cfs 0.090 af
<b>Subcatchment 30S: Subcat #30</b>	Runoff Area=58,317 sf 10.64% Impervious Runoff Depth>0.54" Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=65 Runoff=0.53 cfs 0.060 af
<b>Subcatchment 31S: Subcat #31</b>	Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>1.44" Tc=6.0 min CN=82 Runoff=0.75 cfs 0.054 af
<b>Subcatchment 32S: Subcat #32</b>	Runoff Area=40,270 sf 63.40% Impervious Runoff Depth>1.58" Tc=6.0 min CN=84 Runoff=1.68 cfs 0.122 af
<b>Subcatchment 34S: Subcat #34</b>	Runoff Area=1,936 sf 0.00% Impervious Runoff Depth>0.67" Tc=6.0 min CN=68 Runoff=0.03 cfs 0.002 af
<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=62,561 sf 28.86% Impervious Runoff Depth>0.21" Flow Length=380' Tc=15.2 min CN=55 Runoff=0.11 cfs 0.026 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>0.16" Flow Length=563' Tc=39.5 min CN=53 Runoff=0.07 cfs 0.032 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>0.46" Flow Length=478' Tc=32.0 min CN=63 Runoff=0.34 cfs 0.061 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=43.0' S=0.0105 '/' Capacity=16.58 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.030 L=63.5' S=0.1339 '/' Capacity=43.48 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.030 L=230.5' S=0.0130 '/' Capacity=13.56 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30dR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.030 L=11.0' S=0.1364 '/' Capacity=43.88 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 33aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.016 L=50.0' S=0.0198 '/' Capacity=31.35 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 34aR: Overland Flow</b>	Avg. Flow Depth=0.16' Max Vel=2.51 fps Inflow=2.56 cfs 0.186 af n=0.016 L=35.0' S=0.0140 '/' Capacity=26.36 cfs Outflow=2.37 cfs 0.187 af
<b>Reach 34bR: Overland Flow</b>	Avg. Flow Depth=0.18' Max Vel=2.25 fps Inflow=2.37 cfs 0.187 af n=0.016 L=194.0' S=0.0103 '/' Capacity=22.62 cfs Outflow=2.36 cfs 0.186 af

**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 2YR-24HR Rainfall=3.08"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 2

<b>Reach 34cR: Overland Flow</b>	Avg. Flow Depth=0.13' Max Vel=3.75 fps Inflow=2.36 cfs 0.186 af n=0.030 L=36.0' S=0.1597 '/ Capacity=47.49 cfs Outflow=2.36 cfs 0.186 af
<b>Reach 34dR: Overland Flow</b>	Avg. Flow Depth=0.21' Max Vel=1.73 fps Inflow=2.36 cfs 0.186 af n=0.030 L=43.0' S=0.0174 '/ Capacity=15.69 cfs Outflow=2.33 cfs 0.186 af
<b>Reach 71aR: Wooded Swale</b>	Avg. Flow Depth=0.16' Max Vel=0.76 fps Inflow=0.34 cfs 0.070 af n=0.035 L=125.0' S=0.0064 '/ Capacity=79.88 cfs Outflow=0.34 cfs 0.069 af
<b>Reach 71bR: Riprap Swale</b>	Avg. Flow Depth=0.12' Max Vel=0.79 fps Inflow=0.34 cfs 0.069 af n=0.041 L=147.7' S=0.0135 '/ Capacity=31.94 cfs Outflow=0.33 cfs 0.069 af
<b>Reach 72R: Roadside Swale</b>	Avg. Flow Depth=0.09' Max Vel=0.92 fps Inflow=0.31 cfs 0.038 af n=0.022 L=495.6' S=0.0060 '/ Capacity=33.12 cfs Outflow=0.28 cfs 0.038 af
<b>Reach 200R: Final Reach #200</b>	Inflow=0.78 cfs 0.123 af Outflow=0.78 cfs 0.123 af
<b>Reach 300R: Final Reach #300</b>	Inflow=2.33 cfs 0.276 af Outflow=2.33 cfs 0.276 af
<b>Reach 400R: Final Reach #400</b>	Inflow=0.40 cfs 0.095 af Outflow=0.40 cfs 0.095 af
<b>Pond 30P: Existing Infiltration/Trench</b>	Peak Elev=183.58' Storage=516 cf Inflow=0.53 cfs 0.060 af Discarded=0.15 cfs 0.043 af Primary=0.28 cfs 0.015 af Secondary=0.00 cfs 0.000 af Outflow=0.42 cfs 0.059 af
<b>Pond 70P: Existing Catch Basin</b>	Peak Elev=180.42' Storage=0.000 af Inflow=0.40 cfs 0.095 af 18.0" Round Culvert n=0.012 L=62.8' S=0.0180 '/ Outflow=0.40 cfs 0.095 af
<b>Pond 71P: Existing Catch Basin</b>	Peak Elev=188.18' Inflow=0.34 cfs 0.070 af 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/ Outflow=0.34 cfs 0.070 af
<b>Pond 72P: Existing Depression</b>	Peak Elev=196.04' Storage=103 cf Inflow=0.34 cfs 0.061 af Discarded=0.02 cfs 0.021 af Primary=0.31 cfs 0.038 af Outflow=0.33 cfs 0.059 af
<b>Pond E01P: Existing Catch Basin</b>	Peak Elev=184.47' Storage=12 cf Inflow=0.75 cfs 0.054 af 15.0" Round Culvert n=0.012 L=57.0' S=0.0049 '/ Outflow=0.82 cfs 0.054 af
<b>Pond E02P: Existing Catch Basin</b>	Peak Elev=184.45' Storage=75 cf Inflow=2.50 cfs 0.176 af Discarded=0.01 cfs 0.005 af Primary=2.67 cfs 0.171 af Outflow=2.68 cfs 0.176 af
<b>Pond E03P: Existing Catch Basin</b>	Peak Elev=184.28' Storage=59 cf Inflow=0.28 cfs 0.015 af Primary=0.23 cfs 0.014 af Secondary=0.00 cfs 0.000 af Outflow=0.23 cfs 0.014 af
<b>Pond E04P: Existing Catch Basin</b>	Peak Elev=184.40' Storage=59 cf Inflow=2.69 cfs 0.187 af Outflow=2.56 cfs 0.186 af

**Total Runoff Area = 16.942 ac Runoff Volume = 0.570 af Average Runoff Depth = 0.40"**  
**82.88% Pervious = 14.041 ac 17.12% Impervious = 2.901 ac**

**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 10YR-24HR Rainfall=4.65"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 3

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 2S: Subcat #2</b>	Runoff Area=163,452 sf 2.33% Impervious Runoff Depth>1.16" Flow Length=301' Tc=15.8 min CN=61 Runoff=3.31 cfs 0.362 af
<b>Subcatchment 3S: Subcat #3</b>	Runoff Area=222,064 sf 1.16% Impervious Runoff Depth>0.80" Flow Length=682' Tc=43.7 min CN=55 Runoff=1.76 cfs 0.339 af
<b>Subcatchment 30S: Subcat #30</b>	Runoff Area=58,317 sf 10.64% Impervious Runoff Depth>1.42" Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=65 Runoff=1.73 cfs 0.159 af
<b>Subcatchment 31S: Subcat #31</b>	Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>2.77" Tc=6.0 min CN=82 Runoff=1.43 cfs 0.104 af
<b>Subcatchment 32S: Subcat #32</b>	Runoff Area=40,270 sf 63.40% Impervious Runoff Depth>2.95" Tc=6.0 min CN=84 Runoff=3.12 cfs 0.227 af
<b>Subcatchment 34S: Subcat #34</b>	Runoff Area=1,936 sf 0.00% Impervious Runoff Depth>1.63" Tc=6.0 min CN=68 Runoff=0.08 cfs 0.006 af
<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=62,561 sf 28.86% Impervious Runoff Depth>0.81" Flow Length=380' Tc=15.2 min CN=55 Runoff=0.76 cfs 0.097 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>0.69" Flow Length=563' Tc=39.5 min CN=53 Runoff=0.68 cfs 0.134 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>1.28" Flow Length=478' Tc=32.0 min CN=63 Runoff=1.20 cfs 0.169 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=43.0' S=0.0105 '/' Capacity=16.58 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.030 L=63.5' S=0.1339 '/' Capacity=43.48 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.030 L=230.5' S=0.0130 '/' Capacity=13.56 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30dR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.030 L=11.0' S=0.1364 '/' Capacity=43.88 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 33aR: Overland Flow</b>	Avg. Flow Depth=0.03' Max Vel=1.01 fps Inflow=0.08 cfs 0.001 af n=0.016 L=50.0' S=0.0198 '/' Capacity=31.35 cfs Outflow=0.08 cfs 0.001 af
<b>Reach 34aR: Overland Flow</b>	Avg. Flow Depth=0.22' Max Vel=3.09 fps Inflow=4.93 cfs 0.356 af n=0.016 L=35.0' S=0.0140 '/' Capacity=26.36 cfs Outflow=4.62 cfs 0.357 af
<b>Reach 34bR: Overland Flow</b>	Avg. Flow Depth=0.24' Max Vel=2.76 fps Inflow=4.66 cfs 0.358 af n=0.016 L=194.0' S=0.0103 '/' Capacity=22.62 cfs Outflow=4.60 cfs 0.357 af

**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 10YR-24HR Rainfall=4.65"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 4

**Reach 34cR: Overland Flow** Avg. Flow Depth=0.17' Max Vel=4.62 fps Inflow=4.60 cfs 0.357 af  
 n=0.030 L=36.0' S=0.1597 '/ Capacity=47.49 cfs Outflow=4.61 cfs 0.357 af

**Reach 34dR: Overland Flow** Avg. Flow Depth=0.28' Max Vel=2.14 fps Inflow=4.61 cfs 0.357 af  
 n=0.030 L=43.0' S=0.0174 '/ Capacity=15.69 cfs Outflow=4.60 cfs 0.357 af

**Reach 71aR: Wooded Swale** Avg. Flow Depth=0.34' Max Vel=1.26 fps Inflow=1.78 cfs 0.277 af  
 n=0.035 L=125.0' S=0.0064 '/ Capacity=79.88 cfs Outflow=1.78 cfs 0.277 af

**Reach 71bR: Riprap Swale** Avg. Flow Depth=0.26' Max Vel=1.32 fps Inflow=1.78 cfs 0.277 af  
 n=0.041 L=147.7' S=0.0135 '/ Capacity=31.94 cfs Outflow=1.77 cfs 0.277 af

**Reach 72R: Roadside Swale** Avg. Flow Depth=0.19' Max Vel=1.45 fps Inflow=1.16 cfs 0.144 af  
 n=0.022 L=495.6' S=0.0060 '/ Capacity=33.12 cfs Outflow=1.12 cfs 0.144 af

**Reach 200R: Final Reach #200** Inflow=3.31 cfs 0.362 af  
 Outflow=3.31 cfs 0.362 af

**Reach 300R: Final Reach #300** Inflow=4.69 cfs 0.696 af  
 Outflow=4.69 cfs 0.696 af

**Reach 400R: Final Reach #400** Inflow=2.18 cfs 0.373 af  
 Outflow=2.18 cfs 0.373 af

**Pond 30P: Existing Infiltration/Trench** Peak Elev=183.82' Storage=1,764 cf Inflow=1.73 cfs 0.159 af  
 Discarded=0.61 cfs 0.122 af Primary=0.47 cfs 0.028 af Secondary=0.00 cfs 0.000 af Outflow=0.94 cfs 0.150 af

**Pond 70P: Existing Catch Basin** Peak Elev=180.82' Storage=0.000 af Inflow=2.18 cfs 0.373 af  
 18.0" Round Culvert n=0.012 L=62.8' S=0.0180 '/ Outflow=2.18 cfs 0.373 af

**Pond 71P: Existing Catch Basin** Peak Elev=188.60' Inflow=1.78 cfs 0.277 af  
 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/ Outflow=1.78 cfs 0.277 af

**Pond 72P: Existing Depression** Peak Elev=196.14' Storage=138 cf Inflow=1.20 cfs 0.169 af  
 Discarded=0.02 cfs 0.023 af Primary=1.16 cfs 0.144 af Outflow=1.18 cfs 0.167 af

**Pond E01P: Existing Catch Basin** Peak Elev=184.80' Storage=16 cf Inflow=1.43 cfs 0.104 af  
 15.0" Round Culvert n=0.012 L=57.0' S=0.0049 '/ Outflow=1.43 cfs 0.104 af

**Pond E02P: Existing Catch Basin** Peak Elev=184.75' Storage=108 cf Inflow=4.48 cfs 0.331 af  
 Discarded=0.01 cfs 0.005 af Primary=5.01 cfs 0.325 af Outflow=5.02 cfs 0.330 af

**Pond E03P: Existing Catch Basin** Peak Elev=184.60' Storage=63 cf Inflow=0.47 cfs 0.028 af  
 Primary=0.27 cfs 0.026 af Secondary=0.08 cfs 0.001 af Outflow=0.27 cfs 0.027 af

**Pond E04P: Existing Catch Basin** Peak Elev=184.63' Storage=68 cf Inflow=5.26 cfs 0.358 af  
 Outflow=4.93 cfs 0.356 af

**Total Runoff Area = 16.942 ac Runoff Volume = 1.597 af Average Runoff Depth = 1.13"**  
**82.88% Pervious = 14.041 ac 17.12% Impervious = 2.901 ac**

**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 5

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 2S: Subcat #2</b>	Runoff Area=163,452 sf 2.33% Impervious Runoff Depth>1.91" Flow Length=301' Tc=15.8 min CN=61 Runoff=5.85 cfs 0.598 af
<b>Subcatchment 3S: Subcat #3</b>	Runoff Area=222,064 sf 1.16% Impervious Runoff Depth>1.42" Flow Length=682' Tc=43.7 min CN=55 Runoff=3.57 cfs 0.605 af
<b>Subcatchment 30S: Subcat #30</b>	Runoff Area=58,317 sf 10.64% Impervious Runoff Depth>2.25" Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=65 Runoff=2.87 cfs 0.251 af
<b>Subcatchment 31S: Subcat #31</b>	Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>3.86" Tc=6.0 min CN=82 Runoff=1.99 cfs 0.145 af
<b>Subcatchment 32S: Subcat #32</b>	Runoff Area=40,270 sf 63.40% Impervious Runoff Depth>4.07" Tc=6.0 min CN=84 Runoff=4.26 cfs 0.314 af
<b>Subcatchment 34S: Subcat #34</b>	Runoff Area=1,936 sf 0.00% Impervious Runoff Depth>2.52" Tc=6.0 min CN=68 Runoff=0.13 cfs 0.009 af
<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=62,561 sf 28.86% Impervious Runoff Depth>1.44" Flow Length=380' Tc=15.2 min CN=55 Runoff=1.58 cfs 0.172 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>1.28" Flow Length=563' Tc=39.5 min CN=53 Runoff=1.48 cfs 0.246 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>2.07" Flow Length=478' Tc=32.0 min CN=63 Runoff=2.04 cfs 0.273 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.01' Max Vel=0.28 fps Inflow=0.01 cfs 0.000 af n=0.022 L=43.0' S=0.0105 '/' Capacity=16.58 cfs Outflow=0.01 cfs 0.000 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.01' Max Vel=0.56 fps Inflow=0.01 cfs 0.000 af n=0.030 L=63.5' S=0.1339 '/' Capacity=43.48 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.01' Max Vel=0.18 fps Inflow=0.00 cfs 0.000 af n=0.030 L=230.5' S=0.0130 '/' Capacity=13.56 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30dR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.41 fps Inflow=0.00 cfs 0.000 af n=0.030 L=11.0' S=0.1364 '/' Capacity=43.88 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 33aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.016 L=50.0' S=0.0198 '/' Capacity=31.35 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 34aR: Overland Flow</b>	Avg. Flow Depth=0.25' Max Vel=3.36 fps Inflow=6.40 cfs 0.513 af n=0.016 L=35.0' S=0.0140 '/' Capacity=26.36 cfs Outflow=6.08 cfs 0.513 af
<b>Reach 34bR: Overland Flow</b>	Avg. Flow Depth=0.27' Max Vel=3.03 fps Inflow=6.08 cfs 0.513 af n=0.016 L=194.0' S=0.0103 '/' Capacity=22.62 cfs Outflow=6.18 cfs 0.512 af

**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 6

<b>Reach 34cR: Overland Flow</b>	Avg. Flow Depth=0.20' Max Vel=5.07 fps Inflow=6.18 cfs 0.512 af n=0.030 L=36.0' S=0.1597 '/' Capacity=47.49 cfs Outflow=6.19 cfs 0.512 af
<b>Reach 34dR: Overland Flow</b>	Avg. Flow Depth=0.33' Max Vel=2.35 fps Inflow=6.19 cfs 0.512 af n=0.030 L=43.0' S=0.0174 '/' Capacity=15.69 cfs Outflow=6.20 cfs 0.512 af
<b>Reach 71aR: Wooded Swale</b>	Avg. Flow Depth=0.46' Max Vel=1.53 fps Inflow=3.40 cfs 0.492 af n=0.035 L=125.0' S=0.0064 '/' Capacity=79.88 cfs Outflow=3.39 cfs 0.492 af
<b>Reach 71bR: Riprap Swale</b>	Avg. Flow Depth=0.35' Max Vel=1.61 fps Inflow=3.39 cfs 0.492 af n=0.041 L=147.7' S=0.0135 '/' Capacity=31.94 cfs Outflow=3.39 cfs 0.491 af
<b>Reach 72R: Roadside Swale</b>	Avg. Flow Depth=0.25' Max Vel=1.71 fps Inflow=2.00 cfs 0.247 af n=0.022 L=495.6' S=0.0060 '/' Capacity=33.12 cfs Outflow=1.95 cfs 0.246 af
<b>Reach 200R: Final Reach #200</b>	Inflow=5.85 cfs 0.598 af Outflow=5.85 cfs 0.598 af
<b>Reach 300R: Final Reach #300</b>	Inflow=6.68 cfs 1.117 af Outflow=6.68 cfs 1.117 af
<b>Reach 400R: Final Reach #400</b>	Inflow=4.22 cfs 0.663 af Outflow=4.22 cfs 0.663 af
<b>Pond 30P: Existing Infiltration/Trench</b>	Peak Elev=183.96' Storage=3,170 cf Inflow=2.87 cfs 0.251 af Discarded=0.93 cfs 0.189 af Primary=0.65 cfs 0.053 af Secondary=0.01 cfs 0.000 af Outflow=1.47 cfs 0.242 af
<b>Pond 70P: Existing Catch Basin</b>	Peak Elev=181.13' Storage=0.000 af Inflow=4.22 cfs 0.663 af 18.0" Round Culvert n=0.012 L=62.8' S=0.0180 '/' Outflow=4.22 cfs 0.663 af
<b>Pond 71P: Existing Catch Basin</b>	Peak Elev=188.93' Inflow=3.40 cfs 0.492 af 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/' Outflow=3.40 cfs 0.492 af
<b>Pond 72P: Existing Depression</b>	Peak Elev=196.21' Storage=160 cf Inflow=2.04 cfs 0.273 af Discarded=0.02 cfs 0.024 af Primary=2.00 cfs 0.247 af Outflow=2.02 cfs 0.271 af
<b>Pond E01P: Existing Catch Basin</b>	Peak Elev=185.04' Storage=19 cf Inflow=1.99 cfs 0.145 af 15.0" Round Culvert n=0.012 L=57.0' S=0.0049 '/' Outflow=1.92 cfs 0.145 af
<b>Pond E02P: Existing Catch Basin</b>	Peak Elev=184.97' Storage=135 cf Inflow=6.18 cfs 0.459 af Discarded=0.01 cfs 0.006 af Primary=6.73 cfs 0.453 af Outflow=6.73 cfs 0.459 af
<b>Pond E03P: Existing Catch Basin</b>	Peak Elev=184.31' Storage=60 cf Inflow=0.65 cfs 0.053 af Primary=0.42 cfs 0.052 af Secondary=0.00 cfs 0.000 af Outflow=0.42 cfs 0.052 af
<b>Pond E04P: Existing Catch Basin</b>	Peak Elev=184.75' Storage=75 cf Inflow=6.85 cfs 0.514 af Outflow=6.40 cfs 0.513 af

**Total Runoff Area = 16.942 ac Runoff Volume = 2.614 af Average Runoff Depth = 1.85"**  
**82.88% Pervious = 14.041 ac 17.12% Impervious = 2.901 ac**

**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 7

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 2S: Subcat #2</b>	Runoff Area=163,452 sf 2.33% Impervious Runoff Depth>2.71" Flow Length=301' Tc=15.8 min CN=61 Runoff=8.53 cfs 0.846 af
<b>Subcatchment 3S: Subcat #3</b>	Runoff Area=222,064 sf 1.16% Impervious Runoff Depth>2.11" Flow Length=682' Tc=43.7 min CN=55 Runoff=5.59 cfs 0.897 af
<b>Subcatchment 30S: Subcat #30</b>	Runoff Area=58,317 sf 10.64% Impervious Runoff Depth>3.11" Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=65 Runoff=4.03 cfs 0.347 af
<b>Subcatchment 31S: Subcat #31</b>	Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>4.93" Tc=6.0 min CN=82 Runoff=2.52 cfs 0.186 af
<b>Subcatchment 32S: Subcat #32</b>	Runoff Area=40,270 sf 63.40% Impervious Runoff Depth>5.16" Tc=6.0 min CN=84 Runoff=5.34 cfs 0.397 af
<b>Subcatchment 34S: Subcat #34</b>	Runoff Area=1,936 sf 0.00% Impervious Runoff Depth>3.42" Tc=6.0 min CN=68 Runoff=0.17 cfs 0.013 af
<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=62,561 sf 28.86% Impervious Runoff Depth>2.13" Flow Length=380' Tc=15.2 min CN=55 Runoff=2.49 cfs 0.255 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>1.93" Flow Length=563' Tc=39.5 min CN=53 Runoff=2.39 cfs 0.372 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>2.89" Flow Length=478' Tc=32.0 min CN=63 Runoff=2.91 cfs 0.382 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.09' Max Vel=1.01 fps Inflow=0.37 cfs 0.010 af n=0.022 L=43.0' S=0.0105 '/' Capacity=16.58 cfs Outflow=0.35 cfs 0.010 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.05' Max Vel=1.92 fps Inflow=0.35 cfs 0.010 af n=0.030 L=63.5' S=0.1339 '/' Capacity=43.48 cfs Outflow=0.32 cfs 0.010 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.08' Max Vel=0.80 fps Inflow=0.32 cfs 0.010 af n=0.030 L=230.5' S=0.0130 '/' Capacity=13.56 cfs Outflow=0.26 cfs 0.010 af
<b>Reach 30dR: Overland Flow</b>	Avg. Flow Depth=0.05' Max Vel=1.80 fps Inflow=0.26 cfs 0.010 af n=0.030 L=11.0' S=0.1364 '/' Capacity=43.88 cfs Outflow=0.26 cfs 0.010 af
<b>Reach 33aR: Overland Flow</b>	Avg. Flow Depth=0.06' Max Vel=1.37 fps Inflow=0.32 cfs 0.004 af n=0.016 L=50.0' S=0.0198 '/' Capacity=31.35 cfs Outflow=0.25 cfs 0.004 af
<b>Reach 34aR: Overland Flow</b>	Avg. Flow Depth=0.29' Max Vel=3.68 fps Inflow=7.96 cfs 0.661 af n=0.016 L=35.0' S=0.0140 '/' Capacity=26.36 cfs Outflow=8.20 cfs 0.661 af
<b>Reach 34bR: Overland Flow</b>	Avg. Flow Depth=0.31' Max Vel=3.28 fps Inflow=8.23 cfs 0.665 af n=0.016 L=194.0' S=0.0103 '/' Capacity=22.62 cfs Outflow=7.96 cfs 0.664 af



**23-017 Existing Analysis Ex TCAM**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying &amp; Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 8

**Reach 34cR: Overland Flow** Avg. Flow Depth=0.22' Max Vel=5.47 fps Inflow=7.96 cfs 0.664 af  
 n=0.030 L=36.0' S=0.1597 '/ Capacity=47.49 cfs Outflow=7.94 cfs 0.664 af

**Reach 34dR: Overland Flow** Avg. Flow Depth=0.36' Max Vel=2.54 fps Inflow=7.94 cfs 0.664 af  
 n=0.030 L=43.0' S=0.0174 '/ Capacity=15.69 cfs Outflow=7.91 cfs 0.664 af

**Reach 71aR: Wooded Swale** Avg. Flow Depth=0.56' Max Vel=1.74 fps Inflow=5.16 cfs 0.725 af  
 n=0.035 L=125.0' S=0.0064 '/ Capacity=79.88 cfs Outflow=5.16 cfs 0.724 af

**Reach 71bR: Riprap Swale** Avg. Flow Depth=0.43' Max Vel=1.83 fps Inflow=5.16 cfs 0.724 af  
 n=0.041 L=147.7' S=0.0135 '/ Capacity=31.94 cfs Outflow=5.15 cfs 0.723 af

**Reach 72R: Roadside Swale** Avg. Flow Depth=0.31' Max Vel=1.90 fps Inflow=2.86 cfs 0.354 af  
 n=0.022 L=495.6' S=0.0060 '/ Capacity=33.12 cfs Outflow=2.82 cfs 0.353 af

**Reach 200R: Final Reach #200** Inflow=8.53 cfs 0.857 af  
 Outflow=8.53 cfs 0.857 af

**Reach 300R: Final Reach #300** Inflow=9.01 cfs 1.561 af  
 Outflow=9.01 cfs 1.561 af

**Reach 400R: Final Reach #400** Inflow=6.45 cfs 0.978 af  
 Outflow=6.45 cfs 0.978 af

**Pond 30P: Existing Infiltration/Trench** Peak Elev=184.03' Storage=4,209 cf Inflow=4.03 cfs 0.347 af  
 Discarded=1.03 cfs 0.249 af Primary=0.70 cfs 0.079 af Secondary=0.37 cfs 0.010 af Outflow=1.87 cfs 0.338 af

**Pond 70P: Existing Catch Basin** Peak Elev=181.46' Storage=0.000 af Inflow=6.45 cfs 0.978 af  
 18.0" Round Culvert n=0.012 L=62.8' S=0.0180 '/ Outflow=6.45 cfs 0.978 af

**Pond 71P: Existing Catch Basin** Peak Elev=189.23' Inflow=5.16 cfs 0.725 af  
 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/ Outflow=5.16 cfs 0.725 af

**Pond 72P: Existing Depression** Peak Elev=196.26' Storage=179 cf Inflow=2.91 cfs 0.382 af  
 Discarded=0.02 cfs 0.026 af Primary=2.86 cfs 0.354 af Outflow=2.89 cfs 0.380 af

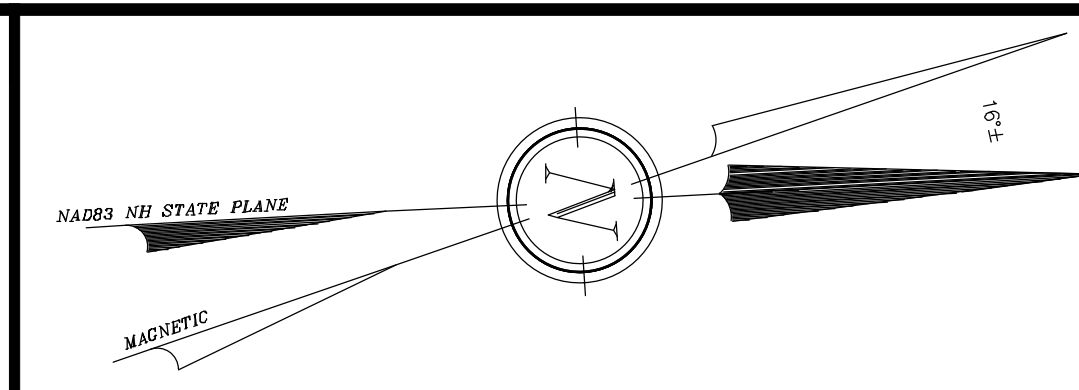
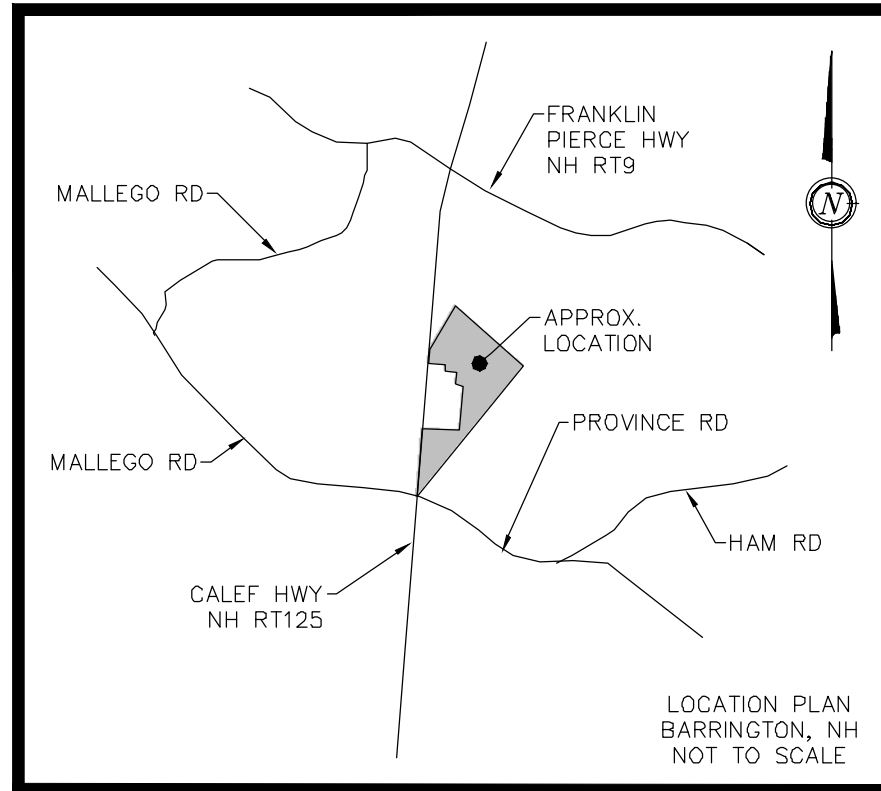
**Pond E01P: Existing Catch Basin** Peak Elev=185.20' Storage=21 cf Inflow=2.52 cfs 0.186 af  
 15.0" Round Culvert n=0.012 L=57.0' S=0.0049 '/ Outflow=2.54 cfs 0.185 af

**Pond E02P: Existing Catch Basin** Peak Elev=185.13' Storage=156 cf Inflow=7.88 cfs 0.583 af  
 Discarded=0.01 cfs 0.006 af Primary=7.86 cfs 0.576 af Outflow=7.86 cfs 0.582 af

**Pond E03P: Existing Catch Basin** Peak Elev=184.59' Storage=63 cf Inflow=0.70 cfs 0.079 af  
 Primary=0.59 cfs 0.074 af Secondary=0.32 cfs 0.004 af Outflow=0.59 cfs 0.078 af

**Pond E04P: Existing Catch Basin** Peak Elev=184.86' Storage=83 cf Inflow=8.01 cfs 0.662 af  
 Outflow=7.96 cfs 0.661 af

**Total Runoff Area = 16.942 ac Runoff Volume = 3.694 af Average Runoff Depth = 2.62"**  
**82.88% Pervious = 14.041 ac 17.12% Impervious = 2.901 ac**



- NOTES:**
- 1.) OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 1A.) APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 2.) TAX MAP 238, LOT 44
  - 3.) LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
  - 4.) S.C.R.D. BOOK 2948, PAGE 332
  - 5.) VERTICAL DATUM BASED ON NAVD83 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
  - 6.) THE INTENT OF THIS PLAN IS TO SHOW THE EXISTING WATERSHED CONDITIONS OF TAX MAP 238, LOT 44 AS OF THE DATE OF SURVEY.
  - 7.) SEE EXISTING CONDITIONS PACKAGE BY BS&E INCLUDED IN THE SITE CIVIL SET.

SEE CORRESPONDING SITE PLAN FOR TAX MAP 238, LOT 44-1

FINAL REACH 400 POINT OF ANALYSIS POINT DISCHARGE  
SEE PLAN SET DB2023-017, ATTACHED  
C - RECEIVING WATERS - MALLEGO BROOK

EASTERN WETLAND EDGE, SOUTH OF EXISTING DRIVEWAY  
LIMIT OF ANALYSIS (TYP.)

EDGE OF POORLY DRAINED JURISDICTIONAL WETLANDS (TYP.)  
BY: DEIDRA BENJAMIN, CWS #295

FINAL REACH 200 POINT OF ANALYSIS NON-POINT DISCHARGE  
SEE PLAN SET DB2023-017, ATTACHED  
C - RECEIVING WATERS - MALLEGO BROOK

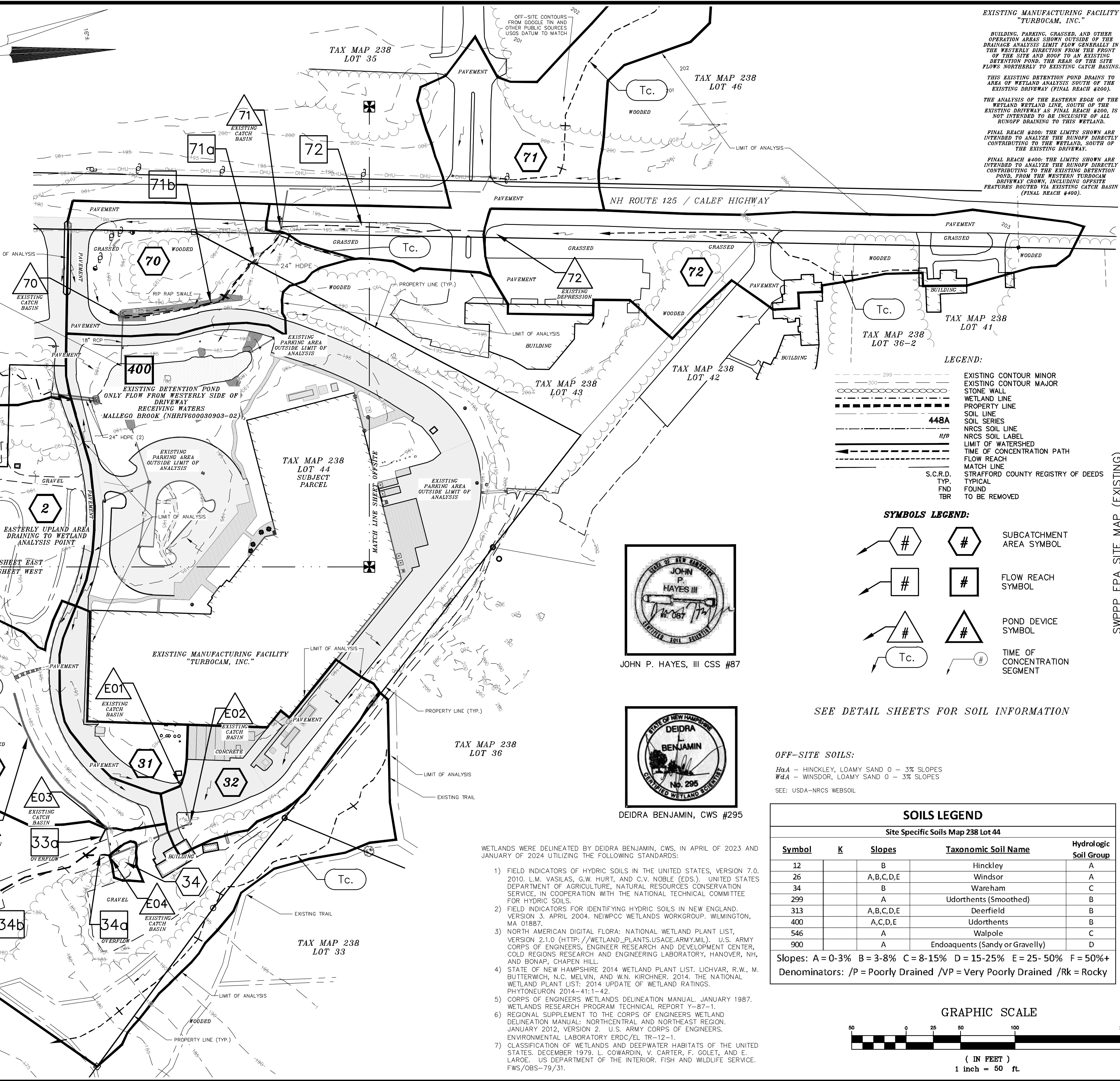
EXISTING WETLANDS FROM EASTERLY UPLAND AREA NON-INCLUSIVE OF ALL FLOW TO POINT RECEIVING WATERS MALLEGO BROOK (NHRIV600030903-02)

EASTERLY UPLAND AREA DRAINING TO WETLAND ANALYSIS POINT

EASTERLY UPLAND AREA DRAINING TO WETLAND ANALYSIS POINT

FINAL REACH 300 POINT OF ANALYSIS NON-POINT DISCHARGE  
SEE PLAN SET DB2023-017, ATTACHED  
C - RECEIVING WATERS - MALLEGO BROOK

SOUTHEAST PROPERTY LINE MALLEGO BROOK (NHRIV600030903-02)



**EXISTING MANUFACTURING FACILITY "TURBOCAM, INC."**

BUILDING, PARKING, GRASSED, AND OTHER OPERATION AREAS SHOWN OUTSIDE OF THE DRAINAGE ANALYSIS LIMIT FLOW GENERALLY IN THE WESTERLY DIRECTION FROM THE FRONT OF THE SITE AND ROOF TO AN EXISTING DETENTION POND. THE REAR OF THE SITE FLOWS NORTHERLY TO EXISTING CATCH BASINS.

THIS EXISTING DETENTION POND DRAINS TO AREA OF WETLAND ANALYSIS SOUTH OF THE EXISTING DRIVEWAY (FINAL REACH #200).

THE ANALYSIS OF THE EASTERN EDGE OF THE WETLAND WETLAND LINE, SOUTH OF THE EXISTING DRIVEWAY AS FINAL REACH #200, IS NOT INTENDED TO BE INCLUSIVE OF ALL RUNOFF DRAWING TO THIS WETLAND.

FINAL REACH #200: THE LIMITS SHOWN ARE INTENDED TO ANALYZE THE RUNOFF DIRECTLY CONTRIBUTING TO THE WETLAND, SOUTH OF THE EXISTING DRIVEWAY.

FINAL REACH #400: THE LIMITS SHOWN ARE INTENDED TO ANALYZE THE RUNOFF DIRECTLY CONTRIBUTING TO THE EXISTING DETENTION POND, FROM THE WESTERN TURBOCAM DRIVEWAY CROWN, INCLUDING OFFSITE FEATURES ROUTED VIA EXISTING CATCH BASIN (FINAL REACH #400).

- LEGEND:**
- EXISTING CONTOUR MINOR
  - EXISTING CONTOUR MAJOR
  - STONE WALL
  - WETLAND LINE
  - PROPERTY LINE
  - SOIL LINE
  - SOIL SERIES
  - NRCS SOIL LINE
  - NRCS SOIL LABEL
  - 448A
  - LIMIT OF WATERSHED
  - TIME OF CONCENTRATION PATH
  - FLOW REACH
  - MATCH LINE
  - S.C.R.D.
  - TYP.
  - FND
  - TBR

- SYMBOLS LEGEND:**
- # SUBCATCHMENT AREA SYMBOL
  - # FLOW REACH SYMBOL
  - # POND DEVICE SYMBOL
  - Tc. TIME OF CONCENTRATION SEGMENT

SEE DETAIL SHEETS FOR SOIL INFORMATION

**OFF-SITE SOILS:**

HsA - HINCKLEY, LOAMY SAND 0 - 3% SLOPES  
WdA - WINDSOR, LOAMY SAND 0 - 3% SLOPES

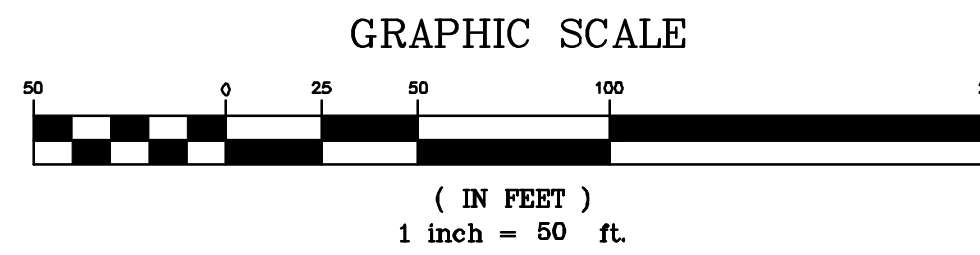
SEE: USDA-NRCS WEBSOIL

**SOILS LEGEND**

Site Specific Soils Map 238 Lot 44

Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoaquents (Sandy or Gravelly)	D

Slopes: A = 0-3% B = 3-8% C = 8-15% D = 15-25% E = 25-50% F = 50%+  
Denominators: /P = Poorly Drained /VP = Very Poorly Drained /Rk = Rocky



REVISION	DATE	DESCRIPTION

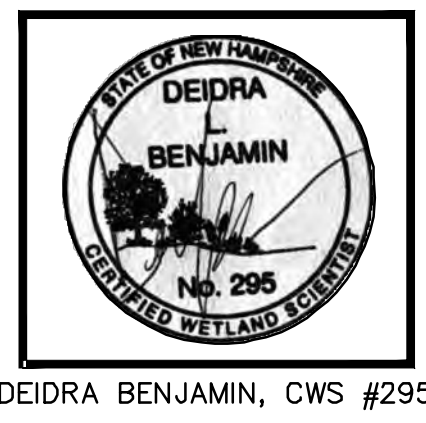
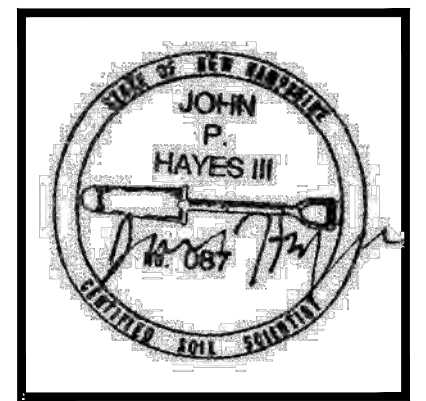
SWPPP EPA SITE MAP (EXISTING)  
W-1 EXISTING WATERSHED PLAN

FOR  
TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 50 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017

SHEET 1 OF 8

- WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS, IN APRIL OF 2023 AND JANUARY OF 2024 UTILIZING THE FOLLOWING STANDARDS:
- 1) FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0, 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.). UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL COMMITTEE FOR HYDRIC SOILS.
  - 2) FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, VERSION 3, APRIL 2004. NEWPPC WETLANDS WORKGROUP. WILMINGTON, MA 01887.
  - 3) NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, VERSION 2.1.0 (HTTP://WETLAND\_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD REGIONS RESEARCH AND ENGINEERING LABORATORY, HANOVER, NH, AND BONAP, CHAFEN HILL.
  - 4) STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST. LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER, 2014. THE NATIONAL WETLAND PLANT LIST: 2014 UPDATE OF WETLAND RATINGS. PHYTONEURON 2014-41:1-42.
  - 5) CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH PROGRAM TECHNICAL REPORT Y-87-1.
  - 6) REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTH-CENTRAL AND NORTH-EAST REGION, JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS. ENVIRONMENTAL LABORATORY ERDC/EL TR-12-1.
  - 7) CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES, DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. US DEPARTMENT OF THE INTERIOR, FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.



**NOTES:**

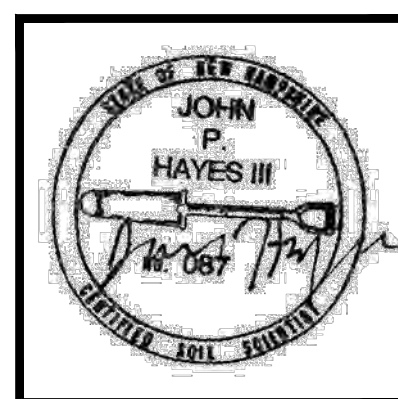
- OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- TAX MAP 238, LOT 44
- LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
- S.C.R.D. BOOK 2948, PAGE 332
- VERTICAL DATUM BASED ON NAVD88 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
- THE INTENT OF THIS PLAN IS TO SHOW THE EXISTING WATERSHED CONDITIONS OF TAX MAP 238, LOT 44 AS OF THE DATE OF SURVEY.
- SEE EXISTING CONDITIONS PACKAGE BY BS&E INCLUDED IN THE SITE CIVIL SET.

**LEGEND:**

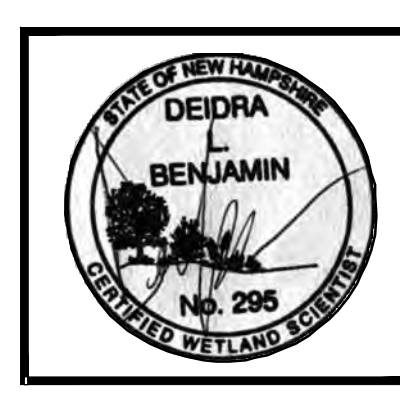
- EXISTING CONTOUR MINOR
- EXISTING CONTOUR MAJOR
- STONE WALL
- WETLAND LINE
- PROPERTY LINE
- SOIL LINE
- SOIL SERIES
- NRCS SOIL LINE
- NRCS SOIL LABEL
- LIMIT OF WATERSHED
- MATCH LINE
- TIME OF CONCENTRATION PATH
- FLOW REACH
- CONCENTRATION SEGMENT
- S.C.R.D. TYP.
- FND
- FOUND
- TBR
- TO BE REMOVED

**SYMBOLS LEGEND:**

- # SUBCATCHMENT AREA SYMBOL
- # FLOW REACH SYMBOL
- # POND DEVICE SYMBOL
- Tc. TIME OF CONCENTRATION SEGMENT



JOHN P. HAYES, III CSS #87

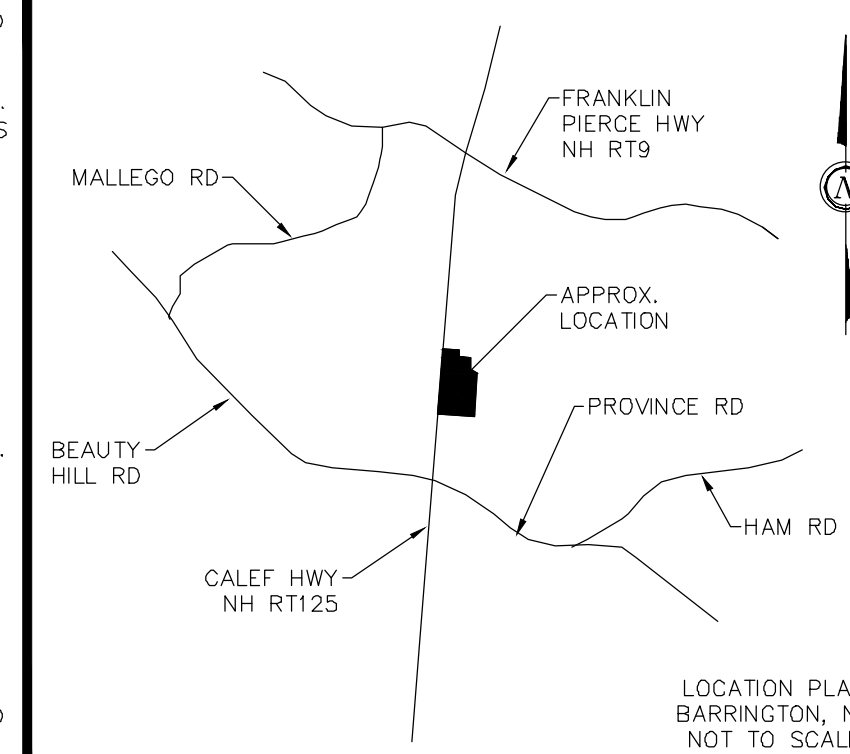


DEIDRA BENJAMIN, CWS #295

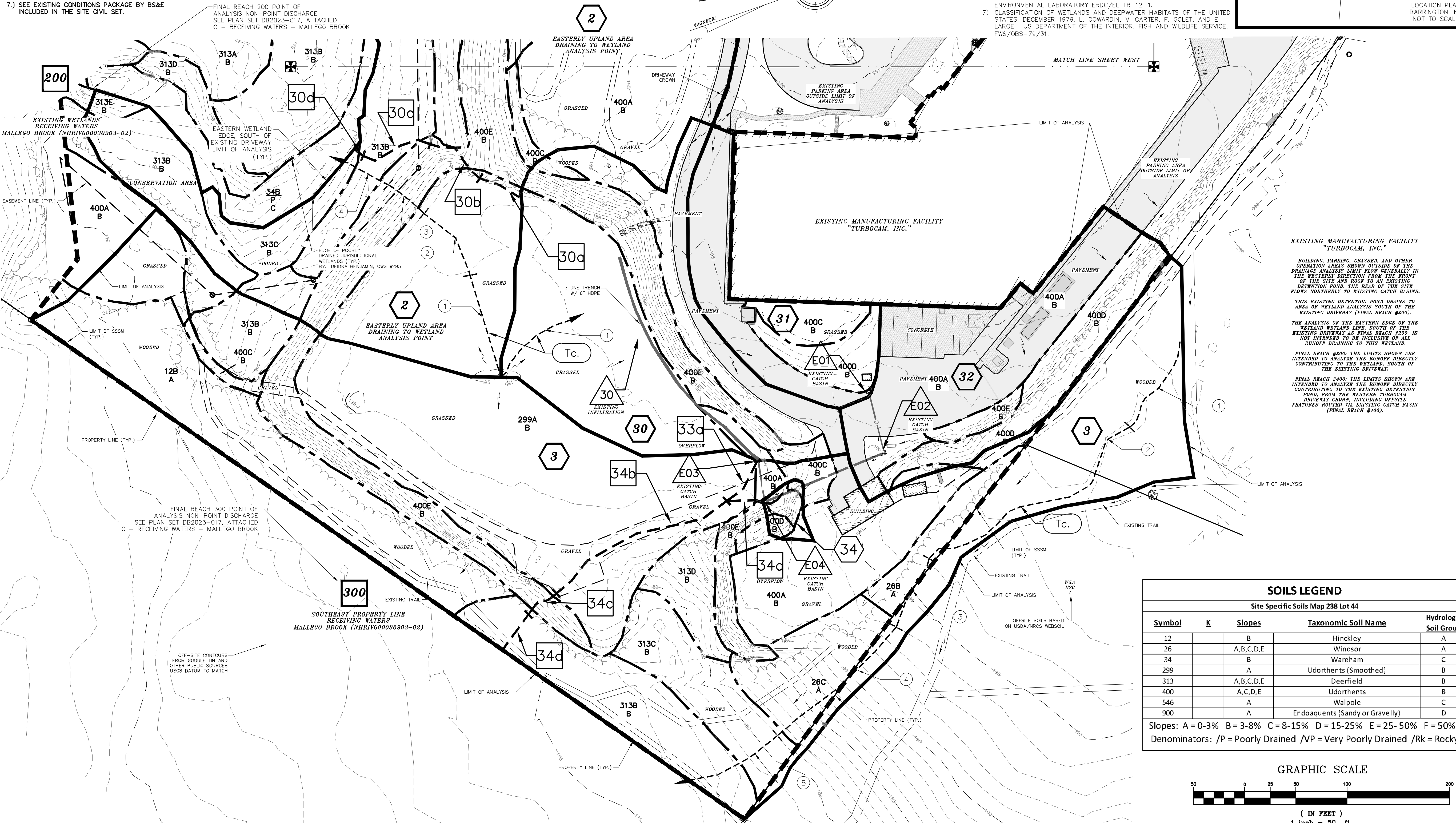


WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS, IN APRIL OF 2023 AND JANUARY OF 2024 UTILIZING THE FOLLOWING STANDARDS:

- FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0. 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.), UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL COMMITTEE FOR HYDRIC SOILS.
- FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, VERSION 3, APRIL 2004. NEWPPC WETLANDS WORKGROUP, WILMINGTON, MA 01887.
- NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, VERSION 2.1.0 (HTTP://WETLAND\_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD REGIONS RESEARCH AND ENGINEERING LABORATORY, HANOVER, NH, AND BONAP, CHAPEN HILL.
- STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST, LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER. 2014. THE NATIONAL WETLAND PLANT LIST: 2014 UPDATE OF WETLAND RATINGS. PHYTONEURON 2014-41:1-42.
- CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH PROGRAM TECHNICAL REPORT Y-87-1.
- REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTHCENTRAL AND NORTHEAST REGION, JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS. ENVIRONMENTAL LABORATORY ERDC/EL TR-12-1.
- CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES, DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. US DEPARTMENT OF THE INTERIOR, FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.



LOCATION PLAN BARRINGTON, NH NOT TO SCALE



**EXISTING MANUFACTURING FACILITY "TURBOCAM, INC."**

BUILDING, PARKING, GRASSED, AND OTHER OPERATION AREAS SHOWN OUTSIDE OF THE DRAINAGE ANALYSIS LIMIT FROM GENERALLY IN THE WESTERLY DIRECTION FROM THE FRONT OF THE SITE AND ROOF TO AN EXISTING DETENTION POND. THE REAR OF THE SITE FLOWS NORTHERLY TO EXISTING CATCH BASINS.

THIS EXISTING DETENTION POND DRAINS TO AREA OF WETLAND ANALYSIS SOUTH OF THE EXISTING DRIVEWAY (FINAL REACH #200).

THE ANALYSIS OF THE EASTERN EDGE OF THE WETLAND WETLAND LINE, SOUTH OF THE EXISTING DRIVEWAY AS FINAL REACH #200, IS NOT INTENDED TO BE INCLUSIVE OF ALL RUNOFF DRAINING TO THIS WETLAND.

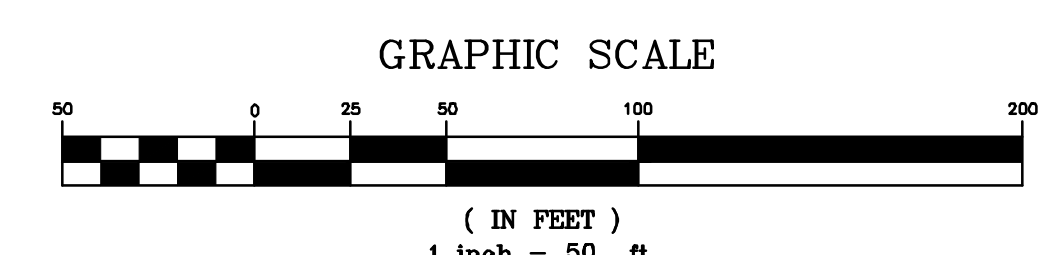
FINAL REACH #200: THE LIMITS SHOWN ARE INTENDED TO ANALYZE THE RUNOFF DIRECTLY CONTRIBUTING TO THE EXISTING DETENTION POND, FROM THE WESTERN TURBOCAM DRIVEWAY CROWN, INCLUDING OFFSITE FEATURES SOUTHERLY VIA EXISTING CATCH BASIN (FINAL REACH #400).

**SOILS LEGEND**

Site Specific Soils Map 238 Lot 44

Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udoorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udoorthents	B
546		A	Walpole	C
900		A	Endoqaquents (Sandy or Gravelly)	D

Slopes: A = 0-3% B = 3-8% C = 8-15% D = 15-25% E = 25-50% F = 50%+  
Denominators: /P = Poorly Drained /VP = Very Poorly Drained /Rk = Rocky



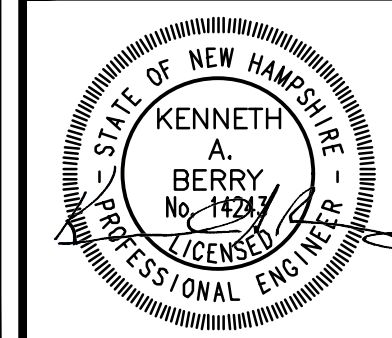
REVISION	DATE	DESCRIPTION

SWPPP EPA SITE MAP (EXISTING)

W-1 EXISTING WATERSHED PLAN EAST

FOR TURBOCAM, INC.  
LAND OF VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

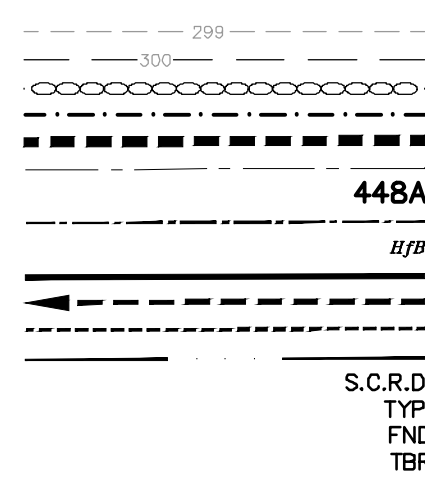
BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 50 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017



**NOTES:**

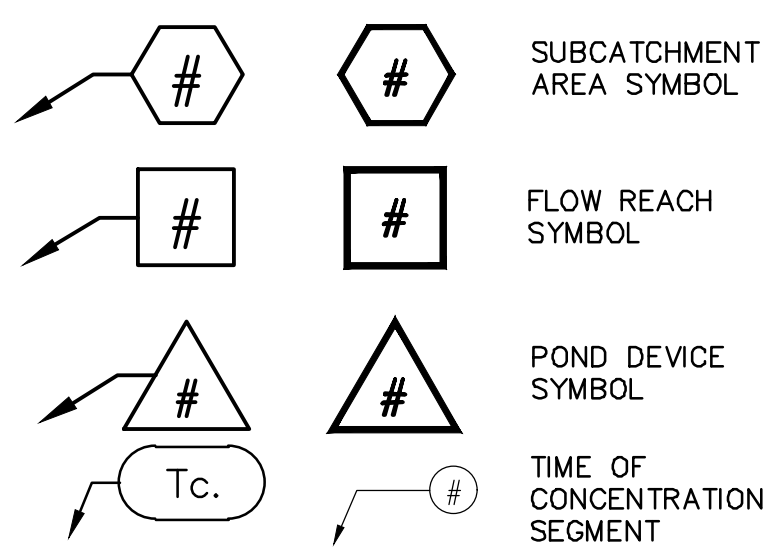
- OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- TAX MAP 238, LOT 44
- LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
- S.C.R.D. BOOK 2948, PAGE 332
- VERTICAL DATUM BASED ON NAVD88 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
- THE INTENT OF THIS PLAN IS TO SHOW THE EXISTING WATERSHED CONDITIONS OF TAX MAP 238, LOT 44 AS OF THE DATE OF SURVEY.
- SEE EXISTING CONDITIONS PACKAGE BY BS&E INCLUDED IN THE SITE CIVIL SET.

**LEGEND:**

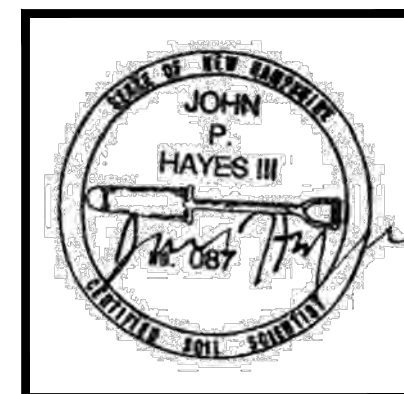


EXISTING CONTOUR MINOR  
EXISTING CONTOUR MAJOR  
STONE WALL  
WETLAND LINE  
PROPERTY LINE  
SOIL LINE  
NRCS SOIL LINE  
NRCS SOIL LABEL  
LIMIT OF WATERSHED  
MATCH LINE  
TIME OF CONCENTRATION PATH  
FLOW REACH  
STRAFFORD COUNTY REGISTRY OF DEEDS  
TYPICAL  
FOUND  
TO BE REMOVED

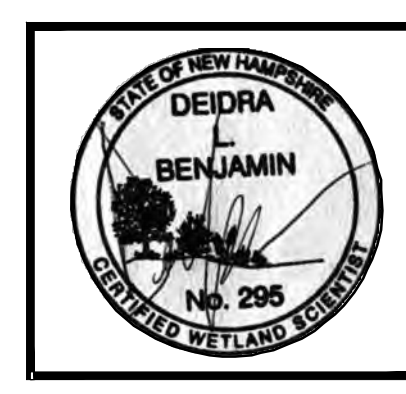
**SYMBOLS LEGEND:**



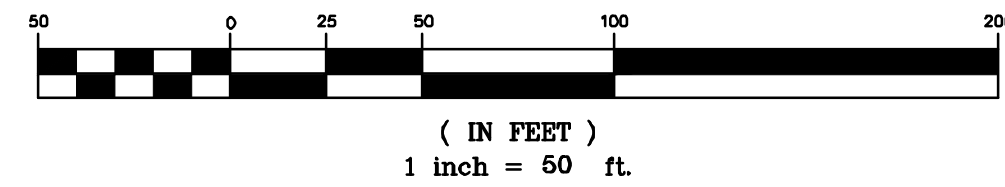
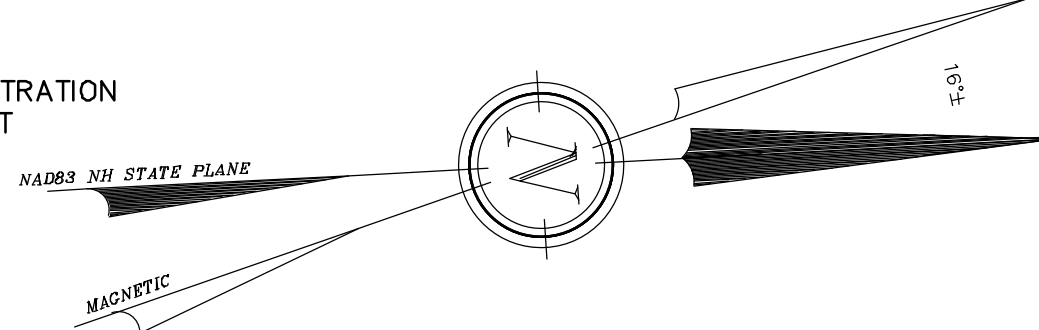
SUBCATCHMENT AREA SYMBOL  
FLOW REACH SYMBOL  
POND DEVICE SYMBOL  
TIME OF CONCENTRATION SEGMENT



JOHN P. HAYES, III CSS #87

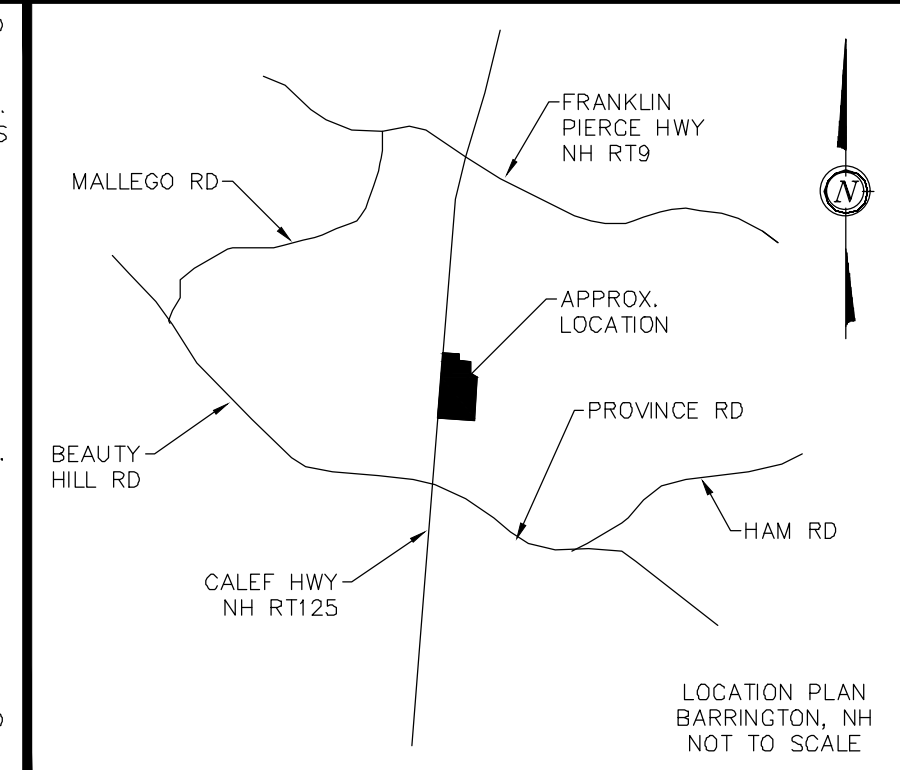


DEIDRA BENJAMIN, CWS #295



WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS, IN APRIL OF 2023 AND JANUARY OF 2024 UTILIZING THE FOLLOWING STANDARDS:

- FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0. 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.). UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL COMMITTEE FOR HYDRIC SOILS.
- FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, VERSION 3, APRIL 2004. NEWPPC WETLANDS WORKGROUP, WILMINGTON, MA 01887.
- NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, VERSION 2.1.0 (HTTP://WETLAND\_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD REGIONS RESEARCH AND ENGINEERING LABORATORY, HANOVER, NH, AND BONAP, CHAPEN HILL.
- STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST, LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER. 2014. THE NATIONAL WETLAND PLANT LIST: 2014 UPDATE OF WETLAND RATINGS. PHYTONEURON 2014-41:1-42.
- CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH PROGRAM TECHNICAL REPORT Y-87-1.
- REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTH-CENTRAL AND NORTHEAST REGION. JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS. ENVIRONMENTAL LABORATORY ERO/VEL TR-12-1.
- CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES. DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. US DEPARTMENT OF THE INTERIOR, FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.



SOILS LEGEND				
Site Specific Soils Map 238 Lot 44				
Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoaquents (Sandy or Gravelly)	D

Slopes: A=0-3% B=3-8% C=8-15% D=15-25% E=25-50% F=50%+  
Denominators: /P = Poorly Drained /VP = Very Poorly Drained /Rk = Rocky

**OFF-SITE SOILS:**

H<sub>a</sub>A - HINCKLEY, LOAMY SAND 0 - 3% SLOPES  
W<sub>d</sub>A - WINDSOR, LOAMY SAND 0 - 3% SLOPES  
SEE: USDA-NRCS WEBSOIL

**EXISTING MANUFACTURING FACILITY "TURBOCAM, INC."**

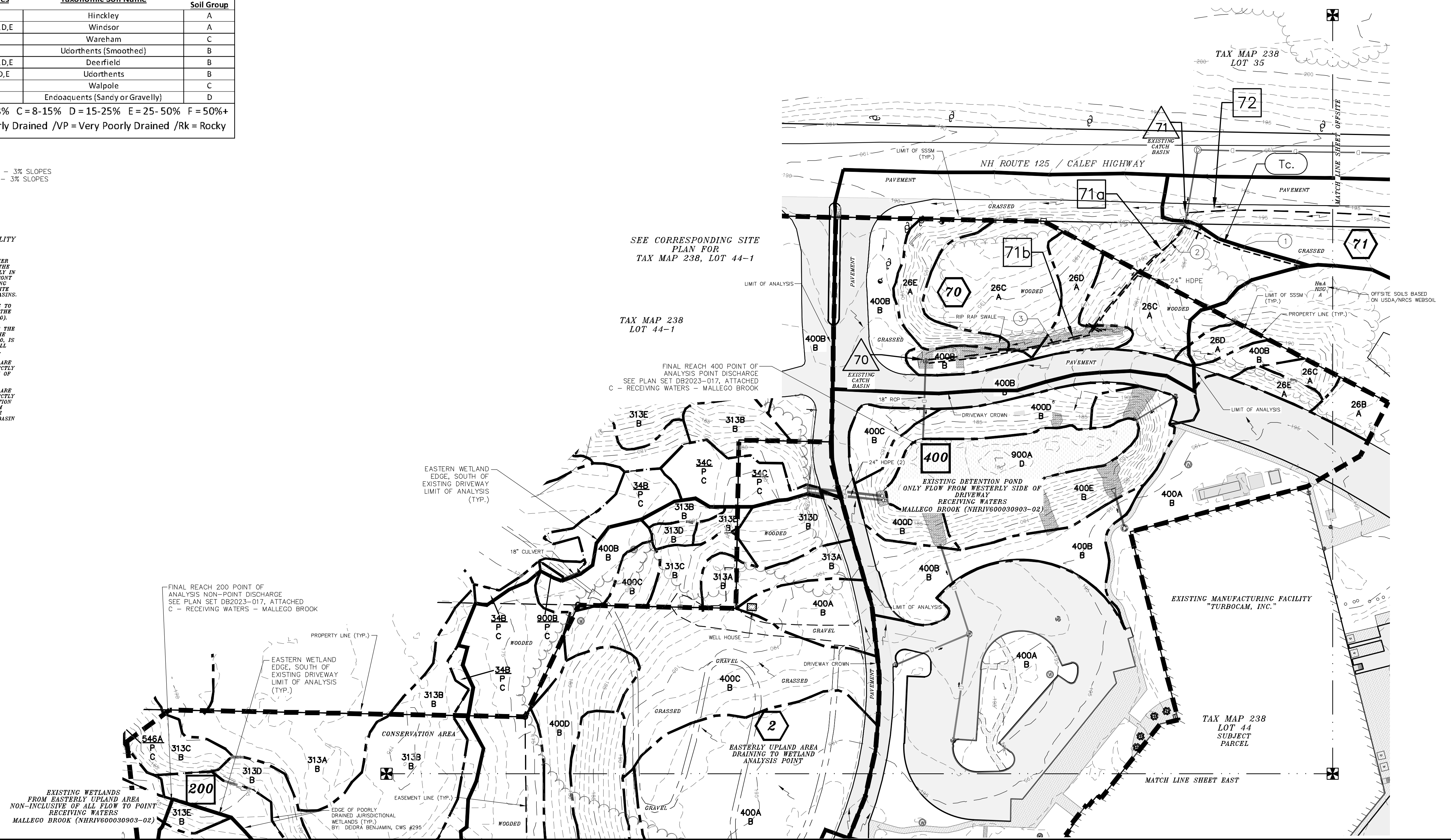
BUILDING, PARKING, GRASSED, AND OTHER OPERATION AREAS SHOWN OUTSIDE OF THE DRAINAGE ANALYSIS LIMIT FLOW GENERALLY IN THE WESTERLY DIRECTION FROM THE FRONT OF THE SITE AND ROOF TO AN EXISTING DETENTION POND. THE REAR OF THE SITE FLOWS NORTHERLY TO EXISTING CATCH BASINS.

THIS EXISTING DETENTION POND DRAINS TO AREA OF WETLAND ANALYSIS SOUTH OF THE EXISTING DRIVEWAY (FINAL REACH #200).

THE ANALYSIS OF THE EASTERN EDGE OF THE WETLAND WETLAND LINE, SOUTH OF THE EXISTING DRIVEWAY AS FINAL REACH #300, IS NOT INTENDED TO BE INCLUSIVE OF ALL RUNOFF DRAINING TO THIS WETLAND.

FINAL REACH #200: THE LIMITS SHOWN ARE INTENDED TO ANALYZE THE RUNOFF DIRECTLY CONTRIBUTING TO THE WETLAND, SOUTH OF THE EXISTING DRIVEWAY.

FINAL REACH #400: THE LIMITS SHOWN ARE INTENDED TO ANALYZE THE RUNOFF DIRECTLY CONTRIBUTING TO THE EXISTING DETENTION POND, FROM THE WESTERN TURBOCAM DRIVEWAY CROWN, INCLUDING OFFSITE FEATURES ROUTED VIA EXISTING CATCH BASIN (FINAL REACH #400).



SEE CORRESPONDING SITE PLAN FOR TAX MAP 238, LOT 44-1

TAX MAP 238 LOT 44-1

FINAL REACH 400 POINT OF ANALYSIS POINT DISCHARGE SEE PLAN SET DB2023-017, ATTACHED C - RECEIVING WATERS - MALLEGO BROOK

EXISTING MANUFACTURING FACILITY "TURBOCAM, INC."

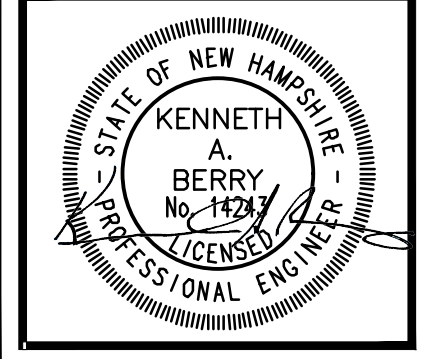
TAX MAP 238 LOT 44 SUBJECT PARCEL

SWPPP EPA SITE MAP (EXISTING)

W-1 EXISTING WATERSHED PLAN WEST

FOR TURBOCAM, INC. LAND OF VIRTUOUS REALTY, LLC NH ROUTE 125/CALEF HIGHWAY BARRINGTON, N.H. TAX MAP 238, LOT 44

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 50 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017



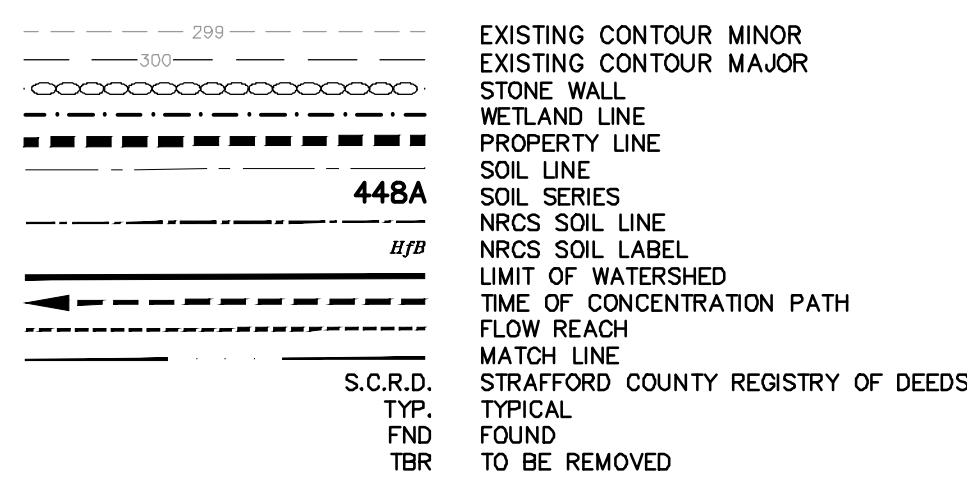
SHEET 3 OF 8

REVISION	DATE	DESCRIPTION

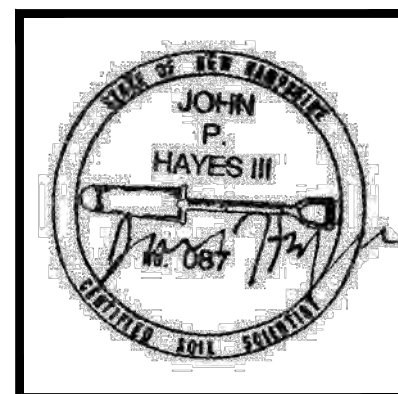
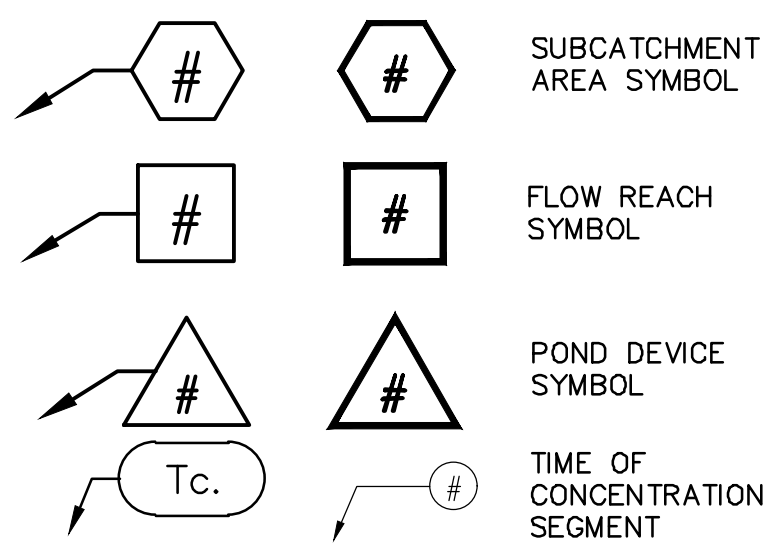
**NOTES:**

- OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- TAX MAP 238, LOT 44
- LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
- S.C.R.D. BOOK 2948, PAGE 332
- VERTICAL DATUM BASED ON NAVD88 ELEVATIONS.  
HORIZONTAL COORDINATES BASED ON NAD83.  
COORDINATES GATHERED USING CARLSON BRX7  
SURVEY GRADE GPS RECEIVERS.
- THE INTENT OF THIS PLAN IS TO SHOW THE  
EXISTING WATERSHED CONDITIONS OF TAX MAP  
238, LOT 44 AS OF THE DATE OF SURVEY.
- SEE EXISTING CONDITIONS PACKAGE BY BS&E  
INCLUDED IN THE SITE CIVIL SET.

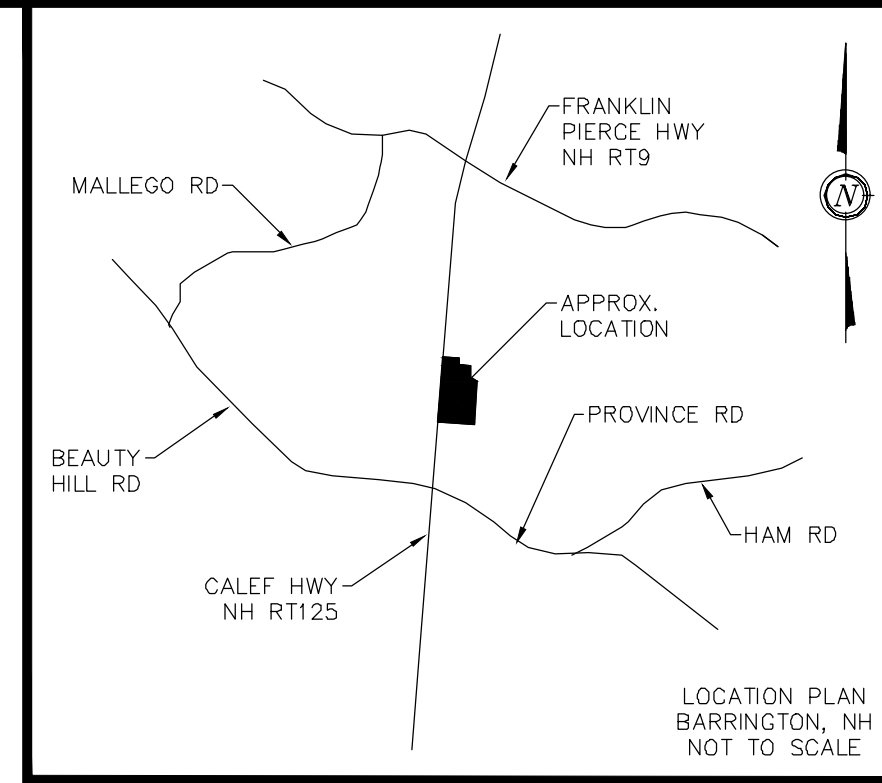
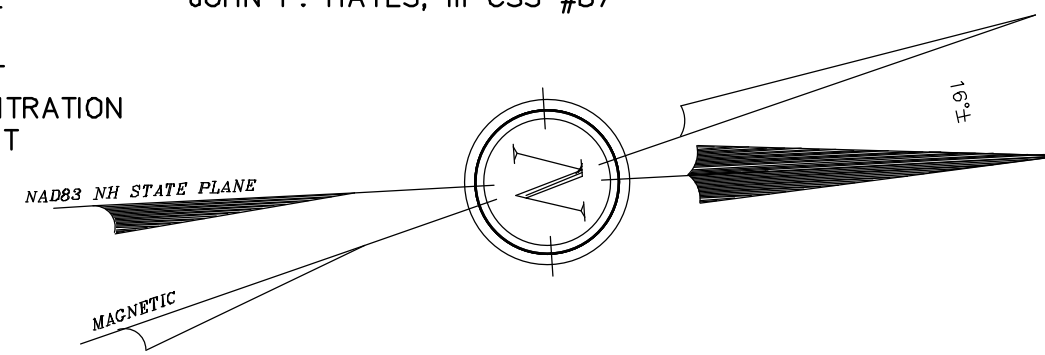
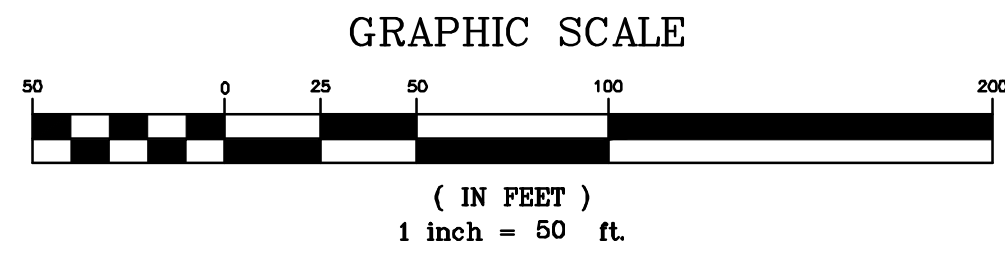
**LEGEND:**



**SYMBOLS LEGEND:**



JOHN P. HAYES, III CSS #87

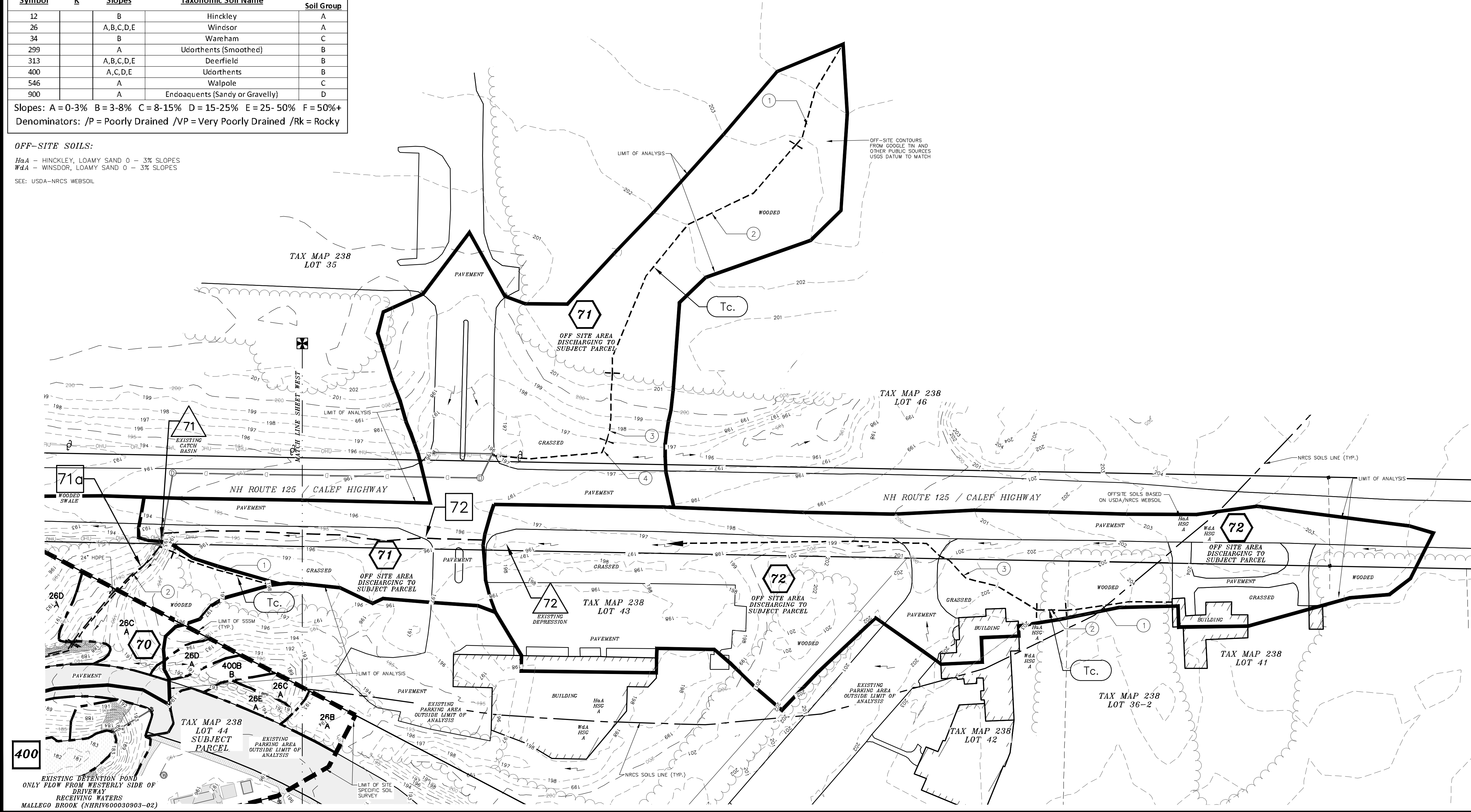


**SOILS LEGEND**  
Site Specific Soils Map 238 Lot 44

Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoaquents (Sandy or Gravelly)	D

Slopes: A=0-3% B=3-8% C=8-15% D=15-25% E=25-50% F=50%+  
Denominators: /P = Poorly Drained /VP = Very Poorly Drained /Rk = Rocky

**OFF-SITE SOILS:**  
HaA - HINCKLEY, LOAMY SAND 0 - 3% SLOPES  
WdA - WINDSOR, LOAMY SAND 0 - 3% SLOPES  
SEE: USDA-NRCS WEBSITE

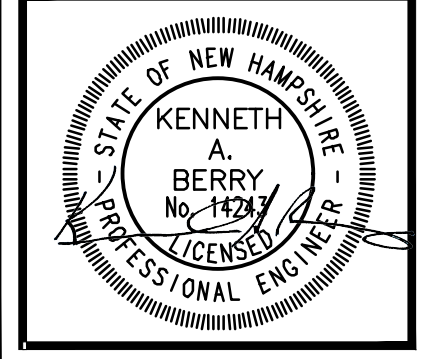


SWPPP EPA SITE MAP (EXISTING)

W-1 EXISTING WATERSHED PLAN OFFSITE

FOR  
TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 50 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017



## **Appendix II - Proposed Conditions Analysis**

25 Yr - 24 Hr. Full Summary

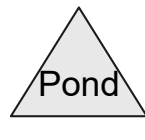
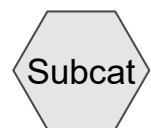
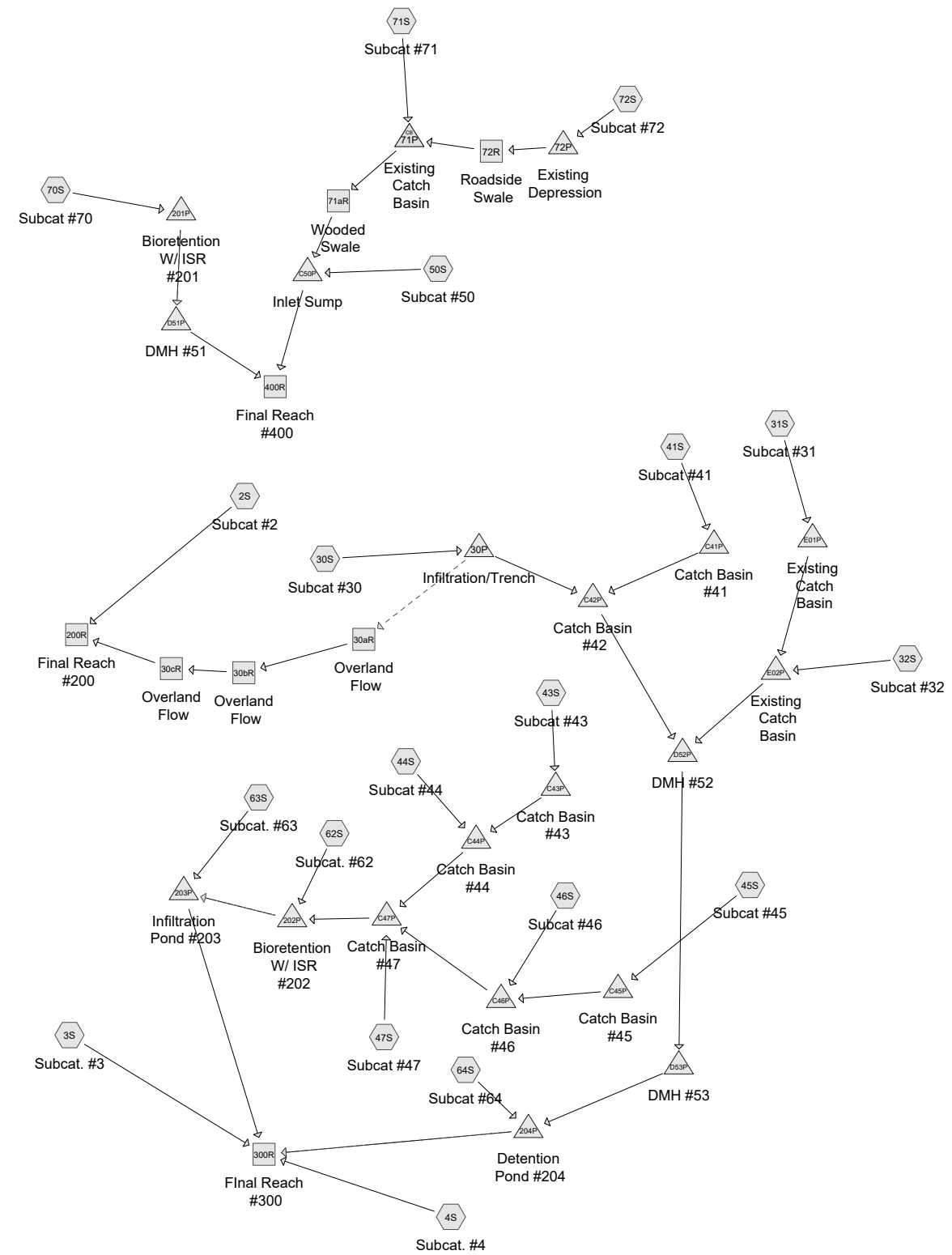
2 Yr - 24 Hr. Node Listing

10 Yr -24 Hr. Node Listing

25 Yr -24 Hr. Node Listing

50 Yr - 24 Hr. Node Listing

50 YR-24-Hr. Swale Capacity Analysis



**Routing Diagram for 23-017 Pro Analysis Ex TCAM Site Mods**  
 Prepared by Berry Surveying & Engineering, Printed 4/17/2024  
 HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

## 23-017 Pro Analysis Ex TCAM Site Mods

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 1

### Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2.514	39	>75% Grass cover, Good, HSG A (3S, 4S, 45S, 50S, 62S, 63S, 64S, 70S, 71S, 72S)
5.641	61	>75% Grass cover, Good, HSG B (2S, 3S, 4S, 30S, 31S, 32S, 41S, 43S, 44S, 45S, 62S, 63S, 64S, 70S)
0.005	96	Gravel surface, HSG A (4S)
0.453	96	Gravel surface, HSG B (2S, 3S, 4S, 30S, 41S, 43S, 44S, 45S, 62S, 63S, 64S)
1.746	98	Paved parking, HSG A (45S, 46S, 70S, 71S, 72S)
2.042	98	Paved parking, HSG B (31S, 32S, 41S, 43S, 44S, 45S, 46S, 62S, 70S)
0.073	98	Paved roads w/curbs & sewers, HSG B (47S)
0.007	98	Roofs, HSG A (45S)
0.094	98	Roofs, HSG B (30S, 31S, 32S, 43S, 45S)
0.086	98	Unconnected pavement, HSG B (2S)
0.018	98	Unconnected roofs, HSG B (2S)
2.421	30	Woods, Good, HSG A (3S, 4S, 50S, 64S, 70S, 71S, 72S)
1.847	55	Woods, Good, HSG B (2S, 3S, 4S, 32S)
<b>16.948</b>	<b>62</b>	<b>TOTAL AREA</b>



## 23-017 Pro Analysis Ex TCAM Site Mods

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 2

### Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
6.694	HSG A	3S, 4S, 45S, 46S, 50S, 62S, 63S, 64S, 70S, 71S, 72S
10.254	HSG B	2S, 3S, 4S, 30S, 31S, 32S, 41S, 43S, 44S, 45S, 46S, 47S, 62S, 63S, 64S, 70S
0.000	HSG C	
0.000	HSG D	
0.000	Other	
<b>16.948</b>		<b>TOTAL AREA</b>

**23-017 Pro Analysis Ex TCAM Site Mods**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 3

**Ground Covers (all nodes)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
2.514	5.641	0.000	0.000	0.000	8.155	>75% Grass cover, Good	2S, 3S, 4S, 30S,  31S,  32S,  41S,  43S,  44S,  45S,  50S,  62S,  63S,  64S,  70S,  71S, 72S
0.005	0.453	0.000	0.000	0.000	0.459	Gravel surface	2S, 3S, 4S, 30S,  41S,  43S,  44S,  45S,  62S,  63S, 64S

**23-017 Pro Analysis Ex TCAM Site Mods**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 4

**Ground Covers (all nodes) (continued)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
1.746	2.042	0.000	0.000	0.000	3.788	Paved parking	31S, 32S, 41S, 43S, 44S, 45S, 46S, 62S, 70S, 71S, 72S
0.000	0.073	0.000	0.000	0.000	0.073	Paved roads w/curbs & sewers	47S
0.007	0.094	0.000	0.000	0.000	0.100	Roofs	30S, 31S, 32S, 43S, 45S
0.000	0.086	0.000	0.000	0.000	0.086	Unconnected pavement	2S
0.000	0.018	0.000	0.000	0.000	0.018	Unconnected roofs	2S
2.421	1.847	0.000	0.000	0.000	4.269	Woods, Good	2S, 3S, 4S, 32S, 50S, 64S, 70S, 71S, 72S

**23-017 Pro Analysis Ex TCAM Site Mods**

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 5

**Ground Covers (all nodes) (continued)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
<b>6.694</b>	<b>10.254</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>16.948</b>	<b>TOTAL AREA</b>	

## 23-017 Pro Analysis Ex TCAM Site Mods

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 6

### Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	30P	183.15	183.15	1.0	0.0000	0.012	6.0	0.0	0.0
2	71P	187.90	187.80	10.2	0.0098	0.012	18.0	0.0	0.0
3	201P	182.00	181.70	33.0	0.0091	0.012	6.0	0.0	0.0
4	201P	182.00	181.70	26.0	0.0115	0.012	15.0	0.0	0.0
5	202P	173.75	173.50	30.0	0.0083	0.012	6.0	0.0	0.0
6	202P	173.75	173.50	30.0	0.0083	0.012	15.0	0.0	0.0
7	204P	176.25	176.00	29.0	0.0086	0.012	18.0	0.0	0.0
8	C41P	184.00	180.25	54.7	0.0686	0.012	15.0	0.0	0.0
9	C42P	180.15	179.81	63.0	0.0054	0.012	15.0	0.0	0.0
10	C43P	180.50	179.10	60.0	0.0233	0.012	15.0	0.0	0.0
11	C44P	179.00	178.60	48.0	0.0083	0.012	15.0	0.0	0.0
12	C45P	181.40	180.10	87.2	0.0149	0.012	15.0	0.0	0.0
13	C46P	180.00	178.60	68.0	0.0206	0.012	15.0	0.0	0.0
14	C47P	178.50	178.25	40.0	0.0063	0.012	18.0	0.0	0.0
15	C50P	183.50	182.95	107.5	0.0051	0.012	18.0	0.0	0.0
16	D51P	181.60	181.25	68.0	0.0051	0.012	18.0	0.0	0.0
17	D52P	179.71	179.10	110.0	0.0055	0.012	24.0	0.0	0.0
18	D53P	179.00	178.00	120.0	0.0083	0.012	24.0	0.0	0.0
19	E01P	183.50	183.22	57.0	0.0049	0.012	15.0	0.0	0.0
20	E02P	183.02	179.71	122.2	0.0271	0.012	24.0	0.0	0.0

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment 2S: Subcat #2** Runoff Area=164,530 sf 2.77% Impervious Runoff Depth>1.91"  
Flow Length=298' Tc=16.6 min UI Adjusted CN=61 Runoff=5.80 cfs 0.602 af

**Subcatchment 3S: Subcat. #3** Runoff Area=46,611 sf 0.00% Impervious Runoff Depth>0.94"  
Flow Length=158' Slope=0.0200 '/' Tc=11.3 min CN=48 Runoff=0.69 cfs 0.084 af

**Subcatchment 4S: Subcat. #4** Runoff Area=55,483 sf 0.00% Impervious Runoff Depth>0.45"  
Flow Length=674' Tc=43.2 min CN=40 Runoff=0.15 cfs 0.048 af

**Subcatchment 30S: Subcat #30** Runoff Area=47,823 sf 4.45% Impervious Runoff Depth>2.08"  
Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=63 Runoff=2.15 cfs 0.190 af

**Subcatchment 31S: Subcat #31** Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>3.86"  
Tc=6.0 min CN=82 Runoff=1.99 cfs 0.145 af

**Subcatchment 32S: Subcat #32** Runoff Area=37,918 sf 67.33% Impervious Runoff Depth>4.18"  
Tc=6.0 min CN=85 Runoff=4.10 cfs 0.303 af

**Subcatchment 41S: Subcat #41** Runoff Area=7,421 sf 61.70% Impervious Runoff Depth>4.49"  
Flow Length=342' Tc=14.7 min CN=88 Runoff=0.66 cfs 0.064 af

**Subcatchment 43S: Subcat #43** Runoff Area=15,256 sf 64.41% Impervious Runoff Depth>4.39"  
Flow Length=100' Tc=7.0 min CN=87 Runoff=1.67 cfs 0.128 af

**Subcatchment 44S: Subcat #44** Runoff Area=14,458 sf 76.68% Impervious Runoff Depth>4.83"  
Flow Length=98' Tc=7.7 min CN=91 Runoff=1.66 cfs 0.133 af

**Subcatchment 45S: Subcat #45** Runoff Area=16,893 sf 94.23% Impervious Runoff Depth>5.39"  
Flow Length=330' Tc=6.0 min CN=96 Runoff=2.15 cfs 0.174 af

**Subcatchment 46S: Subcat #46** Runoff Area=7,602 sf 100.00% Impervious Runoff Depth>5.63"  
Tc=6.0 min CN=98 Runoff=0.98 cfs 0.082 af

**Subcatchment 47S: Subcat #47** Runoff Area=3,200 sf 100.00% Impervious Runoff Depth>5.63"  
Tc=6.0 min CN=98 Runoff=0.41 cfs 0.034 af

**Subcatchment 50S: Subcat #50** Runoff Area=11,704 sf 0.00% Impervious Runoff Depth>0.15"  
Flow Length=182' Tc=11.7 min CN=33 Runoff=0.01 cfs 0.003 af

**Subcatchment 62S: Subcat. #62** Runoff Area=45,124 sf 0.15% Impervious Runoff Depth>2.16"  
Flow Length=165' Tc=14.3 min CN=64 Runoff=1.95 cfs 0.187 af

**Subcatchment 63S: Subcat. #63** Runoff Area=16,040 sf 0.00% Impervious Runoff Depth>1.67"  
Flow Length=150' Tc=10.8 min CN=58 Runoff=0.56 cfs 0.051 af

**Subcatchment 64S: Subcat #64** Runoff Area=7,675 sf 0.00% Impervious Runoff Depth>1.67"  
Tc=6.0 min CN=58 Runoff=0.31 cfs 0.025 af

<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=51,128 sf 43.96% Impervious Runoff Depth>2.43" Flow Length=345' Tc=14.1 min CN=67 Runoff=2.53 cfs 0.237 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>1.28" Flow Length=563' Tc=39.5 min CN=53 Runoff=1.48 cfs 0.246 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>2.07" Flow Length=478' Tc=32.0 min CN=63 Runoff=2.04 cfs 0.273 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=151.0' S=0.0063 '/' Capacity=12.85 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=35.0' S=0.2286 '/' Capacity=77.47 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.035 L=58.0' S=0.0948 '/' Capacity=31.37 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 71aR: Wooded Swale</b>	Avg. Flow Depth=0.52' Max Vel=1.28 fps Inflow=3.40 cfs 0.492 af n=0.035 L=78.5' S=0.0038 '/' Capacity=61.73 cfs Outflow=3.39 cfs 0.492 af
<b>Reach 72R: Roadside Swale</b>	Avg. Flow Depth=0.25' Max Vel=1.71 fps Inflow=2.00 cfs 0.247 af n=0.022 L=495.6' S=0.0060 '/' Capacity=33.12 cfs Outflow=1.95 cfs 0.246 af
<b>Reach 200R: Final Reach #200</b>	Inflow=5.80 cfs 0.602 af Outflow=5.80 cfs 0.602 af
<b>Reach 300R: Final Reach #300</b>	Inflow=4.82 cfs 1.034 af Outflow=4.82 cfs 1.034 af
<b>Reach 400R: Final Reach #400</b>	Inflow=4.05 cfs 0.668 af Outflow=4.05 cfs 0.668 af
<b>Pond 30P: Infiltration/Trench</b>	Peak Elev=183.82' Storage=1,384 cf Inflow=2.15 cfs 0.190 af Discarded=0.59 cfs 0.081 af Primary=0.51 cfs 0.107 af Secondary=0.00 cfs 0.000 af Outflow=1.10 cfs 0.188 af
<b>Pond 71P: Existing Catch Basin</b>	Peak Elev=188.93' Inflow=3.40 cfs 0.492 af 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/' Outflow=3.40 cfs 0.492 af
<b>Pond 72P: Existing Depression</b>	Peak Elev=196.21' Storage=160 cf Inflow=2.04 cfs 0.273 af Discarded=0.02 cfs 0.024 af Primary=2.00 cfs 0.247 af Outflow=2.02 cfs 0.271 af
<b>Pond 201P: Bioretention W/ ISR #201</b>	Peak Elev=185.45' Storage=4,286 cf Inflow=2.53 cfs 0.237 af Primary=0.02 cfs 0.023 af Secondary=0.66 cfs 0.150 af Outflow=0.68 cfs 0.173 af
<b>Pond 202P: Bioretention W/ ISR #202</b>	Peak Elev=179.19' Storage=18,074 cf Inflow=8.23 cfs 0.739 af Primary=0.09 cfs 0.103 af Secondary=0.60 cfs 0.463 af Tertiary=0.00 cfs 0.000 af Outflow=0.68 cfs 0.566 af
<b>Pond 203P: Infiltration Pond #203</b>	Peak Elev=174.58' Storage=4,820 cf Inflow=1.08 cfs 0.617 af Discarded=0.21 cfs 0.233 af Primary=0.52 cfs 0.277 af Outflow=0.73 cfs 0.510 af
<b>Pond 204P: Detention Pond #204</b>	Peak Elev=178.85' Storage=4,793 cf Inflow=7.28 cfs 0.630 af Primary=4.16 cfs 0.625 af Secondary=0.00 cfs 0.000 af Outflow=4.16 cfs 0.625 af

**Pond C41P: Catch Basin #41** Peak Elev=184.38' Storage=5 cf Inflow=0.66 cfs 0.064 af  
15.0" Round Culvert n=0.012 L=54.7' S=0.0686 ' Outflow=0.66 cfs 0.064 af

**Pond C42P: Catch Basin #42** Peak Elev=181.09' Storage=12 cf Inflow=1.14 cfs 0.171 af  
15.0" Round Culvert n=0.012 L=63.0' S=0.0054 ' Outflow=1.14 cfs 0.171 af

**Pond C43P: Catch Basin #43** Peak Elev=181.13' Storage=8 cf Inflow=1.67 cfs 0.128 af  
15.0" Round Culvert n=0.012 L=60.0' S=0.0233 ' Outflow=1.67 cfs 0.128 af

**Pond C44P: Catch Basin #44** Peak Elev=180.36' Storage=17 cf Inflow=3.33 cfs 0.262 af  
15.0" Round Culvert n=0.012 L=48.0' S=0.0083 ' Outflow=3.32 cfs 0.262 af

**Pond C45P: Catch Basin #45** Peak Elev=182.13' Storage=9 cf Inflow=2.15 cfs 0.174 af  
15.0" Round Culvert n=0.012 L=87.2' S=0.0149 ' Outflow=2.15 cfs 0.174 af

**Pond C46P: Catch Basin #46** Peak Elev=180.91' Storage=11 cf Inflow=3.13 cfs 0.256 af  
15.0" Round Culvert n=0.012 L=68.0' S=0.0206 ' Outflow=3.14 cfs 0.256 af

**Pond C47P: Catch Basin #47** Peak Elev=180.08' Storage=20 cf Inflow=6.84 cfs 0.552 af  
18.0" Round Culvert n=0.012 L=40.0' S=0.0063 ' Outflow=6.84 cfs 0.552 af

**Pond C50P: Inlet Sump** Peak Elev=184.47' Storage=12 cf Inflow=3.39 cfs 0.495 af  
18.0" Round Culvert n=0.012 L=107.5' S=0.0051 ' Outflow=3.39 cfs 0.495 af

**Pond D51P: DMH #51** Peak Elev=182.01' Storage=0.000 af Inflow=0.68 cfs 0.173 af  
18.0" Round Culvert n=0.012 L=68.0' S=0.0051 ' Outflow=0.68 cfs 0.173 af

**Pond D52P: DMH #52** Peak Elev=180.99' Storage=16 cf Inflow=6.97 cfs 0.606 af  
24.0" Round Culvert n=0.012 L=110.0' S=0.0055 ' Outflow=6.97 cfs 0.606 af

**Pond D53P: DMH #53** Peak Elev=180.16' Storage=15 cf Inflow=6.97 cfs 0.606 af  
24.0" Round Culvert n=0.012 L=120.0' S=0.0083 ' Outflow=6.97 cfs 0.606 af

**Pond E01P: Existing Catch Basin** Peak Elev=184.38' Storage=11 cf Inflow=1.99 cfs 0.145 af  
15.0" Round Culvert n=0.012 L=57.0' S=0.0049 ' Outflow=1.98 cfs 0.145 af

**Pond E02P: Existing Catch Basin** Peak Elev=184.10' Storage=224 cf Inflow=6.08 cfs 0.448 af  
Discarded=0.02 cfs 0.013 af Primary=6.08 cfs 0.435 af Outflow=6.09 cfs 0.448 af

**Total Runoff Area = 16.948 ac Runoff Volume = 3.011 af Average Runoff Depth = 2.13"**  
**76.01% Pervious = 12.882 ac 23.99% Impervious = 4.066 ac**



**Summary for Subcatchment 2S: Subcat #2**

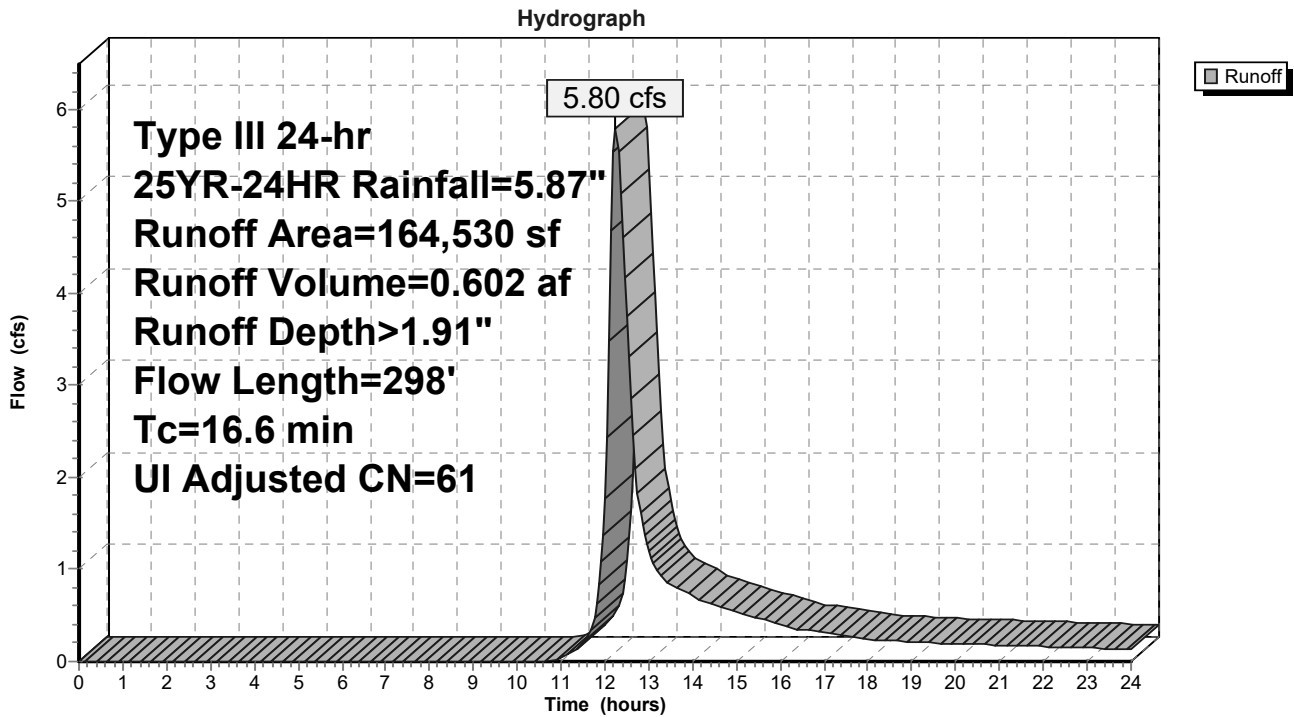
Runoff = 5.80 cfs @ 12.25 hrs, Volume= 0.602 af, Depth> 1.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Adj	Description
93,834	61		>75% Grass cover, Good, HSG B
3,750	98		Unconnected pavement, HSG B
800	98		Unconnected roofs, HSG B
7,747	96		Gravel surface, HSG B
58,399	55		Woods, Good, HSG B
164,530	62	61	Weighted Average, UI Adjusted
159,980			97.23% Pervious Area
4,550			2.77% Impervious Area
4,550			100.00% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.2	100	0.0100	0.13		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
2.6	105	0.0095	0.68		<b>Shallow Concentrated Flow, Segment #2</b> Short Grass Pasture Kv= 7.0 fps
0.2	35	0.2290	3.35		<b>Shallow Concentrated Flow, Segment #3</b> Short Grass Pasture Kv= 7.0 fps
0.6	58	0.0950	1.54		<b>Shallow Concentrated Flow, Segment #4</b> Woodland Kv= 5.0 fps
16.6	298	Total			

### Subcatchment 2S: Subcat #2



**Summary for Subcatchment 3S: Subcat. #3**

Runoff = 0.69 cfs @ 12.21 hrs, Volume= 0.084 af, Depth> 0.94"

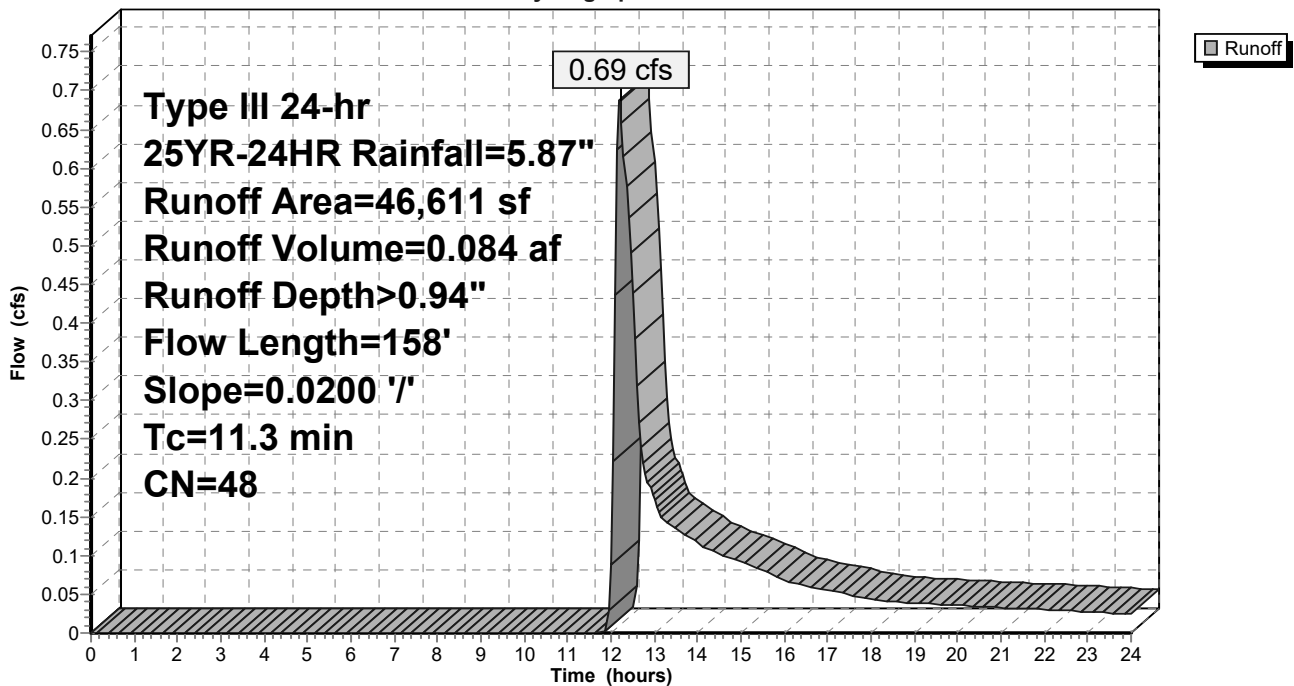
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
6,149	39	>75% Grass cover, Good, HSG A
16,252	30	Woods, Good, HSG A
14,263	61	>75% Grass cover, Good, HSG B
8,052	55	Woods, Good, HSG B
1,895	96	Gravel surface, HSG B
46,611	48	Weighted Average
46,611		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0	100	0.0200	0.17		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
0.2	12	0.0200	0.99		<b>Shallow Concentrated Flow, Segment #2</b> Short Grass Pasture Kv= 7.0 fps
1.1	46	0.0200	0.71		<b>Shallow Concentrated Flow, Segment #3</b> Woodland Kv= 5.0 fps
11.3	158	Total			

**Subcatchment 3S: Subcat. #3**

Hydrograph



**Summary for Subcatchment 4S: Subcat. #4**

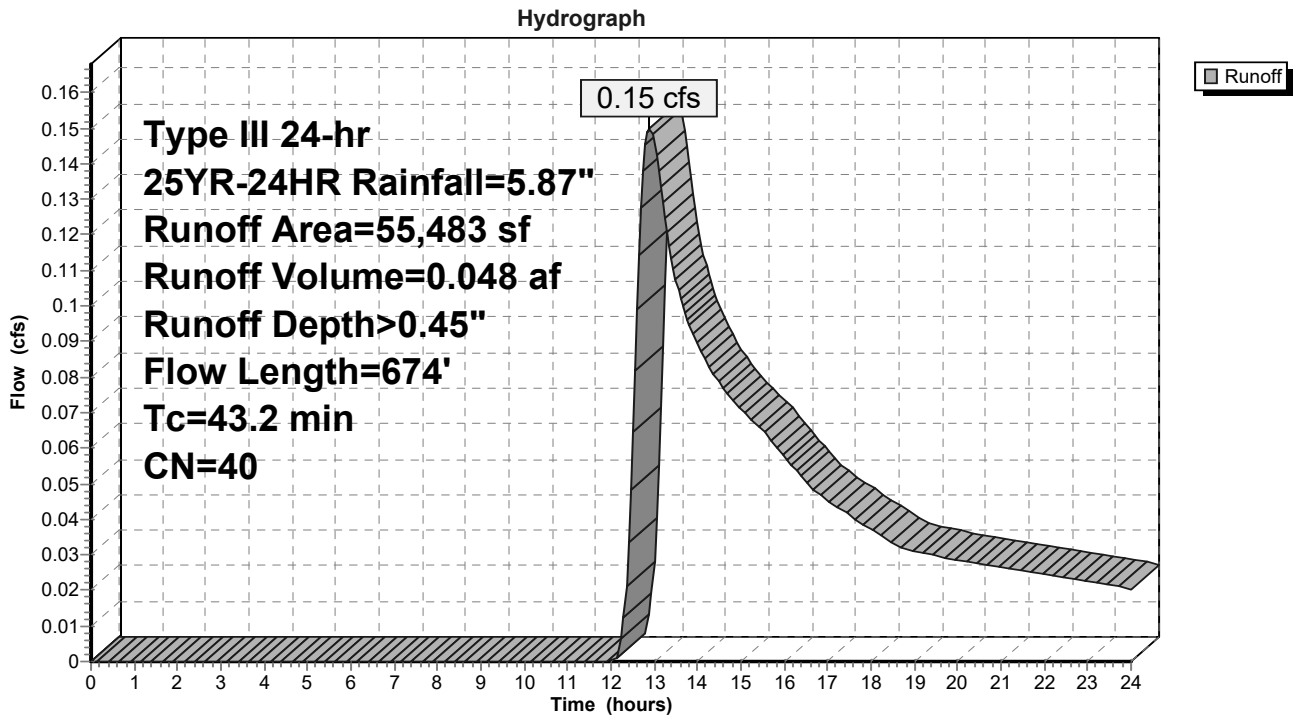
Runoff = 0.15 cfs @ 12.90 hrs, Volume= 0.048 af, Depth> 0.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
3,884	39	>75% Grass cover, Good, HSG A
34,310	30	Woods, Good, HSG A
238	96	Gravel surface, HSG A
8,394	61	>75% Grass cover, Good, HSG B
8,339	55	Woods, Good, HSG B
318	96	Gravel surface, HSG B
55,483	40	Weighted Average
55,483		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
28.9	100	0.0100	0.06		<b>Sheet Flow, Segment #1</b> Woods: Light underbrush n= 0.400 P2= 3.08"
10.0	252	0.0070	0.42		<b>Shallow Concentrated Flow, Segment #2</b> Woodland Kv= 5.0 fps
1.5	74	0.0270	0.82		<b>Shallow Concentrated Flow, Segment #3</b> Woodland Kv= 5.0 fps
1.8	157	0.0828	1.44		<b>Shallow Concentrated Flow, Segment #4</b> Woodland Kv= 5.0 fps
1.0	91	0.0440	1.47		<b>Shallow Concentrated Flow, Segment #5</b> Short Grass Pasture Kv= 7.0 fps
43.2	674	Total			

### Subcatchment 4S: Subcat. #4



**Summary for Subcatchment 30S: Subcat #30**

Runoff = 2.15 cfs @ 12.17 hrs, Volume= 0.190 af, Depth> 2.08"

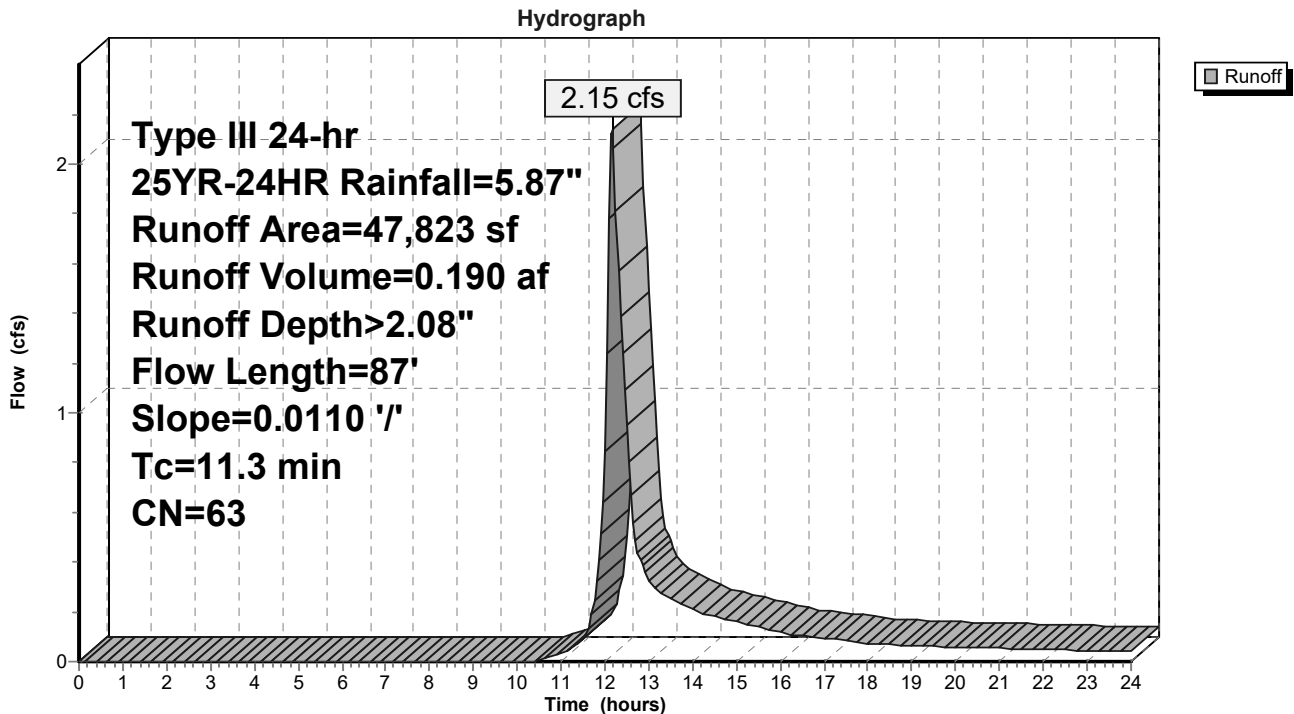
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
45,515	61	>75% Grass cover, Good, HSG B
2,126	98	Roofs, HSG B
182	96	Gravel surface, HSG B
47,823	63	Weighted Average
45,697		95.55% Pervious Area
2,126		4.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.3	87	0.0110	0.13		Sheet Flow, Segment #1

Grass: Short n= 0.150 P2= 3.08"

**Subcatchment 30S: Subcat #30**



**Summary for Subcatchment 31S: Subcat #31**

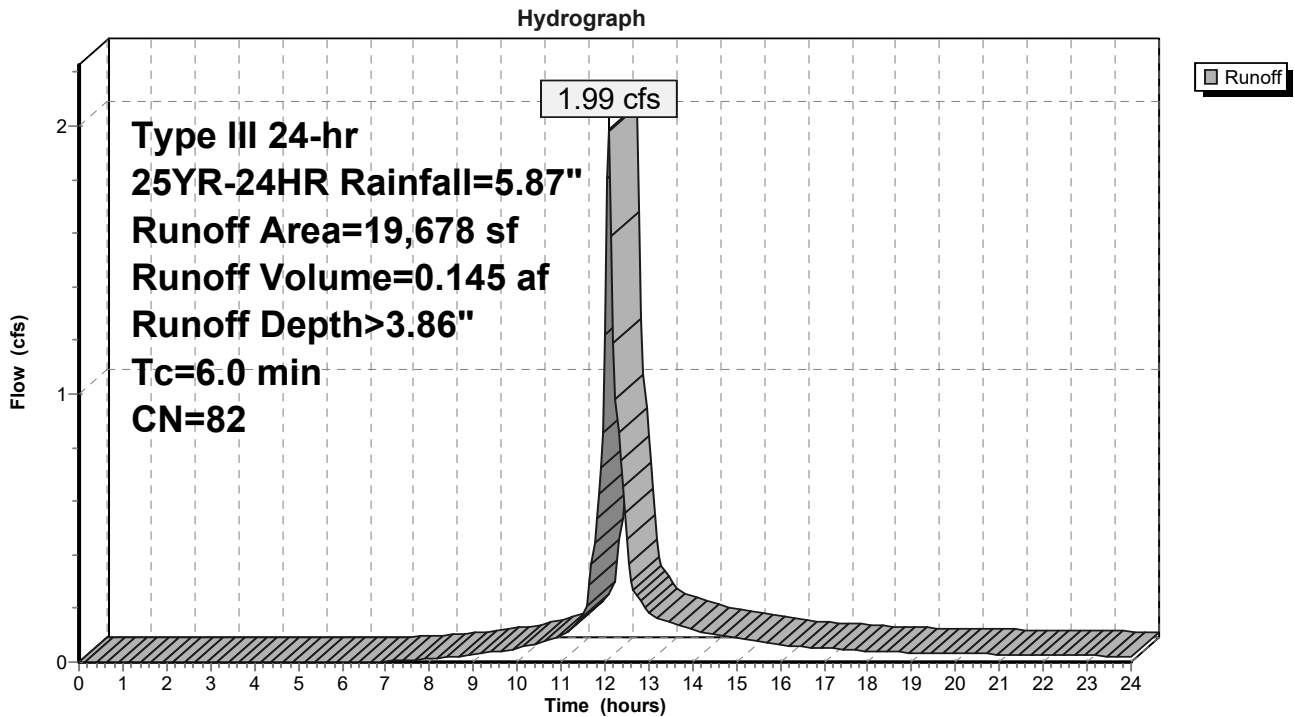
Runoff = 1.99 cfs @ 12.09 hrs, Volume= 0.145 af, Depth> 3.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
57	98	Roofs, HSG B
8,646	61	>75% Grass cover, Good, HSG B
10,975	98	Paved parking, HSG B
19,678	82	Weighted Average
8,646		43.94% Pervious Area
11,032		56.06% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Entry

**Subcatchment 31S: Subcat #31**



**Summary for Subcatchment 32S: Subcat #32**

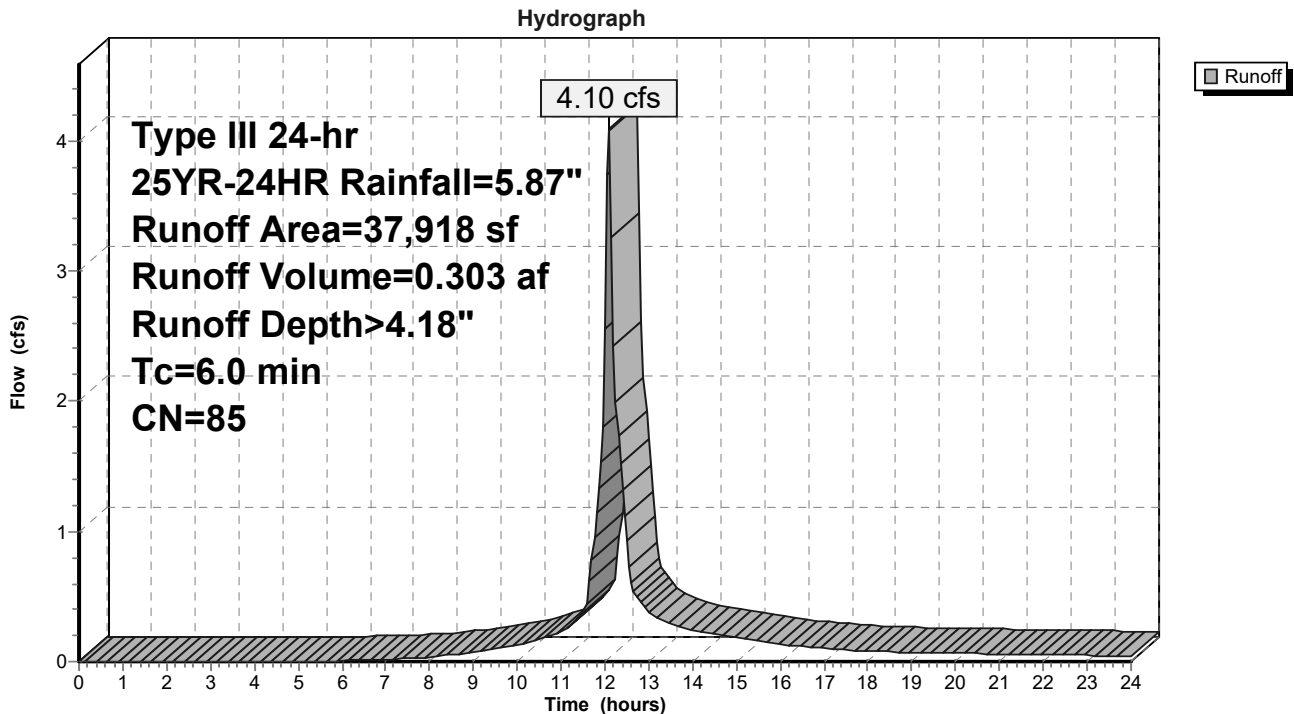
Runoff = 4.10 cfs @ 12.09 hrs, Volume= 0.303 af, Depth> 4.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
341	98	Roofs, HSG B
6,716	61	>75% Grass cover, Good, HSG B
25,189	98	Paved parking, HSG B
5,672	55	Woods, Good, HSG B
37,918	85	Weighted Average
12,388		32.67% Pervious Area
25,530		67.33% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Entry

**Subcatchment 32S: Subcat #32**





**Summary for Subcatchment 41S: Subcat #41**

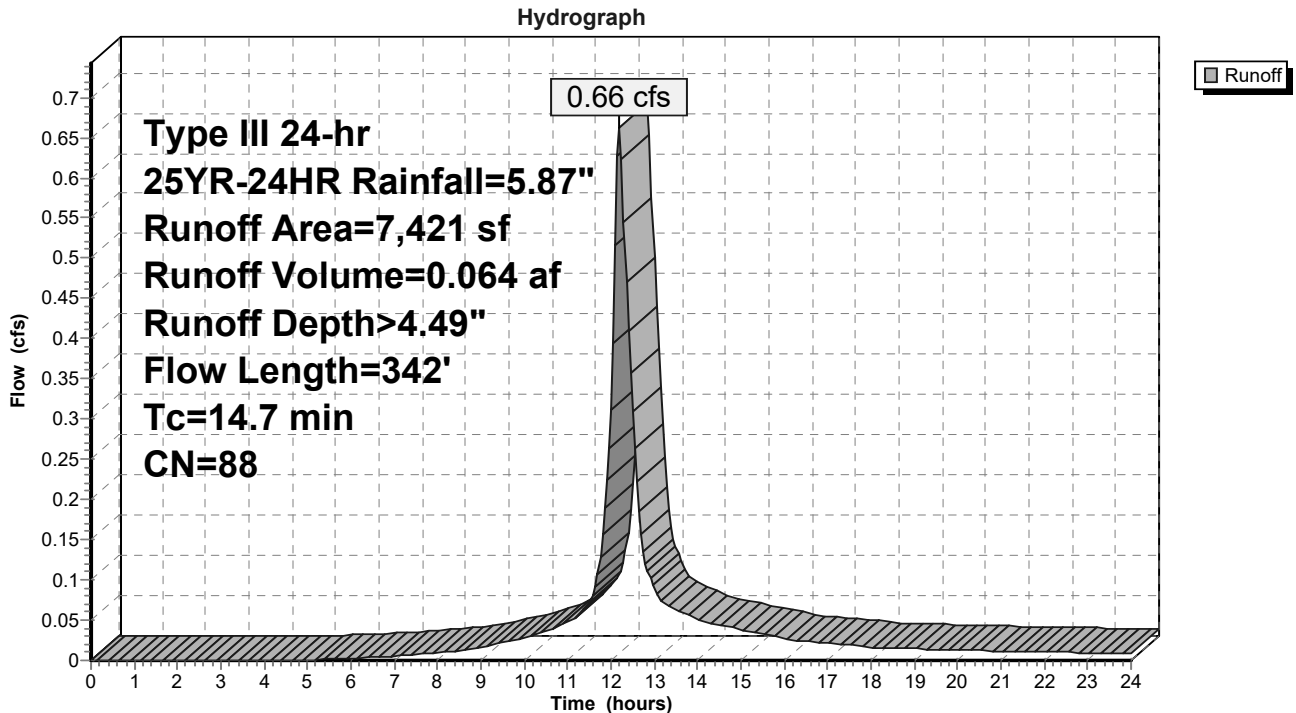
Runoff = 0.66 cfs @ 12.20 hrs, Volume= 0.064 af, Depth> 4.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
1,989	61	>75% Grass cover, Good, HSG B
4,579	98	Paved parking, HSG B
853	96	Gravel surface, HSG B
7,421	88	Weighted Average
2,842		38.30% Pervious Area
4,579		61.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.2	100	0.0100	0.13		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
1.5	242	0.0186	2.77		<b>Shallow Concentrated Flow, Segment #2</b> Paved Kv= 20.3 fps
14.7	342	Total			

**Subcatchment 41S: Subcat #41**



**Summary for Subcatchment 43S: Subcat #43**

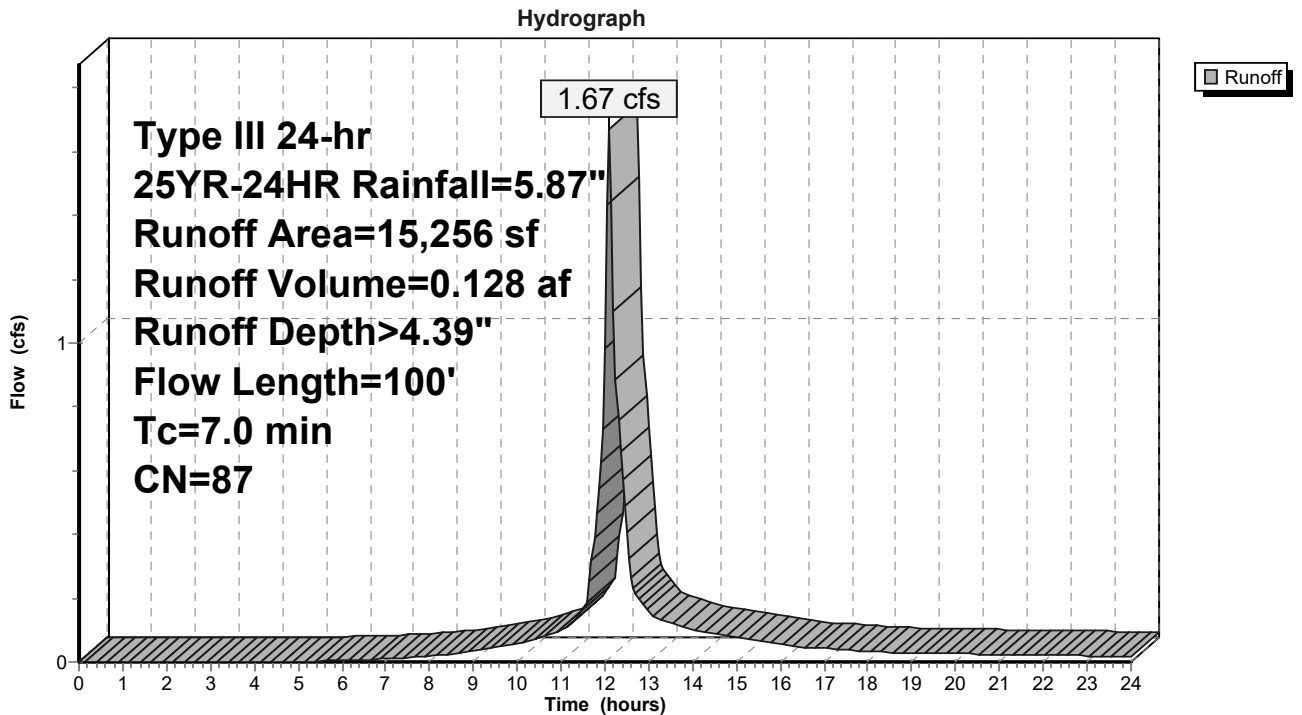
Runoff = 1.67 cfs @ 12.10 hrs, Volume= 0.128 af, Depth> 4.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
417	98	Roofs, HSG B
4,449	61	>75% Grass cover, Good, HSG B
9,410	98	Paved parking, HSG B
980	96	Gravel surface, HSG B
15,256	87	Weighted Average
5,429		35.59% Pervious Area
9,827		64.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.9	40	0.0693	0.23		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
4.1	60	0.0663	0.24		<b>Sheet Flow, Segment #2</b> Grass: Short n= 0.150 P2= 3.08"
7.0	100	Total			

**Subcatchment 43S: Subcat #43**



**Summary for Subcatchment 44S: Subcat #44**

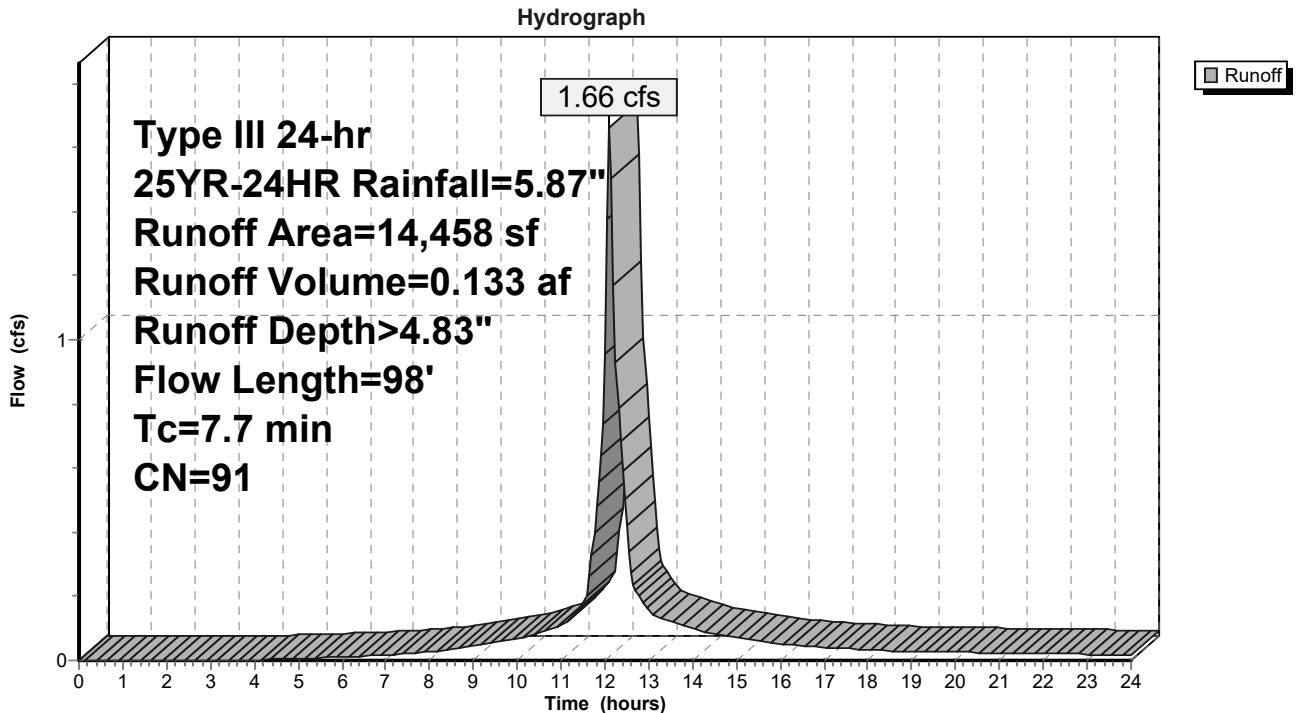
Runoff = 1.66 cfs @ 12.11 hrs, Volume= 0.133 af, Depth> 4.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
2,611	61	>75% Grass cover, Good, HSG B
11,087	98	Paved parking, HSG B
760	96	Gravel surface, HSG B
14,458	91	Weighted Average
3,371		23.32% Pervious Area
11,087		76.68% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.4	37	0.1002	0.26		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
5.3	61	0.0370	0.19		<b>Sheet Flow, Segment #2</b> Grass: Short n= 0.150 P2= 3.08"
7.7	98	Total			

**Subcatchment 44S: Subcat #44**



**Summary for Subcatchment 45S: Subcat #45**

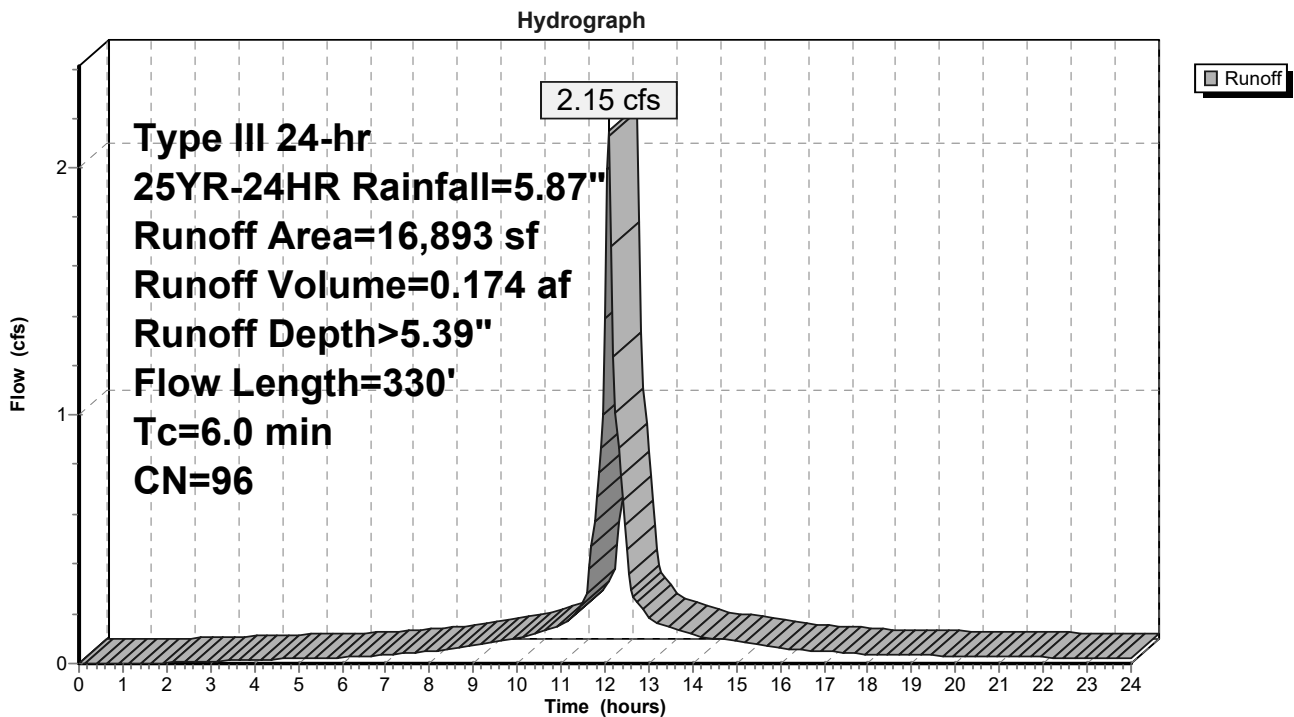
Runoff = 2.15 cfs @ 12.09 hrs, Volume= 0.174 af, Depth> 5.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
290	98	Roofs, HSG A
12	39	>75% Grass cover, Good, HSG A
5,939	98	Paved parking, HSG A
1,139	98	Roofs, HSG B
784	61	>75% Grass cover, Good, HSG B
8,550	98	Paved parking, HSG B
179	96	Gravel surface, HSG B
16,893	96	Weighted Average
975		5.77% Pervious Area
15,918		94.23% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	18	0.3010	0.35		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
2.9	18	0.0140	0.10		<b>Sheet Flow, Segment #2</b> Grass: Short n= 0.150 P2= 3.08"
1.3	294	0.0365	3.88		<b>Shallow Concentrated Flow, Segment #3</b> Paved Kv= 20.3 fps
5.1	330	Total, Increased to minimum Tc = 6.0 min			

### Subcatchment 45S: Subcat #45



**Summary for Subcatchment 46S: Subcat #46**

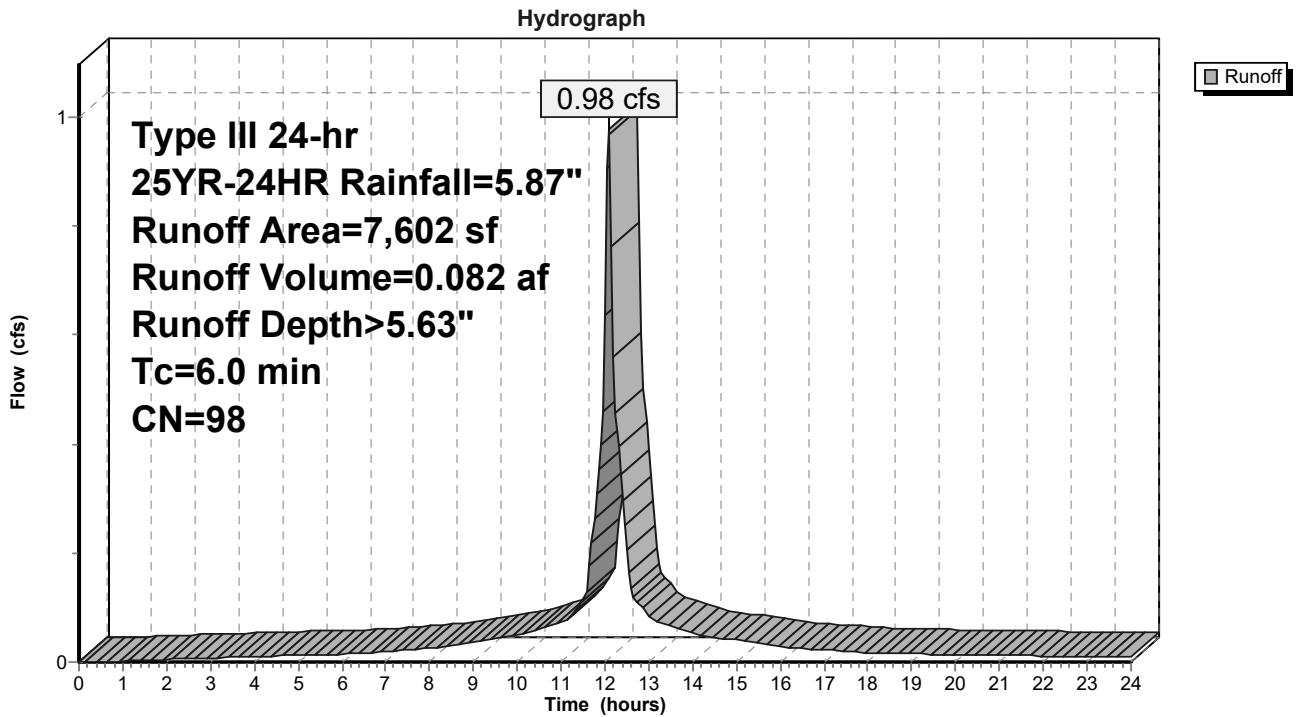
Runoff = 0.98 cfs @ 12.09 hrs, Volume= 0.082 af, Depth> 5.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
74	98	Paved parking, HSG A
7,528	98	Paved parking, HSG B
7,602	98	Weighted Average
7,602		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Entry

**Subcatchment 46S: Subcat #46**



**Summary for Subcatchment 47S: Subcat #47**

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 0.034 af, Depth> 5.63"

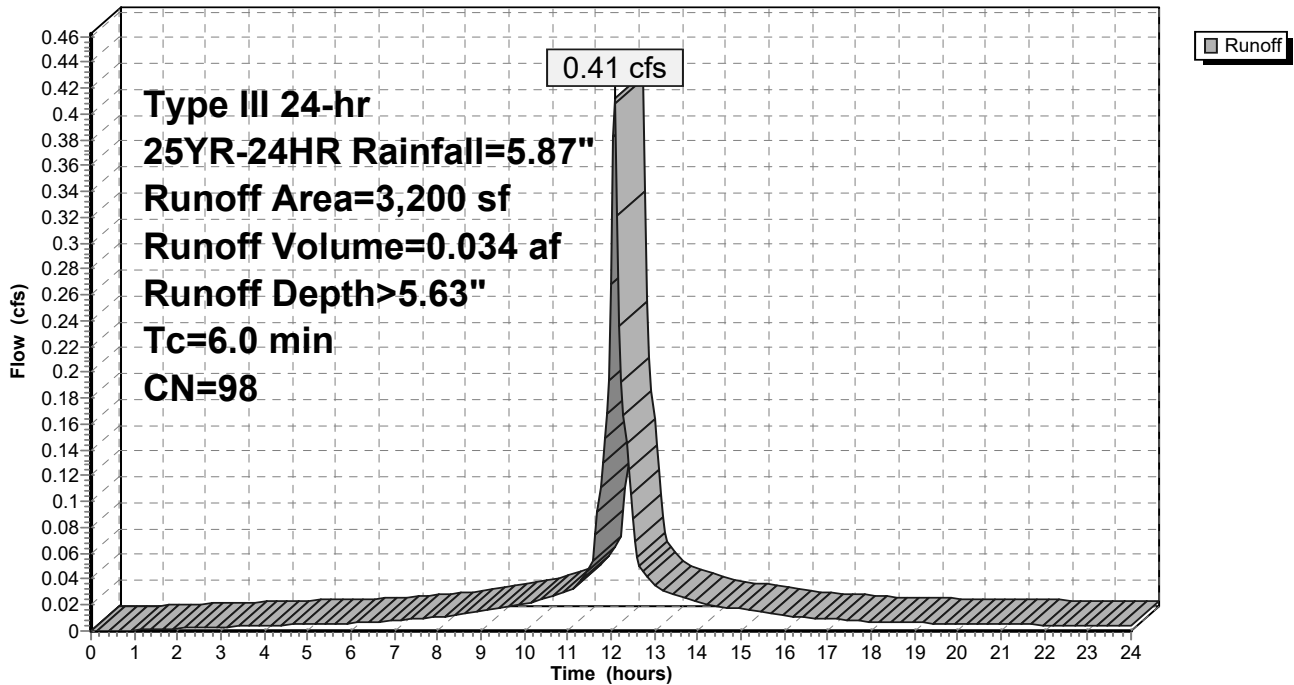
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
3,200	98	Paved roads w/curbs & sewers, HSG B
3,200		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Entry

**Subcatchment 47S: Subcat #47**

Hydrograph



**Summary for Subcatchment 50S: Subcat #50**

Runoff = 0.01 cfs @ 14.77 hrs, Volume= 0.003 af, Depth> 0.15"

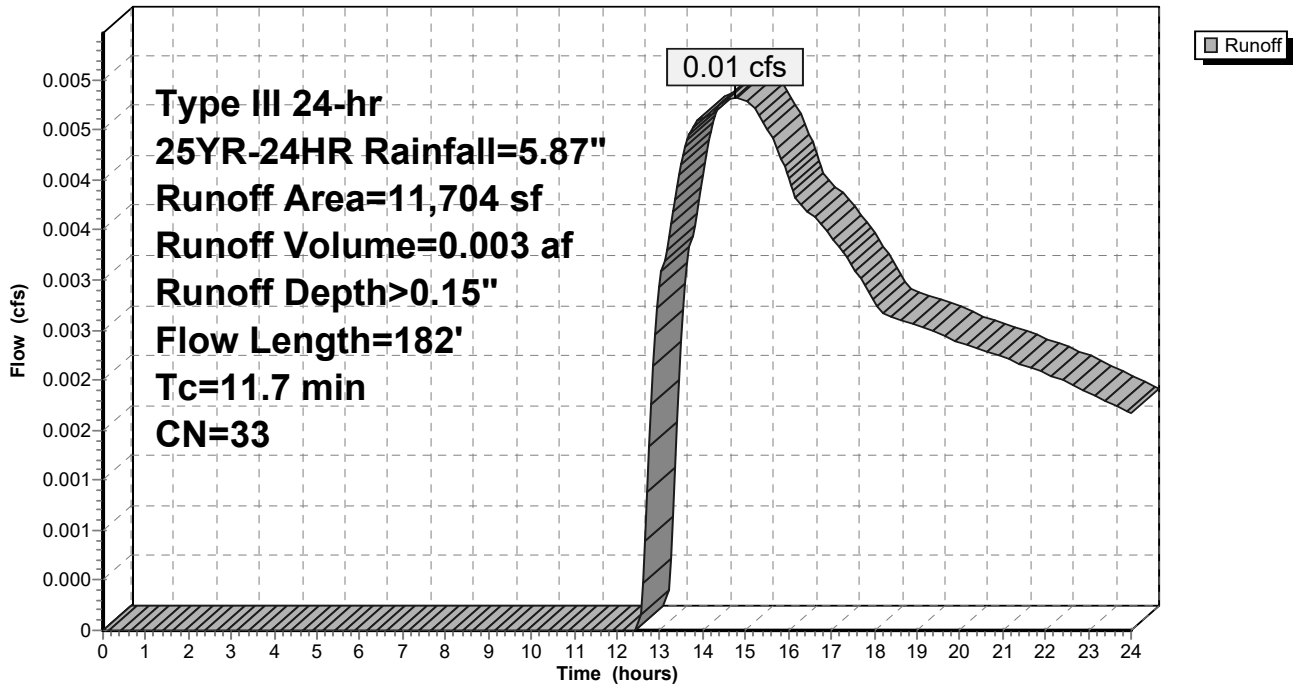
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
3,587	39	>75% Grass cover, Good, HSG A
8,117	30	Woods, Good, HSG A
11,704	33	Weighted Average
11,704		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	94	0.0319	0.20		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
3.8	88	0.0060	0.39		<b>Shallow Concentrated Flow, Segment #2</b> Woodland Kv= 5.0 fps
11.7	182	Total			

**Subcatchment 50S: Subcat #50**

Hydrograph





**Summary for Subcatchment 62S: Subcat. #62**

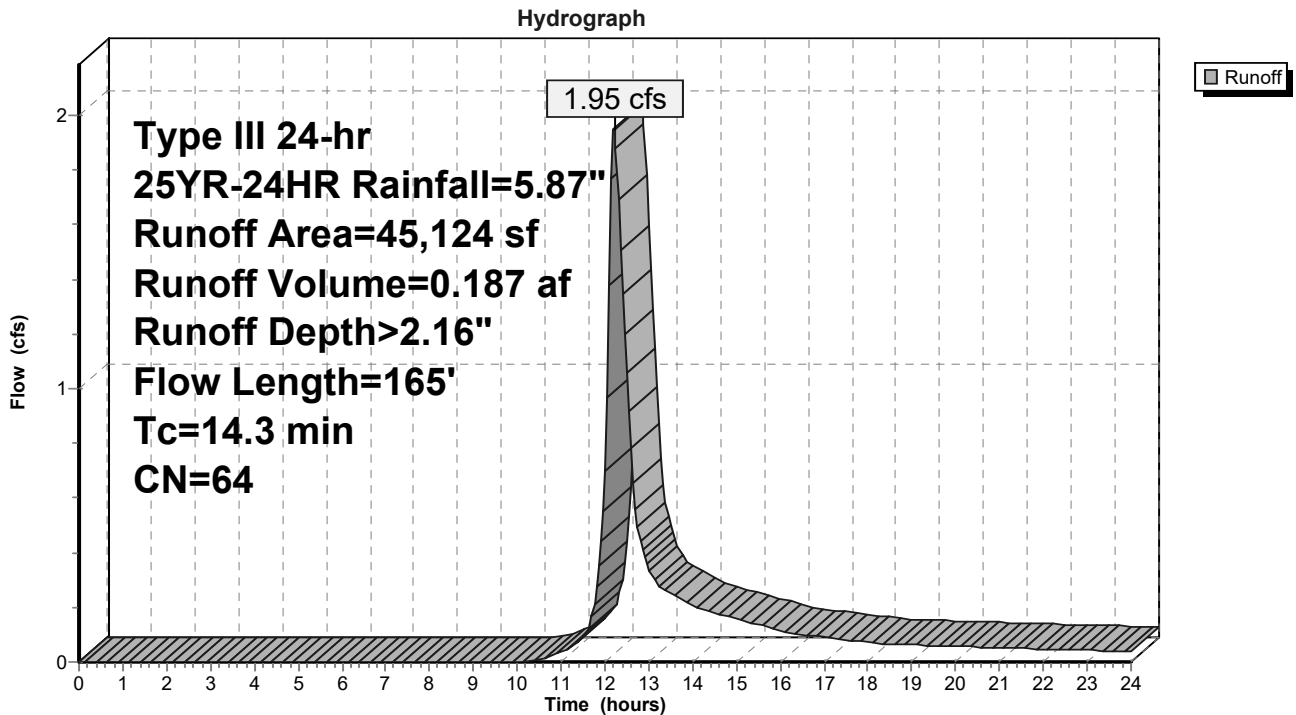
Runoff = 1.95 cfs @ 12.21 hrs, Volume= 0.187 af, Depth> 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
3,607	39	>75% Grass cover, Good, HSG A
66	98	Paved parking, HSG B
35,835	61	>75% Grass cover, Good, HSG B
5,616	96	Gravel surface, HSG B
45,124	64	Weighted Average
45,058		99.85% Pervious Area
66		0.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.2	100	0.0100	0.13		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
1.1	65	0.0200	0.99		<b>Shallow Concentrated Flow, Segment #2</b> Short Grass Pasture Kv= 7.0 fps
14.3	165	Total			

**Subcatchment 62S: Subcat. #62**



**Summary for Subcatchment 63S: Subcat. #63**

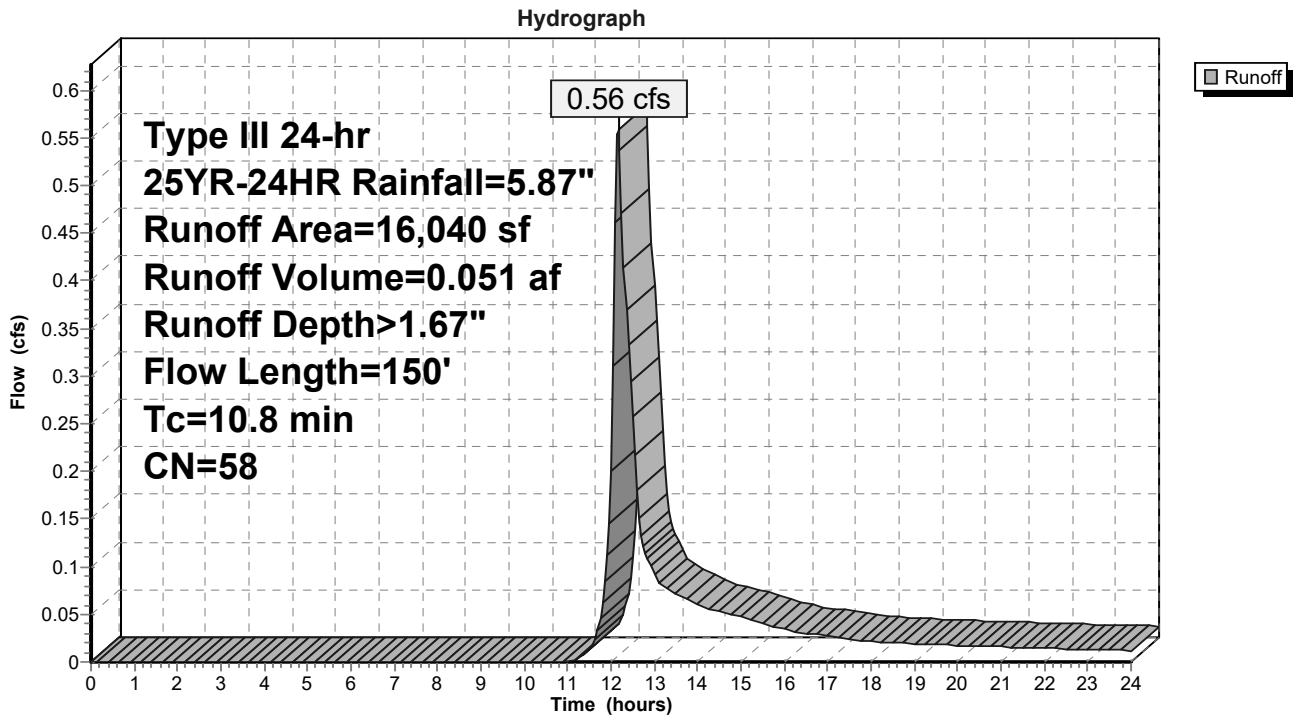
Runoff = 0.56 cfs @ 12.17 hrs, Volume= 0.051 af, Depth> 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
3,476	39	>75% Grass cover, Good, HSG A
11,805	61	>75% Grass cover, Good, HSG B
759	96	Gravel surface, HSG B
16,040	58	Weighted Average
16,040		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.5	100	0.0175	0.16		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
0.2	21	0.0595	1.71		<b>Shallow Concentrated Flow, Segment #2</b> Short Grass Pasture Kv= 7.0 fps
0.1	29	0.2410	3.44		<b>Shallow Concentrated Flow, Segment #3</b> Short Grass Pasture Kv= 7.0 fps
10.8	150	Total			

**Subcatchment 63S: Subcat. #63**



**Summary for Subcatchment 64S: Subcat #64**

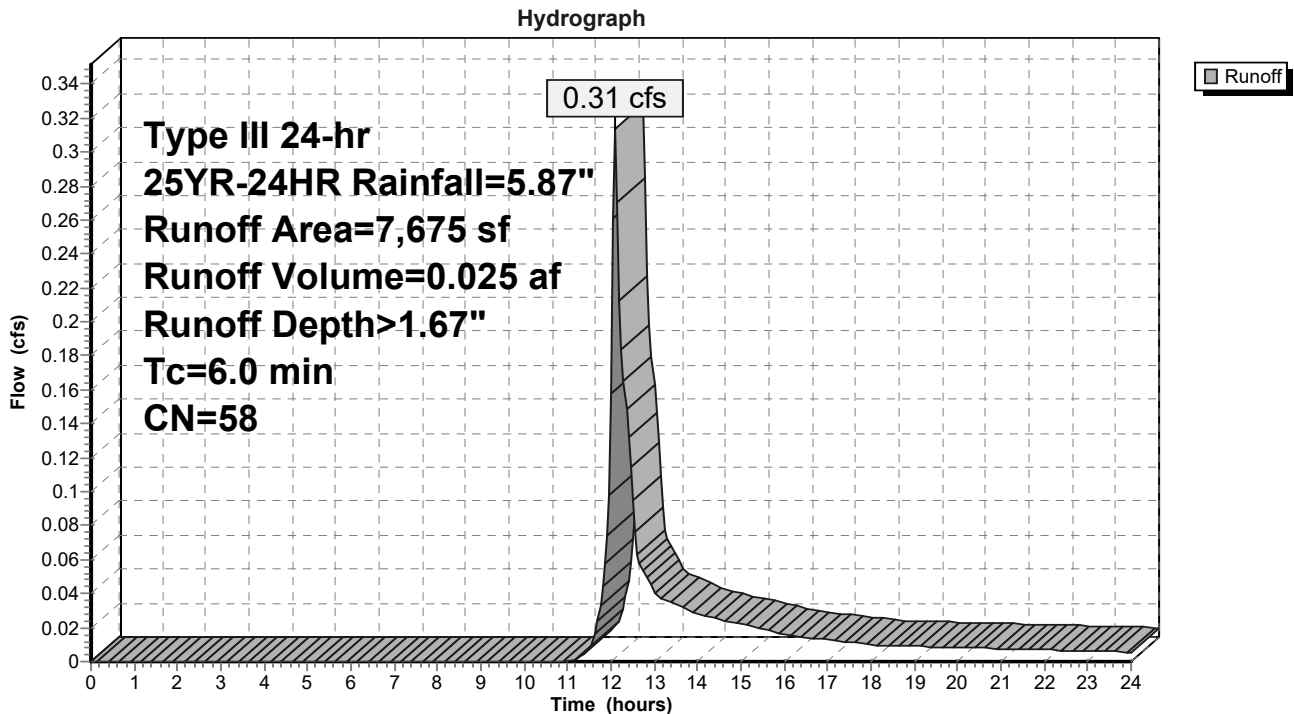
Runoff = 0.31 cfs @ 12.10 hrs, Volume= 0.025 af, Depth> 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
1,389	39	>75% Grass cover, Good, HSG A
178	30	Woods, Good, HSG A
265	96	Gravel surface, HSG B
5,653	61	>75% Grass cover, Good, HSG B
190	96	Gravel surface, HSG B
7,675	58	Weighted Average
7,675		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Entry

**Subcatchment 64S: Subcat #64**



**Summary for Subcatchment 70S: Subcat #70**

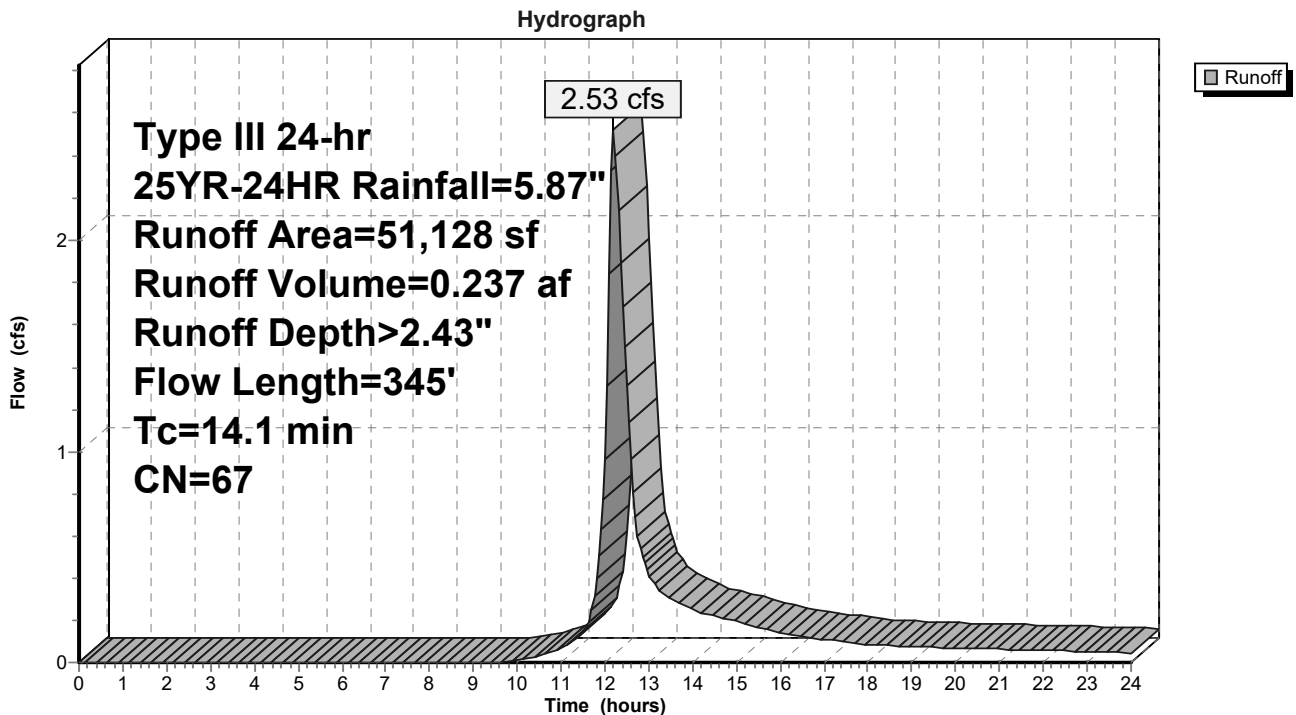
Runoff = 2.53 cfs @ 12.21 hrs, Volume= 0.237 af, Depth> 2.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
19,630	39	>75% Grass cover, Good, HSG A
10,924	98	Paved parking, HSG A
3,813	30	Woods, Good, HSG A
5,208	61	>75% Grass cover, Good, HSG B
11,553	98	Paved parking, HSG B
51,128	67	Weighted Average
28,651		56.04% Pervious Area
22,477		43.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0	100	0.0200	0.17		<b>Sheet Flow, Segment #1</b> Grass: Short n= 0.150 P2= 3.08"
4.1	245	0.0204	1.00		<b>Shallow Concentrated Flow, Segment #2</b> Short Grass Pasture Kv= 7.0 fps
14.1	345	Total			

**Subcatchment 70S: Subcat #70**



**Summary for Subcatchment 71S: Subcat #71**

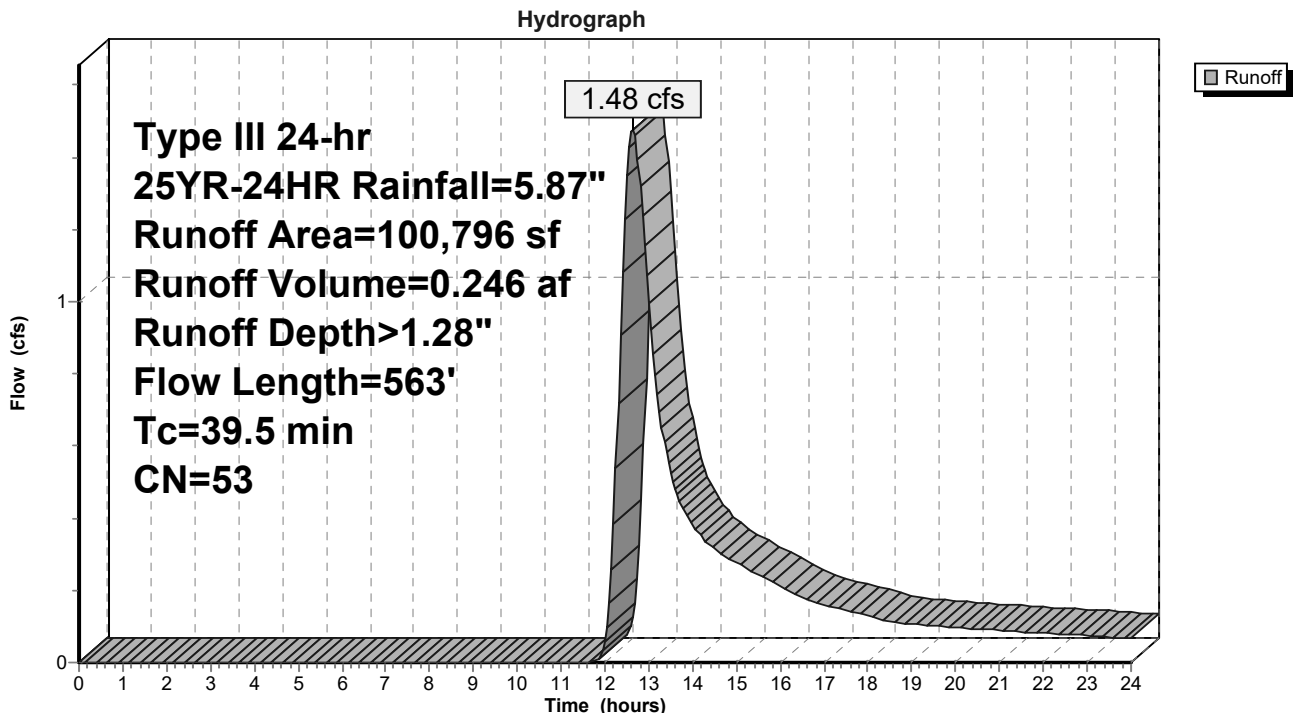
Runoff = 1.48 cfs @ 12.63 hrs, Volume= 0.246 af, Depth> 1.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
35,048	39	>75% Grass cover, Good, HSG A
29,681	98	Paved parking, HSG A
36,067	30	Woods, Good, HSG A
100,796	53	Weighted Average
71,115		70.55% Pervious Area
29,681		29.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.6	100	0.0150	0.07		<b>Sheet Flow, Segment #1</b> Woods: Light underbrush n= 0.400 P2= 3.08"
11.4	285	0.0070	0.42		<b>Shallow Concentrated Flow, Segment #2</b> Woodland Kv= 5.0 fps
0.6	65	0.0615	1.74		<b>Shallow Concentrated Flow, Segment #3</b> Short Grass Pasture Kv= 7.0 fps
2.9	113	0.0088	0.66		<b>Shallow Concentrated Flow, Segment #4</b> Short Grass Pasture Kv= 7.0 fps
39.5	563	Total			

**Subcatchment 71S: Subcat #71**



**Summary for Subcatchment 72S: Subcat #72**

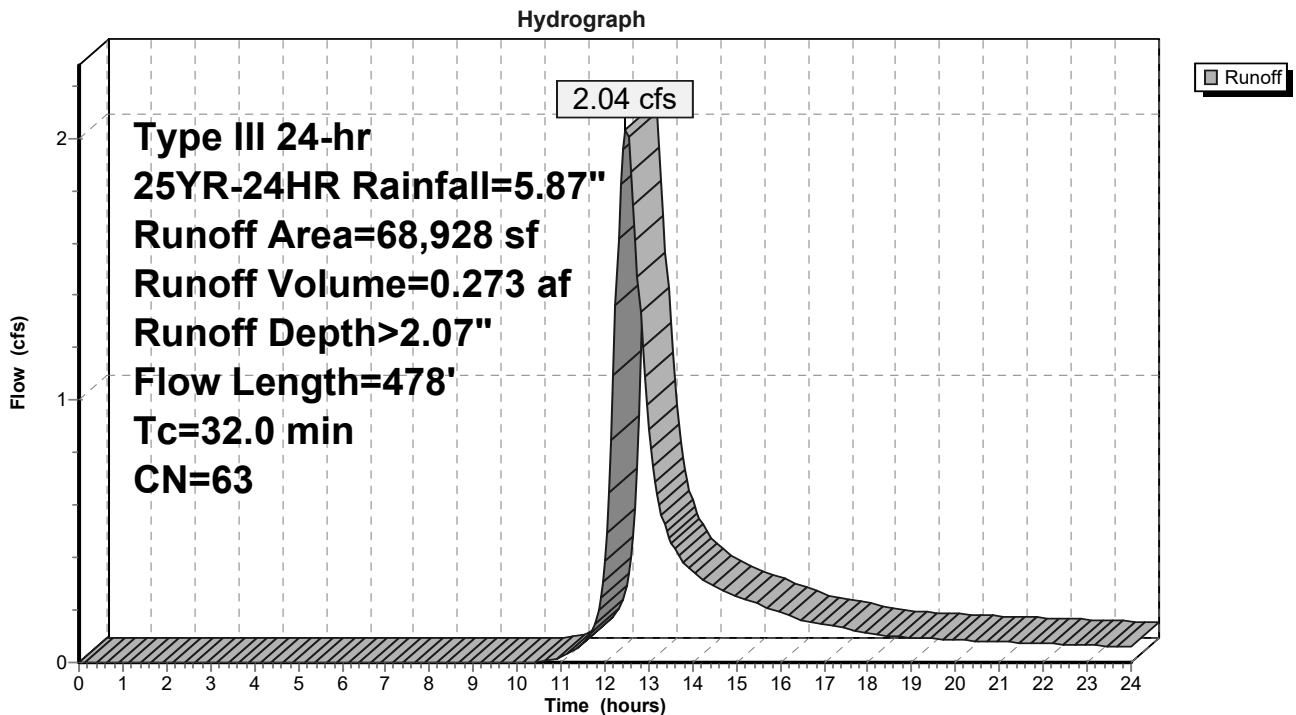
Runoff = 2.04 cfs @ 12.48 hrs, Volume= 0.273 af, Depth> 2.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 25YR-24HR Rainfall=5.87"

Area (sf)	CN	Description
32,729	39	>75% Grass cover, Good, HSG A
29,456	98	Paved parking, HSG A
6,743	30	Woods, Good, HSG A
68,928	63	Weighted Average
39,472		57.27% Pervious Area
29,456		42.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.6	100	0.0150	0.07		<b>Sheet Flow, Segment #1</b> Woods: Light underbrush n= 0.400 P2= 3.08"
0.7	27	0.0150	0.61		<b>Shallow Concentrated Flow, Segment #2</b> Woodland Kv= 5.0 fps
6.7	351	0.0157	0.88		<b>Shallow Concentrated Flow, Segment #3</b> Short Grass Pasture Kv= 7.0 fps
32.0	478	Total			

**Subcatchment 72S: Subcat #72**



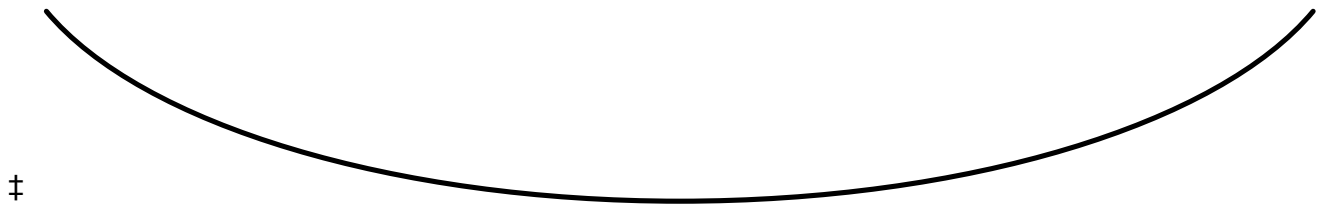
Summary for Reach 30aR: Overland Flow

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

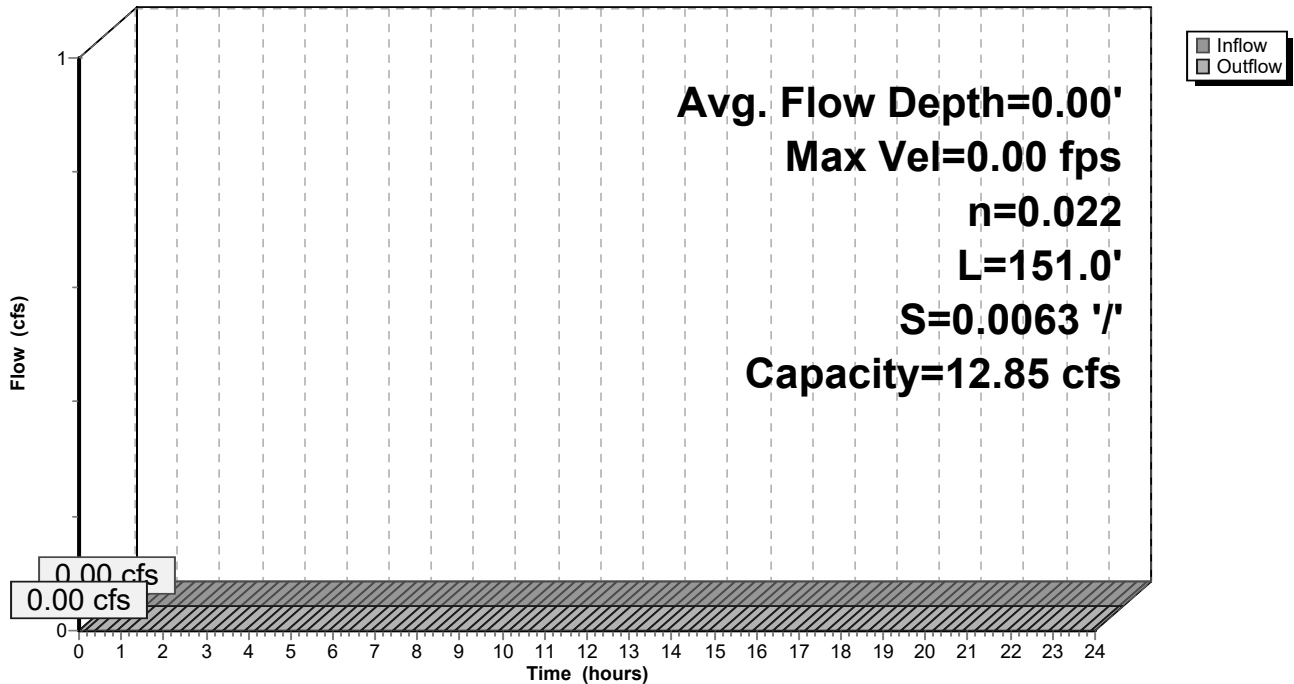
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 12.85 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight
Length= 151.0' Slope= 0.0063 '/
Inlet Invert= 183.95', Outlet Invert= 183.00'



Reach 30aR: Overland Flow

Hydrograph



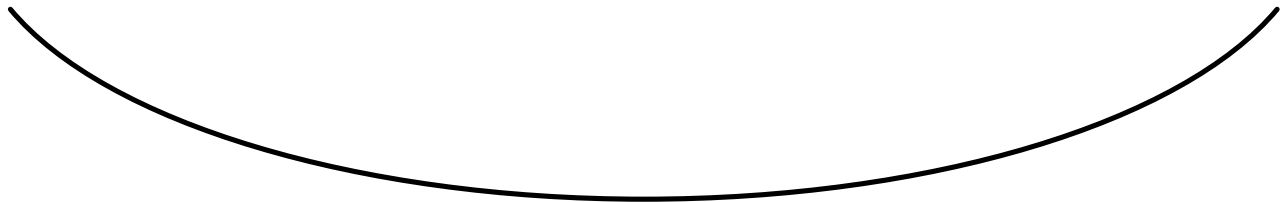
Summary for Reach 30bR: Overland Flow

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

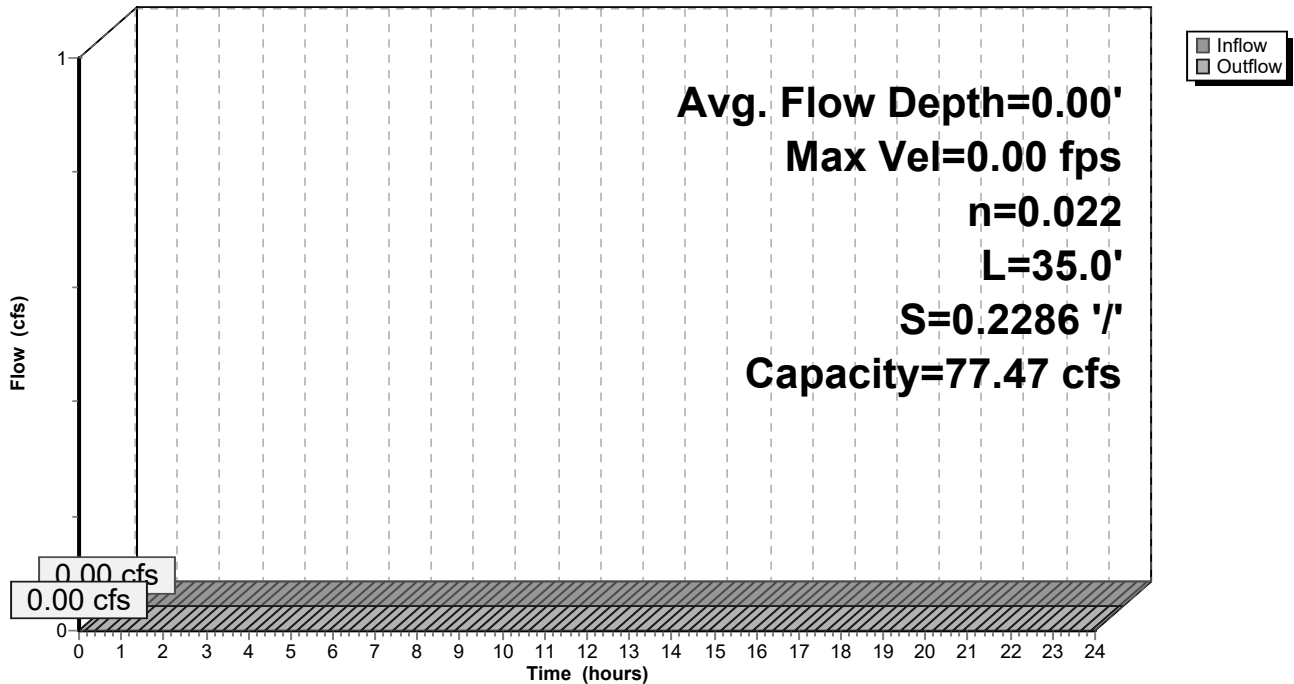
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 77.47 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight
Length= 35.0' Slope= 0.2286 '/'
Inlet Invert= 183.00', Outlet Invert= 175.00'



Reach 30bR: Overland Flow

Hydrograph





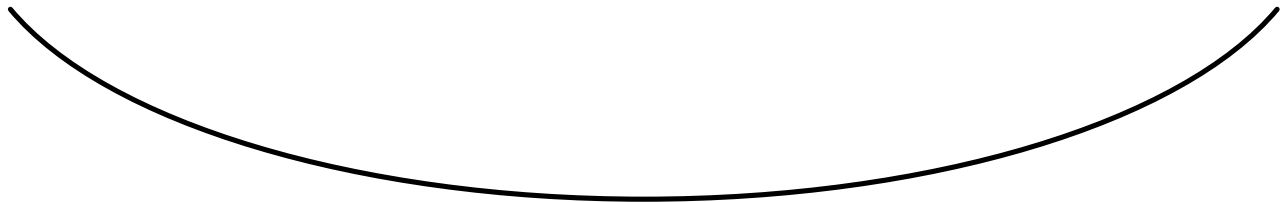
Summary for Reach 30cR: Overland Flow

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

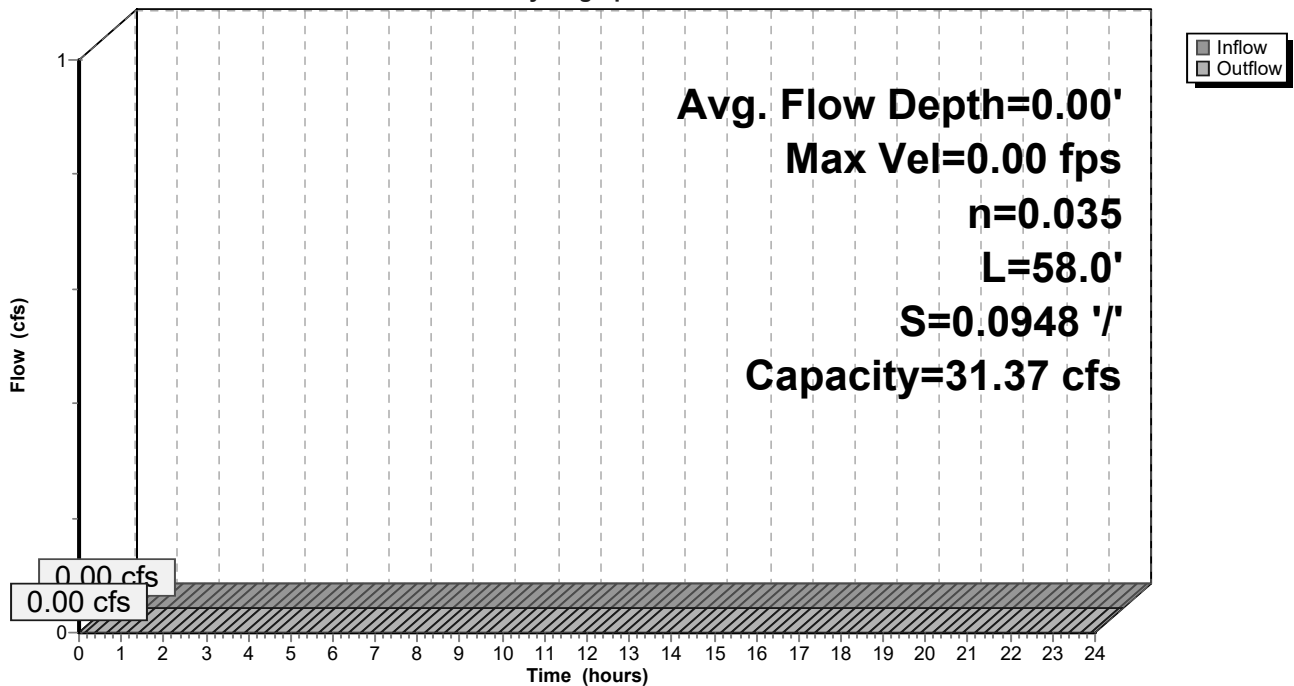
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 31.37 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.035 Earth, dense weeds
Length= 58.0' Slope= 0.0948 '/'
Inlet Invert= 175.00', Outlet Invert= 169.50'



Reach 30cR: Overland Flow

Hydrograph



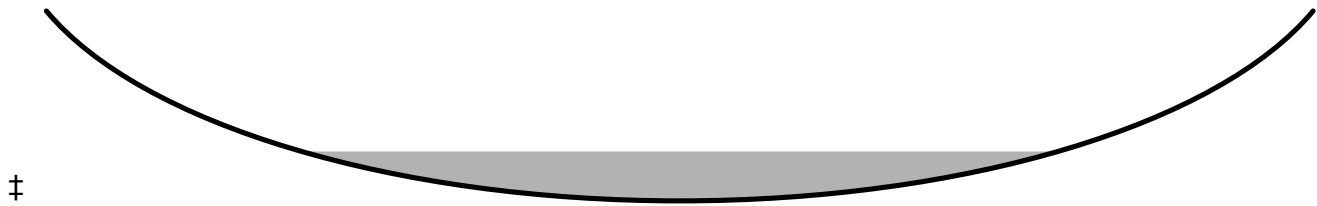
### Summary for Reach 71aR: Wooded Swale

Inflow Area = 3.896 ac, 34.84% Impervious, Inflow Depth > 1.52" for 25YR-24HR event  
 Inflow = 3.40 cfs @ 12.58 hrs, Volume= 0.492 af  
 Outflow = 3.39 cfs @ 12.59 hrs, Volume= 0.492 af, Atten= 0%, Lag= 0.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 1.28 fps, Min. Travel Time= 1.0 min  
 Avg. Velocity = 0.62 fps, Avg. Travel Time= 2.1 min

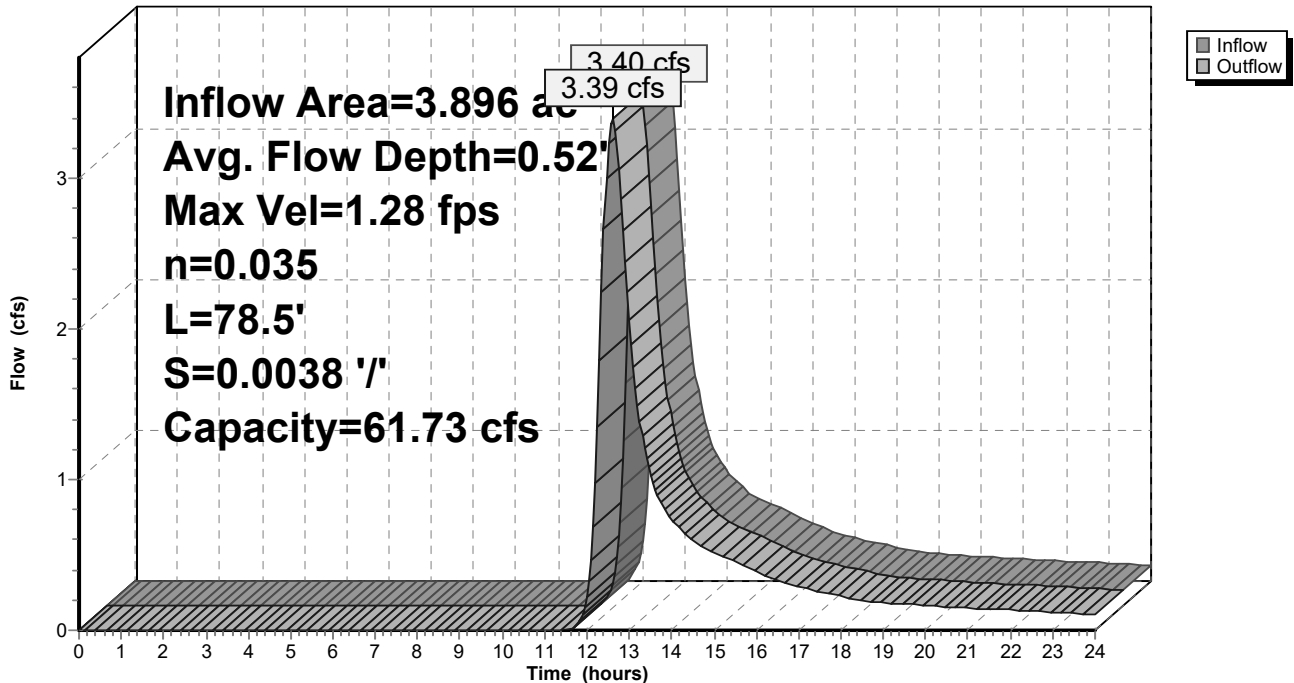
Peak Storage= 208 cf @ 12.59 hrs  
 Average Depth at Peak Storage= 0.52'  
 Bank-Full Depth= 2.00' Flow Area= 20.0 sf, Capacity= 61.73 cfs

15.00' x 2.00' deep Parabolic Channel, n= 0.035  
 Length= 78.5' Slope= 0.0038 '/  
 Inlet Invert= 187.80', Outlet Invert= 187.50'



### Reach 71aR: Wooded Swale

Hydrograph



### Summary for Reach 72R: Roadside Swale

[80] Warning: Exceeded Pond 72P by 0.02' @ 12.25 hrs (1.74 cfs 0.063 af)

Inflow Area = 1.582 ac, 42.73% Impervious, Inflow Depth > 1.87" for 25YR-24HR event  
 Inflow = 2.00 cfs @ 12.49 hrs, Volume= 0.247 af  
 Outflow = 1.95 cfs @ 12.55 hrs, Volume= 0.246 af, Atten= 2%, Lag= 3.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 1.71 fps, Min. Travel Time= 4.8 min  
 Avg. Velocity = 0.74 fps, Avg. Travel Time= 11.2 min

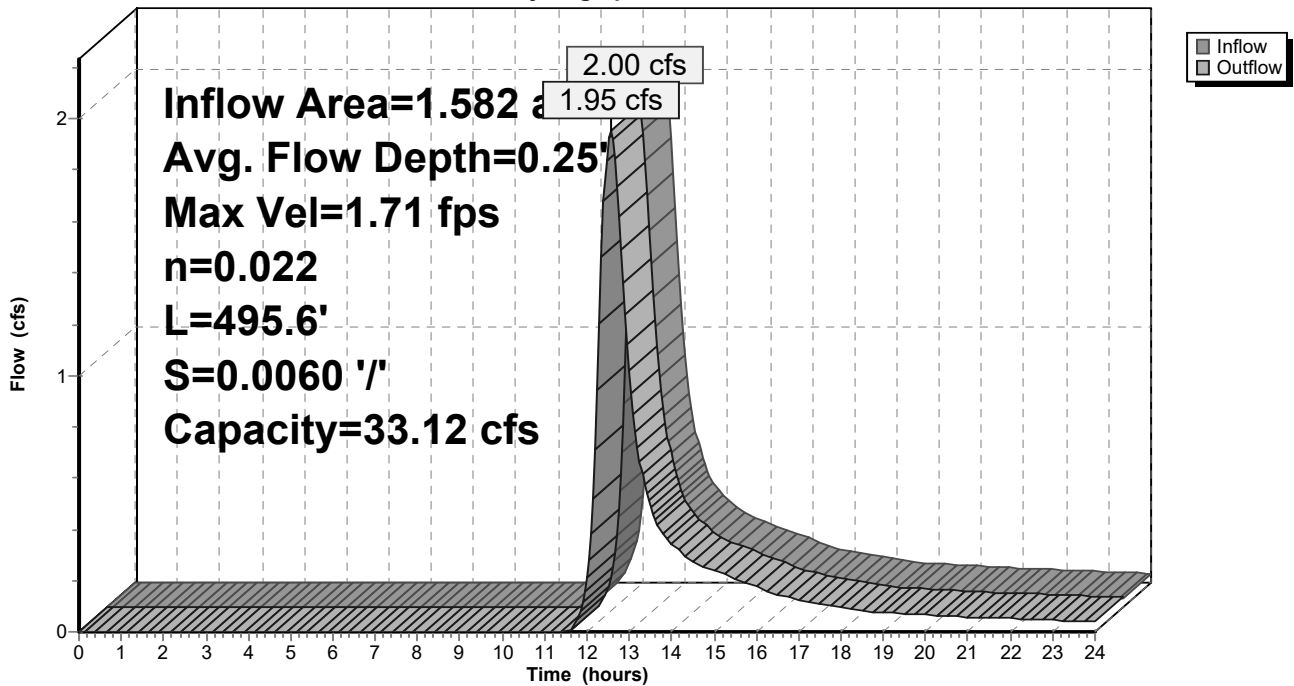
Peak Storage= 566 cf @ 12.55 hrs  
 Average Depth at Peak Storage= 0.25'  
 Bank-Full Depth= 1.00' Flow Area= 9.0 sf, Capacity= 33.12 cfs

3.00' x 1.00' deep channel, n= 0.022  
 Side Slope Z-value= 6.0 '/' Top Width= 15.00'  
 Length= 495.6' Slope= 0.0060 '/'  
 Inlet Invert= 195.95', Outlet Invert= 193.00'



### Reach 72R: Roadside Swale

Hydrograph



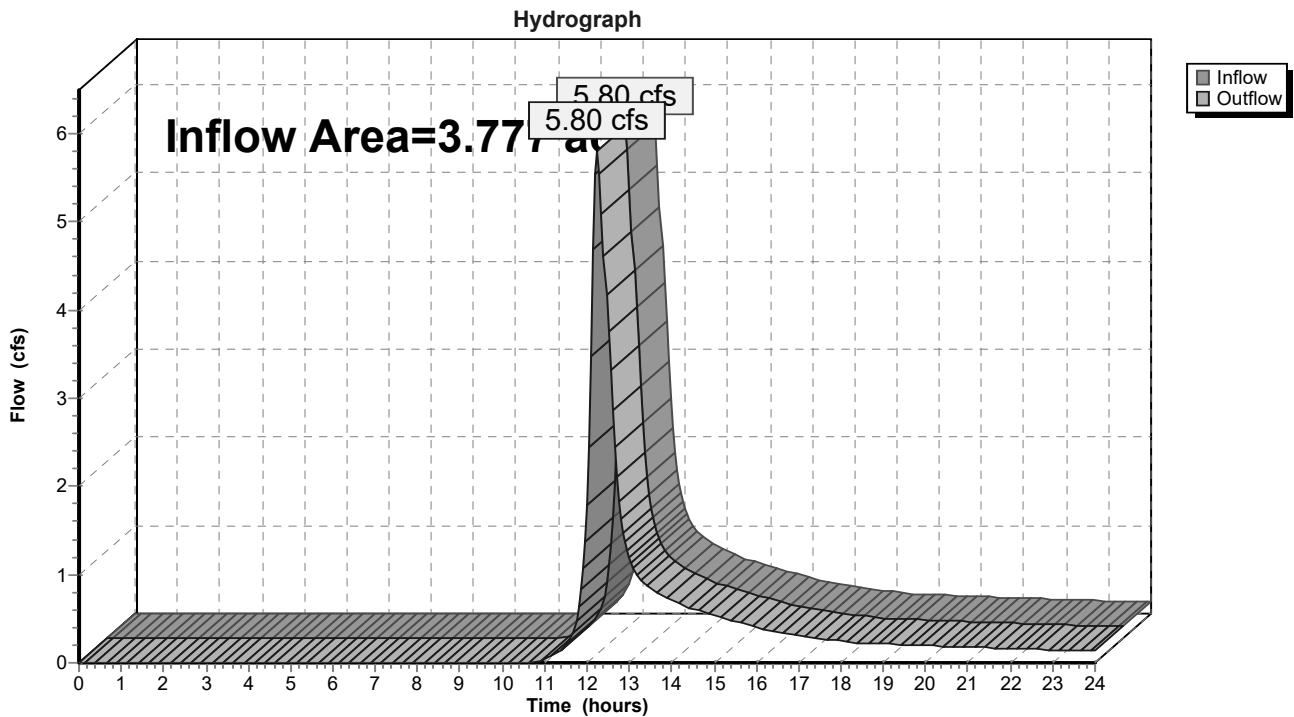
### Summary for Reach 200R: Final Reach #200

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 3.777 ac, 2.77% Impervious, Inflow Depth > 1.91" for 25YR-24HR event  
Inflow = 5.80 cfs @ 12.25 hrs, Volume= 0.602 af  
Outflow = 5.80 cfs @ 12.25 hrs, Volume= 0.602 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach 200R: Final Reach #200



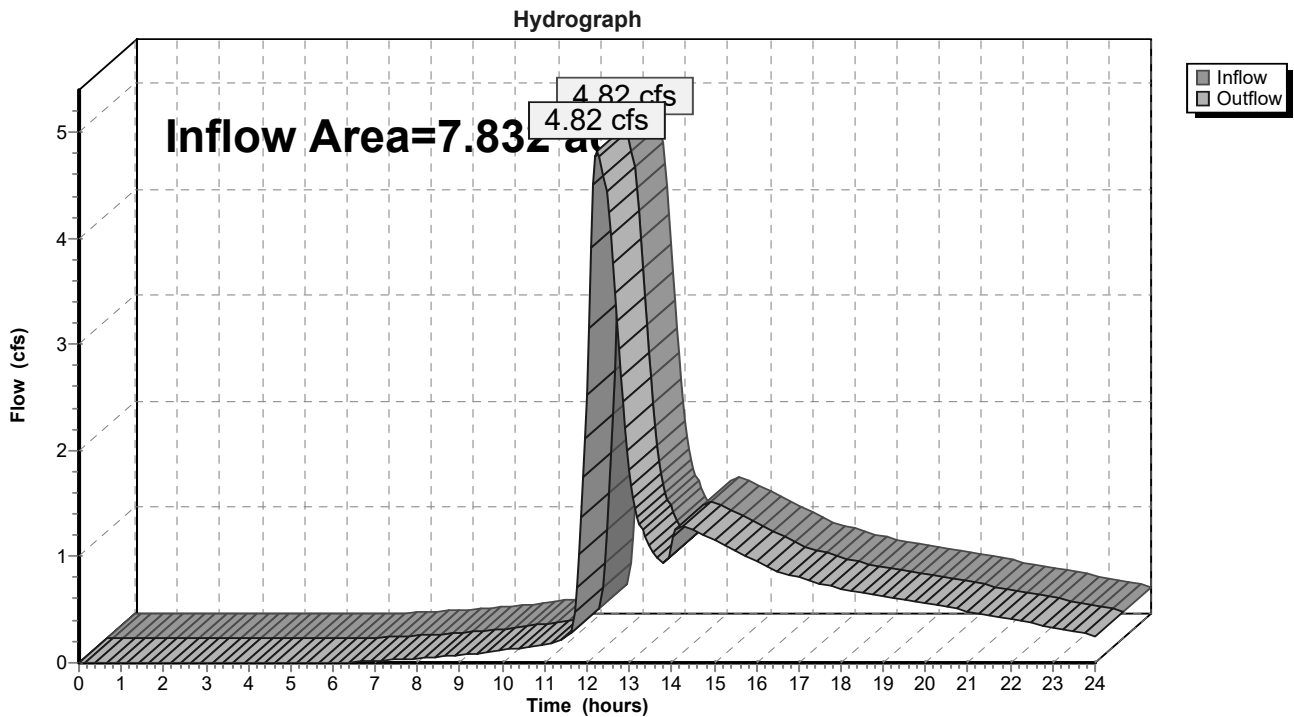
### Summary for Reach 300R: Final Reach #300

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 7.832 ac, 26.66% Impervious, Inflow Depth > 1.58" for 25YR-24HR event  
Inflow = 4.82 cfs @ 12.25 hrs, Volume= 1.034 af  
Outflow = 4.82 cfs @ 12.25 hrs, Volume= 1.034 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach 300R: Final Reach #300



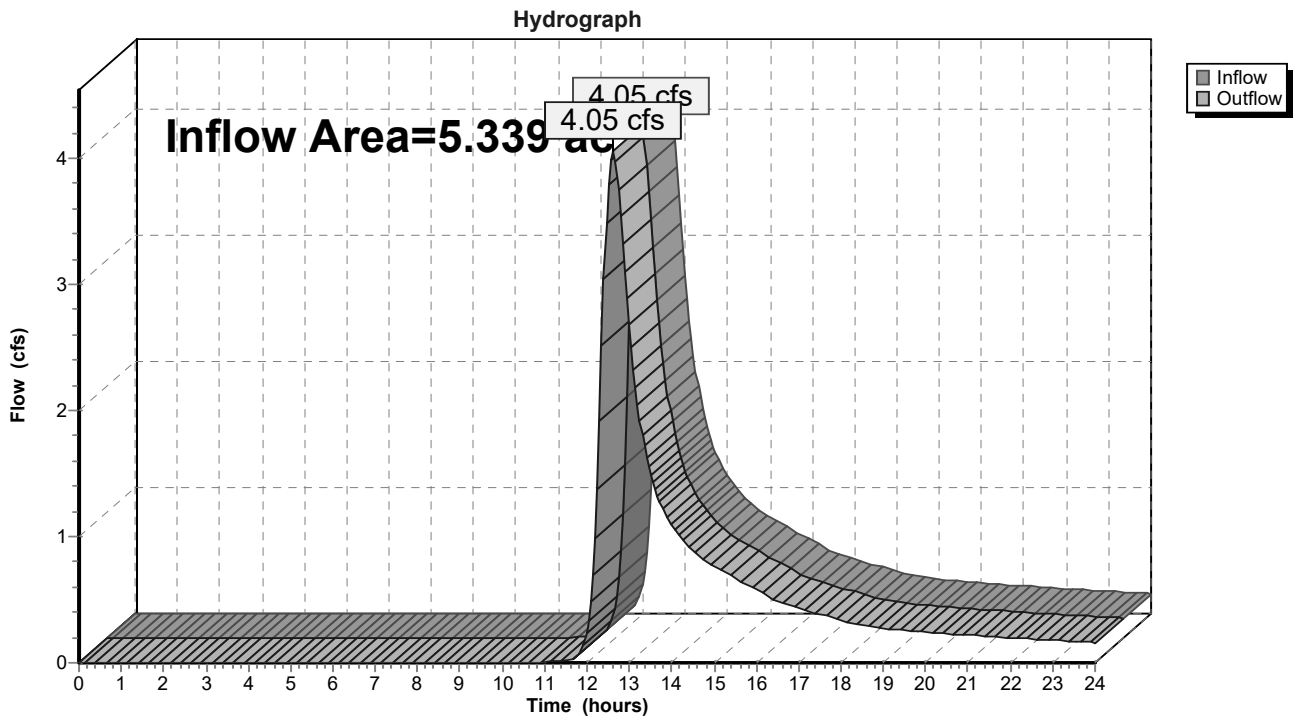
### Summary for Reach 400R: Final Reach #400

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 5.339 ac, 35.09% Impervious, Inflow Depth > 1.50" for 25YR-24HR event  
Inflow = 4.05 cfs @ 12.61 hrs, Volume= 0.668 af  
Outflow = 4.05 cfs @ 12.61 hrs, Volume= 0.668 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

### Reach 400R: Final Reach #400



**Summary for Pond 30P: Infiltration/Trench**

Inflow Area = 1.098 ac, 4.45% Impervious, Inflow Depth > 2.08" for 25YR-24HR event  
 Inflow = 2.15 cfs @ 12.17 hrs, Volume= 0.190 af  
 Outflow = 1.10 cfs @ 12.45 hrs, Volume= 0.188 af, Atten= 49%, Lag= 16.7 min  
 Discarded = 0.59 cfs @ 12.45 hrs, Volume= 0.081 af  
 Primary = 0.51 cfs @ 12.45 hrs, Volume= 0.107 af  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 183.82' @ 12.45 hrs Surf.Area= 8,493 sf Storage= 1,384 cf  
 Flood Elev= 184.10' Surf.Area= 14,837 sf Storage= 4,848 cf

Plug-Flow detention time= 17.4 min calculated for 0.188 af (99% of inflow)  
 Center-of-Mass det. time= 11.5 min ( 871.3 - 859.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	183.50'	6,797 cf	<b>Ponding Area (Irregular)</b> Listed below (Recalc)
#2	182.75'	162 cf	<b>Stone Trench (Irregular)</b> Listed below (Recalc)
			480 cf Overall - 75 cf Embedded = 405 cf x 40.0% Voids
#3	183.15'	42 cf	<b>6.0" Round 6" HDPE N-12</b> Inside #2
			L= 215.0'
			75 cf Overall - 1.0" Wall Thickness = 42 cf
		7,001 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
183.50	538	154.3	0	0	538
183.75	6,179	527.1	712	712	20,753
184.00	14,357	677.3	2,496	3,208	35,149
184.25	14,357	677.3	3,589	6,797	35,319

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
182.75	480	432.4	0	0	480
183.75	480	432.4	480	480	912

Device	Routing	Invert	Outlet Devices
#1	Discarded	182.75'	<b>3.000 in/hr Infiltration over Surface area</b>
#2	Primary	183.15'	<b>6.0" Round 6" HDPE N-12</b> L= 1.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 183.15' / 183.15' S= 0.0000 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#3	Secondary	183.95'	<b>10.0' long x 10.0' breadth Overflow to Wetland</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

**Discarded OutFlow** Max=0.59 cfs @ 12.45 hrs HW=183.82' (Free Discharge)

↳ **1=Infiltration** (Exfiltration Controls 0.59 cfs)

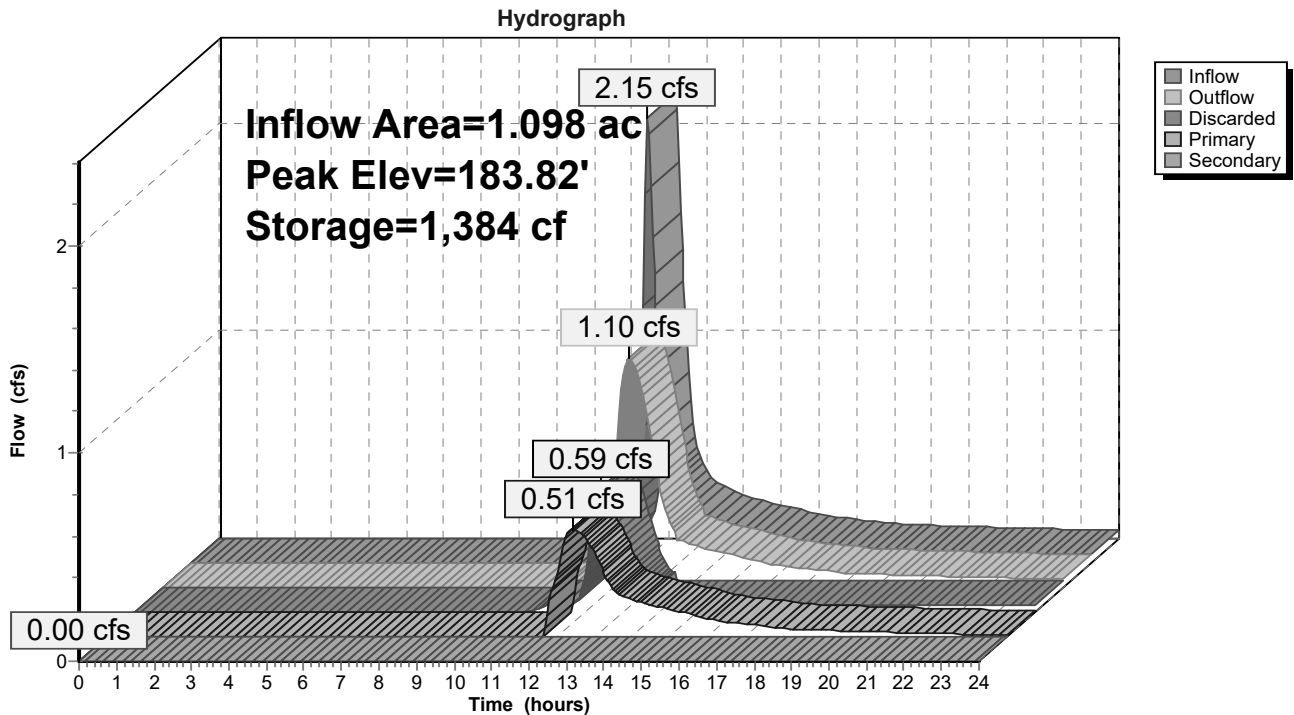
**Primary OutFlow** Max=0.51 cfs @ 12.45 hrs HW=183.82' TW=180.76' (Dynamic Tailwater)

↳ **2=6" HDPE N-12** (Barrel Controls 0.51 cfs @ 2.62 fps)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=182.75' TW=183.95' (Dynamic Tailwater)

↳ **3=Overflow to Wetland** ( Controls 0.00 cfs)

### Pond 30P: Infiltration/Trench





### Summary for Pond 71P: Existing Catch Basin

[57] Hint: Peaked at 188.93' (Flood elevation advised)

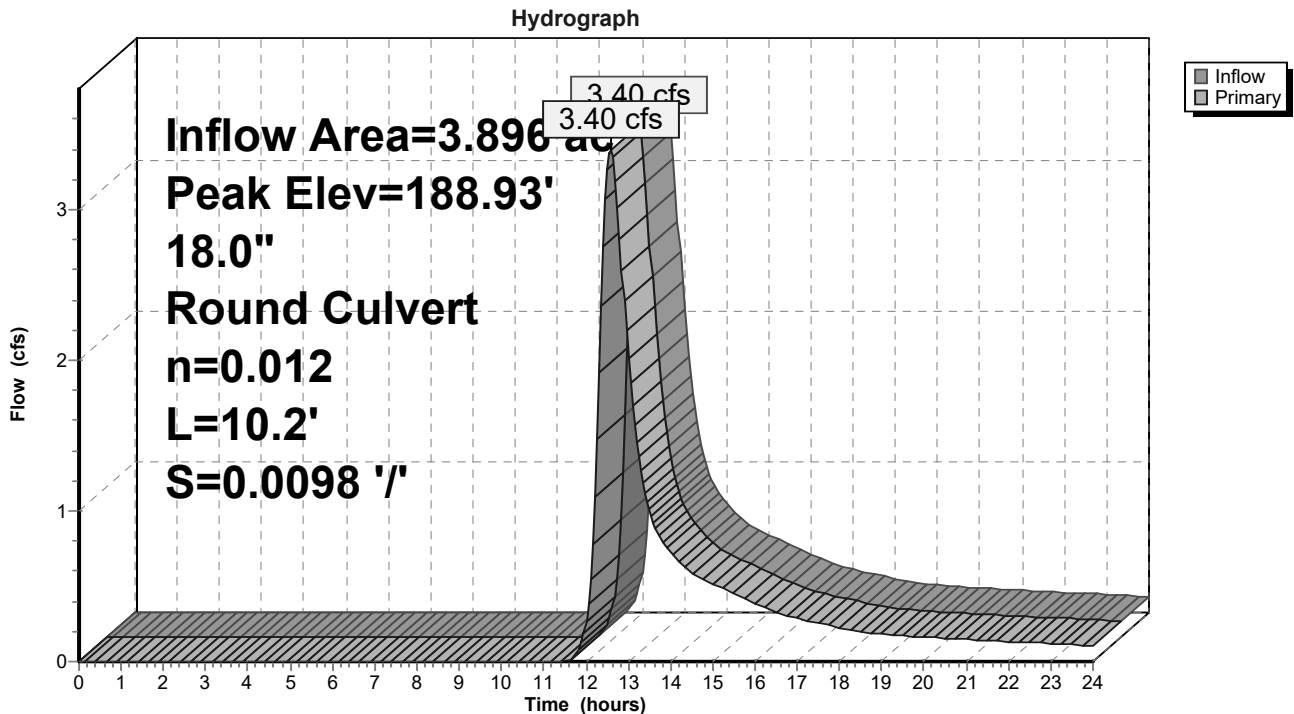
Inflow Area = 3.896 ac, 34.84% Impervious, Inflow Depth > 1.52" for 25YR-24HR event  
 Inflow = 3.40 cfs @ 12.58 hrs, Volume= 0.492 af  
 Outflow = 3.40 cfs @ 12.58 hrs, Volume= 0.492 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.40 cfs @ 12.58 hrs, Volume= 0.492 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 188.93' @ 12.58 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	187.90'	<b>18.0" Round 18" RCP</b> L= 10.2' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 187.90' / 187.80' S= 0.0098 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.39 cfs @ 12.58 hrs HW=188.92' TW=188.32' (Dynamic Tailwater)  
 ↳ 1=18" RCP (Barrel Controls 3.39 cfs @ 3.72 fps)

### Pond 71P: Existing Catch Basin



**Summary for Pond 72P: Existing Depression**

[58] Hint: Peaked 0.21' above defined flood level

Inflow Area = 1.582 ac, 42.73% Impervious, Inflow Depth > 2.07" for 25YR-24HR event  
 Inflow = 2.04 cfs @ 12.48 hrs, Volume= 0.273 af  
 Outflow = 2.02 cfs @ 12.49 hrs, Volume= 0.271 af, Atten= 1%, Lag= 0.5 min  
 Discarded = 0.02 cfs @ 11.95 hrs, Volume= 0.024 af  
 Primary = 2.00 cfs @ 12.49 hrs, Volume= 0.247 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 196.21' @ 12.59 hrs Surf.Area= 333 sf Storage= 160 cf  
 Flood Elev= 196.00' Surf.Area= 333 sf Storage= 91 cf

Plug-Flow detention time= 6.1 min calculated for 0.270 af (99% of inflow)  
 Center-of-Mass det. time= 2.2 min ( 878.0 - 875.8 )

Volume	Invert	Avail.Storage	Storage Description			
#1	195.50'	257 cf	<b>Ponding Area (Irregular)</b> Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
195.50	55	32.6	0	0	55	
195.75	179	63.6	28	28	293	
196.00	333	92.1	63	91	646	
196.50	333	92.1	167	257	692	

Device	Routing	Invert	Outlet Devices									
#1	Discarded	195.50'	<b>3.000 in/hr Infiltration over Surface area</b>									
#2	Primary	195.95'	<b>20.0' long x 50.0' breadth Overflow over DW</b>									
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60									
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63									

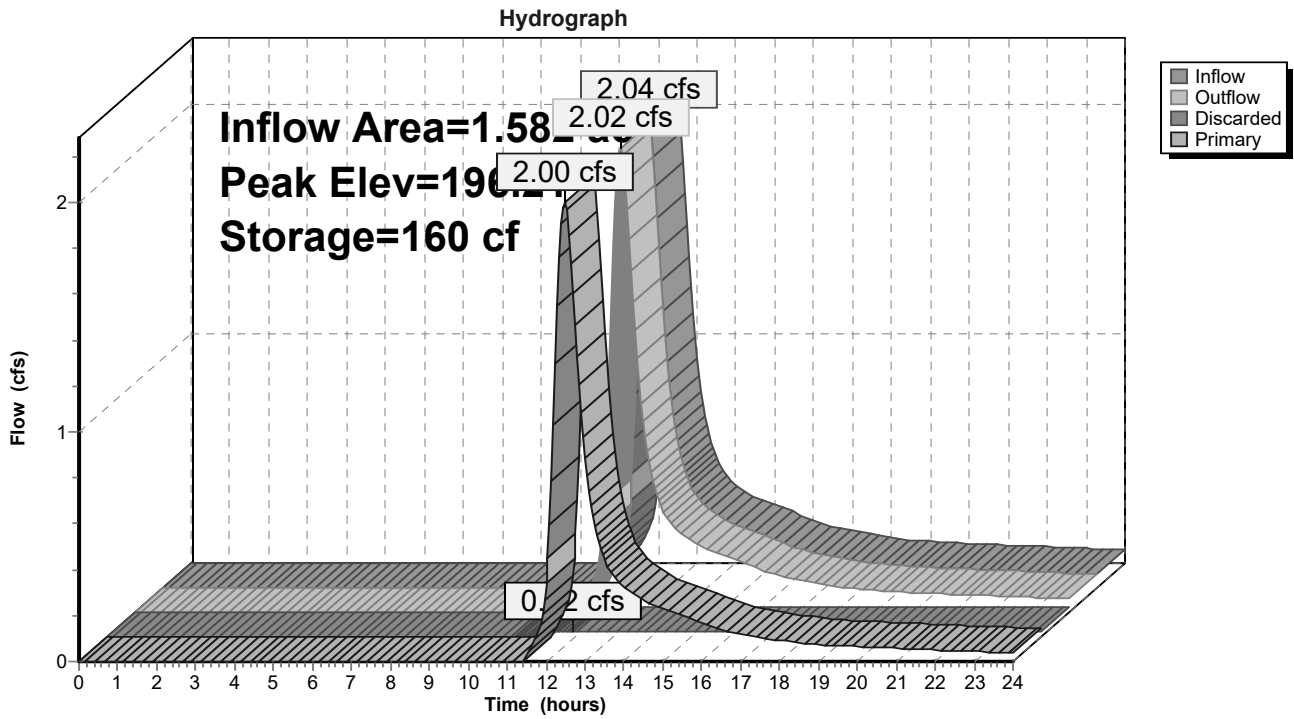
**Discarded OutFlow** Max=0.02 cfs @ 11.95 hrs HW=196.00' (Free Discharge)

↳ **1=Infiltration** (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=0.00 cfs @ 12.49 hrs HW=196.20' TW=196.20' (Dynamic Tailwater)

↳ **2=Overflow over DW** ( Controls 0.00 cfs)

### Pond 72P: Existing Depression



**Summary for Pond 201P: Bioretention W/ ISR #201**

Inflow Area = 1.174 ac, 43.96% Impervious, Inflow Depth > 2.43" for 25YR-24HR event  
 Inflow = 2.53 cfs @ 12.21 hrs, Volume= 0.237 af  
 Outflow = 0.68 cfs @ 12.71 hrs, Volume= 0.173 af, Atten= 73%, Lag= 30.1 min  
 Primary = 0.02 cfs @ 12.71 hrs, Volume= 0.023 af  
 Secondary = 0.66 cfs @ 12.71 hrs, Volume= 0.150 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 185.45' @ 12.71 hrs Surf.Area= 1,118 sf Storage= 4,286 cf  
 Flood Elev= 186.00' Surf.Area= 1,118 sf Storage= 6,720 cf

Plug-Flow detention time= 175.9 min calculated for 0.173 af (73% of inflow)  
 Center-of-Mass det. time= 82.6 min ( 934.8 - 852.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	182.00'	112 cf	<b>Stone (Irregular)</b> Listed below (Recalc) -Impervious 280 cf Overall x 40.0% Voids
#2	182.25'	335 cf	<b>Bio Media (Irregular)</b> Listed below (Recalc) 1,677 cf Overall x 20.0% Voids
#3	184.00'	1,032 cf	<b>Sediment Forebay (Irregular)</b> Listed below (Recalc) -Impervious
#4	183.75'	2,979 cf	<b>Cell (Irregular)</b> Listed below (Recalc) -Impervious
#5	185.50'	2,262 cf	<b>Open Water Storage (Irregular)</b> Listed below (Recalc) -Impervious
		6,720 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
182.00	1,118	165.5	0	0	1,118
182.25	1,118	165.5	280	280	1,159

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
182.25	1,118	165.5	0	0	1,118
183.75	1,118	165.5	1,677	1,677	1,366

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
184.00	160	53.5	0	0	160
185.00	822	223.7	448	448	3,917
185.50	1,551	276.6	584	1,032	6,027

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
183.75	1,118	165.5	0	0	1,118
184.00	1,459	183.1	321	321	1,608
185.00	1,877	199.3	1,664	1,985	2,137
185.50	2,101	207.6	994	2,979	2,424

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 46

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
185.50	3,838	302.2	0	0	3,838
186.00	5,247	335.0	2,262	2,262	5,509

Device	Routing	Invert	Outlet Devices
#1	Primary	182.00'	<b>6.0" Round 6" HDPE N-12</b> L= 33.0' Ke= 0.500 Inlet / Outlet Invert= 182.00' / 181.70' S= 0.0091 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#2	Secondary	182.00'	<b>15.0" Round 15" HDPE N-12</b> L= 26.0' Ke= 0.500 Inlet / Outlet Invert= 182.00' / 181.70' S= 0.0115 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf
#3	Device 1	182.00'	<b>0.7" Vert. 0.75" Orifice</b> C= 0.600
#4	Device 3	182.25'	<b>10.000 in/hr Bio Media over Surface area</b>
#5	Device 2	184.90'	<b>6.0" W x 10.0" H Vert. 6"W x 10" T Notch</b> C= 0.600
#6	Device 2	185.75'	<b>48.0" Horiz. 48" Outlet Structure</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.71 hrs HW=185.45' TW=182.01' (Dynamic Tailwater)

1=6" HDPE N-12 (Passes 0.02 cfs of 1.47 cfs potential flow)

3=0.75" Orifice (Orifice Controls 0.02 cfs @ 8.91 fps)

4=Bio Media (Passes 0.02 cfs of 0.26 cfs potential flow)

**Secondary OutFlow** Max=0.66 cfs @ 12.71 hrs HW=185.45' TW=182.01' (Dynamic Tailwater)

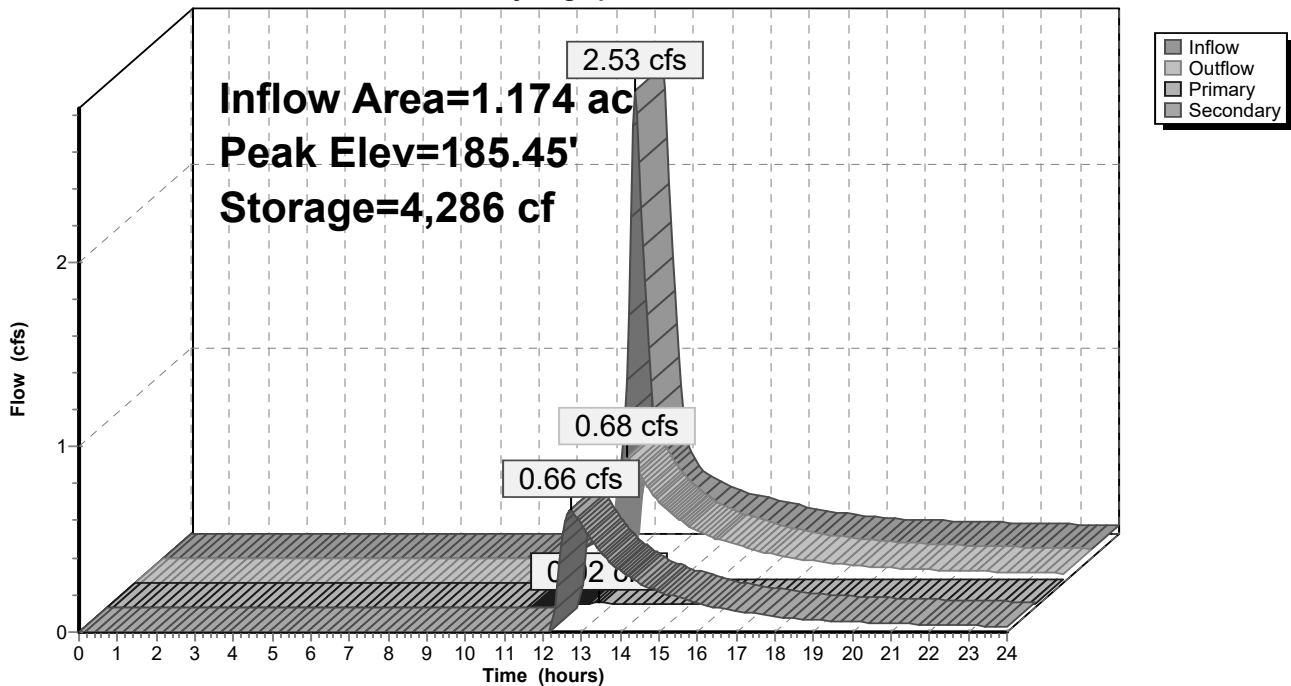
2=15" HDPE N-12 (Passes 0.66 cfs of 9.93 cfs potential flow)

5=6"W x 10" T Notch (Orifice Controls 0.66 cfs @ 2.38 fps)

6=48" Outlet Structure ( Controls 0.00 cfs)

**Pond 201P: Bioretention W/ ISR #201**

Hydrograph



**Summary for Pond 202P: Bioretention W/ ISR #202**

Inflow Area = 2.354 ac, 46.52% Impervious, Inflow Depth > 3.77" for 25YR-24HR event  
 Inflow = 8.23 cfs @ 12.11 hrs, Volume= 0.739 af  
 Outflow = 0.68 cfs @ 13.37 hrs, Volume= 0.566 af, Atten= 92%, Lag= 75.8 min  
 Primary = 0.09 cfs @ 12.99 hrs, Volume= 0.103 af  
 Secondary = 0.60 cfs @ 13.69 hrs, Volume= 0.463 af  
 Tertiary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 179.19' @ 13.69 hrs Surf.Area= 1,981 sf Storage= 18,074 cf  
 Flood Elev= 180.50' Surf.Area= 1,981 sf Storage= 28,868 cf

Plug-Flow detention time= 304.9 min calculated for 0.564 af (76% of inflow)  
 Center-of-Mass det. time= 219.9 min ( 1,012.9 - 793.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	173.75'	198 cf	<b>Stone (Irregular)</b> Listed below (Recalc) -Impervious 495 cf Overall x 40.0% Voids
#2	174.00'	594 cf	<b>Bio Media (Irregular)</b> Listed below (Recalc) 2,972 cf Overall x 20.0% Voids
#3	175.50'	903 cf	<b>Sediment Forebay (Irregular)</b> Listed below (Recalc) -Impervious
#4	175.50'	6,102 cf	<b>Cell (Irregular)</b> Listed below (Recalc) -Impervious
#5	177.50'	21,071 cf	<b>Open Water Storage (Irregular)</b> Listed below (Recalc) -Impervious
		28,868 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
173.75	1,981	351.5	0	0	1,981
174.00	1,981	351.5	495	495	2,069

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
174.00	1,981	351.5	0	0	1,981
175.50	1,981	351.5	2,972	2,972	2,508

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
175.50	243	61.9	0	0	243
176.00	337	70.7	144	144	342
177.00	563	89.6	445	590	596
177.50	693	98.2	313	903	732

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
175.50	1,981	351.5	0	0	1,981
176.00	2,509	360.4	1,120	1,120	2,516
177.00	3,604	379.0	3,040	4,160	3,671
177.50	4,170	387.1	1,942	6,102	4,201

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 48

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
177.50	4,925	448.8	0	0	4,925
178.00	5,605	458.3	2,631	2,631	5,647
179.00	7,008	477.1	6,293	8,924	7,123
180.00	8,468	496.0	7,726	16,651	8,665
180.50	9,219	505.4	4,420	21,071	9,455

Device	Routing	Invert	Outlet Devices
#1	Primary	173.75'	<b>6.0" Round 6" HDPE N-12</b> L= 30.0' Ke= 0.500 Inlet / Outlet Invert= 173.75' / 173.50' S= 0.0083 ' / Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#2	Secondary	173.75'	<b>15.0" Round 15" HDPE N-12</b> L= 30.0' Ke= 0.500 Inlet / Outlet Invert= 173.75' / 173.50' S= 0.0083 ' / Cc= 0.900 n= 0.012, Flow Area= 1.23 sf
#3	Device 1	173.75'	<b>1.2" Vert. 1.25" Orifice</b> C= 0.600
#4	Device 3	174.00'	<b>10.000 in/hr Bio Media over Surface area</b>
#5	Device 2	177.00'	<b>4.0" Vert. 4" Orifice</b> C= 0.600
#6	Device 2	179.75'	<b>48.0" Horiz. 48" Outlet Structure</b> C= 0.600 Limited to weir flow at low heads
#7	Tertiary	180.00'	<b>10.0' long x 8.5' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.45 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.65 2.66 2.67 2.69 2.71

**Primary OutFlow** Max=0.09 cfs @ 12.99 hrs HW=179.15' TW=173.82' (Dynamic Tailwater)

↑ **1=6" HDPE N-12** (Passes 0.09 cfs of 1.91 cfs potential flow)

↑ **3=1.25" Orifice** (Orifice Controls 0.09 cfs @ 11.12 fps)

↑ **4=Bio Media** (Passes 0.09 cfs of 0.46 cfs potential flow)

**Secondary OutFlow** Max=0.60 cfs @ 13.69 hrs HW=179.19' TW=174.35' (Dynamic Tailwater)

↑ **2=15" HDPE N-12** (Passes 0.60 cfs of 12.97 cfs potential flow)

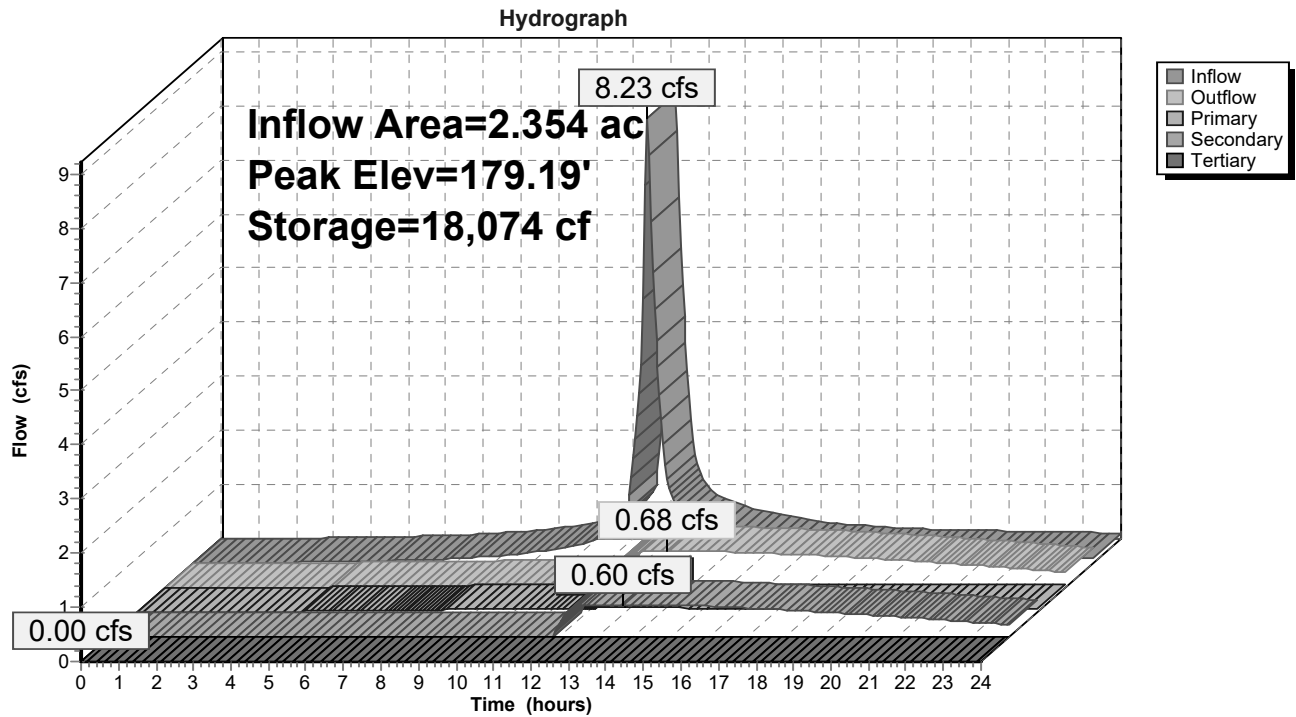
↑ **5=4" Orifice** (Orifice Controls 0.60 cfs @ 6.85 fps)

↑ **6=48" Outlet Structure** ( Controls 0.00 cfs)

**Tertiary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=173.75' TW=172.50' (Dynamic Tailwater)

↑ **7=Spillway** ( Controls 0.00 cfs)

### Pond 202P: Bioretention W/ ISR #202





**Summary for Pond 203P: Infiltration Pond #203**

Inflow Area = 2.722 ac, 40.23% Impervious, Inflow Depth > 2.72" for 25YR-24HR event  
 Inflow = 1.08 cfs @ 12.20 hrs, Volume= 0.617 af  
 Outflow = 0.73 cfs @ 14.44 hrs, Volume= 0.510 af, Atten= 32%, Lag= 134.8 min  
 Discarded = 0.21 cfs @ 14.44 hrs, Volume= 0.233 af  
 Primary = 0.52 cfs @ 14.44 hrs, Volume= 0.277 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 174.58' @ 14.44 hrs Surf.Area= 3,058 sf Storage= 4,820 cf  
 Flood Elev= 175.00' Surf.Area= 3,385 sf Storage= 6,181 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 50.7 min ( 1,051.9 - 1,001.2 )

Volume	Invert	Avail.Storage	Storage Description			
#1	172.50'	6,181 cf	<b>Open Water Storage (Irregular)</b> Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
172.50	1,574	204.2	0	0	1,574	
173.00	1,967	225.0	883	883	2,292	
174.00	2,638	239.5	2,294	3,178	2,877	
175.00	3,385	258.5	3,004	6,181	3,670	

Device	Routing	Invert	Outlet Devices																		
#1	Discarded	172.50'	<b>3.000 in/hr Infiltration over Surface area</b>																		
#2	Primary	174.50'	<b>10.0' long x 7.0' breadth Spillway</b>																		
			Head (feet)	0.20	0.40	0.60	0.80	1.00	1.20	1.40	1.60	1.80	2.00	2.50	3.00	3.50	4.00	4.50	5.00	5.50	
			Coef. (English)	2.40	2.52	2.70	2.68	2.68	2.67	2.66	2.65	2.65	2.65	2.65	2.65	2.65	2.66	2.68	2.70	2.73	2.78

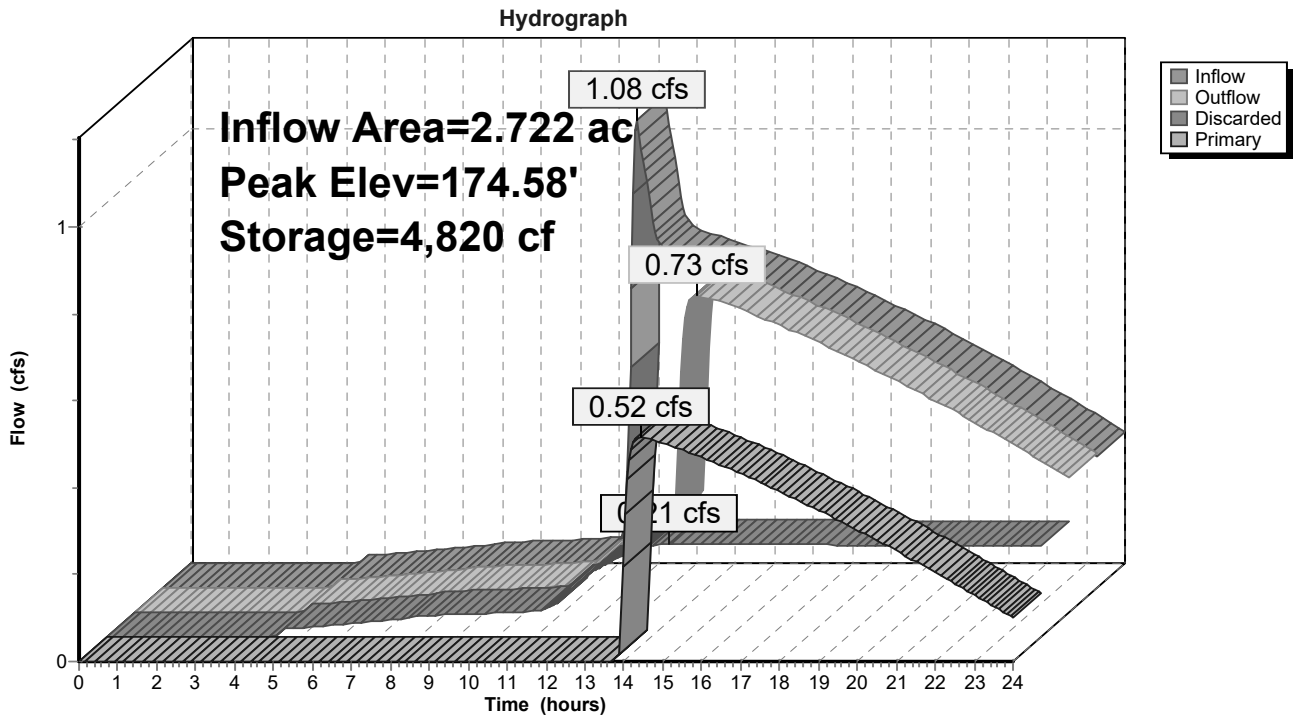
**Discarded OutFlow** Max=0.21 cfs @ 14.44 hrs HW=174.58' (Free Discharge)

↑1=Infiltration (Exfiltration Controls 0.21 cfs)

**Primary OutFlow** Max=0.52 cfs @ 14.44 hrs HW=174.58' TW=0.00' (Dynamic Tailwater)

↑2=Spillway (Weir Controls 0.52 cfs @ 0.67 fps)

### Pond 203P: Infiltration Pond #203



**Summary for Pond 204P: Detention Pond #204**

Inflow Area = 2.767 ac, 35.90% Impervious, Inflow Depth > 2.73" for 25YR-24HR event  
 Inflow = 7.28 cfs @ 12.10 hrs, Volume= 0.630 af  
 Outflow = 4.16 cfs @ 12.27 hrs, Volume= 0.625 af, Atten= 43%, Lag= 10.2 min  
 Primary = 4.16 cfs @ 12.27 hrs, Volume= 0.625 af  
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 178.85' @ 12.27 hrs Surf.Area= 2,707 sf Storage= 4,793 cf  
 Flood Elev= 180.00' Surf.Area= 3,505 sf Storage= 8,346 cf

Plug-Flow detention time= 38.8 min calculated for 0.624 af (99% of inflow)  
 Center-of-Mass det. time= 33.7 min ( 848.1 - 814.4 )

Volume	Invert	Avail.Storage	Storage Description			
#1	176.25'	8,346 cf	<b>Open Water Storage (Irregular)</b> Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
176.25	4	4.0	0	0	4	
176.50	1,320	155.0	116	116	1,915	
177.00	1,578	170.7	724	840	2,330	
178.00	2,183	202.8	1,872	2,712	3,302	
179.00	2,803	222.5	2,487	5,199	4,002	
180.00	3,505	242.8	3,147	8,346	4,789	

Device	Routing	Invert	Outlet Devices	
#1	Primary	176.25'	<b>18.0" Round 18" HDPE N-12</b> L= 29.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 176.25' / 176.00' S= 0.0086 1/1 Cc= 0.900 n= 0.012, Flow Area= 1.77 sf	
#2	Device 1	176.25'	<b>3.0" Vert. 3" Orifice</b> C= 0.600	
#3	Device 1	177.25'	<b>8.0" Vert. 8" Orifice (2) X 2.00</b> C= 0.600	
#4	Device 1	179.25'	<b>48.0" Horiz. 48" Outlet Structure</b> C= 0.600 Limited to weir flow at low heads	
#5	Secondary	179.50'	<b>10.0' long x 9.0' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.46 2.55 2.70 2.69 2.68 2.68 2.67 2.64 2.64 2.64 2.65 2.64 2.65 2.65 2.66 2.67 2.69	

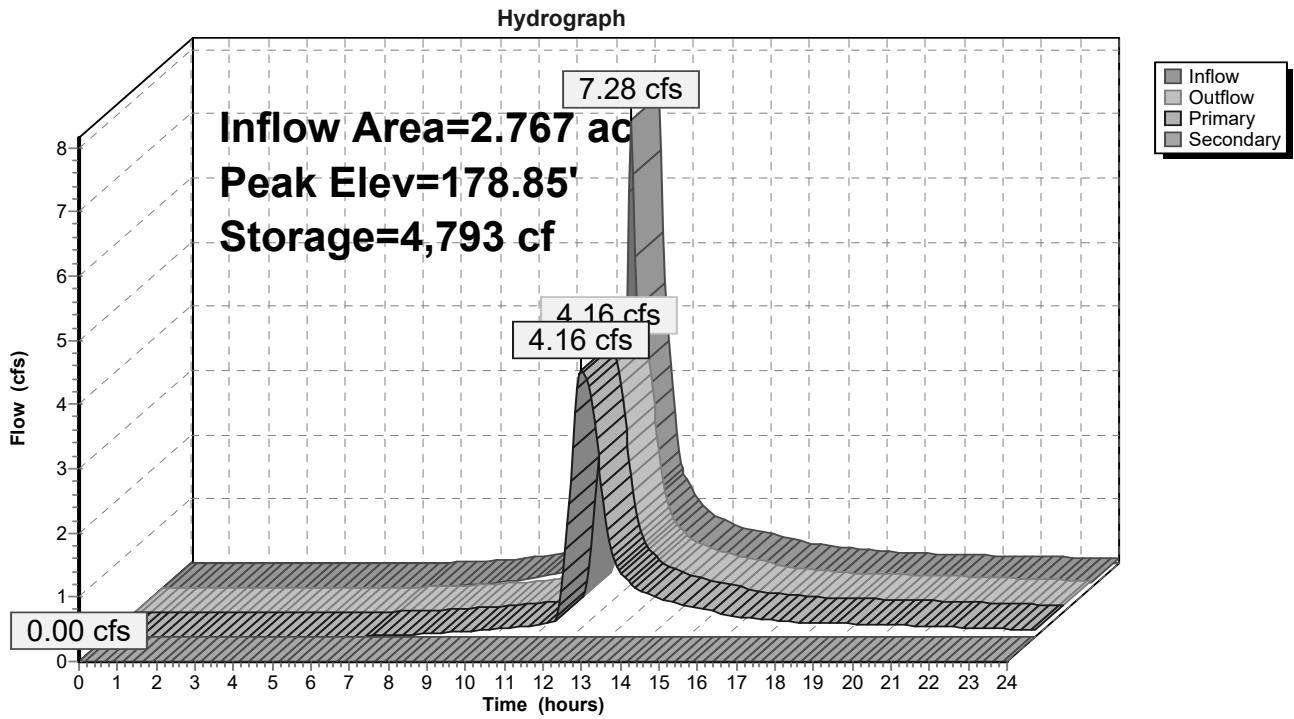
**Primary OutFlow** Max=4.15 cfs @ 12.27 hrs HW=178.85' TW=0.00' (Dynamic Tailwater)

- 1=18" HDPE N-12 (Passes 4.15 cfs of 11.56 cfs potential flow)
- 2=3" Orifice (Orifice Controls 0.37 cfs @ 7.57 fps)
- 3=8" Orifice (2) (Orifice Controls 3.78 cfs @ 5.41 fps)
- 4=48" Outlet Structure ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=176.25' TW=0.00' (Dynamic Tailwater)

- 5=Spillway ( Controls 0.00 cfs)

### Pond 204P: Detention Pond #204



**Summary for Pond C41P: Catch Basin #41**

Inflow Area = 0.170 ac, 61.70% Impervious, Inflow Depth > 4.49" for 25YR-24HR event  
 Inflow = 0.66 cfs @ 12.20 hrs, Volume= 0.064 af  
 Outflow = 0.66 cfs @ 12.20 hrs, Volume= 0.064 af, Atten= 0%, Lag= 0.1 min  
 Primary = 0.66 cfs @ 12.20 hrs, Volume= 0.064 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 184.38' @ 12.20 hrs Surf.Area= 13 sf Storage= 5 cf  
 Flood Elev= 191.00' Surf.Area= 13 sf Storage= 88 cf

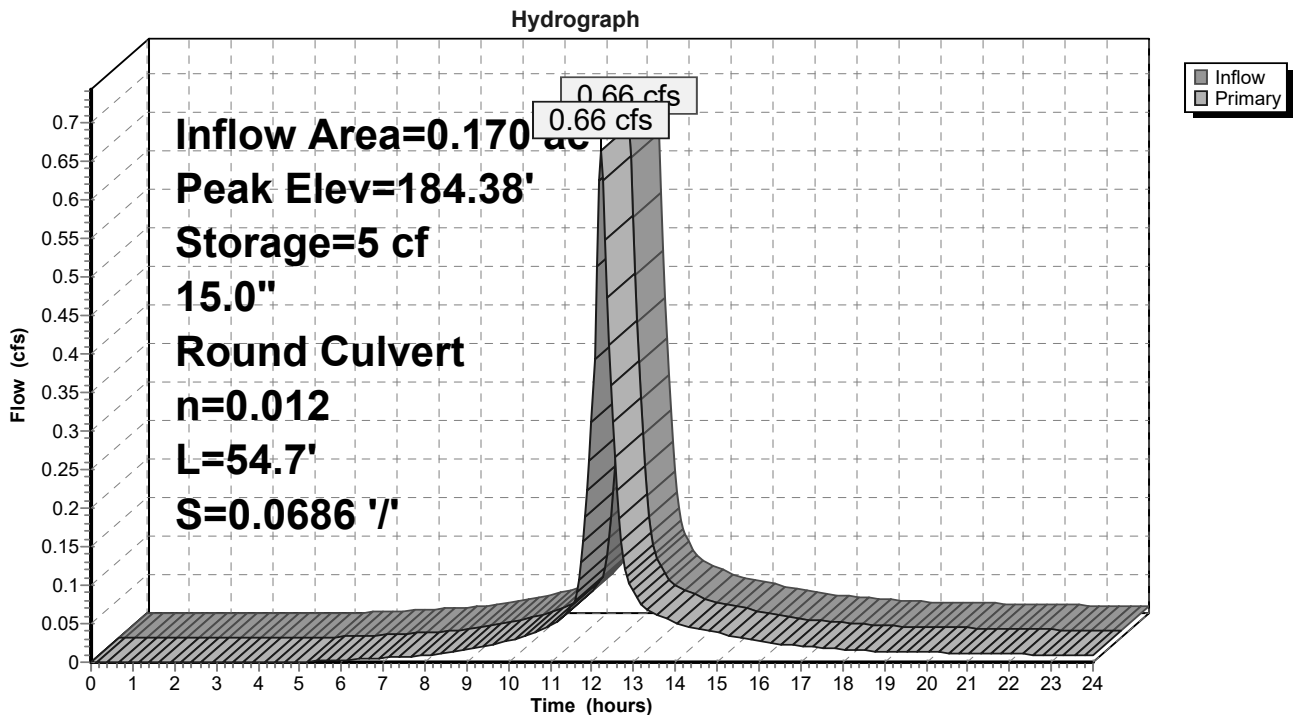
Plug-Flow detention time= 0.4 min calculated for 0.064 af (100% of inflow)  
 Center-of-Mass det. time= 0.3 min ( 799.1 - 798.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	184.00'	88 cf	<b>4.00'D x 7.00'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	184.00'	<b>15.0" Round 15" HDPE N-12</b> L= 54.7' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 184.00' / 180.25' S= 0.0686 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

**Primary OutFlow** Max=0.66 cfs @ 12.20 hrs HW=184.38' TW=181.06' (Dynamic Tailwater)  
 ↳ 1=15" HDPE N-12 (Inlet Controls 0.66 cfs @ 2.10 fps)

**Pond C41P: Catch Basin #41**



**Summary for Pond C42P: Catch Basin #42**

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 1.268 ac, 12.14% Impervious, Inflow Depth > 1.62" for 25YR-24HR event  
 Inflow = 1.14 cfs @ 12.22 hrs, Volume= 0.171 af  
 Outflow = 1.14 cfs @ 12.22 hrs, Volume= 0.171 af, Atten= 0%, Lag= 0.3 min  
 Primary = 1.14 cfs @ 12.22 hrs, Volume= 0.171 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 181.09' @ 12.17 hrs Surf.Area= 13 sf Storage= 12 cf  
 Flood Elev= 184.50' Surf.Area= 13 sf Storage= 55 cf

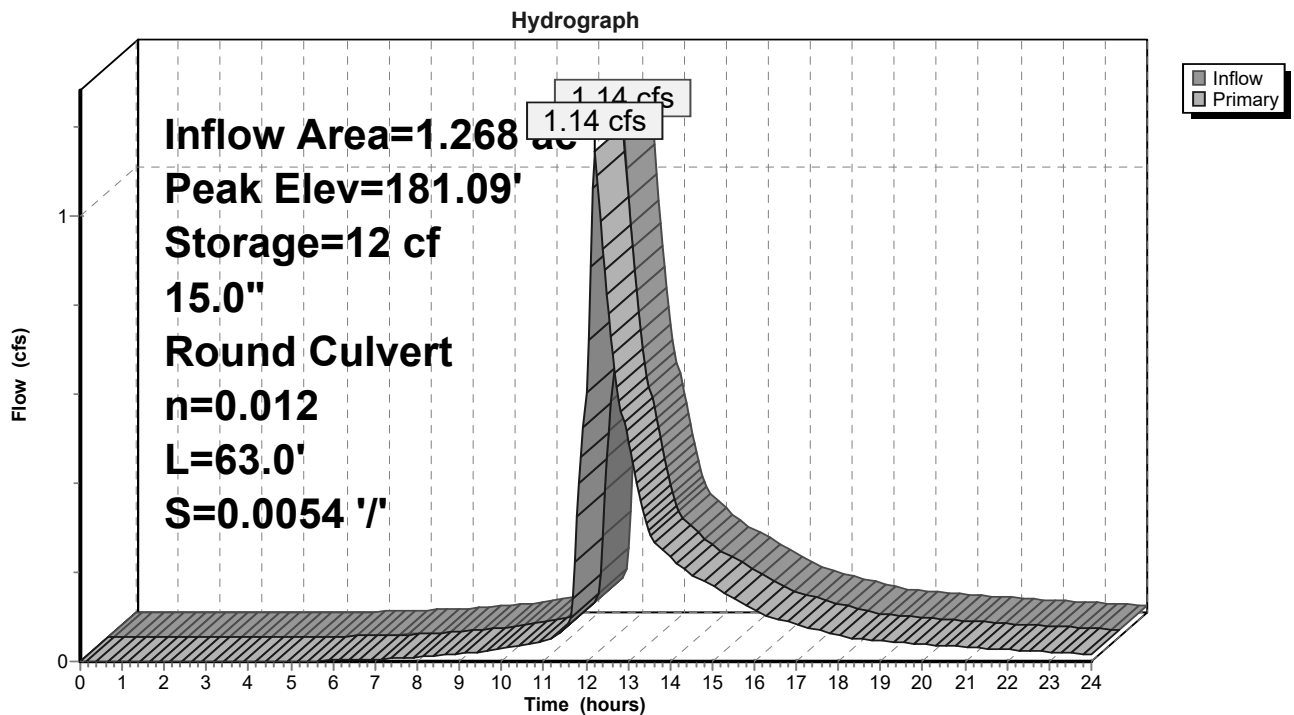
Plug-Flow detention time= 0.3 min calculated for 0.171 af (100% of inflow)  
 Center-of-Mass det. time= 0.2 min ( 837.7 - 837.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	180.15'	136 cf	<b>4.00'D x 10.85'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	180.15'	<b>15.0" Round 15" HDPE N-12</b> L= 63.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 180.15' / 179.81' S= 0.0054 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

**Primary OutFlow** Max=1.62 cfs @ 12.22 hrs HW=181.03' TW=180.77' (Dynamic Tailwater)  
 ←1=15" HDPE N-12 (Outlet Controls 1.62 cfs @ 2.48 fps)

**Pond C42P: Catch Basin #42**



**Summary for Pond C43P: Catch Basin #43**

Inflow Area = 0.350 ac, 64.41% Impervious, Inflow Depth > 4.39" for 25YR-24HR event  
 Inflow = 1.67 cfs @ 12.10 hrs, Volume= 0.128 af  
 Outflow = 1.67 cfs @ 12.10 hrs, Volume= 0.128 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.67 cfs @ 12.10 hrs, Volume= 0.128 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 181.13' @ 12.10 hrs Surf.Area= 13 sf Storage= 8 cf  
 Flood Elev= 186.00' Surf.Area= 13 sf Storage= 69 cf

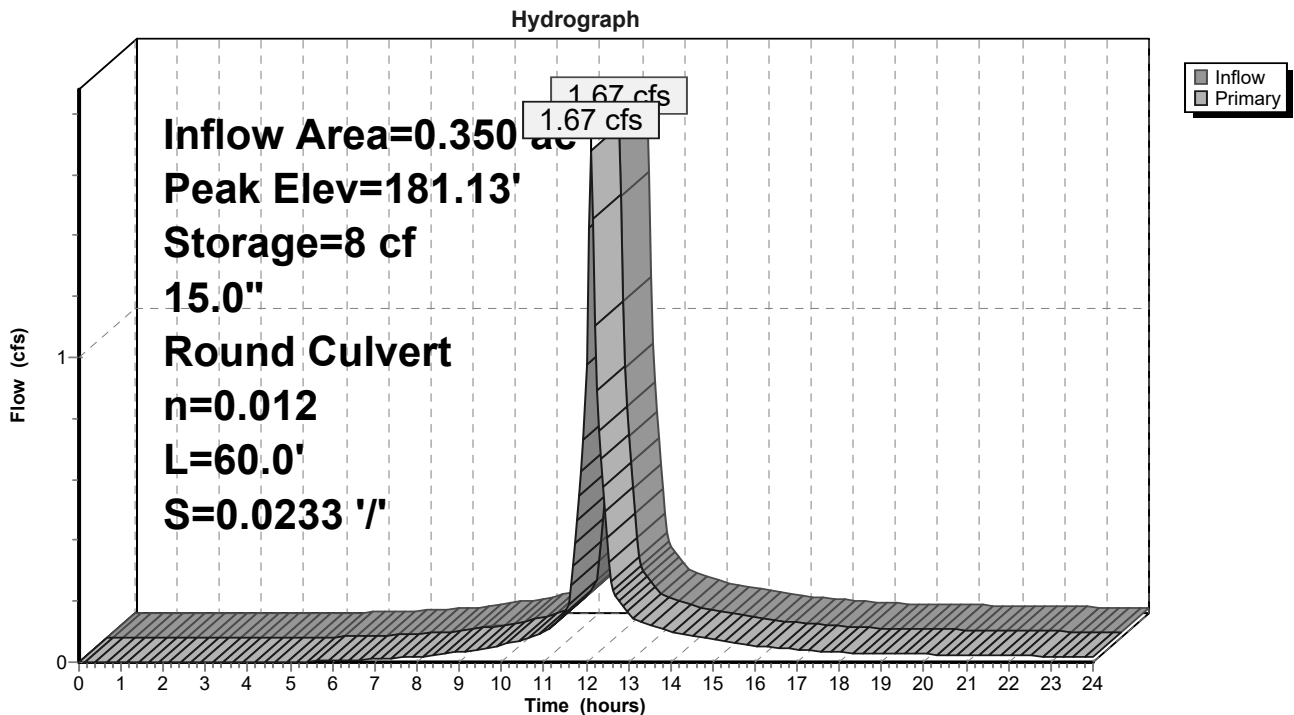
Plug-Flow detention time= 0.3 min calculated for 0.128 af (100% of inflow)  
 Center-of-Mass det. time= 0.2 min ( 795.9 - 795.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	180.50'	69 cf	<b>4.00'D x 5.50'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	180.50'	<b>15.0" Round 15" HDPE N-12</b> L= 60.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 180.50' / 179.10' S= 0.0233 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

**Primary OutFlow** Max=1.67 cfs @ 12.10 hrs HW=181.13' TW=180.30' (Dynamic Tailwater)  
 ↳ 1=15" HDPE N-12 (Inlet Controls 1.67 cfs @ 2.70 fps)

**Pond C43P: Catch Basin #43**



**Summary for Pond C44P: Catch Basin #44**

Inflow Area = 0.682 ac, 70.38% Impervious, Inflow Depth > 4.60" for 25YR-24HR event  
 Inflow = 3.33 cfs @ 12.10 hrs, Volume= 0.262 af  
 Outflow = 3.32 cfs @ 12.11 hrs, Volume= 0.262 af, Atten= 0%, Lag= 0.1 min  
 Primary = 3.32 cfs @ 12.11 hrs, Volume= 0.262 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 180.36' @ 12.13 hrs Surf.Area= 13 sf Storage= 17 cf  
 Flood Elev= 183.50' Surf.Area= 13 sf Storage= 57 cf

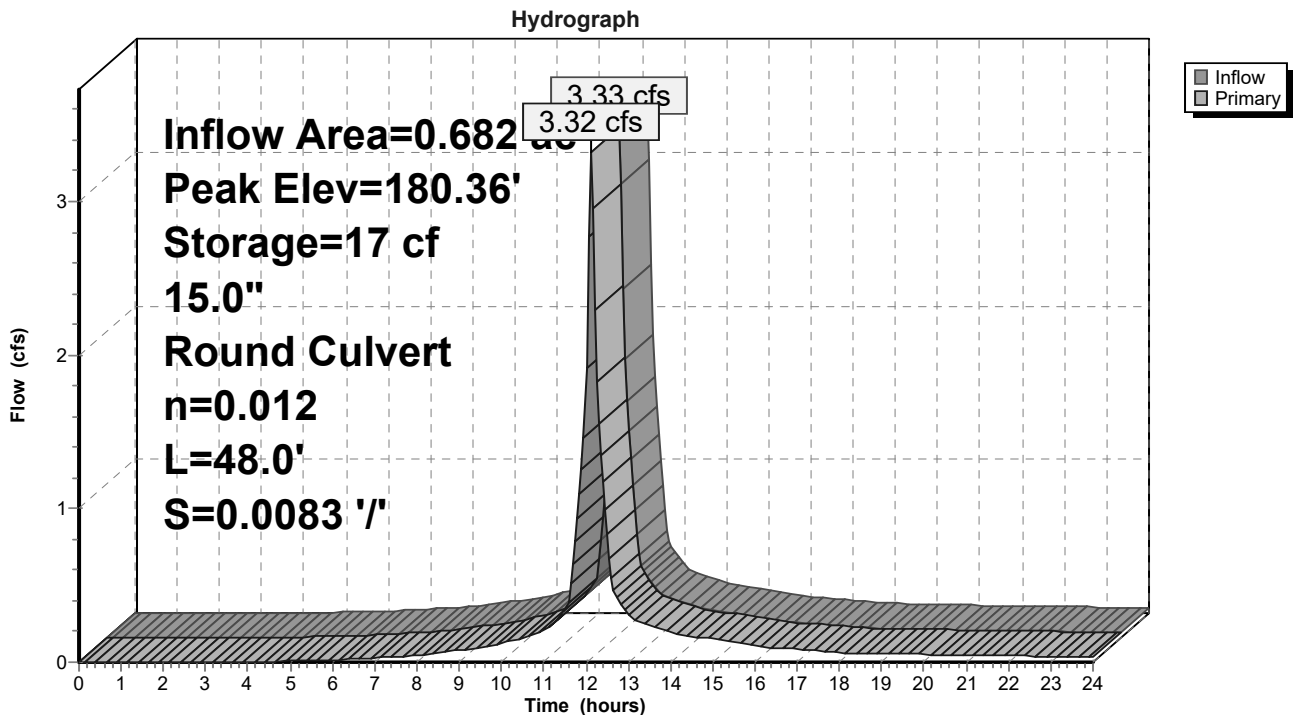
Plug-Flow detention time= 0.2 min calculated for 0.262 af (100% of inflow)  
 Center-of-Mass det. time= 0.2 min ( 789.3 - 789.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	179.00'	57 cf	<b>4.00'D x 4.50'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	179.00'	<b>15.0" Round 15" HDPE N-12</b> L= 48.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 179.00' / 178.60' S= 0.0083 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

**Primary OutFlow** Max=2.74 cfs @ 12.11 hrs HW=180.30' TW=180.06' (Dynamic Tailwater)  
 ←1=15" HDPE N-12 (Outlet Controls 2.74 cfs @ 2.66 fps)

**Pond C44P: Catch Basin #44**





**Summary for Pond C45P: Catch Basin #45**

Inflow Area = 0.388 ac, 94.23% Impervious, Inflow Depth > 5.39" for 25YR-24HR event  
 Inflow = 2.15 cfs @ 12.09 hrs, Volume= 0.174 af  
 Outflow = 2.15 cfs @ 12.09 hrs, Volume= 0.174 af, Atten= 0%, Lag= 0.1 min  
 Primary = 2.15 cfs @ 12.09 hrs, Volume= 0.174 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 182.13' @ 12.09 hrs Surf.Area= 13 sf Storage= 9 cf  
 Flood Elev= 185.90' Surf.Area= 13 sf Storage= 57 cf

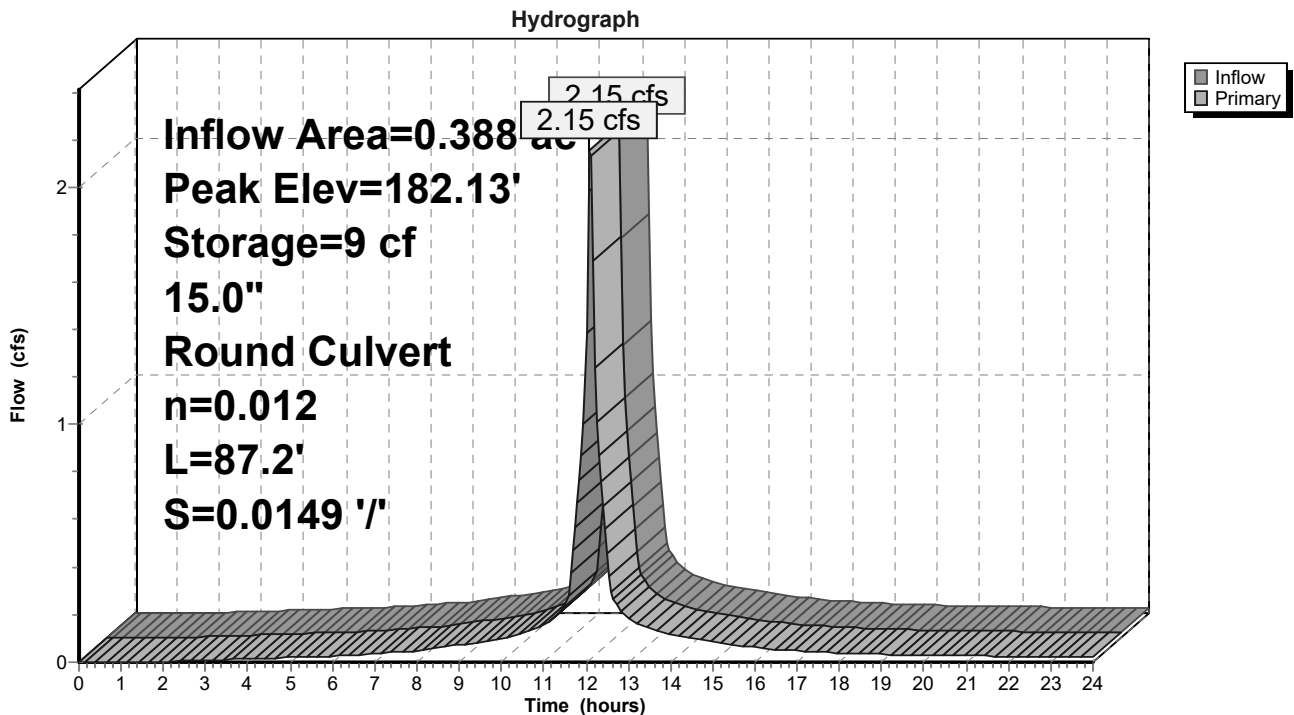
Plug-Flow detention time= 0.2 min calculated for 0.174 af (100% of inflow)  
 Center-of-Mass det. time= 0.2 min ( 758.5 - 758.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	181.40'	57 cf	<b>4.00'D x 4.50'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	181.40'	<b>15.0" Round 15" HDPE N-12</b> L= 87.2' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 181.40' / 180.10' S= 0.0149 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

**Primary OutFlow** Max=2.10 cfs @ 12.09 hrs HW=182.12' TW=180.90' (Dynamic Tailwater)  
 ↳ 1=15" HDPE N-12 (Inlet Controls 2.10 cfs @ 2.88 fps)

**Pond C45P: Catch Basin #45**



**Summary for Pond C46P: Catch Basin #46**

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 0.562 ac, 96.02% Impervious, Inflow Depth > 5.47" for 25YR-24HR event  
 Inflow = 3.13 cfs @ 12.09 hrs, Volume= 0.256 af  
 Outflow = 3.14 cfs @ 12.09 hrs, Volume= 0.256 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.14 cfs @ 12.09 hrs, Volume= 0.256 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 180.91' @ 12.09 hrs Surf.Area= 13 sf Storage= 11 cf  
 Flood Elev= 184.50' Surf.Area= 13 sf Storage= 57 cf

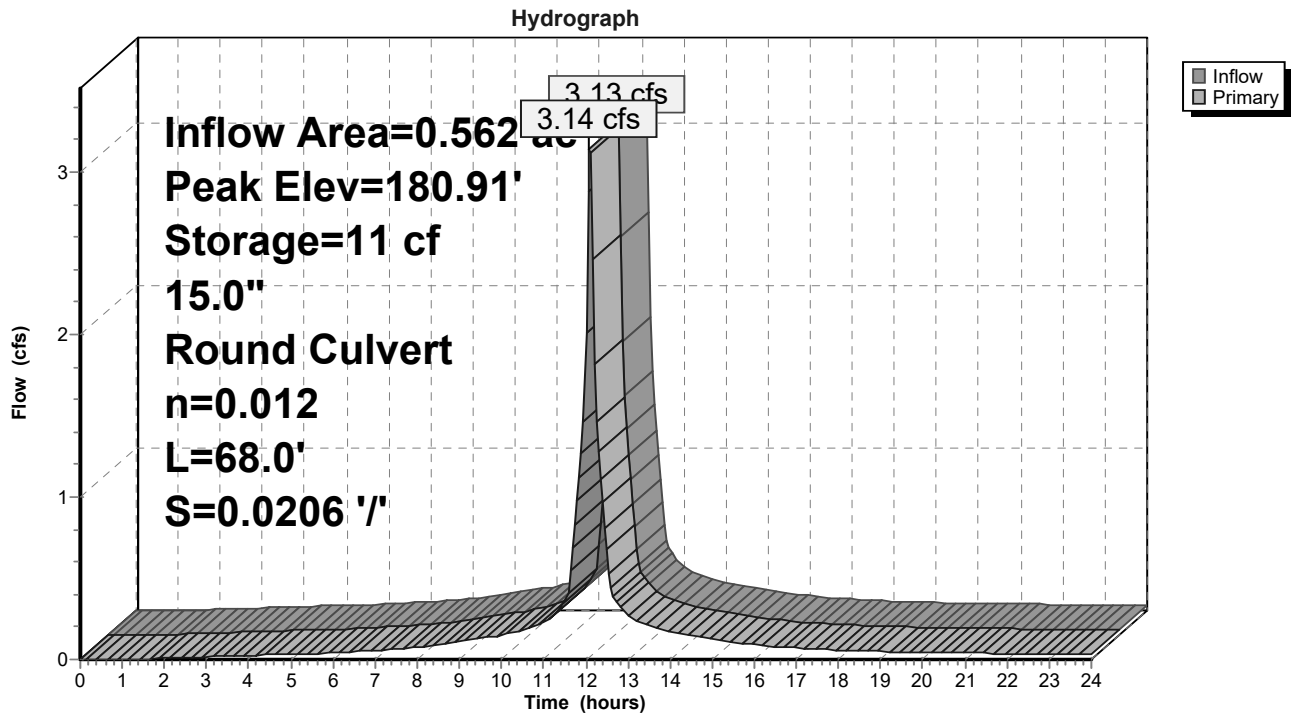
Plug-Flow detention time= 0.2 min calculated for 0.256 af (100% of inflow)  
 Center-of-Mass det. time= 0.2 min ( 754.3 - 754.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	180.00'	57 cf	<b>4.00'D x 4.50'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	180.00'	<b>15.0" Round 15" HDPE N-12</b> L= 68.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 180.00' / 178.60' S= 0.0206 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

**Primary OutFlow** Max=3.02 cfs @ 12.09 hrs HW=180.90' TW=180.04' (Dynamic Tailwater)  
 ←1=15" HDPE N-12 (Outlet Controls 3.02 cfs @ 4.47 fps)

**Pond C46P: Catch Basin #46**



**Summary for Pond C47P: Catch Basin #47**

Inflow Area = 1.318 ac, 82.97% Impervious, Inflow Depth > 5.03" for 25YR-24HR event  
 Inflow = 6.84 cfs @ 12.10 hrs, Volume= 0.552 af  
 Outflow = 6.84 cfs @ 12.10 hrs, Volume= 0.552 af, Atten= 0%, Lag= 0.0 min  
 Primary = 6.84 cfs @ 12.10 hrs, Volume= 0.552 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 180.08' @ 12.10 hrs Surf.Area= 13 sf Storage= 20 cf  
 Flood Elev= 183.75' Surf.Area= 13 sf Storage= 66 cf

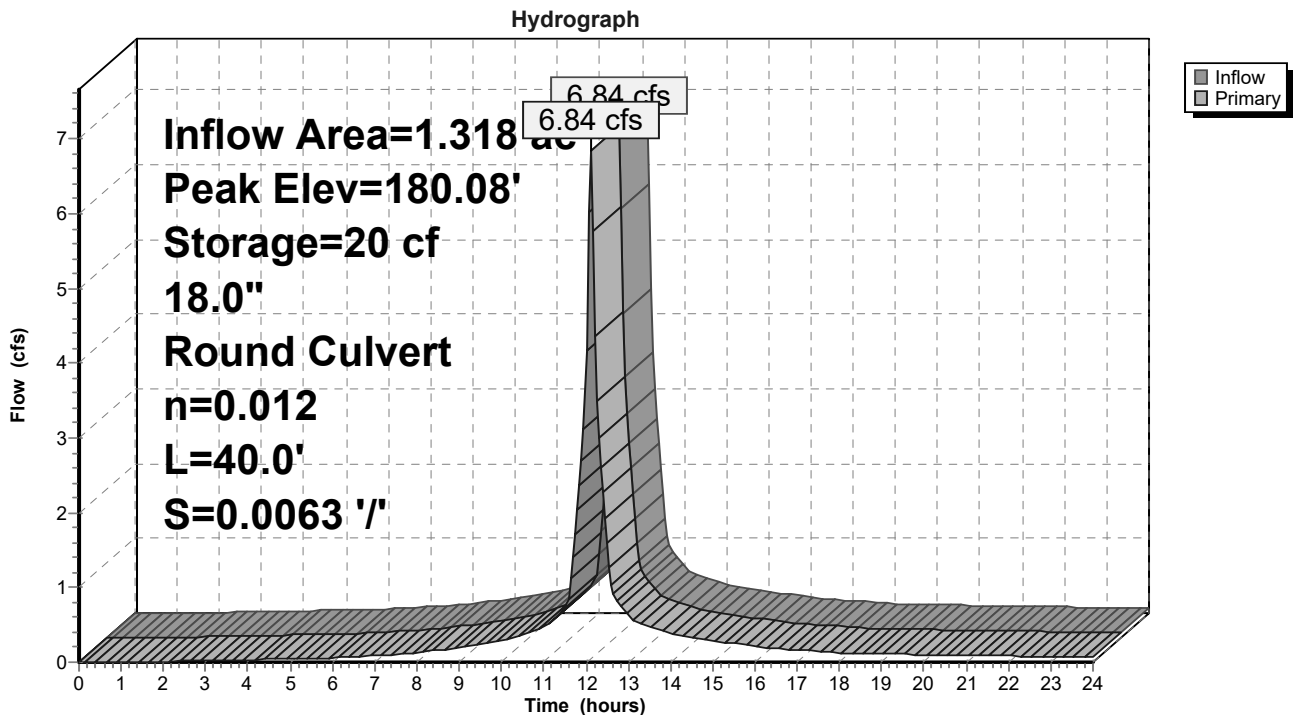
Plug-Flow detention time= 0.2 min calculated for 0.551 af (100% of inflow)  
 Center-of-Mass det. time= 0.2 min ( 770.5 - 770.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	178.50'	66 cf	<b>4.00'D x 5.25'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	178.50'	<b>18.0" Round 18" HDPE N-12</b> L= 40.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 178.50' / 178.25' S= 0.0063 '/ Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

**Primary OutFlow** Max=6.78 cfs @ 12.10 hrs HW=180.07' TW=177.91' (Dynamic Tailwater)  
 ↳ 1=18" HDPE N-12 (Barrel Controls 6.78 cfs @ 4.57 fps)

**Pond C47P: Catch Basin #47**



**Summary for Pond C50P: Inlet Sump**

Inflow Area = 4.165 ac, 32.60% Impervious, Inflow Depth > 1.43" for 25YR-24HR event  
 Inflow = 3.39 cfs @ 12.59 hrs, Volume= 0.495 af  
 Outflow = 3.39 cfs @ 12.60 hrs, Volume= 0.495 af, Atten= 0%, Lag= 0.0 min  
 Primary = 3.39 cfs @ 12.60 hrs, Volume= 0.495 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 184.47' @ 12.60 hrs Surf.Area= 13 sf Storage= 12 cf  
 Flood Elev= 190.00' Surf.Area= 1,083 sf Storage= 1,167 cf

Plug-Flow detention time= 0.1 min calculated for 0.494 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 890.5 - 890.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	187.50'	1,117 cf	<b>Ponding Area (Irregular)</b> Listed below (Recalc)
#2	183.50'	50 cf	<b>4.00'D x 4.00'H 4' Structure</b>
		1,167 cf	Total Available Storage

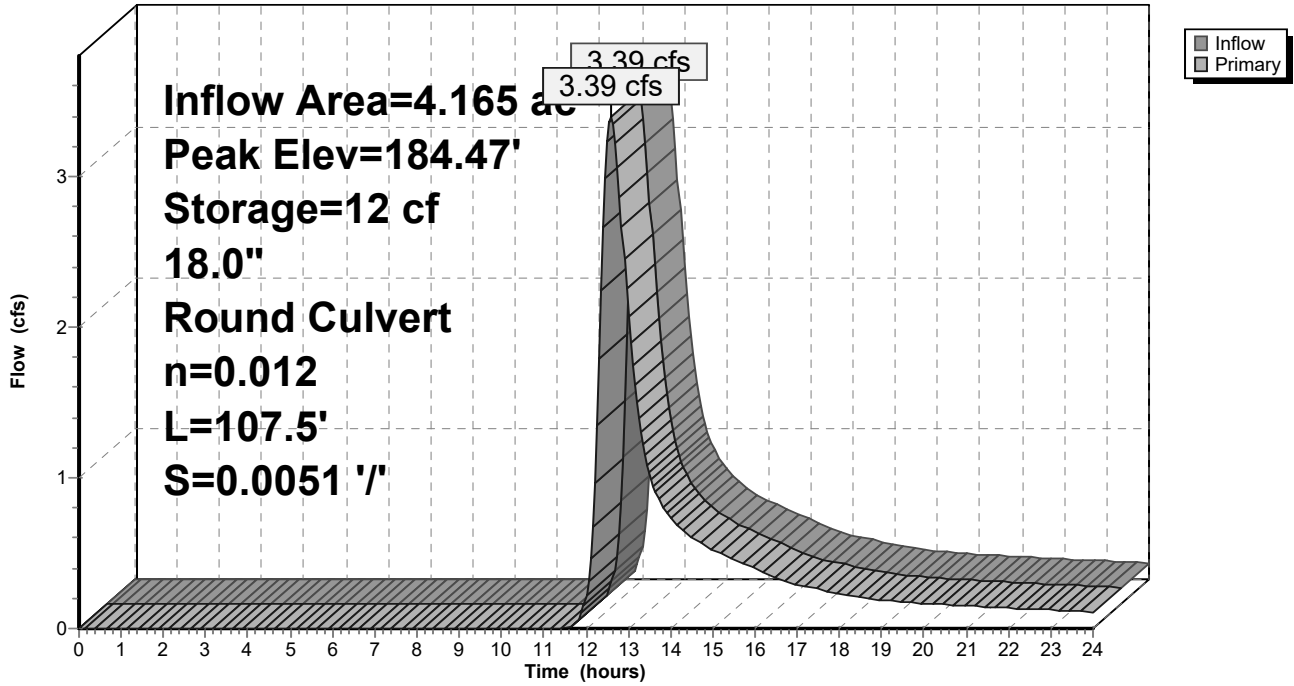
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
187.50	71	52.1	0	0	71
188.00	156	61.5	55	55	161
189.00	483	101.0	304	360	678
190.00	1,070	143.1	757	1,117	1,505

Device	Routing	Invert	Outlet Devices
#1	Primary	183.50'	<b>18.0" Round 18" HDPE N-12</b> L= 107.5' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 183.50' / 182.95' S= 0.0051 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

**Primary OutFlow** Max=3.39 cfs @ 12.60 hrs HW=184.47' TW=0.00' (Dynamic Tailwater)  
 ←1=18" HDPE N-12 (Barrel Controls 3.39 cfs @ 3.98 fps)

### Pond C50P: Inlet Sump

Hydrograph



**Summary for Pond D51P: DMH #51**

Inflow Area = 1.174 ac, 43.96% Impervious, Inflow Depth > 1.77" for 25YR-24HR event  
 Inflow = 0.68 cfs @ 12.71 hrs, Volume= 0.173 af  
 Outflow = 0.68 cfs @ 12.73 hrs, Volume= 0.173 af, Atten= 0%, Lag= 1.3 min  
 Primary = 0.68 cfs @ 12.73 hrs, Volume= 0.173 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 182.01' @ 12.73 hrs Surf.Area= 0.000 ac Storage= 0.000 af  
 Flood Elev= 185.75' Surf.Area= 0.000 ac Storage= 0.001 af

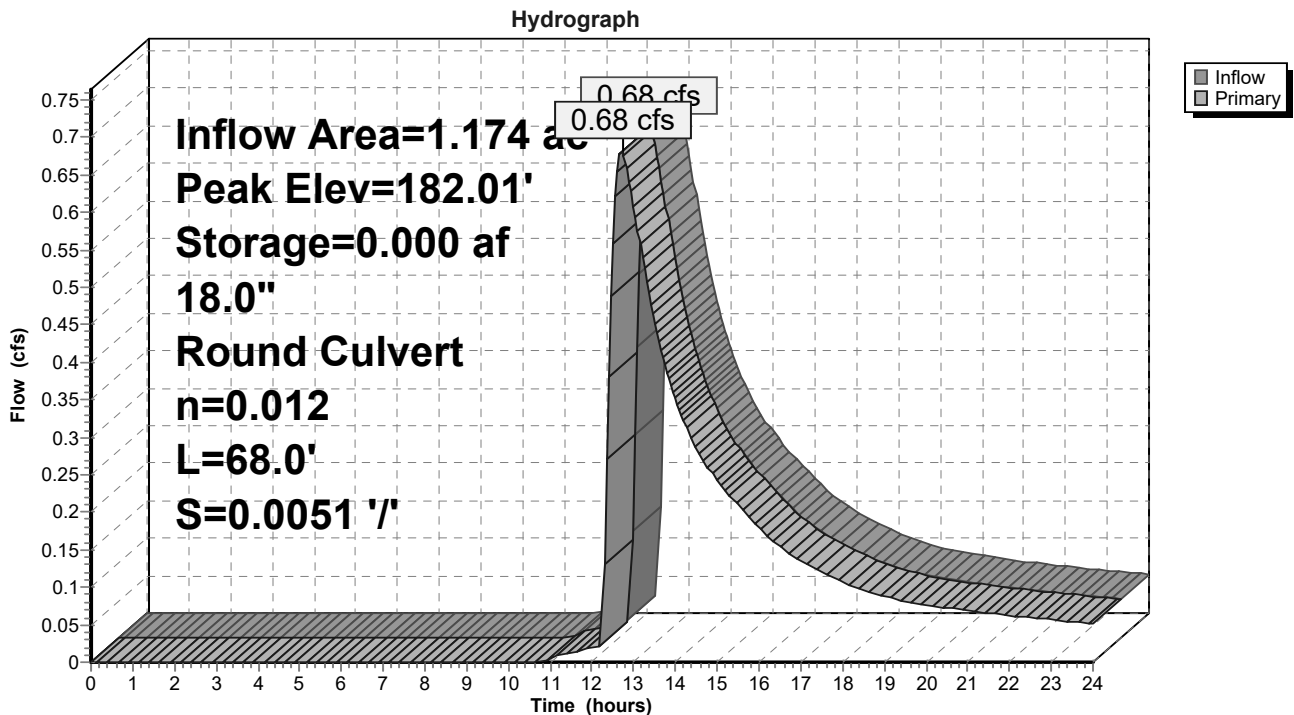
Plug-Flow detention time= 0.2 min calculated for 0.173 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 934.9 - 934.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	181.60'	0.001 af	<b>4.00'D x 4.05'H Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	181.60'	<b>18.0" Round 18" HDPE N-12</b> L= 68.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 181.60' / 181.25' S= 0.0051 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

**Primary OutFlow** Max=0.68 cfs @ 12.73 hrs HW=182.01' TW=0.00' (Dynamic Tailwater)  
 ↳ 1=18" HDPE N-12 (Barrel Controls 0.68 cfs @ 2.62 fps)

**Pond D51P: DMH #51**



**Summary for Pond D52P: DMH #52**

[80] Warning: Exceeded Pond C42P by 0.08' @ 12.05 hrs (0.69 cfs 0.005 af)

Inflow Area = 2.590 ac, 38.34% Impervious, Inflow Depth > 2.81" for 25YR-24HR event  
 Inflow = 6.97 cfs @ 12.10 hrs, Volume= 0.606 af  
 Outflow = 6.97 cfs @ 12.10 hrs, Volume= 0.606 af, Atten= 0%, Lag= 0.0 min  
 Primary = 6.97 cfs @ 12.10 hrs, Volume= 0.606 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 180.99' @ 12.11 hrs Surf.Area= 13 sf Storage= 16 cf  
 Flood Elev= 190.11' Surf.Area= 13 sf Storage= 131 cf

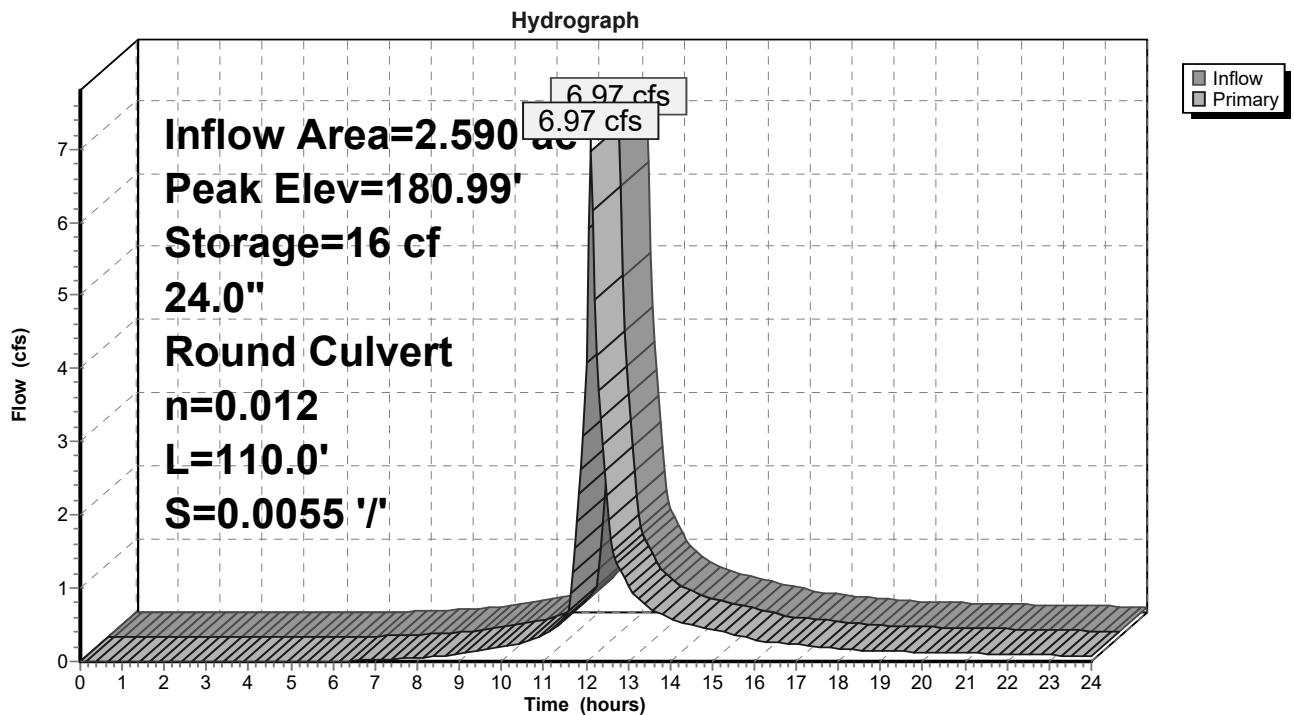
Plug-Flow detention time= 0.1 min calculated for 0.606 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 812.1 - 812.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	179.71'	131 cf	<b>4.00'D x 10.40'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	179.71'	<b>24.0" Round 24" HDPE N-12</b> L= 110.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 179.71' / 179.10' S= 0.0055 '/' Cc= 0.900 n= 0.012, Flow Area= 3.14 sf

**Primary OutFlow** Max=6.55 cfs @ 12.10 hrs HW=180.98' TW=180.16' (Dynamic Tailwater)  
 ←1=24" HDPE N-12 (Outlet Controls 6.55 cfs @ 4.42 fps)

**Pond D52P: DMH #52**



**Summary for Pond D53P: DMH #53**

Inflow Area = 2.590 ac, 38.34% Impervious, Inflow Depth > 2.81" for 25YR-24HR event  
 Inflow = 6.97 cfs @ 12.10 hrs, Volume= 0.606 af  
 Outflow = 6.97 cfs @ 12.10 hrs, Volume= 0.606 af, Atten= 0%, Lag= 0.0 min  
 Primary = 6.97 cfs @ 12.10 hrs, Volume= 0.606 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 180.16' @ 12.10 hrs Surf.Area= 13 sf Storage= 15 cf  
 Flood Elev= 191.95' Surf.Area= 13 sf Storage= 163 cf

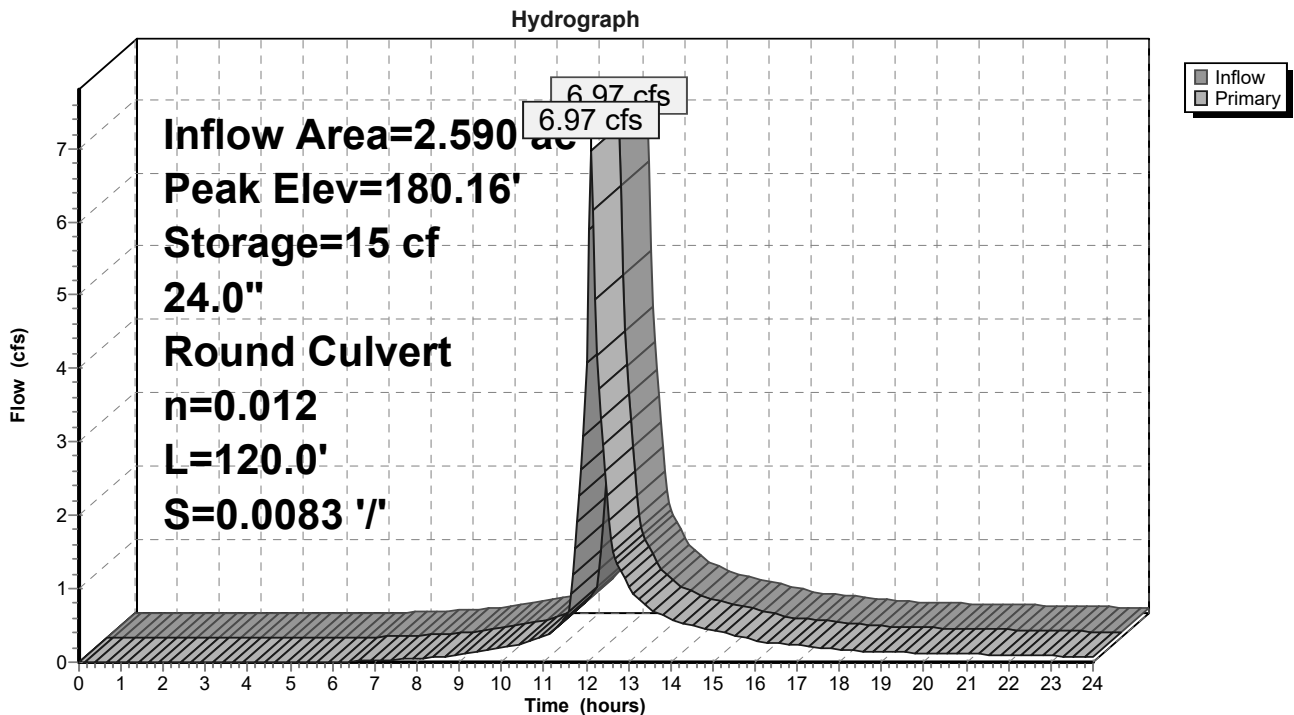
Plug-Flow detention time= 0.1 min calculated for 0.606 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 812.1 - 812.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	179.00'	163 cf	<b>4.00'D x 12.95'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	179.00'	<b>24.0" Round 24" HDPE N-12</b> L= 120.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 179.00' / 178.00' S= 0.0083 '/ Cc= 0.900 n= 0.012, Flow Area= 3.14 sf

**Primary OutFlow** Max=6.91 cfs @ 12.10 hrs HW=180.16' TW=178.47' (Dynamic Tailwater)  
 ↳ 1=24" HDPE N-12 (Inlet Controls 6.91 cfs @ 3.66 fps)

**Pond D53P: DMH #53**





**Summary for Pond E01P: Existing Catch Basin**

Inflow Area = 0.452 ac, 56.06% Impervious, Inflow Depth > 3.86" for 25YR-24HR event  
 Inflow = 1.99 cfs @ 12.09 hrs, Volume= 0.145 af  
 Outflow = 1.98 cfs @ 12.09 hrs, Volume= 0.145 af, Atten= 0%, Lag= 0.0 min  
 Primary = 1.98 cfs @ 12.09 hrs, Volume= 0.145 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 184.38' @ 12.12 hrs Surf.Area= 13 sf Storage= 11 cf  
 Flood Elev= 190.33' Surf.Area= 13 sf Storage= 86 cf

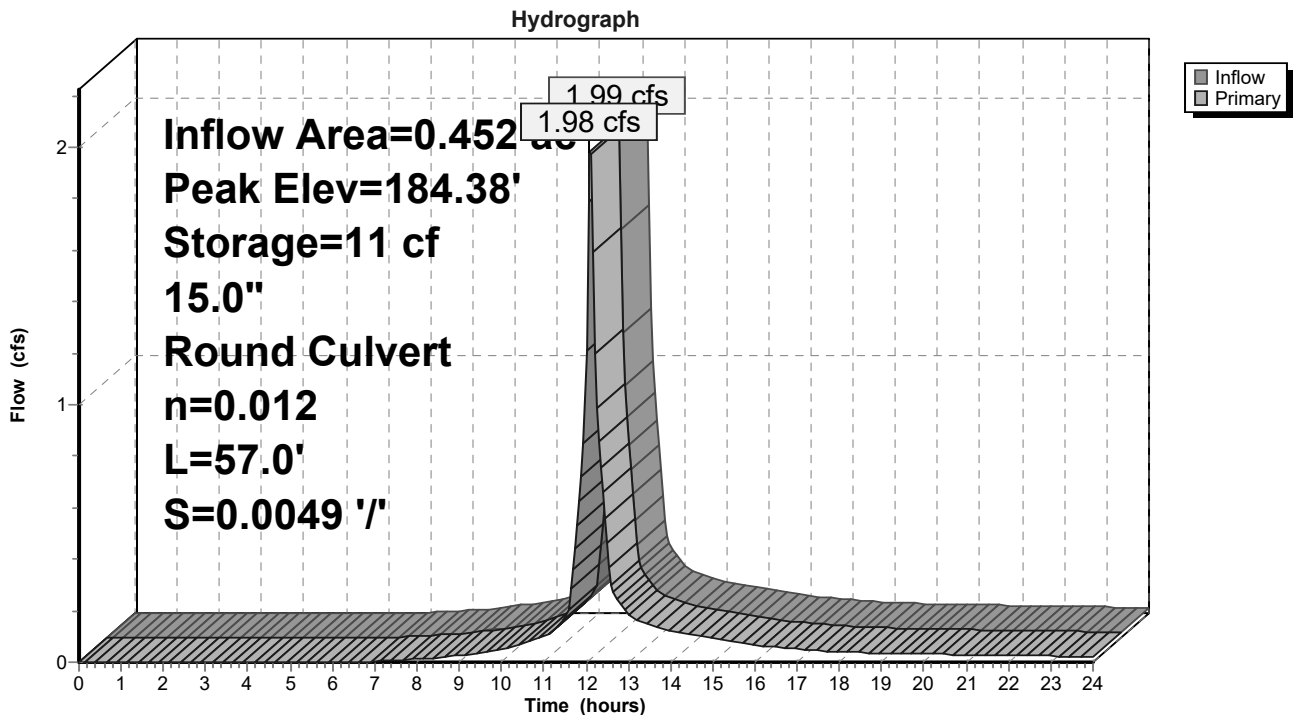
Plug-Flow detention time= 0.3 min calculated for 0.145 af (100% of inflow)  
 Center-of-Mass det. time= 0.2 min ( 809.5 - 809.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	183.50'	86 cf	<b>4.00'D x 6.83'H 4' Structure</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	183.50'	<b>15.0" Round 15" HDPE N-12</b> L= 57.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 183.50' / 183.22' S= 0.0049 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

**Primary OutFlow** Max=1.65 cfs @ 12.09 hrs HW=184.36' TW=184.08' (Dynamic Tailwater)  
 ←1=15" HDPE N-12 (Outlet Controls 1.65 cfs @ 2.61 fps)

**Pond E01P: Existing Catch Basin**



**Summary for Pond E02P: Existing Catch Basin**

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 1.322 ac, 63.48% Impervious, Inflow Depth > 4.07" for 25YR-24HR event  
 Inflow = 6.08 cfs @ 12.09 hrs, Volume= 0.448 af  
 Outflow = 6.09 cfs @ 12.10 hrs, Volume= 0.448 af, Atten= 0%, Lag= 0.5 min  
 Discarded = 0.02 cfs @ 12.05 hrs, Volume= 0.013 af  
 Primary = 6.08 cfs @ 12.10 hrs, Volume= 0.435 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 184.10' @ 12.10 hrs Surf.Area= 244 sf Storage= 224 cf  
 Flood Elev= 189.42' Surf.Area= 0 sf Storage= 464 cf

Plug-Flow detention time= 1.2 min calculated for 0.447 af (100% of inflow)  
 Center-of-Mass det. time= 0.9 min ( 804.6 - 803.7 )

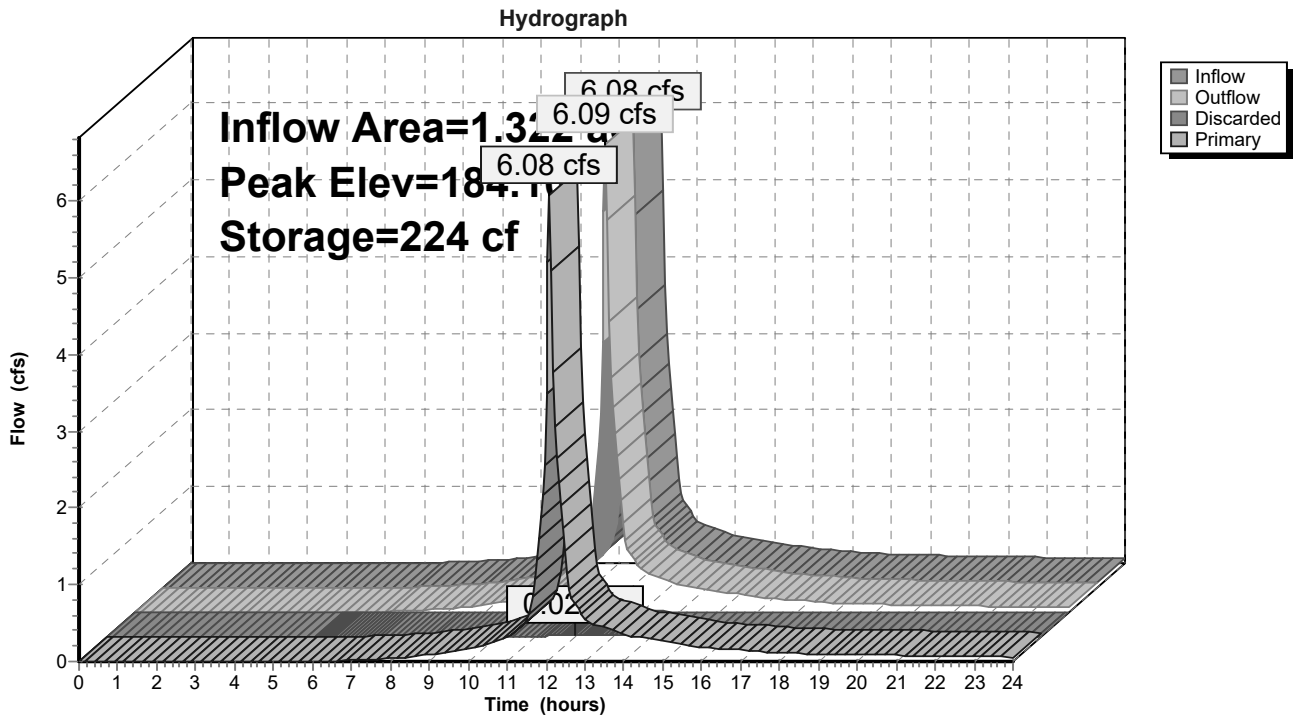
Volume	Invert	Avail.Storage	Storage Description
#1	183.02'	80 cf	<b>4.00'D x 6.40'H 4' Structure-Impervious</b>
#2	183.02'	384 cf	<b>24.0" Round 24" HDPE N-12 Perf</b> L= 122.2'
		464 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	183.02'	<b>24.0" Round 24" HDPE N-12 Perf</b> L= 122.2' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 183.02' / 179.71' S= 0.0271 '/' Cc= 0.900 n= 0.012, Flow Area= 3.14 sf
#2	Discarded	183.02'	<b>3.000 in/hr Infiltration over Surface area</b>

**Discarded OutFlow** Max=0.02 cfs @ 12.05 hrs HW=184.01' (Free Discharge)  
 ↳2=Infiltration (Exfiltration Controls 0.02 cfs)

**Primary OutFlow** Max=6.04 cfs @ 12.10 hrs HW=184.09' TW=180.98' (Dynamic Tailwater)  
 ↳1=24" HDPE N-12 Perf (Inlet Controls 6.04 cfs @ 3.53 fps)

### Pond E02P: Existing Catch Basin



Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

<b>Subcatchment 2S: Subcat #2</b>	Runoff Area=164,530 sf 2.77% Impervious Runoff Depth>0.39" Flow Length=298' Tc=16.6 min UI Adjusted CN=61 Runoff=0.77 cfs 0.124 af
<b>Subcatchment 3S: Subcat. #3</b>	Runoff Area=46,611 sf 0.00% Impervious Runoff Depth>0.07" Flow Length=158' Slope=0.0200 '/' Tc=11.3 min CN=48 Runoff=0.01 cfs 0.006 af
<b>Subcatchment 4S: Subcat. #4</b>	Runoff Area=55,483 sf 0.00% Impervious Runoff Depth>0.00" Flow Length=674' Tc=43.2 min CN=40 Runoff=0.00 cfs 0.000 af
<b>Subcatchment 30S: Subcat #30</b>	Runoff Area=47,823 sf 4.45% Impervious Runoff Depth>0.47" Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=63 Runoff=0.34 cfs 0.043 af
<b>Subcatchment 31S: Subcat #31</b>	Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>1.44" Tc=6.0 min CN=82 Runoff=0.75 cfs 0.054 af
<b>Subcatchment 32S: Subcat #32</b>	Runoff Area=37,918 sf 67.33% Impervious Runoff Depth>1.65" Tc=6.0 min CN=85 Runoff=1.66 cfs 0.120 af
<b>Subcatchment 41S: Subcat #41</b>	Runoff Area=7,421 sf 61.70% Impervious Runoff Depth>1.88" Flow Length=342' Tc=14.7 min CN=88 Runoff=0.29 cfs 0.027 af
<b>Subcatchment 43S: Subcat #43</b>	Runoff Area=15,256 sf 64.41% Impervious Runoff Depth>1.81" Flow Length=100' Tc=7.0 min CN=87 Runoff=0.71 cfs 0.053 af
<b>Subcatchment 44S: Subcat #44</b>	Runoff Area=14,458 sf 76.68% Impervious Runoff Depth>2.14" Flow Length=98' Tc=7.7 min CN=91 Runoff=0.77 cfs 0.059 af
<b>Subcatchment 45S: Subcat #45</b>	Runoff Area=16,893 sf 94.23% Impervious Runoff Depth>2.63" Flow Length=330' Tc=6.0 min CN=96 Runoff=1.09 cfs 0.085 af
<b>Subcatchment 46S: Subcat #46</b>	Runoff Area=7,602 sf 100.00% Impervious Runoff Depth>2.85" Tc=6.0 min CN=98 Runoff=0.51 cfs 0.041 af
<b>Subcatchment 47S: Subcat #47</b>	Runoff Area=3,200 sf 100.00% Impervious Runoff Depth>2.85" Tc=6.0 min CN=98 Runoff=0.21 cfs 0.017 af
<b>Subcatchment 50S: Subcat #50</b>	Runoff Area=11,704 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=182' Tc=11.7 min CN=33 Runoff=0.00 cfs 0.000 af
<b>Subcatchment 62S: Subcat. #62</b>	Runoff Area=45,124 sf 0.15% Impervious Runoff Depth>0.50" Flow Length=165' Tc=14.3 min CN=64 Runoff=0.33 cfs 0.043 af
<b>Subcatchment 63S: Subcat. #63</b>	Runoff Area=16,040 sf 0.00% Impervious Runoff Depth>0.30" Flow Length=150' Tc=10.8 min CN=58 Runoff=0.05 cfs 0.009 af
<b>Subcatchment 64S: Subcat #64</b>	Runoff Area=7,675 sf 0.00% Impervious Runoff Depth>0.30" Tc=6.0 min CN=58 Runoff=0.03 cfs 0.004 af

<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=51,128 sf 43.96% Impervious Runoff Depth>0.62" Flow Length=345' Tc=14.1 min CN=67 Runoff=0.53 cfs 0.061 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>0.16" Flow Length=563' Tc=39.5 min CN=53 Runoff=0.07 cfs 0.032 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>0.46" Flow Length=478' Tc=32.0 min CN=63 Runoff=0.34 cfs 0.061 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=151.0' S=0.0063 '/' Capacity=12.85 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=35.0' S=0.2286 '/' Capacity=77.47 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.035 L=58.0' S=0.0948 '/' Capacity=31.37 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 71aR: Wooded Swale</b>	Avg. Flow Depth=0.18' Max Vel=0.63 fps Inflow=0.34 cfs 0.070 af n=0.035 L=78.5' S=0.0038 '/' Capacity=61.73 cfs Outflow=0.34 cfs 0.069 af
<b>Reach 72R: Roadside Swale</b>	Avg. Flow Depth=0.09' Max Vel=0.92 fps Inflow=0.31 cfs 0.038 af n=0.022 L=495.6' S=0.0060 '/' Capacity=33.12 cfs Outflow=0.28 cfs 0.038 af
<b>Reach 200R: Final Reach #200</b>	Inflow=0.77 cfs 0.124 af Outflow=0.77 cfs 0.124 af
<b>Reach 300R: Final Reach #300</b>	Inflow=1.55 cfs 0.217 af Outflow=1.55 cfs 0.217 af
<b>Reach 400R: Final Reach #400</b>	Inflow=0.36 cfs 0.089 af Outflow=0.36 cfs 0.089 af
<b>Pond 30P: Infiltration/Trench</b>	Peak Elev=183.51' Storage=162 cf Inflow=0.34 cfs 0.043 af Discarded=0.08 cfs 0.027 af Primary=0.21 cfs 0.016 af Secondary=0.00 cfs 0.000 af Outflow=0.28 cfs 0.043 af
<b>Pond 71P: Existing Catch Basin</b>	Peak Elev=188.18' Inflow=0.34 cfs 0.070 af 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/' Outflow=0.34 cfs 0.070 af
<b>Pond 72P: Existing Depression</b>	Peak Elev=196.04' Storage=103 cf Inflow=0.34 cfs 0.061 af Discarded=0.02 cfs 0.021 af Primary=0.31 cfs 0.038 af Outflow=0.33 cfs 0.059 af
<b>Pond 201P: Bioretention W/ ISR #201</b>	Peak Elev=184.56' Storage=1,815 cf Inflow=0.53 cfs 0.061 af Primary=0.02 cfs 0.020 af Secondary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.020 af
<b>Pond 202P: Bioretention W/ ISR #202</b>	Peak Elev=177.39' Storage=7,276 cf Inflow=3.42 cfs 0.299 af Primary=0.07 cfs 0.084 af Secondary=0.20 cfs 0.085 af Tertiary=0.00 cfs 0.000 af Outflow=0.27 cfs 0.169 af
<b>Pond 203P: Infiltration Pond #203</b>	Peak Elev=173.49' Storage=1,933 cf Inflow=0.29 cfs 0.178 af Discarded=0.16 cfs 0.158 af Primary=0.00 cfs 0.000 af Outflow=0.16 cfs 0.158 af
<b>Pond 204P: Detention Pond #204</b>	Peak Elev=177.73' Storage=2,147 cf Inflow=2.61 cfs 0.211 af Primary=1.55 cfs 0.211 af Secondary=0.00 cfs 0.000 af Outflow=1.55 cfs 0.211 af

**Pond C41P: Catch Basin #41** Peak Elev=184.25' Storage=3 cf Inflow=0.29 cfs 0.027 af  
15.0" Round Culvert n=0.012 L=54.7' S=0.0686 '/' Outflow=0.29 cfs 0.027 af

**Pond C42P: Catch Basin #42** Peak Elev=180.56' Storage=5 cf Inflow=0.45 cfs 0.042 af  
15.0" Round Culvert n=0.012 L=63.0' S=0.0054 '/' Outflow=0.45 cfs 0.042 af

**Pond C43P: Catch Basin #43** Peak Elev=180.89' Storage=5 cf Inflow=0.71 cfs 0.053 af  
15.0" Round Culvert n=0.012 L=60.0' S=0.0233 '/' Outflow=0.71 cfs 0.053 af

**Pond C44P: Catch Basin #44** Peak Elev=179.74' Storage=9 cf Inflow=1.47 cfs 0.112 af  
15.0" Round Culvert n=0.012 L=48.0' S=0.0083 '/' Outflow=1.47 cfs 0.112 af

**Pond C45P: Catch Basin #45** Peak Elev=181.90' Storage=6 cf Inflow=1.09 cfs 0.085 af  
15.0" Round Culvert n=0.012 L=87.2' S=0.0149 '/' Outflow=1.09 cfs 0.085 af

**Pond C46P: Catch Basin #46** Peak Elev=180.61' Storage=8 cf Inflow=1.60 cfs 0.126 af  
15.0" Round Culvert n=0.012 L=68.0' S=0.0206 '/' Outflow=1.60 cfs 0.126 af

**Pond C47P: Catch Basin #47** Peak Elev=179.47' Storage=12 cf Inflow=3.27 cfs 0.256 af  
18.0" Round Culvert n=0.012 L=40.0' S=0.0063 '/' Outflow=3.27 cfs 0.256 af

**Pond C50P: Inlet Sump** Peak Elev=183.78' Storage=4 cf Inflow=0.34 cfs 0.069 af  
18.0" Round Culvert n=0.012 L=107.5' S=0.0051 '/' Outflow=0.34 cfs 0.069 af

**Pond D51P: DMH #51** Peak Elev=181.67' Storage=0.000 af Inflow=0.02 cfs 0.020 af  
18.0" Round Culvert n=0.012 L=68.0' S=0.0051 '/' Outflow=0.02 cfs 0.020 af

**Pond D52P: DMH #52** Peak Elev=180.44' Storage=9 cf Inflow=2.59 cfs 0.207 af  
24.0" Round Culvert n=0.012 L=110.0' S=0.0055 '/' Outflow=2.59 cfs 0.207 af

**Pond D53P: DMH #53** Peak Elev=179.67' Storage=8 cf Inflow=2.59 cfs 0.207 af  
24.0" Round Culvert n=0.012 L=120.0' S=0.0083 '/' Outflow=2.60 cfs 0.207 af

**Pond E01P: Existing Catch Basin** Peak Elev=183.97' Storage=6 cf Inflow=0.75 cfs 0.054 af  
15.0" Round Culvert n=0.012 L=57.0' S=0.0049 '/' Outflow=0.75 cfs 0.054 af

**Pond E02P: Existing Catch Basin** Peak Elev=183.66' Storage=115 cf Inflow=2.40 cfs 0.174 af  
Discarded=0.02 cfs 0.010 af Primary=2.38 cfs 0.165 af Outflow=2.40 cfs 0.174 af

**Total Runoff Area = 16.948 ac Runoff Volume = 0.840 af Average Runoff Depth = 0.59"**  
**76.01% Pervious = 12.882 ac 23.99% Impervious = 4.066 ac**

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment 2S: Subcat #2** Runoff Area=164,530 sf 2.77% Impervious Runoff Depth>1.16"  
Flow Length=298' Tc=16.6 min UI Adjusted CN=61 Runoff=3.28 cfs 0.365 af

**Subcatchment 3S: Subcat. #3** Runoff Area=46,611 sf 0.00% Impervious Runoff Depth>0.46"  
Flow Length=158' Slope=0.0200 '/' Tc=11.3 min CN=48 Runoff=0.23 cfs 0.041 af

**Subcatchment 4S: Subcat. #4** Runoff Area=55,483 sf 0.00% Impervious Runoff Depth>0.16"  
Flow Length=674' Tc=43.2 min CN=40 Runoff=0.03 cfs 0.017 af

**Subcatchment 30S: Subcat #30** Runoff Area=47,823 sf 4.45% Impervious Runoff Depth>1.29"  
Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=63 Runoff=1.26 cfs 0.118 af

**Subcatchment 31S: Subcat #31** Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>2.77"  
Tc=6.0 min CN=82 Runoff=1.43 cfs 0.104 af

**Subcatchment 32S: Subcat #32** Runoff Area=37,918 sf 67.33% Impervious Runoff Depth>3.04"  
Tc=6.0 min CN=85 Runoff=3.02 cfs 0.221 af

**Subcatchment 41S: Subcat #41** Runoff Area=7,421 sf 61.70% Impervious Runoff Depth>3.33"  
Flow Length=342' Tc=14.7 min CN=88 Runoff=0.50 cfs 0.047 af

**Subcatchment 43S: Subcat #43** Runoff Area=15,256 sf 64.41% Impervious Runoff Depth>3.24"  
Flow Length=100' Tc=7.0 min CN=87 Runoff=1.25 cfs 0.094 af

**Subcatchment 44S: Subcat #44** Runoff Area=14,458 sf 76.68% Impervious Runoff Depth>3.64"  
Flow Length=98' Tc=7.7 min CN=91 Runoff=1.27 cfs 0.101 af

**Subcatchment 45S: Subcat #45** Runoff Area=16,893 sf 94.23% Impervious Runoff Depth>4.18"  
Flow Length=330' Tc=6.0 min CN=96 Runoff=1.69 cfs 0.135 af

**Subcatchment 46S: Subcat #46** Runoff Area=7,602 sf 100.00% Impervious Runoff Depth>4.41"  
Tc=6.0 min CN=98 Runoff=0.77 cfs 0.064 af

**Subcatchment 47S: Subcat #47** Runoff Area=3,200 sf 100.00% Impervious Runoff Depth>4.41"  
Tc=6.0 min CN=98 Runoff=0.33 cfs 0.027 af

**Subcatchment 50S: Subcat #50** Runoff Area=11,704 sf 0.00% Impervious Runoff Depth>0.02"  
Flow Length=182' Tc=11.7 min CN=33 Runoff=0.00 cfs 0.000 af

**Subcatchment 62S: Subcat. #62** Runoff Area=45,124 sf 0.15% Impervious Runoff Depth>1.35"  
Flow Length=165' Tc=14.3 min CN=64 Runoff=1.16 cfs 0.117 af

**Subcatchment 63S: Subcat. #63** Runoff Area=16,040 sf 0.00% Impervious Runoff Depth>0.98"  
Flow Length=150' Tc=10.8 min CN=58 Runoff=0.29 cfs 0.030 af

**Subcatchment 64S: Subcat #64** Runoff Area=7,675 sf 0.00% Impervious Runoff Depth>0.98"  
Tc=6.0 min CN=58 Runoff=0.17 cfs 0.014 af

<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=51,128 sf 43.96% Impervious Runoff Depth>1.56" Flow Length=345' Tc=14.1 min CN=67 Runoff=1.57 cfs 0.152 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>0.69" Flow Length=563' Tc=39.5 min CN=53 Runoff=0.68 cfs 0.134 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>1.28" Flow Length=478' Tc=32.0 min CN=63 Runoff=1.20 cfs 0.169 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=151.0' S=0.0063 '/' Capacity=12.85 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=35.0' S=0.2286 '/' Capacity=77.47 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.035 L=58.0' S=0.0948 '/' Capacity=31.37 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 71aR: Wooded Swale</b>	Avg. Flow Depth=0.38' Max Vel=1.05 fps Inflow=1.78 cfs 0.277 af n=0.035 L=78.5' S=0.0038 '/' Capacity=61.73 cfs Outflow=1.78 cfs 0.277 af
<b>Reach 72R: Roadside Swale</b>	Avg. Flow Depth=0.19' Max Vel=1.45 fps Inflow=1.16 cfs 0.144 af n=0.022 L=495.6' S=0.0060 '/' Capacity=33.12 cfs Outflow=1.12 cfs 0.144 af
<b>Reach 200R: Final Reach #200</b>	Inflow=3.28 cfs 0.365 af Outflow=3.28 cfs 0.365 af
<b>Reach 300R: Final Reach #300</b>	Inflow=3.49 cfs 0.599 af Outflow=3.49 cfs 0.599 af
<b>Reach 400R: Final Reach #400</b>	Inflow=1.84 cfs 0.367 af Outflow=1.84 cfs 0.367 af
<b>Pond 30P: Infiltration/Trench</b>	Peak Elev=183.71' Storage=667 cf Inflow=1.26 cfs 0.118 af Discarded=0.36 cfs 0.053 af Primary=0.42 cfs 0.063 af Secondary=0.00 cfs 0.000 af Outflow=0.78 cfs 0.117 af
<b>Pond 71P: Existing Catch Basin</b>	Peak Elev=188.60' Inflow=1.78 cfs 0.277 af 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/' Outflow=1.78 cfs 0.277 af
<b>Pond 72P: Existing Depression</b>	Peak Elev=196.14' Storage=138 cf Inflow=1.20 cfs 0.169 af Discarded=0.02 cfs 0.023 af Primary=1.16 cfs 0.144 af Outflow=1.18 cfs 0.167 af
<b>Pond 201P: Bioretention W/ ISR #201</b>	Peak Elev=185.13' Storage=3,247 cf Inflow=1.57 cfs 0.152 af Primary=0.02 cfs 0.022 af Secondary=0.18 cfs 0.067 af Outflow=0.20 cfs 0.090 af
<b>Pond 202P: Bioretention W/ ISR #202</b>	Peak Elev=178.40' Storage=12,765 cf Inflow=6.06 cfs 0.538 af Primary=0.08 cfs 0.092 af Secondary=0.47 cfs 0.303 af Tertiary=0.00 cfs 0.000 af Outflow=0.55 cfs 0.395 af
<b>Pond 203P: Infiltration Pond #203</b>	Peak Elev=174.56' Storage=4,756 cf Inflow=0.68 cfs 0.425 af Discarded=0.21 cfs 0.221 af Primary=0.32 cfs 0.105 af Outflow=0.53 cfs 0.326 af
<b>Pond 204P: Detention Pond #204</b>	Peak Elev=178.35' Storage=3,509 cf Inflow=5.27 cfs 0.438 af Primary=3.27 cfs 0.436 af Secondary=0.00 cfs 0.000 af Outflow=3.27 cfs 0.436 af



**Pond C41P: Catch Basin #41** Peak Elev=184.33' Storage=4 cf Inflow=0.50 cfs 0.047 af  
15.0" Round Culvert n=0.012 L=54.7' S=0.0686 '/' Outflow=0.50 cfs 0.047 af

**Pond C42P: Catch Basin #42** Peak Elev=180.89' Storage=9 cf Inflow=0.88 cfs 0.110 af  
15.0" Round Culvert n=0.012 L=63.0' S=0.0054 '/' Outflow=0.88 cfs 0.110 af

**Pond C43P: Catch Basin #43** Peak Elev=181.04' Storage=7 cf Inflow=1.25 cfs 0.094 af  
15.0" Round Culvert n=0.012 L=60.0' S=0.0233 '/' Outflow=1.25 cfs 0.094 af

**Pond C44P: Catch Basin #44** Peak Elev=180.08' Storage=14 cf Inflow=2.52 cfs 0.195 af  
15.0" Round Culvert n=0.012 L=48.0' S=0.0083 '/' Outflow=2.51 cfs 0.195 af

**Pond C45P: Catch Basin #45** Peak Elev=182.03' Storage=8 cf Inflow=1.69 cfs 0.135 af  
15.0" Round Culvert n=0.012 L=87.2' S=0.0149 '/' Outflow=1.69 cfs 0.135 af

**Pond C46P: Catch Basin #46** Peak Elev=180.79' Storage=10 cf Inflow=2.47 cfs 0.199 af  
15.0" Round Culvert n=0.012 L=68.0' S=0.0206 '/' Outflow=2.47 cfs 0.199 af

**Pond C47P: Catch Basin #47** Peak Elev=179.81' Storage=16 cf Inflow=5.28 cfs 0.421 af  
18.0" Round Culvert n=0.012 L=40.0' S=0.0063 '/' Outflow=5.28 cfs 0.421 af

**Pond C50P: Inlet Sump** Peak Elev=184.17' Storage=8 cf Inflow=1.78 cfs 0.278 af  
18.0" Round Culvert n=0.012 L=107.5' S=0.0051 '/' Outflow=1.78 cfs 0.277 af

**Pond D51P: DMH #51** Peak Elev=181.82' Storage=0.000 af Inflow=0.20 cfs 0.090 af  
18.0" Round Culvert n=0.012 L=68.0' S=0.0051 '/' Outflow=0.20 cfs 0.090 af

**Pond D52P: DMH #52** Peak Elev=180.78' Storage=13 cf Inflow=5.11 cfs 0.423 af  
24.0" Round Culvert n=0.012 L=110.0' S=0.0055 '/' Outflow=5.11 cfs 0.423 af

**Pond D53P: DMH #53** Peak Elev=179.97' Storage=12 cf Inflow=5.11 cfs 0.423 af  
24.0" Round Culvert n=0.012 L=120.0' S=0.0083 '/' Outflow=5.11 cfs 0.423 af

**Pond E01P: Existing Catch Basin** Peak Elev=184.21' Storage=9 cf Inflow=1.43 cfs 0.104 af  
15.0" Round Culvert n=0.012 L=57.0' S=0.0049 '/' Outflow=1.43 cfs 0.104 af

**Pond E02P: Existing Catch Basin** Peak Elev=183.92' Storage=179 cf Inflow=4.45 cfs 0.325 af  
Discarded=0.02 cfs 0.012 af Primary=4.44 cfs 0.313 af Outflow=4.46 cfs 0.325 af

**Total Runoff Area = 16.948 ac Runoff Volume = 1.951 af Average Runoff Depth = 1.38"**  
**76.01% Pervious = 12.882 ac 23.99% Impervious = 4.066 ac**

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment 2S: Subcat #2** Runoff Area=164,530 sf 2.77% Impervious Runoff Depth>1.91"  
Flow Length=298' Tc=16.6 min UI Adjusted CN=61 Runoff=5.80 cfs 0.602 af

**Subcatchment 3S: Subcat. #3** Runoff Area=46,611 sf 0.00% Impervious Runoff Depth>0.94"  
Flow Length=158' Slope=0.0200 '/' Tc=11.3 min CN=48 Runoff=0.69 cfs 0.084 af

**Subcatchment 4S: Subcat. #4** Runoff Area=55,483 sf 0.00% Impervious Runoff Depth>0.45"  
Flow Length=674' Tc=43.2 min CN=40 Runoff=0.15 cfs 0.048 af

**Subcatchment 30S: Subcat #30** Runoff Area=47,823 sf 4.45% Impervious Runoff Depth>2.08"  
Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=63 Runoff=2.15 cfs 0.190 af

**Subcatchment 31S: Subcat #31** Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>3.86"  
Tc=6.0 min CN=82 Runoff=1.99 cfs 0.145 af

**Subcatchment 32S: Subcat #32** Runoff Area=37,918 sf 67.33% Impervious Runoff Depth>4.18"  
Tc=6.0 min CN=85 Runoff=4.10 cfs 0.303 af

**Subcatchment 41S: Subcat #41** Runoff Area=7,421 sf 61.70% Impervious Runoff Depth>4.49"  
Flow Length=342' Tc=14.7 min CN=88 Runoff=0.66 cfs 0.064 af

**Subcatchment 43S: Subcat #43** Runoff Area=15,256 sf 64.41% Impervious Runoff Depth>4.39"  
Flow Length=100' Tc=7.0 min CN=87 Runoff=1.67 cfs 0.128 af

**Subcatchment 44S: Subcat #44** Runoff Area=14,458 sf 76.68% Impervious Runoff Depth>4.83"  
Flow Length=98' Tc=7.7 min CN=91 Runoff=1.66 cfs 0.133 af

**Subcatchment 45S: Subcat #45** Runoff Area=16,893 sf 94.23% Impervious Runoff Depth>5.39"  
Flow Length=330' Tc=6.0 min CN=96 Runoff=2.15 cfs 0.174 af

**Subcatchment 46S: Subcat #46** Runoff Area=7,602 sf 100.00% Impervious Runoff Depth>5.63"  
Tc=6.0 min CN=98 Runoff=0.98 cfs 0.082 af

**Subcatchment 47S: Subcat #47** Runoff Area=3,200 sf 100.00% Impervious Runoff Depth>5.63"  
Tc=6.0 min CN=98 Runoff=0.41 cfs 0.034 af

**Subcatchment 50S: Subcat #50** Runoff Area=11,704 sf 0.00% Impervious Runoff Depth>0.15"  
Flow Length=182' Tc=11.7 min CN=33 Runoff=0.01 cfs 0.003 af

**Subcatchment 62S: Subcat. #62** Runoff Area=45,124 sf 0.15% Impervious Runoff Depth>2.16"  
Flow Length=165' Tc=14.3 min CN=64 Runoff=1.95 cfs 0.187 af

**Subcatchment 63S: Subcat. #63** Runoff Area=16,040 sf 0.00% Impervious Runoff Depth>1.67"  
Flow Length=150' Tc=10.8 min CN=58 Runoff=0.56 cfs 0.051 af

**Subcatchment 64S: Subcat #64** Runoff Area=7,675 sf 0.00% Impervious Runoff Depth>1.67"  
Tc=6.0 min CN=58 Runoff=0.31 cfs 0.025 af

<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=51,128 sf 43.96% Impervious Runoff Depth>2.43" Flow Length=345' Tc=14.1 min CN=67 Runoff=2.53 cfs 0.237 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>1.28" Flow Length=563' Tc=39.5 min CN=53 Runoff=1.48 cfs 0.246 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>2.07" Flow Length=478' Tc=32.0 min CN=63 Runoff=2.04 cfs 0.273 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=151.0' S=0.0063 '/' Capacity=12.85 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=35.0' S=0.2286 '/' Capacity=77.47 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.035 L=58.0' S=0.0948 '/' Capacity=31.37 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 71aR: Wooded Swale</b>	Avg. Flow Depth=0.52' Max Vel=1.28 fps Inflow=3.40 cfs 0.492 af n=0.035 L=78.5' S=0.0038 '/' Capacity=61.73 cfs Outflow=3.39 cfs 0.492 af
<b>Reach 72R: Roadside Swale</b>	Avg. Flow Depth=0.25' Max Vel=1.71 fps Inflow=2.00 cfs 0.247 af n=0.022 L=495.6' S=0.0060 '/' Capacity=33.12 cfs Outflow=1.95 cfs 0.246 af
<b>Reach 200R: Final Reach #200</b>	Inflow=5.80 cfs 0.602 af Outflow=5.80 cfs 0.602 af
<b>Reach 300R: Final Reach #300</b>	Inflow=4.82 cfs 1.034 af Outflow=4.82 cfs 1.034 af
<b>Reach 400R: Final Reach #400</b>	Inflow=4.05 cfs 0.668 af Outflow=4.05 cfs 0.668 af
<b>Pond 30P: Infiltration/Trench</b>	Peak Elev=183.82' Storage=1,384 cf Inflow=2.15 cfs 0.190 af Discarded=0.59 cfs 0.081 af Primary=0.51 cfs 0.107 af Secondary=0.00 cfs 0.000 af Outflow=1.10 cfs 0.188 af
<b>Pond 71P: Existing Catch Basin</b>	Peak Elev=188.93' Inflow=3.40 cfs 0.492 af 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/' Outflow=3.40 cfs 0.492 af
<b>Pond 72P: Existing Depression</b>	Peak Elev=196.21' Storage=160 cf Inflow=2.04 cfs 0.273 af Discarded=0.02 cfs 0.024 af Primary=2.00 cfs 0.247 af Outflow=2.02 cfs 0.271 af
<b>Pond 201P: Bioretention W/ ISR #201</b>	Peak Elev=185.45' Storage=4,286 cf Inflow=2.53 cfs 0.237 af Primary=0.02 cfs 0.023 af Secondary=0.66 cfs 0.150 af Outflow=0.68 cfs 0.173 af
<b>Pond 202P: Bioretention W/ ISR #202</b>	Peak Elev=179.19' Storage=18,074 cf Inflow=8.23 cfs 0.739 af Primary=0.09 cfs 0.103 af Secondary=0.60 cfs 0.463 af Tertiary=0.00 cfs 0.000 af Outflow=0.68 cfs 0.566 af
<b>Pond 203P: Infiltration Pond #203</b>	Peak Elev=174.58' Storage=4,820 cf Inflow=1.08 cfs 0.617 af Discarded=0.21 cfs 0.233 af Primary=0.52 cfs 0.277 af Outflow=0.73 cfs 0.510 af
<b>Pond 204P: Detention Pond #204</b>	Peak Elev=178.85' Storage=4,793 cf Inflow=7.28 cfs 0.630 af Primary=4.16 cfs 0.625 af Secondary=0.00 cfs 0.000 af Outflow=4.16 cfs 0.625 af

**Pond C41P: Catch Basin #41** Peak Elev=184.38' Storage=5 cf Inflow=0.66 cfs 0.064 af  
15.0" Round Culvert n=0.012 L=54.7' S=0.0686 ' Outflow=0.66 cfs 0.064 af

**Pond C42P: Catch Basin #42** Peak Elev=181.09' Storage=12 cf Inflow=1.14 cfs 0.171 af  
15.0" Round Culvert n=0.012 L=63.0' S=0.0054 ' Outflow=1.14 cfs 0.171 af

**Pond C43P: Catch Basin #43** Peak Elev=181.13' Storage=8 cf Inflow=1.67 cfs 0.128 af  
15.0" Round Culvert n=0.012 L=60.0' S=0.0233 ' Outflow=1.67 cfs 0.128 af

**Pond C44P: Catch Basin #44** Peak Elev=180.36' Storage=17 cf Inflow=3.33 cfs 0.262 af  
15.0" Round Culvert n=0.012 L=48.0' S=0.0083 ' Outflow=3.32 cfs 0.262 af

**Pond C45P: Catch Basin #45** Peak Elev=182.13' Storage=9 cf Inflow=2.15 cfs 0.174 af  
15.0" Round Culvert n=0.012 L=87.2' S=0.0149 ' Outflow=2.15 cfs 0.174 af

**Pond C46P: Catch Basin #46** Peak Elev=180.91' Storage=11 cf Inflow=3.13 cfs 0.256 af  
15.0" Round Culvert n=0.012 L=68.0' S=0.0206 ' Outflow=3.14 cfs 0.256 af

**Pond C47P: Catch Basin #47** Peak Elev=180.08' Storage=20 cf Inflow=6.84 cfs 0.552 af  
18.0" Round Culvert n=0.012 L=40.0' S=0.0063 ' Outflow=6.84 cfs 0.552 af

**Pond C50P: Inlet Sump** Peak Elev=184.47' Storage=12 cf Inflow=3.39 cfs 0.495 af  
18.0" Round Culvert n=0.012 L=107.5' S=0.0051 ' Outflow=3.39 cfs 0.495 af

**Pond D51P: DMH #51** Peak Elev=182.01' Storage=0.000 af Inflow=0.68 cfs 0.173 af  
18.0" Round Culvert n=0.012 L=68.0' S=0.0051 ' Outflow=0.68 cfs 0.173 af

**Pond D52P: DMH #52** Peak Elev=180.99' Storage=16 cf Inflow=6.97 cfs 0.606 af  
24.0" Round Culvert n=0.012 L=110.0' S=0.0055 ' Outflow=6.97 cfs 0.606 af

**Pond D53P: DMH #53** Peak Elev=180.16' Storage=15 cf Inflow=6.97 cfs 0.606 af  
24.0" Round Culvert n=0.012 L=120.0' S=0.0083 ' Outflow=6.97 cfs 0.606 af

**Pond E01P: Existing Catch Basin** Peak Elev=184.38' Storage=11 cf Inflow=1.99 cfs 0.145 af  
15.0" Round Culvert n=0.012 L=57.0' S=0.0049 ' Outflow=1.98 cfs 0.145 af

**Pond E02P: Existing Catch Basin** Peak Elev=184.10' Storage=224 cf Inflow=6.08 cfs 0.448 af  
Discarded=0.02 cfs 0.013 af Primary=6.08 cfs 0.435 af Outflow=6.09 cfs 0.448 af

**Total Runoff Area = 16.948 ac Runoff Volume = 3.011 af Average Runoff Depth = 2.13"**  
**76.01% Pervious = 12.882 ac 23.99% Impervious = 4.066 ac**

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment 2S: Subcat #2** Runoff Area=164,530 sf 2.77% Impervious Runoff Depth>2.71"  
Flow Length=298' Tc=16.6 min UI Adjusted CN=61 Runoff=8.45 cfs 0.852 af

**Subcatchment 3S: Subcat. #3** Runoff Area=46,611 sf 0.00% Impervious Runoff Depth>1.50"  
Flow Length=158' Slope=0.0200 '/' Tc=11.3 min CN=48 Runoff=1.28 cfs 0.133 af

**Subcatchment 4S: Subcat. #4** Runoff Area=55,483 sf 0.00% Impervious Runoff Depth>0.83"  
Flow Length=674' Tc=43.2 min CN=40 Runoff=0.38 cfs 0.089 af

**Subcatchment 30S: Subcat #30** Runoff Area=47,823 sf 4.45% Impervious Runoff Depth>2.91"  
Flow Length=87' Slope=0.0110 '/' Tc=11.3 min CN=63 Runoff=3.07 cfs 0.266 af

**Subcatchment 31S: Subcat #31** Runoff Area=19,678 sf 56.06% Impervious Runoff Depth>4.93"  
Tc=6.0 min CN=82 Runoff=2.52 cfs 0.186 af

**Subcatchment 32S: Subcat #32** Runoff Area=37,918 sf 67.33% Impervious Runoff Depth>5.27"  
Tc=6.0 min CN=85 Runoff=5.11 cfs 0.382 af

**Subcatchment 41S: Subcat #41** Runoff Area=7,421 sf 61.70% Impervious Runoff Depth>5.60"  
Flow Length=342' Tc=14.7 min CN=88 Runoff=0.82 cfs 0.080 af

**Subcatchment 43S: Subcat #43** Runoff Area=15,256 sf 64.41% Impervious Runoff Depth>5.49"  
Flow Length=100' Tc=7.0 min CN=87 Runoff=2.07 cfs 0.160 af

**Subcatchment 44S: Subcat #44** Runoff Area=14,458 sf 76.68% Impervious Runoff Depth>5.95"  
Flow Length=98' Tc=7.7 min CN=91 Runoff=2.02 cfs 0.165 af

**Subcatchment 45S: Subcat #45** Runoff Area=16,893 sf 94.23% Impervious Runoff Depth>6.54"  
Flow Length=330' Tc=6.0 min CN=96 Runoff=2.59 cfs 0.211 af

**Subcatchment 46S: Subcat #46** Runoff Area=7,602 sf 100.00% Impervious Runoff Depth>6.78"  
Tc=6.0 min CN=98 Runoff=1.17 cfs 0.099 af

**Subcatchment 47S: Subcat #47** Runoff Area=3,200 sf 100.00% Impervious Runoff Depth>6.78"  
Tc=6.0 min CN=98 Runoff=0.49 cfs 0.041 af

**Subcatchment 50S: Subcat #50** Runoff Area=11,704 sf 0.00% Impervious Runoff Depth>0.37"  
Flow Length=182' Tc=11.7 min CN=33 Runoff=0.03 cfs 0.008 af

**Subcatchment 62S: Subcat. #62** Runoff Area=45,124 sf 0.15% Impervious Runoff Depth>3.01"  
Flow Length=165' Tc=14.3 min CN=64 Runoff=2.77 cfs 0.260 af

**Subcatchment 63S: Subcat. #63** Runoff Area=16,040 sf 0.00% Impervious Runoff Depth>2.42"  
Flow Length=150' Tc=10.8 min CN=58 Runoff=0.84 cfs 0.074 af

**Subcatchment 64S: Subcat #64** Runoff Area=7,675 sf 0.00% Impervious Runoff Depth>2.42"  
Tc=6.0 min CN=58 Runoff=0.47 cfs 0.036 af

<b>Subcatchment 70S: Subcat #70</b>	Runoff Area=51,128 sf 43.96% Impervious Runoff Depth>3.31" Flow Length=345' Tc=14.1 min CN=67 Runoff=3.50 cfs 0.324 af
<b>Subcatchment 71S: Subcat #71</b>	Runoff Area=100,796 sf 29.45% Impervious Runoff Depth>1.93" Flow Length=563' Tc=39.5 min CN=53 Runoff=2.39 cfs 0.372 af
<b>Subcatchment 72S: Subcat #72</b>	Runoff Area=68,928 sf 42.73% Impervious Runoff Depth>2.89" Flow Length=478' Tc=32.0 min CN=63 Runoff=2.91 cfs 0.382 af
<b>Reach 30aR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=151.0' S=0.0063 '/' Capacity=12.85 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30bR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.022 L=35.0' S=0.2286 '/' Capacity=77.47 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 30cR: Overland Flow</b>	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.035 L=58.0' S=0.0948 '/' Capacity=31.37 cfs Outflow=0.00 cfs 0.000 af
<b>Reach 71aR: Wooded Swale</b>	Avg. Flow Depth=0.63' Max Vel=1.46 fps Inflow=5.16 cfs 0.725 af n=0.035 L=78.5' S=0.0038 '/' Capacity=61.73 cfs Outflow=5.16 cfs 0.724 af
<b>Reach 72R: Roadside Swale</b>	Avg. Flow Depth=0.31' Max Vel=1.90 fps Inflow=2.86 cfs 0.354 af n=0.022 L=495.6' S=0.0060 '/' Capacity=33.12 cfs Outflow=2.82 cfs 0.353 af
<b>Reach 200R: Final Reach #200</b>	Inflow=8.45 cfs 0.852 af Outflow=8.45 cfs 0.852 af
<b>Reach 300R: Final Reach #300</b>	Inflow=6.43 cfs 1.461 af Outflow=6.43 cfs 1.461 af
<b>Reach 400R: Final Reach #400</b>	Inflow=6.44 cfs 0.991 af Outflow=6.44 cfs 0.991 af
<b>Pond 30P: Infiltration/Trench</b>	Peak Elev=183.91' Storage=2,238 cf Inflow=3.07 cfs 0.266 af Discarded=0.79 cfs 0.114 af Primary=0.64 cfs 0.150 af Secondary=0.00 cfs 0.000 af Outflow=1.43 cfs 0.264 af
<b>Pond 71P: Existing Catch Basin</b>	Peak Elev=189.23' Inflow=5.16 cfs 0.725 af 18.0" Round Culvert n=0.012 L=10.2' S=0.0098 '/' Outflow=5.16 cfs 0.725 af
<b>Pond 72P: Existing Depression</b>	Peak Elev=196.26' Storage=179 cf Inflow=2.91 cfs 0.382 af Discarded=0.02 cfs 0.026 af Primary=2.86 cfs 0.354 af Outflow=2.89 cfs 0.380 af
<b>Pond 201P: Bioretention W/ ISR #201</b>	Peak Elev=185.74' Storage=5,447 cf Inflow=3.50 cfs 0.324 af Primary=0.02 cfs 0.025 af Secondary=1.23 cfs 0.234 af Outflow=1.26 cfs 0.259 af
<b>Pond 202P: Bioretention W/ ISR #202</b>	Peak Elev=179.80' Storage=22,760 cf Inflow=10.32 cfs 0.936 af Primary=0.09 cfs 0.112 af Secondary=1.10 cfs 0.597 af Tertiary=0.00 cfs 0.000 af Outflow=1.19 cfs 0.709 af
<b>Pond 203P: Infiltration Pond #203</b>	Peak Elev=174.61' Storage=4,928 cf Inflow=1.46 cfs 0.783 af Discarded=0.21 cfs 0.241 af Primary=0.90 cfs 0.433 af Outflow=1.12 cfs 0.674 af
<b>Pond 204P: Detention Pond #204</b>	Peak Elev=179.30' Storage=6,074 cf Inflow=9.17 cfs 0.818 af Primary=5.24 cfs 0.806 af Secondary=0.00 cfs 0.000 af Outflow=5.24 cfs 0.806 af

**Pond C41P: Catch Basin #41** Peak Elev=184.43' Storage=5 cf Inflow=0.82 cfs 0.080 af  
15.0" Round Culvert n=0.012 L=54.7' S=0.0686 ' /' Outflow=0.82 cfs 0.080 af

**Pond C42P: Catch Basin #42** Peak Elev=181.27' Storage=14 cf Inflow=1.37 cfs 0.230 af  
15.0" Round Culvert n=0.012 L=63.0' S=0.0054 ' /' Outflow=1.38 cfs 0.230 af

**Pond C43P: Catch Basin #43** Peak Elev=181.22' Storage=9 cf Inflow=2.07 cfs 0.160 af  
15.0" Round Culvert n=0.012 L=60.0' S=0.0233 ' /' Outflow=2.07 cfs 0.160 af

**Pond C44P: Catch Basin #44** Peak Elev=180.78' Storage=22 cf Inflow=4.09 cfs 0.325 af  
15.0" Round Culvert n=0.012 L=48.0' S=0.0083 ' /' Outflow=4.06 cfs 0.325 af

**Pond C45P: Catch Basin #45** Peak Elev=182.21' Storage=10 cf Inflow=2.59 cfs 0.211 af  
15.0" Round Culvert n=0.012 L=87.2' S=0.0149 ' /' Outflow=2.59 cfs 0.211 af

**Pond C46P: Catch Basin #46** Peak Elev=181.04' Storage=13 cf Inflow=3.76 cfs 0.310 af  
15.0" Round Culvert n=0.012 L=68.0' S=0.0206 ' /' Outflow=3.76 cfs 0.310 af

**Pond C47P: Catch Basin #47** Peak Elev=180.38' Storage=24 cf Inflow=8.28 cfs 0.676 af  
18.0" Round Culvert n=0.012 L=40.0' S=0.0063 ' /' Outflow=8.28 cfs 0.676 af

**Pond C50P: Inlet Sump** Peak Elev=184.77' Storage=16 cf Inflow=5.19 cfs 0.733 af  
18.0" Round Culvert n=0.012 L=107.5' S=0.0051 ' /' Outflow=5.19 cfs 0.732 af

**Pond D51P: DMH #51** Peak Elev=182.16' Storage=0.000 af Inflow=1.26 cfs 0.259 af  
18.0" Round Culvert n=0.012 L=68.0' S=0.0051 ' /' Outflow=1.25 cfs 0.259 af

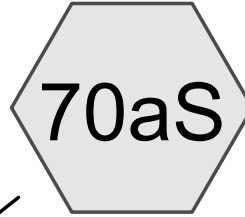
**Pond D52P: DMH #52** Peak Elev=181.18' Storage=19 cf Inflow=8.70 cfs 0.783 af  
24.0" Round Culvert n=0.012 L=110.0' S=0.0055 ' /' Outflow=8.70 cfs 0.783 af

**Pond D53P: DMH #53** Peak Elev=180.33' Storage=17 cf Inflow=8.70 cfs 0.783 af  
24.0" Round Culvert n=0.012 L=120.0' S=0.0083 ' /' Outflow=8.70 cfs 0.783 af

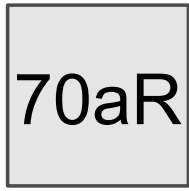
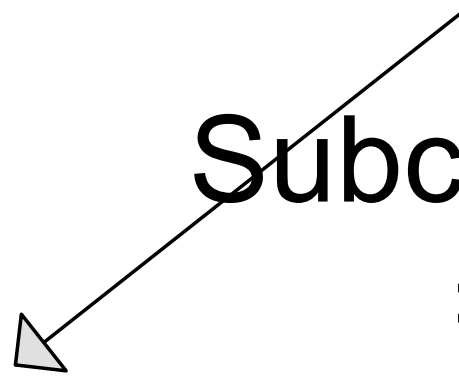
**Pond E01P: Existing Catch Basin** Peak Elev=184.54' Storage=13 cf Inflow=2.52 cfs 0.186 af  
15.0" Round Culvert n=0.012 L=57.0' S=0.0049 ' /' Outflow=2.51 cfs 0.186 af

**Pond E02P: Existing Catch Basin** Peak Elev=184.25' Storage=262 cf Inflow=7.62 cfs 0.568 af  
Discarded=0.02 cfs 0.015 af Primary=7.63 cfs 0.553 af Outflow=7.64 cfs 0.568 af

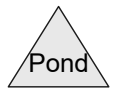
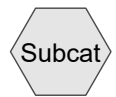
**Total Runoff Area = 16.948 ac Runoff Volume = 4.119 af Average Runoff Depth = 2.92"**  
**76.01% Pervious = 12.882 ac 23.99% Impervious = 4.066 ac**



Subcatchment #70a



Reach #70a





## 23-017 Pro Analysis Ex TCAM Swale

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 2

### Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.112	39	>75% Grass cover, Good, HSG A (70aS)
0.064	98	Paved parking, HSG A (70aS)
0.043	30	Woods, Good, HSG A (70aS)
<b>0.219</b>	<b>55</b>	<b>TOTAL AREA</b>

## 23-017 Pro Analysis Ex TCAM Swale

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 3

### Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.219	HSG A	70aS
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
<b>0.219</b>		<b>TOTAL AREA</b>

## 23-017 Pro Analysis Ex TCAM Swale

Prepared by Berry Surveying & Engineering

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Printed 4/17/2024

Page 4

### Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.112	0.000	0.000	0.000	0.000	0.112	>75% Grass cover, Good	70aS
0.064	0.000	0.000	0.000	0.000	0.064	Paved parking	70aS
0.043	0.000	0.000	0.000	0.000	0.043	Woods, Good	70aS
<b>0.219</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.219</b>	<b>TOTAL AREA</b>	

**23-017 Pro Analysis Ex TCAM Swale**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Page 5

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

**Subcatchment 70aS: Subcatchment #70a** Runoff Area=9,558 sf 29.27% Impervious Runoff Depth>2.13"  
Tc=6.0 min CN=55 Runoff=0.51 cfs 0.039 af

**Reach 70aR: Reach #70a** Avg. Flow Depth=0.11' Max Vel=1.94 fps Inflow=0.51 cfs 0.039 af  
n=0.022 L=13.0' S=0.0192 '/ Capacity=178.16 cfs Outflow=0.51 cfs 0.039 af

**Total Runoff Area = 0.219 ac Runoff Volume = 0.039 af Average Runoff Depth = 2.13"**  
**70.73% Pervious = 0.155 ac 29.27% Impervious = 0.064 ac**

**Summary for Subcatchment 70aS: Subcatchment #70a**

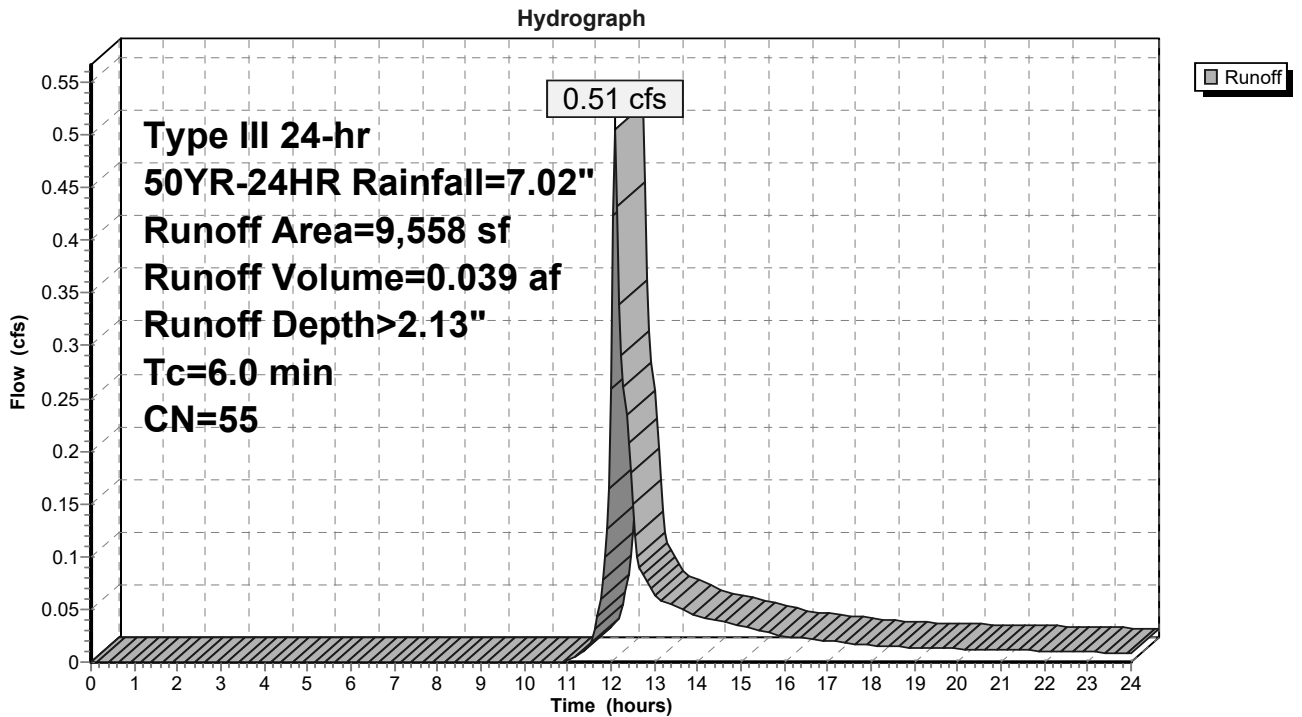
Runoff = 0.51 cfs @ 12.10 hrs, Volume= 0.039 af, Depth> 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 50YR-24HR Rainfall=7.02"

Area (sf)	CN	Description
4,896	39	>75% Grass cover, Good, HSG A
2,798	98	Paved parking, HSG A
1,864	30	Woods, Good, HSG A
9,558	55	Weighted Average
6,760		70.73% Pervious Area
2,798		29.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Entry

**Subcatchment 70aS: Subcatchment #70a**



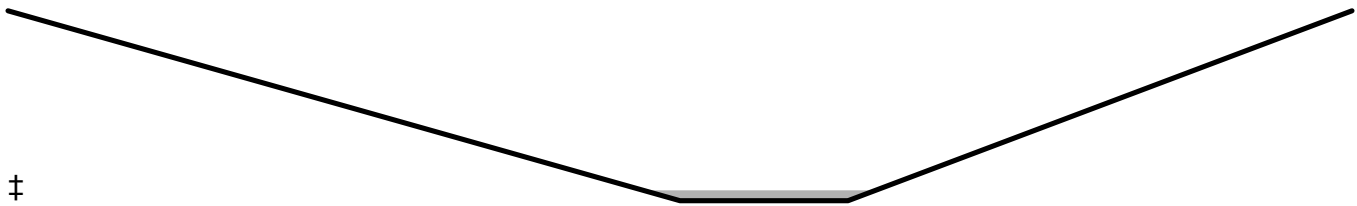
### Summary for Reach 70aR: Reach #70a

Inflow Area = 0.219 ac, 29.27% Impervious, Inflow Depth > 2.13" for 50YR-24HR event  
 Inflow = 0.51 cfs @ 12.10 hrs, Volume= 0.039 af  
 Outflow = 0.51 cfs @ 12.10 hrs, Volume= 0.039 af, Atten= 0%, Lag= 0.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Max. Velocity= 1.94 fps, Min. Travel Time= 0.1 min  
 Avg. Velocity = 0.76 fps, Avg. Travel Time= 0.3 min

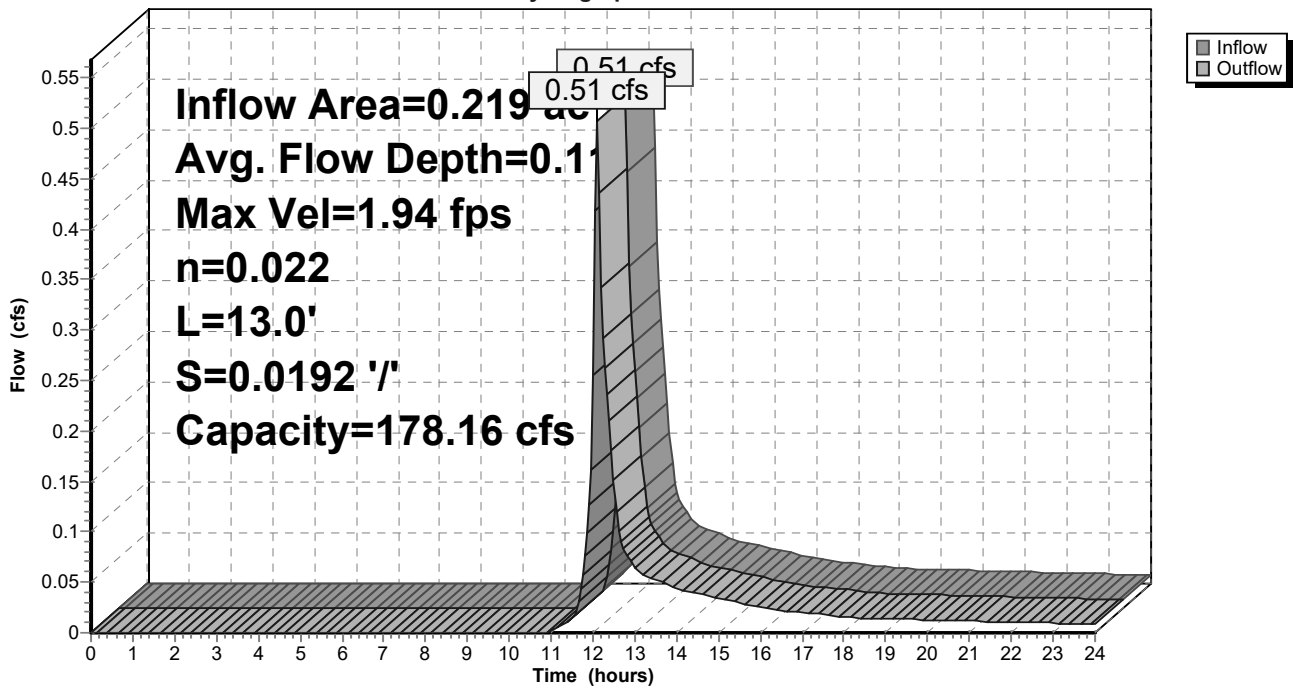
Peak Storage= 3 cf @ 12.10 hrs  
 Average Depth at Peak Storage= 0.11'  
 Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 178.16 cfs

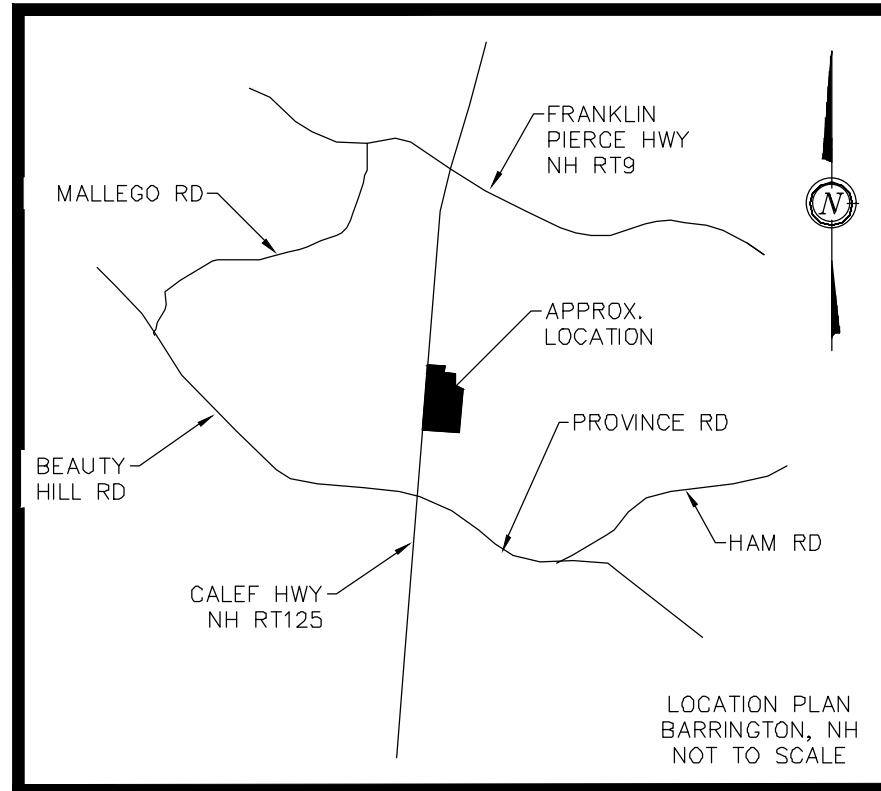
2.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight  
 Side Slope Z-value= 4.0 3.0 '/' Top Width= 16.00'  
 Length= 13.0' Slope= 0.0192 '/'  
 Inlet Invert= 185.50', Outlet Invert= 185.25'



### Reach 70aR: Reach #70a

Hydrograph





- NOTES:**
- 1.) OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 1A.) APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 2.) TAX MAP 238, LOT 44
  - 3.) LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
  - 4.) S.C.R.D. BOOK 2948, PAGE 332
  - 5.) VERTICAL DATUM BASED ON NAVD88 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
  - 6.) THE INTENT OF THIS PLAN IS TO SHOW THE POST CONSTRUCTION DRAINAGE CONDITIONS OF TAX MAP 238, LOT 44 AS OF THE DATE OF SURVEY.
  - 7.) SEE PROPOSED GRADING & DRAINAGE PLAN FOR SLOPES & EROSION & SEDIMENT CONTROL PLAN FOR STOCKPILE AREA AND CONSTRUCTION ENTRANCE.
  - 8.) HYDROCAD USES A SERIES OF NODE SUFFIXES FOR NUMBERING PURPOSES (S=SUBCATCHMENT, P=POND DEVICE, R=REACH), TO SIMPLIFY ANNOTATION THESE SUFFIXES ARE LEFT OFF THE WATERSHED PLANS AND NODE TYPE IS DENOTED BY THE SYMBOL SHAPE ACCORDING TO THE DISPLAYED LEGEND WHICH COINCIDES WITH HYDROCAD GRAPHICS.

SEE CORRESPONDING SITE PLAN FOR TAX MAP 238, LOT 44-1

FINAL REACH 400 POINT OF ANALYSIS POINT DISCHARGE SEE PLAN SET DB2023-017, ATTACHED C - RECEIVING WATERS - MALLEGRO BROOK

EASTERN WETLAND EDGE, SOUTH OF EXISTING DRIVEWAY LIMIT OF ANALYSIS (TYP.)

EDGE OF POORLY DRAINED JURISDICTIONAL WETLANDS (TYP.) BY: DEIDRA BENJAMIN, CWS #295

FINAL REACH 200 POINT OF ANALYSIS NON-POINT DISCHARGE SEE PLAN SET DB2023-017, ATTACHED C - RECEIVING WATERS - MALLEGRO BROOK

EXISTING WETLANDS FROM EASTERLY UPLAND AREA NON-INCLUSIVE OF ALL FLOW TO POINT RECEIVING WATERS MALLEGRO BROOK (NHRIV600030903-02)

CONSERVATION AREA EASEMENT LINE (TYP.)

PROP. LEACHFIELD FOR LOT 44-1

PROP. GABION BASKET, AMPHITHEATER STYLE SEATING

PROP. GAZEBO

EASTERLY UPLAND AREA DRAINING TO WETLAND ANALYSIS POINT

PROP. WELLS

PROP. WELLS

PROP. WELLS

PROP. WELLS

PROP. WELLS

PROP. WELLS

PROP. WELLS

PROP. WELLS

PROP. WELLS

PROP. WELLS

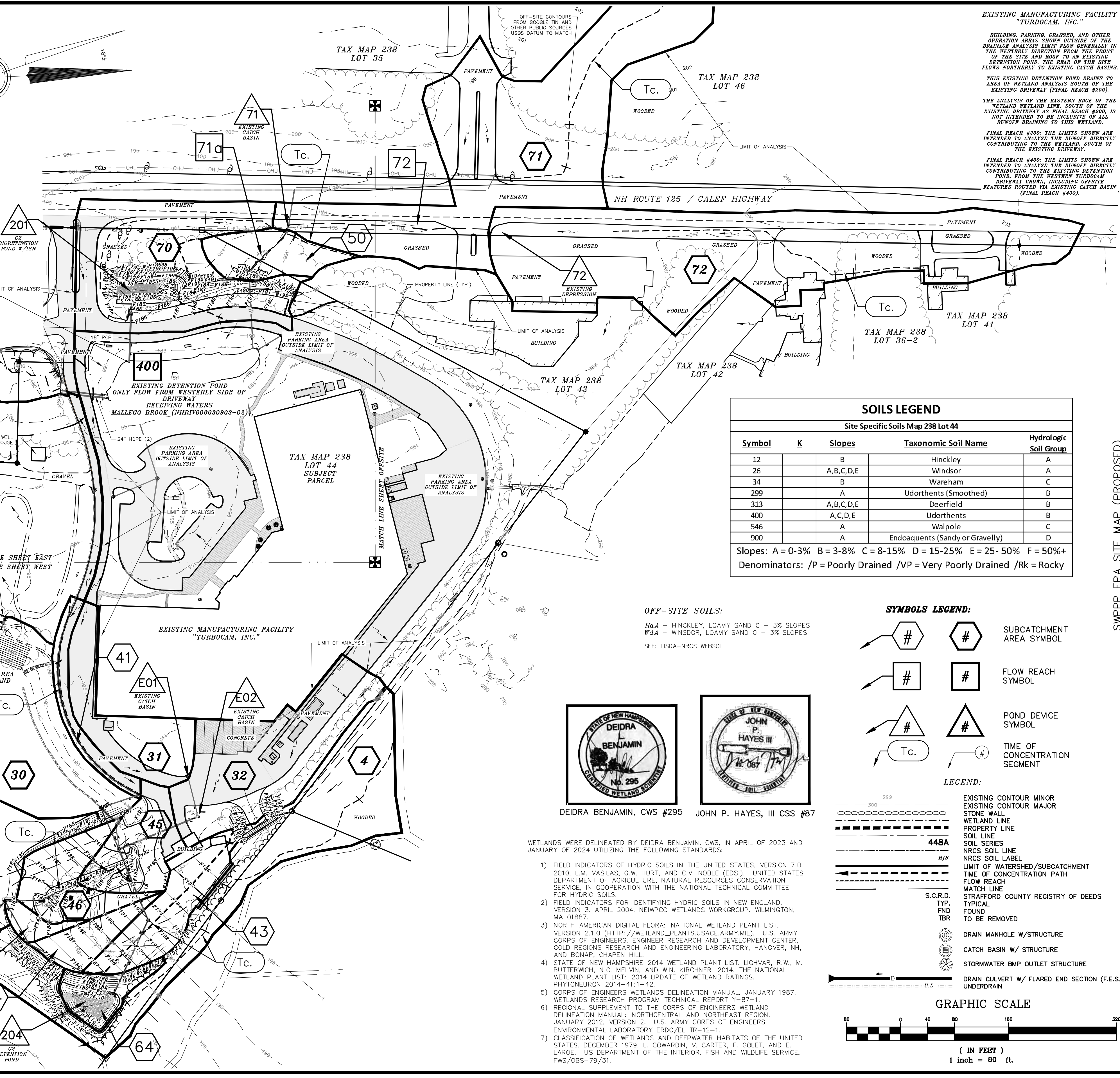
SOUTHEAST PROPERTY LINE RECEIVING WATERS MALLEGRO BROOK (NHRIV600030903-02)

PROP. WELLS

PROP. WELLS

PROP. WELLS

PROP. WELLS



EXISTING MANUFACTURING FACILITY "TURBOCAM, INC."

BUILDING, PARKING, GRASSED, AND OTHER OPERATION AREAS SHOWN OUTSIDE OF THE DRAINAGE ANALYSIS LIMIT. FLOW GENERALLY IN THE WESTERLY DIRECTION FROM THE FRONT OF THE SITE AND NORTH TO AN EXISTING DETENTION POND. THE REAR OF THE SITE FLOWS NORTHERLY TO EXISTING CATCH BASINS.

THIS EXISTING DETENTION POND DRAINS TO AREA OF WETLAND ANALYSIS SOUTH OF THE EXISTING DRIVEWAY (FINAL REACH #200).

THE ANALYSIS OF THE EASTERN EDGE OF THE WETLAND WETLAND LINE, SOUTH OF THE EXISTING DRIVEWAY AS FINAL REACH #400, IS NOT INTENDED TO BE INCLUSIVE OF ALL RUNOFF DRAINING TO THIS WETLAND.

FINAL REACH #200: THE LIMITS SHOWN ARE INTENDED TO ANALYZE THE RUNOFF DIRECTLY CONTRIBUTING TO THE WETLAND, SOUTH OF THE EXISTING DRIVEWAY.

FINAL REACH #400: THE LIMITS SHOWN ARE INTENDED TO ANALYZE THE RUNOFF DIRECTLY CONTRIBUTING TO THE EXISTING DETENTION POND, FROM THE WESTERN TURBOCAM DRIVEWAY CROWN, INCLUDING OFFSITE FEATURES ROUTED VIA EXISTING CATCH BASIN (FINAL REACH #400).

**SOILS LEGEND**

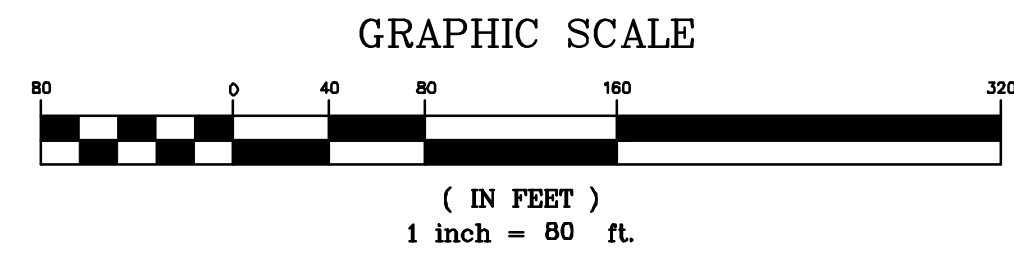
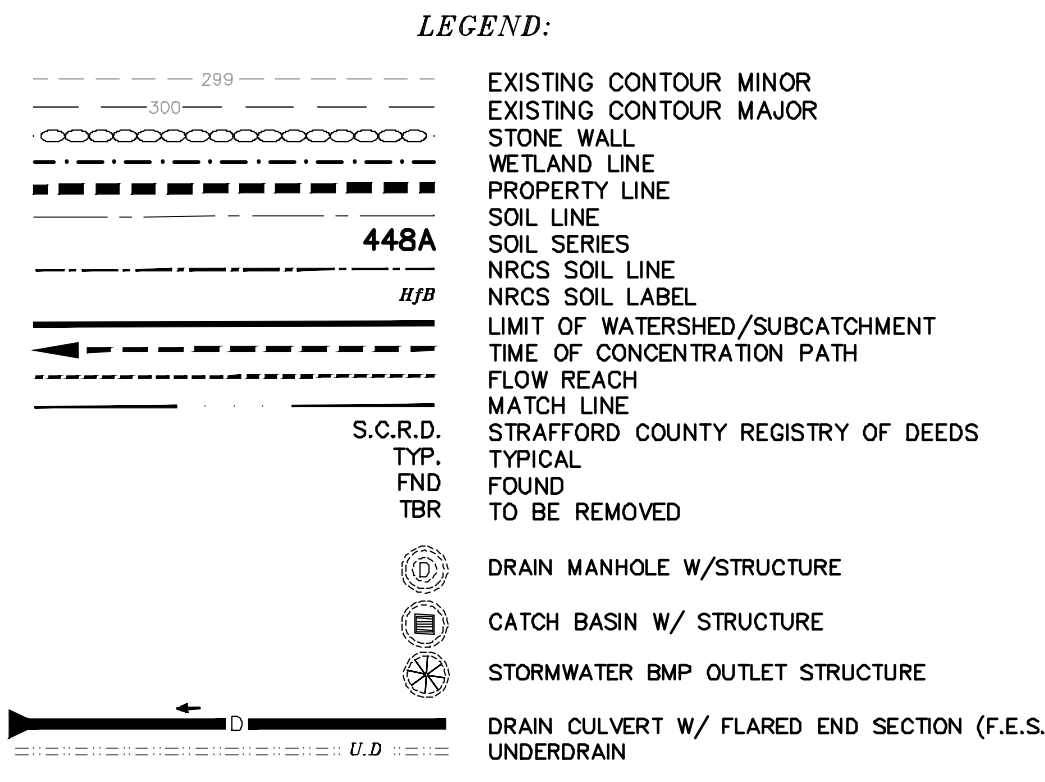
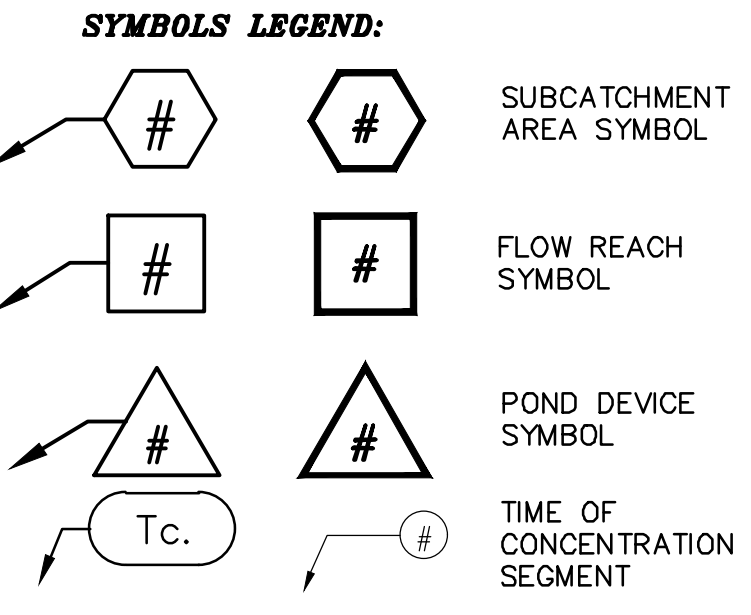
Site Specific Soils Map 238 Lot 44

Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoquents (Sandy or Gravelly)	D

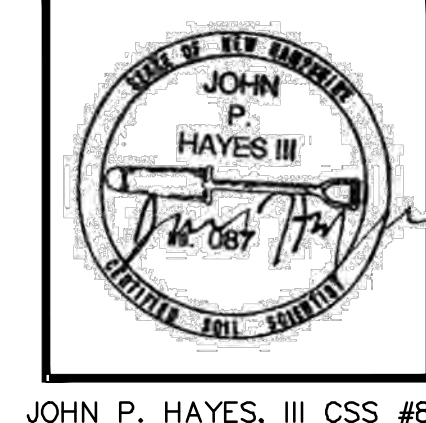
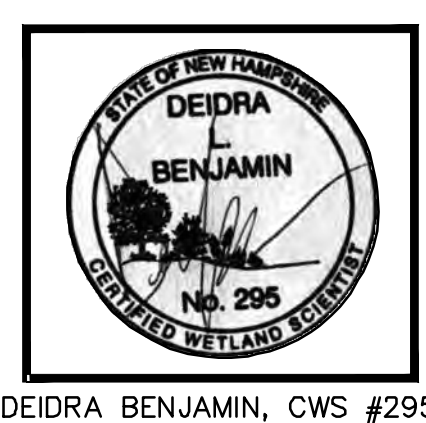
Slopes: A = 0-3% B = 3-8% C = 8-15% D = 15-25% E = 25-50% F = 50%+

Denominators: /P = Poorly Drained /VP = Very Poorly Drained /Rk = Rocky

OFF-SITE SOILS:  
HqA - HINCKLEY, LOAMY SAND 0 - 3% SLOPES  
WdA - WINDSOR, LOAMY SAND 0 - 3% SLOPES  
SEE: USDA-NRCS WEBSOIL



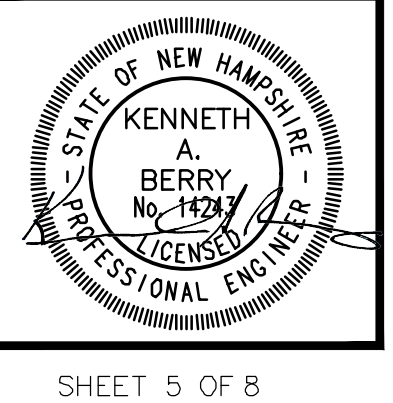
- WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS, IN APRIL OF 2023 AND JANUARY OF 2024 UTILIZING THE FOLLOWING STANDARDS:
- 1) FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0, 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.). UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL COMMITTEE FOR HYDRIC SOILS.
  - 2) FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, VERSION 3, APRIL 2004. NEWPPCC WETLANDS WORKGROUP, WILMINGTON, MA 01887.
  - 3) NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, VERSION 2.1.0 (HTTP://WETLAND\_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD REGIONS RESEARCH AND ENGINEERING LABORATORY, HANOVER, NH, AND BONAP, CHAPEN HILL.
  - 4) STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST. LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER. 2014. THE NATIONAL WETLAND PLANT LIST: 2014 UPDATE OF WETLAND RATINGS. PHYTONEURON 2014-41:1-42.
  - 5) CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH PROGRAM TECHNICAL REPORT Y-87-1.
  - 6) REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTH-CENTRAL AND NORTHEAST REGION, JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS. ENVIRONMENTAL LABORATORY ERDC/EL TR-12-1.
  - 7) CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES, DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. U.S. DEPARTMENT OF THE INTERIOR, FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.



SWPPP EPA SITE MAP (PROPOSED)  
OVERVIEW W-2 PROPOSED WATERSHED PLAN

FOR TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

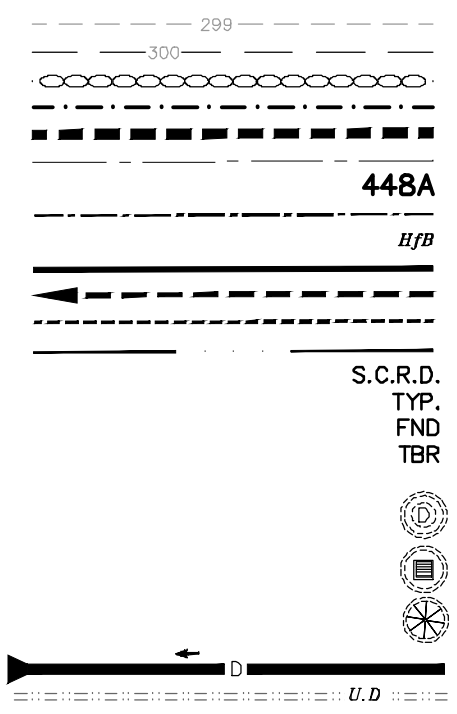
BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 80 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017



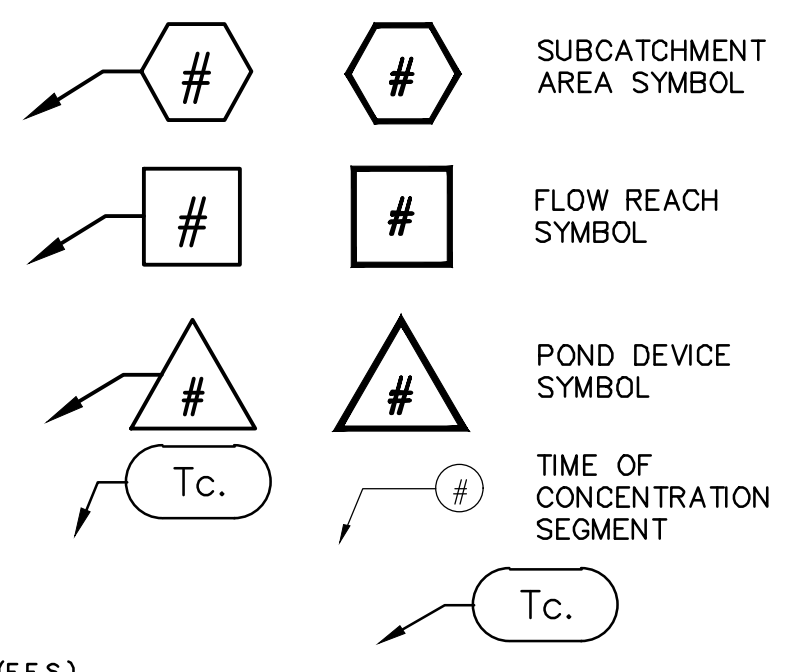
**NOTES:**

- OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- TAX MAP 238, LOT 44
- LOT AREA: 1,217,250 Sq.Ft., 27.94 AC.
- S.C.R.D. BOOK 2948, PAGE 332
- VERTICAL DATUM BASED ON NAVD83 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
- THE INTENT OF THIS PLAN IS TO SHOW THE POST CONSTRUCTION DRAINAGE CONDITIONS OF TAX MAP 238, LOT 44 AS OF THE DATE OF SURVEY.
- SEE PROPOSED GRADING & DRAINAGE PLAN FOR SLOPES & EROSION & SEDIMENT CONTROL PLAN FOR STOCKPILE AREA AND CONSTRUCTION ENTRANCE.
- HYDROCAD USES A SERIES OF NODE SUFFIXES FOR NUMBERING PURPOSES (S=SUBCATCHMENT, P=POND DEVICE, R=REACH), TO SIMPLY ANNOTATION THESE SUFFIXES ARE LEFT OFF THE WATERSHED PLANS AND NODE TYPE IS DENOTED BY THE SYMBOL SHAPE ACCORDING TO THE DISPLAYED LEGEND WHICH COINCIDES WITH HYDROCAD GRAPHICS.

**LEGEND:**



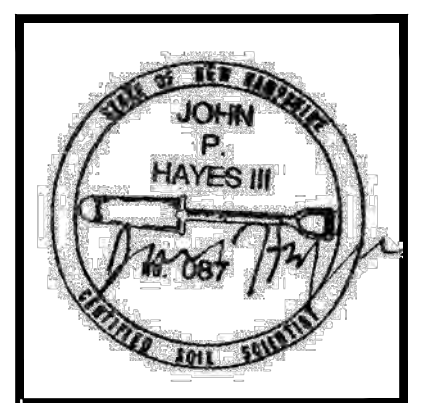
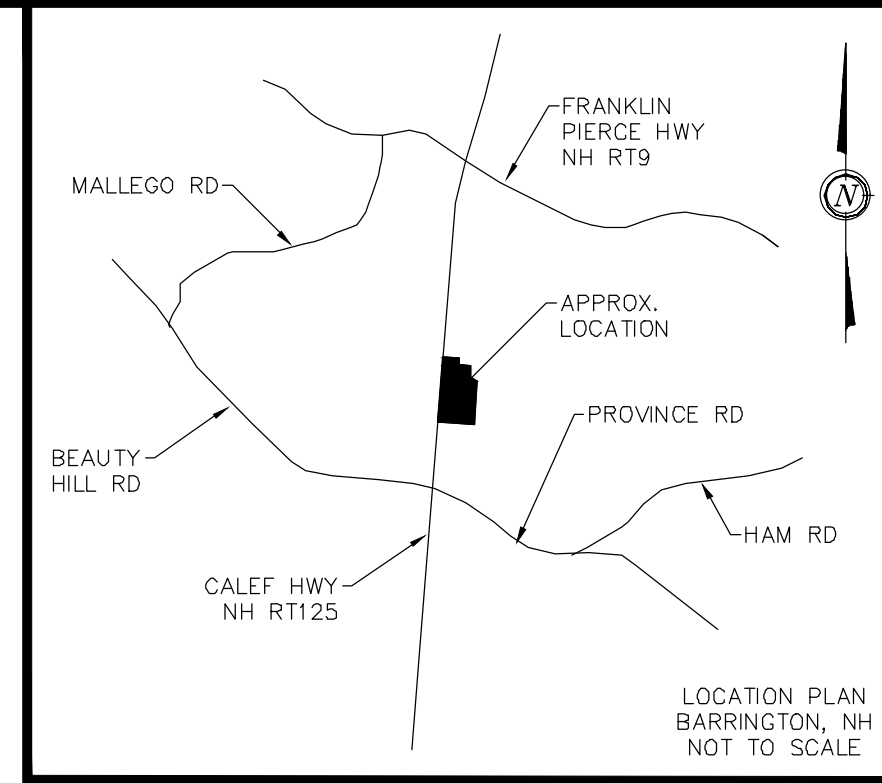
**SYMBOLS LEGEND:**



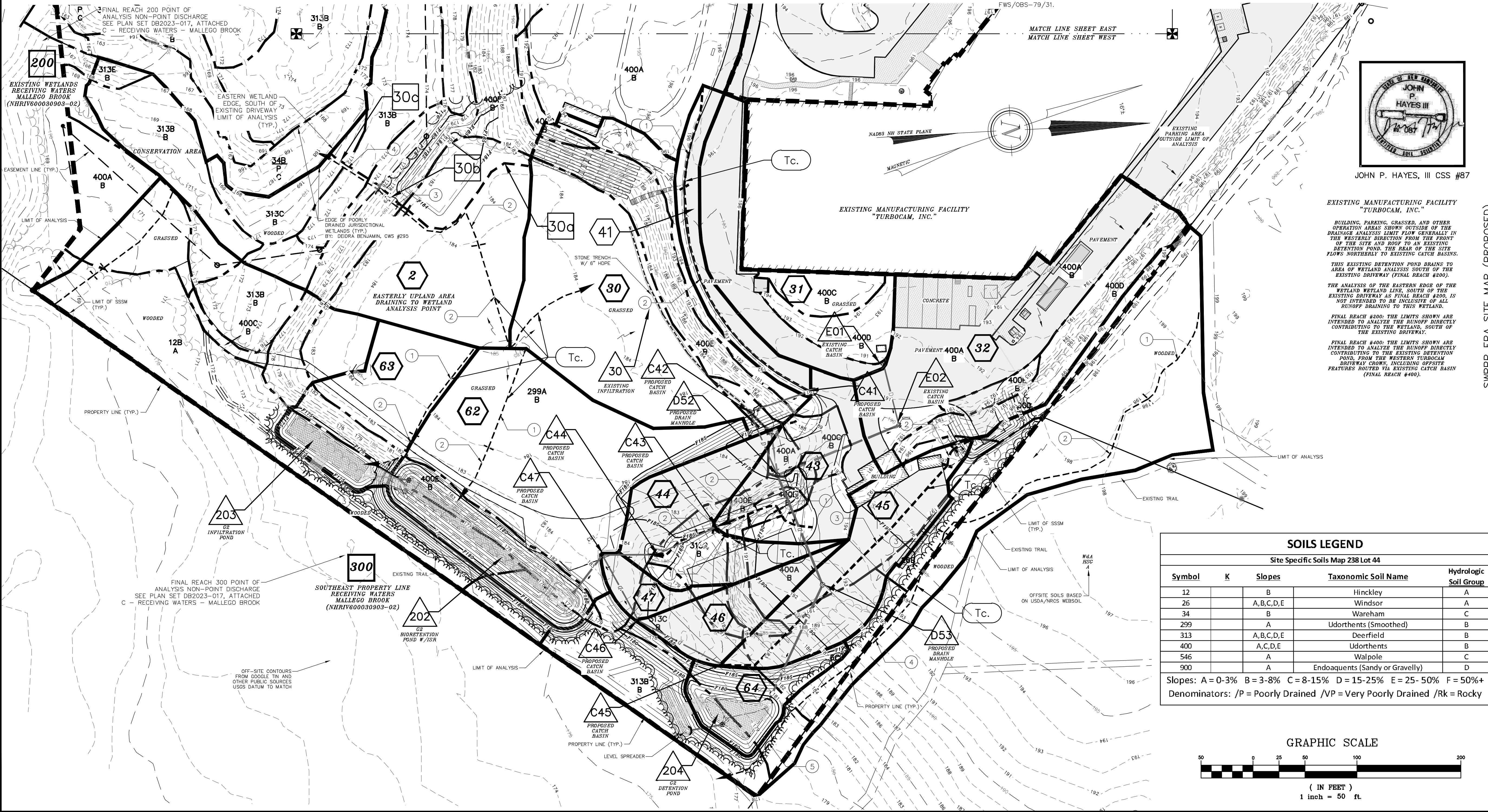
DEIDRA BENJAMIN, CWS #295

WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS, IN APRIL OF 2023 AND JANUARY OF 2024 UTILIZING THE FOLLOWING STANDARDS:

- FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0. 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.). UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL COMMITTEE FOR HYDRIC SOILS.
- FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, VERSION 3. APRIL 2004. NEWPPCC WETLANDS WORKGROUP. WILMINGTON, MA 01887.
- NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, VERSION 2.1.0 (HTTP://WETLAND\_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD REGIONS RESEARCH AND ENGINEERING LABORATORY, HANOVER, NH, AND BONAP, CHAPEL HILL.
- STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST. LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER. 2014. THE NATIONAL WETLAND PLANT LIST: 2014 UPDATE OF WETLAND RATINGS. PHYTONEURON 2014-41:1-42.
- CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH PROGRAM TECHNICAL REPORT Y-87-1.
- REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTHEASTAL AND NORTHEAST REGION, JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS, ENVIRONMENTAL LABORATORY ERDC/EL TR-12-1.
- CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES, DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. U.S. DEPARTMENT OF THE INTERIOR, FISH AND WILDLIFE SERVICE, FWS/OBS-79/31.



JOHN P. HAYES, III CSS #87



**EXISTING MANUFACTURING FACILITY "TURBOCAM, INC."**

BUILDING, PARKING, GRASSED, AND OTHER OPERATION AREAS SHOWN OUTSIDE OF THE DRAINAGE ANALYSIS LIMIT FLOW GENERALLY IN THE WESTERLY DIRECTION FROM THE FRONT OF THE SITE AND ROOF TO AN EXISTING DETENTION POND. THE REAR OF THE SITE FLOWS NORTHERLY TO EXISTING CATCH BASINS.

THIS EXISTING DETENTION POND DRAINS TO AREA OF WETLAND ANALYSIS SOUTH OF THE EXISTING DRIVEWAY (FINAL REACH #200).

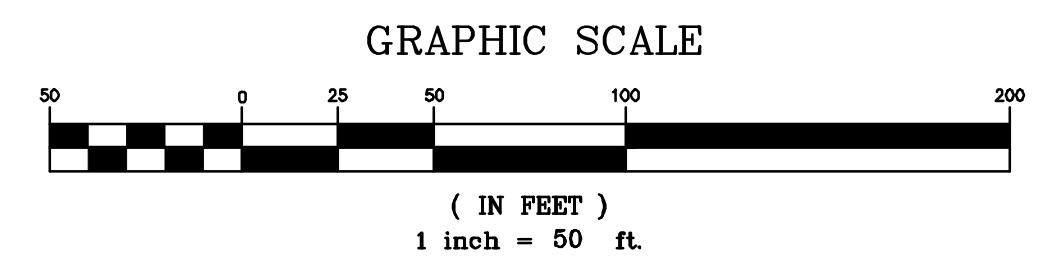
THE ANALYSIS OF THE EASTERN EDGE OF THE WETLAND WETLAND LINE, SOUTH OF THE EXISTING DRIVEWAY AS FINAL REACH #200, IS NOT INTENDED TO BE INCLUSIVE OF ALL RUNOFF DRAINING TO THIS WETLAND.

FINAL REACH #200: THE LIMITS SHOWN ARE INTENDED TO ANALYZE THE RUNOFF DIRECTLY CONTRIBUTING TO THE WETLAND, SOUTH OF THE EXISTING DRIVEWAY.

FINAL REACH #400: THE LIMITS SHOWN ARE INTENDED TO ANALYZE THE RUNOFF DIRECTLY CONTRIBUTING TO THE EXISTING DETENTION POND, FROM THE WESTERN TURBOCAM DRIVEWAY CROWN, INCLUDING OFFSITE FEATURES ROUTED VIA EXISTING CATCH BASIN (FINAL REACH #400).

SOILS LEGEND				
Site Specific Soils Map 238 Lot 44				
Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoqaquents (Sandy or Gravelly)	D

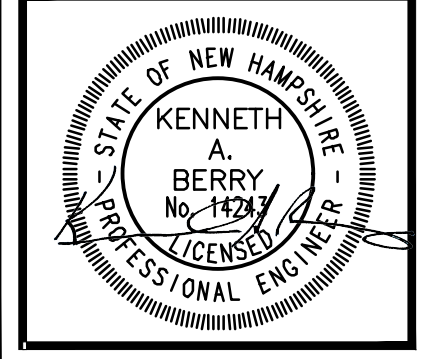
Slopes: A=0-3% B=3-8% C=8-15% D=15-25% E=25-50% F=50%+  
Denominators: /P= Poorly Drained /VP= Very Poorly Drained /Rk= Rocky



SMPPP EPA SITE MAP (PROPOSED) DETAIL W-2 PROPOSED WATERSHED PLAN WEST

FOR TURBOCAM, INC. LAND OF VIRTUOUS REALTY, LLC NH ROUTE 125/CALEF HIGHWAY BARRINGTON, N.H. TAX MAP 238, LOT 44

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 50 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017

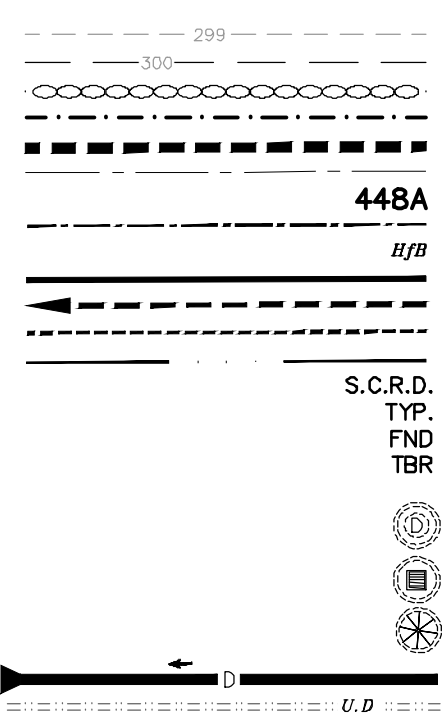




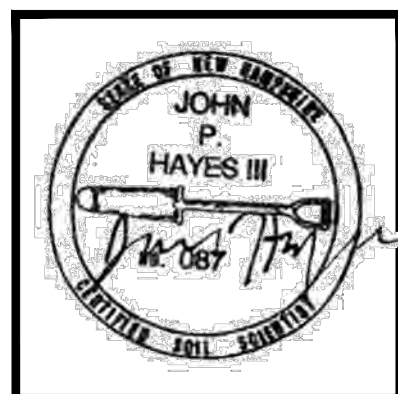
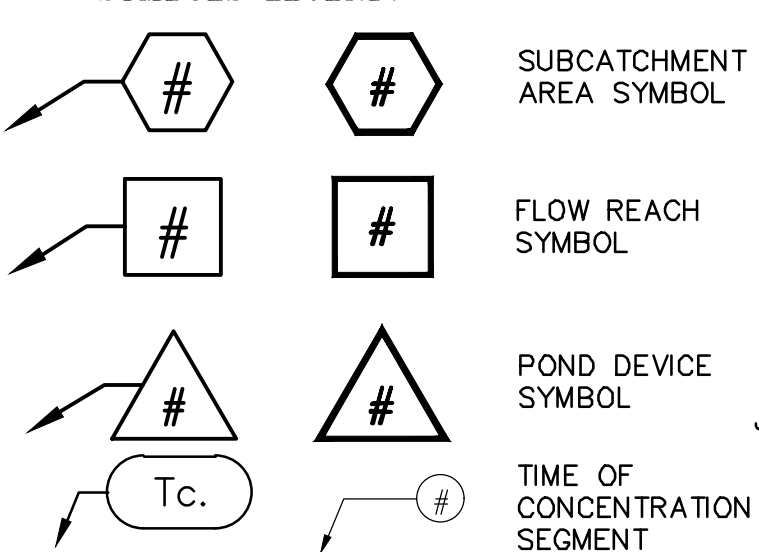
**NOTES:**

- OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- TAX MAP 238, LOT 44
- LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
- S.C.R.D. BOOK 2948, PAGE 332
- VERTICAL DATUM BASED ON NAVD88 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
- THE INTENT OF THIS PLAN IS TO SHOW THE POST CONSTRUCTION DRAINAGE CONDITIONS OF TAX MAP 238, LOT 44 AS OF THE DATE OF SURVEY.
- SEE PROPOSED GRADING & DRAINAGE PLAN FOR SLOPES & EROSION & SEDIMENT CONTROL PLAN FOR STOCKPILE AREA AND CONSTRUCTION ENTRANCE.
- HYDROCAD USES A SERIES OF NODE SUFFIXES FOR NUMBERING PURPOSES (S=SUBCATCHMENT, P=POND DEVICE, R=REACH), TO SIMPLIFY ANNOTATION THESE SUFFIXES ARE LEFT OFF THE WATERSHED PLANS AND NODE TYPE IS DENOTED BY THE SYMBOL SHAPE ACCORDING TO THE DISPLAYED LEGEND WHICH COINCIDES WITH HYDROCAD GRAPHICS.

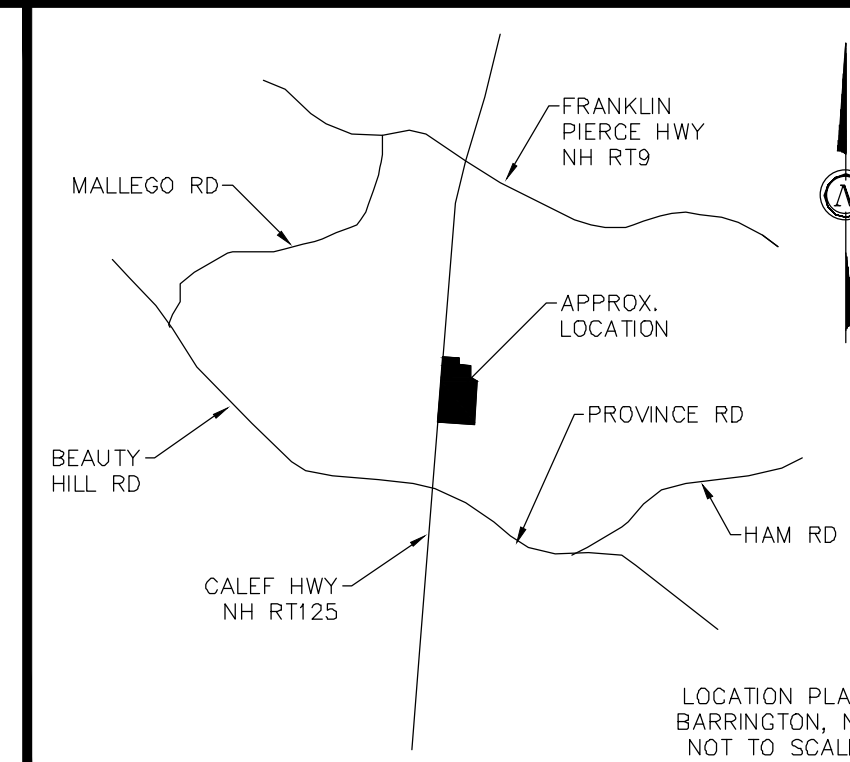
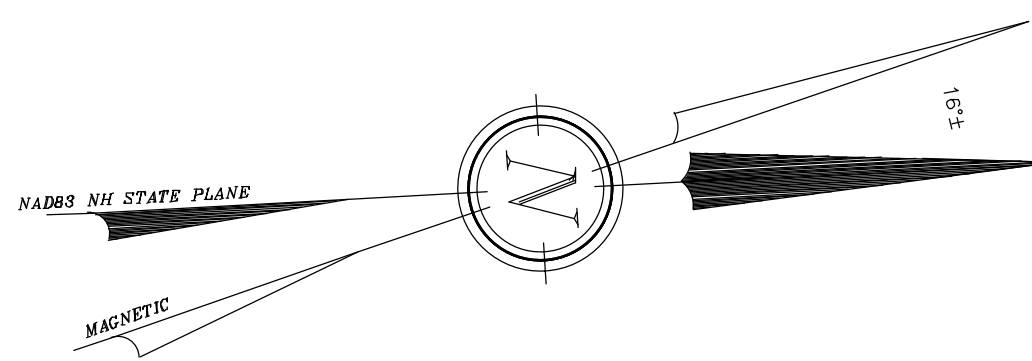
**LEGEND:**



**SYMBOLS LEGEND:**



JOHN P. HAYES, III CSS #87

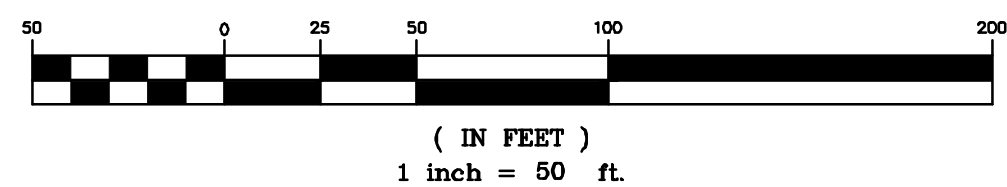


**OFF-SITE SOILS:**  
 H<sub>1</sub>A - HINCKLEY, LOAMY SAND 0 - 3% SLOPES  
 W<sub>1</sub>A - WINDSOR, LOAMY SAND 0 - 3% SLOPES  
 SEE: USDA-NRCS WEBSOIL

SOILS LEGEND				
Site Specific Soils Map 238 Lot 44				
Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoaquents (Sandy or Gravelly)	D

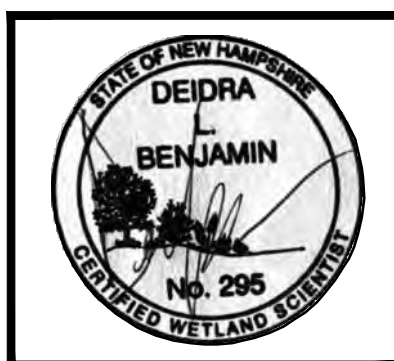
Slopes: A = 0-3% B = 3-8% C = 8-15% D = 15-25% E = 25-50% F = 50%+  
 Denominators: /P = Poorly Drained /VP = Very Poorly Drained /Rk = Rocky

**GRAPHIC SCALE**



WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS, IN APRIL OF 2023 AND JANUARY OF 2024 UTILIZING THE FOLLOWING STANDARDS:

- FIELD INDICATORS FOR HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0, 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.). UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL COMMITTEE FOR HYDRIC SOILS.
- FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, VERSION 3, APRIL 2004. NEWPPC WETLANDS WORKGROUP, WILMINGTON, MA 01887.
- NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, VERSION 2.1.0 (HTTP://WETLAND\_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD REGIONS RESEARCH AND ENGINEERING LABORATORY, HANOVER, NH, AND BONAP, CHAPEN HILL.
- STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST. LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER. 2014. THE NATIONAL WETLAND PLANT LIST: 2014 UPDATE OF WETLAND RATINGS. PHYTONEURON 2014-41:1-42.
- CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH PROGRAM TECHNICAL REPORT Y-87-1.
- REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTH-CENTRAL AND NORTHEAST REGION, JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS, ENVIRONMENTAL LABORATORY ERDC/EL TR-12-1.
- CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES, DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. US DEPARTMENT OF THE INTERIOR, FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.



DEIDRA BENJAMIN, CWS #295

SEE CORRESPONDING SITE PLAN FOR TAX MAP 238, LOT 44-1

FINAL REACH 400 POINT OF ANALYSIS POINT DISCHARGE  
 SEE PLAN SET DB2023-017, ATTACHED  
 C - RECEIVING WATERS - MALLEGO BROOK

FINAL REACH 200 POINT OF ANALYSIS NON-POINT DISCHARGE  
 SEE PLAN SET DB2023-017, ATTACHED  
 C - RECEIVING WATERS - MALLEGO BROOK

EXISTING WETLANDS FROM EASTERLY UPLAND AREA NON-INCLUSIVE OF ALL FLOW TO POINT RECEIVING WATERS MALLEGO BROOK (NHRIV60030903-02)

EASTERN WETLAND EDGE, SOUTH OF EXISTING DRIVEWAY LIMIT OF ANALYSIS (TYP.)

EDGE OF POORLY DRAINED JURISDICTIONAL WETLANDS (TYP.) BY: DEIDRA BENJAMIN, CWS #295



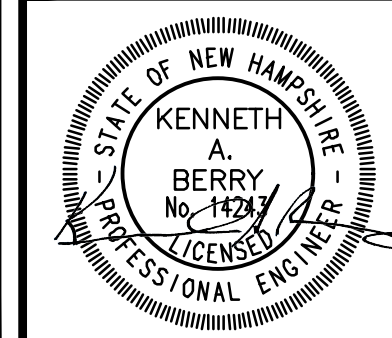
SWPPP EPA SITE MAP (PROPOSED)

DETAIL W-2 PROPOSED WATERSHED PLAN WEST

REVISION	DATE	DESCRIPTION

FOR TURBOCAM, INC.  
 LAND OF VIRTUOUS REALTY, LLC  
 NH ROUTE 125/CALEF HIGHWAY  
 BARRINGTON, N.H.  
 TAX MAP 238, LOT 44

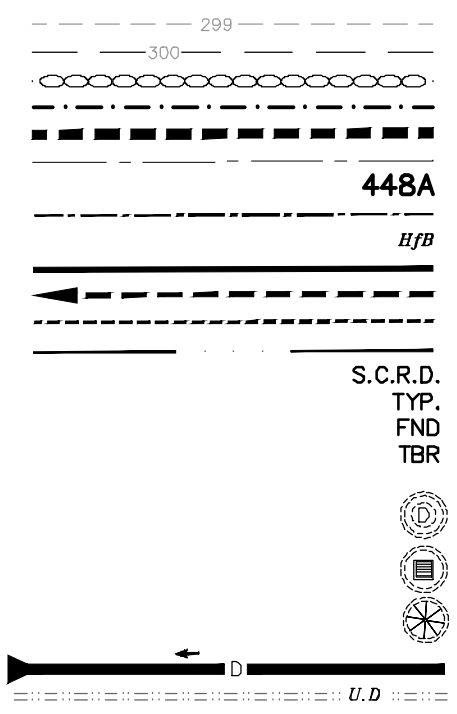
BERRY SURVEYING & ENGINEERING  
 335 SECOND CROWN POINT ROAD  
 BARRINGTON, NH 03825 (603)332-2863  
 SCALE : 1 IN. EQUALS 50 FT.  
 DATE : APRIL 17, 2024  
 FILE NO. : DB 2023 - 017



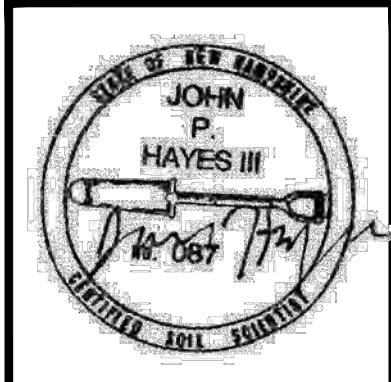
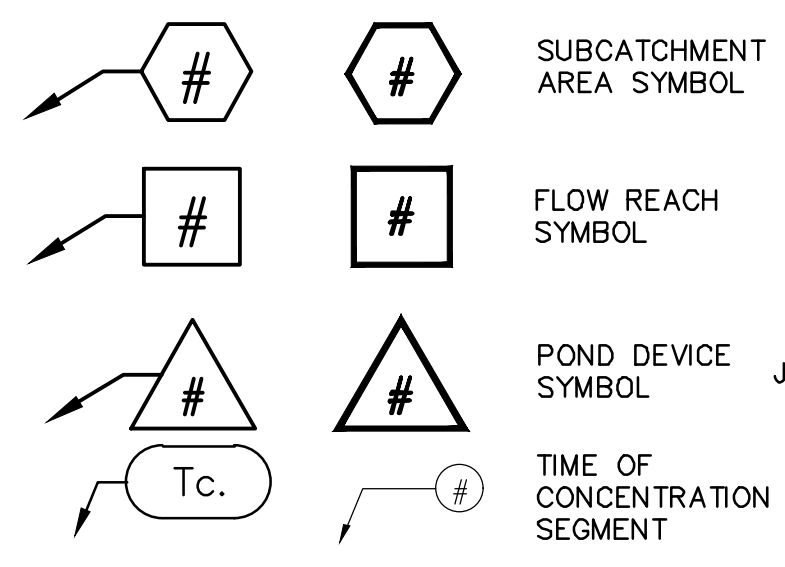
**NOTES:**

- OWNER: VIRTUOUS REALTY, LLC  
807 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- APPLICANT: TURBOCAM, INC.  
807 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- TAX MAP 238, LOT 44
- LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
- S.C.R.D. BOOK 2948, PAGE 332
- VERTICAL DATUM BASED ON NAVD88 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
- THE INTENT OF THIS PLAN IS TO SHOW THE POST CONSTRUCTION DRAINAGE CONDITIONS OF TAX MAP 238, LOT 44 AS OF THE DATE OF SURVEY.
- SEE PROPOSED GRADING & DRAINAGE PLAN FOR SLOPES & EROSION & SEDIMENT CONTROL PLAN FOR STOCKPILE AREA AND CONSTRUCTION ENTRANCE.
- HYDROCAD USES A SERIES OF NODE SUFFIXES FOR NUMBERING PURPOSES (S=SUBCATCHMENT, P=POND DEVICE, R=REACH). TO SIMPLIFY ANNOTATION THESE SUFFIXES ARE LEFT OFF THE WATERSHED PLANS AND NODE TYPE IS DENOTED BY THE SYMBOL SHAPE ACCORDING TO THE DISPLAYED LEGEND WHICH COINCIDES WITH HYDROCAD GRAPHICS.

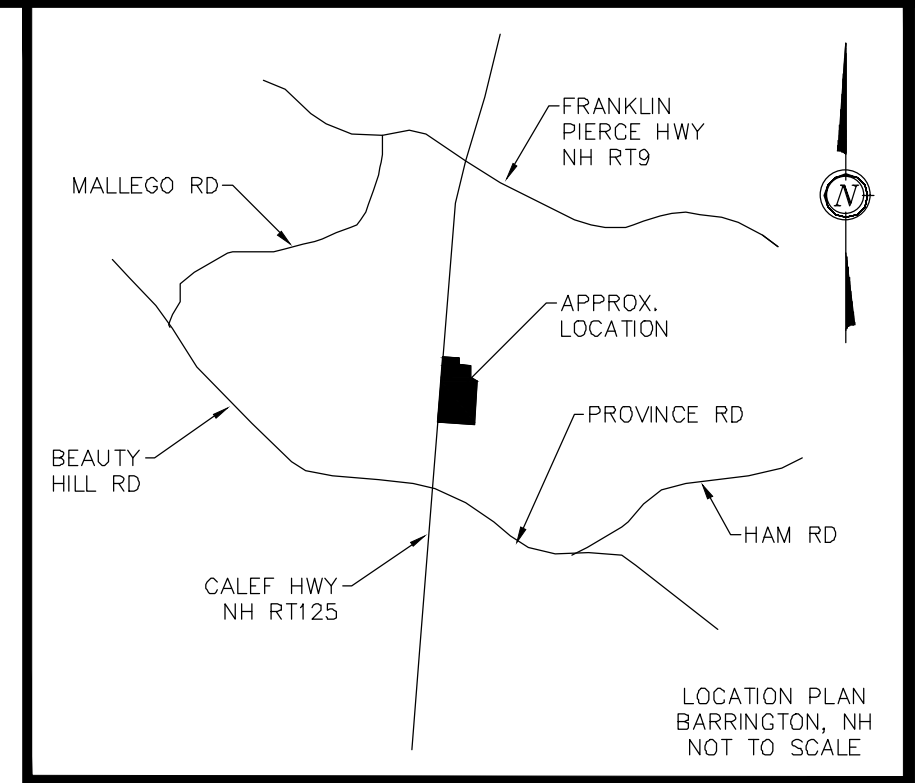
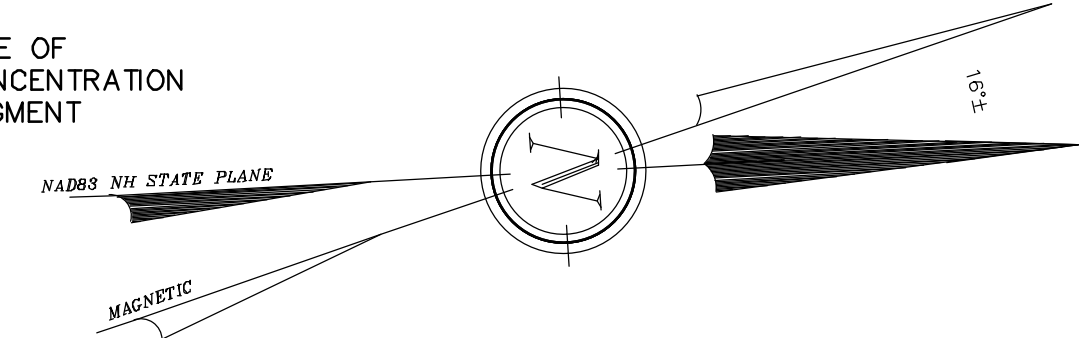
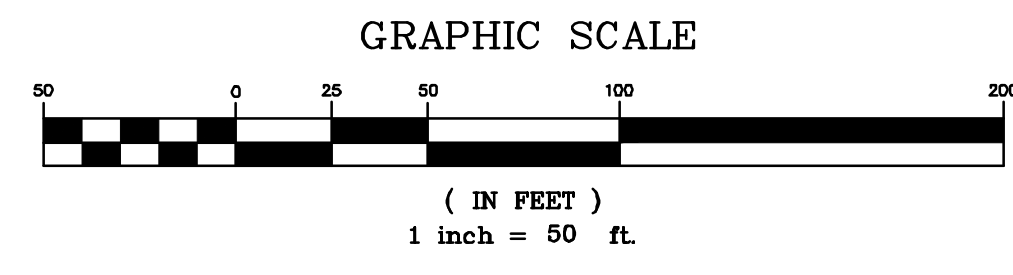
**LEGEND:**



**SYMBOLS LEGEND:**



JOHN P. HAYES, III CSS #87



**SOILS LEGEND**

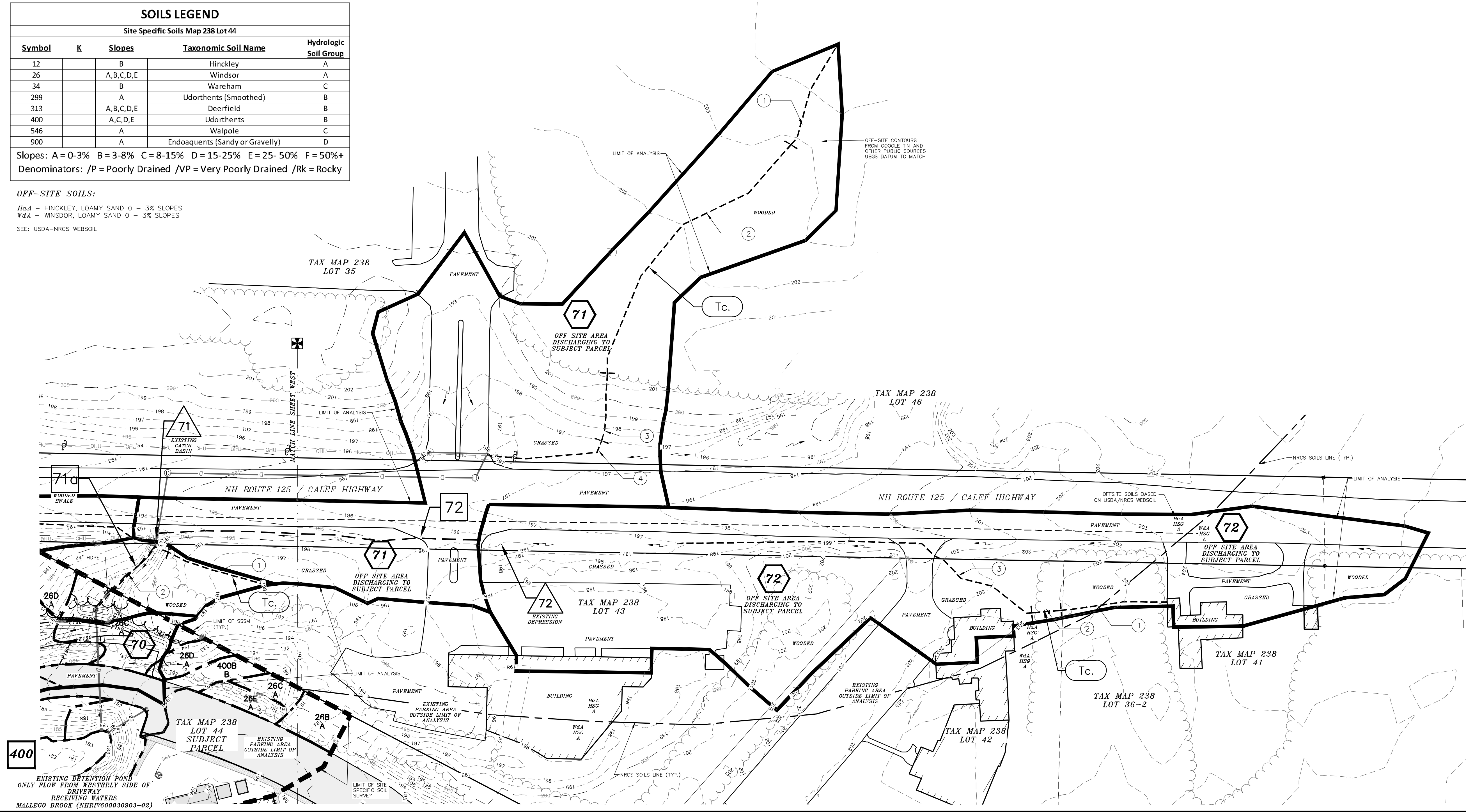
Site Specific Soils Map 238 Lot 44

Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoaquepts (Sandy or Gravelly)	D

Slopes: A = 0-3% B = 3-8% C = 8-15% D = 15-25% E = 25-50% F = 50%+  
Denominators: /P = Poorly Drained /VP = Very Poorly Drained /Rk = Rocky

**OFF-SITE SOILS:**

H<sub>a</sub>A - HINCKLEY, LOAMY SAND 0 - 3% SLOPES  
W<sub>d</sub>A - WINDSOR, LOAMY SAND 0 - 3% SLOPES  
SEE: USDA-NRCS WEBSOIL

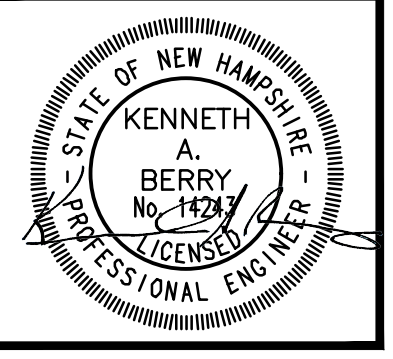


SWPPP EPA SITE MAP (PROPOSED)  
DETAIL W-2 PROPOSED WATERSHED PLAN OFFSITE

REVISION	DATE	DESCRIPTION

FOR  
TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 50 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017



### **Appendix III - Calculations, Charts, & Graphs**

Extreme Precipitation Tables

Rip Rap Calculations

AoT Stormwater Treatment Spreadsheets

NCRS USDA Web-soil Map

Site Specific Soil Survey Report & Plan

Stormwater System Management: Inspection & Maintenance Manual, Plan, Invasive Species & NHDES Green SnoPro Utilization Chart

Infiltration Feasibility Study & Report

Ksat Values for New Hampshire Soils, SSSNNE Special Publication #5, 2009

UNH Stormwater Center Hybrid Bioretention Template

Filtrexx Specifications Sheets

# Extreme Precipitation Tables

## Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Metadata for Point	
Smoothing	Yes
State	
Location	
Latitude	43.205 degrees North
Longitude	70.995 degrees West
Elevation	50 feet
Date/Time	Fri Apr 07 2023 13:37:20 GMT-0400 (Eastern Daylight Time)

### Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.65	0.81	1.03	1yr	0.70	0.98	1.20	1.53	1.97	2.56	2.81	1yr	2.27	2.71	3.12	3.84	4.41	1yr
2yr	0.32	0.49	0.61	0.80	1.01	1.28	2yr	0.87	1.16	1.49	1.89	2.41	3.08	3.43	2yr	2.73	3.30	3.80	4.52	5.15	2yr
5yr	0.37	0.57	0.72	0.96	1.23	1.57	5yr	1.06	1.44	1.84	2.36	3.03	3.89	4.39	5yr	3.45	4.22	4.83	5.70	6.45	5yr
10yr	0.40	0.63	0.80	1.09	1.42	1.84	10yr	1.22	1.69	2.17	2.80	3.61	4.65	5.29	10yr	4.11	5.08	5.81	6.80	7.65	10yr
25yr	0.46	0.74	0.94	1.30	1.72	2.27	25yr	1.49	2.08	2.69	3.50	4.54	5.87	6.77	25yr	5.20	6.51	7.41	8.58	9.59	25yr
50yr	0.52	0.83	1.06	1.49	2.00	2.66	50yr	1.73	2.45	3.17	4.15	5.41	7.02	8.17	50yr	6.21	7.85	8.91	10.24	11.39	50yr
100yr	0.58	0.94	1.21	1.71	2.33	3.12	100yr	2.01	2.88	3.74	4.92	6.43	8.39	9.86	100yr	7.42	9.48	10.72	12.23	13.53	100yr
200yr	0.64	1.05	1.37	1.96	2.71	3.68	200yr	2.34	3.39	4.42	5.85	7.68	10.03	11.90	200yr	8.87	11.44	12.89	14.61	16.07	200yr
500yr	0.76	1.25	1.63	2.37	3.32	4.55	500yr	2.86	4.21	5.49	7.32	9.67	12.70	15.27	500yr	11.24	14.68	16.47	18.49	20.21	500yr

### Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.24	0.36	0.45	0.60	0.74	0.90	1yr	0.64	0.88	0.91	1.25	1.52	1.94	2.49	1yr	1.72	2.39	2.92	3.28	3.96	1yr
2yr	0.31	0.48	0.60	0.81	0.99	1.18	2yr	0.86	1.15	1.36	1.82	2.34	2.99	3.33	2yr	2.65	3.21	3.69	4.41	5.03	2yr
5yr	0.35	0.54	0.67	0.92	1.16	1.40	5yr	1.01	1.37	1.61	2.14	2.77	3.61	4.05	5yr	3.20	3.89	4.50	5.35	6.03	5yr
10yr	0.38	0.59	0.73	1.02	1.32	1.60	10yr	1.14	1.56	1.82	2.43	3.12	4.14	4.69	10yr	3.66	4.51	5.22	6.18	6.90	10yr
25yr	0.44	0.67	0.83	1.19	1.57	1.91	25yr	1.35	1.87	2.12	2.84	3.64	4.94	5.67	25yr	4.37	5.45	6.37	7.49	8.28	25yr
50yr	0.49	0.74	0.92	1.33	1.79	2.20	50yr	1.54	2.15	2.37	3.20	4.08	5.65	6.54	50yr	5.00	6.29	7.40	8.65	9.55	50yr
100yr	0.55	0.83	1.03	1.49	2.05	2.52	100yr	1.77	2.46	2.67	3.60	4.55	6.44	7.54	100yr	5.70	7.25	8.61	10.00	10.92	100yr
200yr	0.61	0.92	1.16	1.68	2.34	2.89	200yr	2.02	2.83	2.99	4.05	5.08	7.34	8.85	200yr	6.50	8.51	10.03	11.55	12.51	200yr
500yr	0.71	1.06	1.37	1.99	2.82	3.50	500yr	2.44	3.42	3.50	4.72	5.91	8.68	10.73	500yr	7.68	10.32	12.28	14.00	14.91	500yr

### Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.43	0.53	0.71	0.87	1.07	1yr	0.75	1.05	1.23	1.72	2.18	2.76	3.02	1yr	2.44	2.90	3.34	4.12	4.73	1yr
2yr	0.33	0.50	0.62	0.84	1.03	1.24	2yr	0.89	1.21	1.46	1.94	2.50	3.19	3.54	2yr	2.82	3.41	3.92	4.66	5.30	2yr
5yr	0.39	0.60	0.75	1.02	1.30	1.57	5yr	1.12	1.54	1.84	2.47	3.16	4.18	4.71	5yr	3.70	4.53	5.18	6.06	6.85	5yr
10yr	0.45	0.70	0.87	1.21	1.56	1.90	10yr	1.35	1.86	2.21	3.01	3.80	5.16	5.87	10yr	4.57	5.64	6.42	7.40	8.32	10yr
25yr	0.55	0.84	1.05	1.49	1.96	2.44	25yr	1.70	2.39	2.84	3.91	4.87	6.83	7.84	25yr	6.04	7.54	8.50	9.75	10.75	25yr
50yr	0.64	0.97	1.21	1.74	2.34	2.94	50yr	2.02	2.87	3.44	4.75	5.90	8.45	9.79	50yr	7.48	9.41	10.54	11.96	13.15	50yr
100yr	0.74	1.12	1.41	2.03	2.79	3.54	100yr	2.41	3.46	4.17	5.80	7.15	10.45	12.22	100yr	9.25	11.75	13.05	14.70	16.05	100yr
200yr	0.86	1.30	1.64	2.38	3.32	4.28	200yr	2.86	4.18	5.06	7.08	8.66	12.98	15.08	200yr	11.49	14.50	16.16	18.04	19.63	200yr
500yr	1.05	1.56	2.01	2.93	4.16	5.48	500yr	3.59	5.35	6.52	9.23	11.17	17.33	20.20	500yr	15.33	19.42	21.45	23.71	25.63	500yr

## RIP RAP CALCULATIONS

23-017 Calef Highway  
TurboCam International Lot 44  
Barrington, NH

**Berry Surveying & Engineering**  
335 Second Crown Point Road  
TURBOCAM, INC., Barrington, NH

17-Apr-24

Rip Rap equations were obtained from the *Stormwater Management and Erosion Control Handbook for Urban and Developing Areas in New Hampshire*. Rip Rap was sized for the 25 year storm event. (Some d50 sizes and T values have been modified)

### TAILWATER < HALF THE Do

$La = (1.8 \times Q) / Do^{3/2} + (7 \times Do)$      $Q =$  Peak Flow &  $Do$  is Pipe Diameter

$W = La + 3 \times Do$  or defined channel width

$d50 = (0.02 \times Q^{4/3}) / (Tw \times Do)$

$Tw =$  Tailwater Depth

$T =$  Largest Stone Size  $\times 1.5$

Culvert or Catch Basin	Tailwater (Feet) Tw	Discharge (C.F.S.) Q	Diameter of Pipe Do	Length of Rip Rap La (feet)	Width of Rip Rap W (feet)	d50-Stone Rip Rap d50(ft.)	Actual Size	Thickness
---------------------------	---------------------------	----------------------------	---------------------------	-----------------------------------	---------------------------------	----------------------------------	----------------	-----------

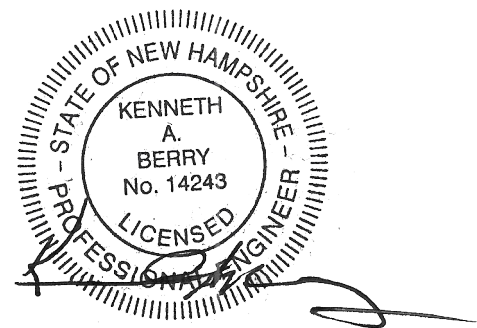
18" HDPE (Pond #C50P)	0.30	3.39	1.50	13.8	18.3	0.23	0.50	1.20
18" RCP (Pond #D51P)	0.30	0.68	1.50	11.2	15.7	0.03	0.50	1.20
24" HDPE (Pond #D53P)	0.40	6.97	2.00	18.4	24.4	0.33	0.50	1.20
18" HDPE (Pond #204P)	0.30	4.16	1.50	14.6	19.1	0.30	0.50	1.20
18" HDPE (Pond #C47P)	0.30	6.84	1.50	17.2	21.7	0.58	0.67	2.00
15" HDPE (Pond #202P)	0.25	0.68	1.25	9.6	13.4	0.04	0.50	1.20

Please note that the designer chose to use the 25 Year Event for the dimensional calculations.

Table 7-24 -- Recommended Rip Rap Gradation Ranges			
d50 Size =	0.5	Feet	6 Inches
% of Weight Smaller Than the Given d50 Size	Size of Stone (Inches)		
	From	To	
100%	9	12	
85%	8	11	
50%	6	9	
15%	2	3	

Table 7-24 -- Recommended Rip Rap Gradation Ranges			
d50 Size =	0.67	Feet	8 Inches
% of Weight Smaller Than the Given d50 Size	Size of Stone (Inches)		
	From	To	
100%	12	16	
85%	10	14	
50%	8	12	
15%	2	4	







## BIORETENTION SYSTEM WITH INTERNAL STORAGE RESERVOIR (UNH Stormwater Center Specification)

**Type/Node Name:** Bioretention W/ ISR #201 (Pond 201P)

Enter the node name in the drainage analysis if applicable.

1.17	ac	A = Area draining to the practice	
0.52	ac	A <sub>i</sub> = Impervious area draining to the practice	
0.44	decimal	I = Percent impervious area draining to the practice, in decimal form	
0.45	unitless	R <sub>v</sub> = Runoff coefficient = 0.05 + (0.9 x I)	
0.52	ac-in	WQV = 1" x R <sub>v</sub> x A	
1,899	cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
190	cf	10% x WQV (check calc for sediment forebay)	
475	cf	25% x WQV (check calc for water stored in saturated zone)	
<b>Sediment Forebay</b>		Method of Pretreatment	
1,032	cf	If pretrt is sed forebay: V <sub>SED</sub> (sediment forebay volume)	≥ 10%WQV
2,170	cf	Volume below lowest orifice <sup>1</sup>	≥ 100%WQV
590	cf	Water stored in voids of saturated zone	≥ 26%WQV
0.04	cfs	2Q <sub>avg</sub> = 2* WQV / 24 hrs * (1hr / 3600 sec) <sup>2</sup>	
184.60	ft	E <sub>WQV</sub> = Elevation of WQV (attach stage-storage table)	
0.02	cfs	Q <sub>WQV</sub> = Discharge at the E <sub>WQV</sub> (attach stage-discharge table)	< 2Q <sub>WQV</sub>
52.75	hours	T <sub>ED</sub> = Drawdown time of extended detention = 2WQV/Q <sub>WQV</sub>	≥ 24-hrs
18.00	in	Depth of Filter Media	≥ 18"
3.00	:1	Pond side slopes	≥ 3:1
Angle Grate		What mechanism is proposed to prevent the outlet structure from clogging (applicable for orifices/weirs with a dimension of ≤6")?	
185.74	ft	Peak elevation of the 50-year storm event (E <sub>50</sub> )	
186.00	ft	Berm elevation of the pond	
YES		E <sub>50</sub> ≤ the berm elevation?	← yes

1. Volume stored above the wetland soil and below the high flow by-pass.

**Designer's Notes:**

26% WQV = 494 CF

1,118 SF Bottom Pond, Stone = 40% Voids, 18" ISR Stone Base

Min ISR height @ Liner Low Point = 1.17'

1.17FT\*1,118 SF= 1,308 CF \* 40% Voids = 523 CF

Triangle of ISR remaining between liner low and high point

Max ISR height above triangle = 0.3'

0.5\*0.3FT\*1,118 SF= 168 CF \* 40% Voids =67 CF

523 CF + 67 CF = 590 CF Total ISR Storage

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

**Summary for Pond 201P: Bioretention W/ ISR #201**

Inflow Area = 1.174 ac, 43.96% Impervious, Inflow Depth > 3.31" for 50YR-24HR event  
 Inflow = 3.50 cfs @ 12.20 hrs, Volume= 0.324 af  
 Outflow = 1.26 cfs @ 12.61 hrs, Volume= 0.259 af, Atten= 64%, Lag= 24.3 min  
 Primary = 0.02 cfs @ 12.54 hrs, Volume= 0.025 af  
 Secondary = 1.23 cfs @ 12.61 hrs, Volume= 0.234 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 185.74' @ 12.61 hrs Surf.Area= 1,118 sf Storage= 5,447 cf  
 Flood Elev= 186.00' Surf.Area= 1,118 sf Storage= 6,720 cf

Plug-Flow detention time= 144.5 min calculated for 0.259 af (80% of inflow)  
 Center-of-Mass det. time= 66.2 min ( 909.4 - 843.2 )

Volume	Invert	Avail.Storage	Storage Description
#1	182.00'	112 cf	<b>Stone (Irregular)</b> Listed below (Recalc) -Impervious 280 cf Overall x 40.0% Voids
#2	182.25'	335 cf	<b>Bio Media (Irregular)</b> Listed below (Recalc) 1,677 cf Overall x 20.0% Voids
#3	184.00'	1,032 cf	<b>Sediment Forebay (Irregular)</b> Listed below (Recalc) -Impervious
#4	183.75'	2,979 cf	<b>Cell (Irregular)</b> Listed below (Recalc) -Impervious
#5	185.50'	2,262 cf	<b>Open Water Storage (Irregular)</b> Listed below (Recalc) -Impervious
		6,720 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
182.00	1,118	165.5	0	0	1,118
182.25	1,118	165.5	280	280	1,159

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
182.25	1,118	165.5	0	0	1,118
183.75	1,118	165.5	1,677	1,677	1,366

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
184.00	160	53.5	0	0	160
185.00	822	223.7	448	448	3,917
185.50	1,551	276.6	584	1,032	6,027

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
183.75	1,118	165.5	0	0	1,118
184.00	1,459	183.1	321	321	1,608
185.00	1,877	199.3	1,664	1,985	2,137
185.50	2,101	207.6	994	2,979	2,424



**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
185.50	3,838	302.2	0	0	3,838
186.00	5,247	335.0	2,262	2,262	5,509

Device	Routing	Invert	Outlet Devices
#1	Primary	182.00'	<b>6.0" Round 6" HDPE N-12</b> L= 33.0' Ke= 0.500 Inlet / Outlet Invert= 182.00' / 181.70' S= 0.0091 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#2	Secondary	182.00'	<b>15.0" Round 15" HDPE N-12</b> L= 26.0' Ke= 0.500 Inlet / Outlet Invert= 182.00' / 181.70' S= 0.0115 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf
#3	Device 1	182.00'	<b>0.7" Vert. 0.75" Orifice</b> C= 0.600
#4	Device 3	182.25'	<b>10.000 in/hr Bio Media over Surface area</b>
#5	Device 2	184.90'	<b>6.0" W x 10.0" H Vert. 6"W x 10" T Notch</b> C= 0.600
#6	Device 2	185.75'	<b>48.0" Horiz. 48" Outlet Structure</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.02 cfs @ 12.54 hrs HW=185.73' TW=182.16' (Dynamic Tailwater)

↑1=6" HDPE N-12 (Passes 0.02 cfs of 1.53 cfs potential flow)

↑3=0.75" Orifice (Orifice Controls 0.02 cfs @ 9.10 fps)

↑4=Bio Media (Passes 0.02 cfs of 0.26 cfs potential flow)

**Secondary OutFlow** Max=1.23 cfs @ 12.61 hrs HW=185.74' TW=182.16' (Dynamic Tailwater)

↑2=15" HDPE N-12 (Passes 1.23 cfs of 10.42 cfs potential flow)

↑5=6"W x 10" T Notch (Orifice Controls 1.23 cfs @ 2.95 fps)

↑6=48" Outlet Structure ( Controls 0.00 cfs)

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

**Stage-Area-Storage for Pond 201P: Bioretention W/ ISR #201**

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
182.00	0	0	184.65	1,118	2,015
182.05	0	22	184.70	1,118	2,129
182.10	0	45	184.75	1,118	2,247
182.15	0	67	184.80	1,118	2,367
182.20	0	89	184.85	1,118	2,490
182.25	<b>1,118</b>	112	<b>184.90</b>	<b>1,118</b>	<b>2,617</b>
182.30	1,118	123	184.95	1,118	2,747
182.35	1,118	134	185.00	1,118	2,880
182.40	1,118	145	185.05	1,118	3,017
182.45	1,118	157	185.10	1,118	3,159
182.50	1,118	168	185.15	1,118	3,304
182.55	1,118	179	185.20	1,118	3,455
182.60	1,118	190	185.25	1,118	3,609
182.65	1,118	201	185.30	1,118	3,769
182.70	1,118	212	185.35	1,118	3,934
182.75	1,118	224	185.40	1,118	4,103
182.80	1,118	235	185.45	1,118	4,278
182.85	1,118	246	185.50	1,118	4,458
182.90	1,118	257	185.55	1,118	4,653
182.95	1,118	268	185.60	1,118	4,855
183.00	1,118	280	185.65	1,118	5,063
183.05	1,118	291	185.70	1,118	5,279
183.10	1,118	302	185.75	1,118	5,501
183.15	1,118	313	185.80	1,118	5,730
183.20	1,118	324	185.85	1,118	5,967
183.25	1,118	335	185.90	1,118	6,210
183.30	1,118	347	185.95	1,118	6,461
183.35	1,118	358	186.00	1,118	<b>6,720</b>
183.40	1,118	369			
183.45	1,118	380			
183.50	1,118	391			
183.55	1,118	402			
183.60	1,118	414			
183.65	1,118	425			
183.70	1,118	436			
<b>183.75</b>	<b>1,118</b>	<b>447</b>			
183.80	1,118	505			
183.85	1,118	565			
183.90	1,118	630			
183.95	1,118	697			
184.00	1,118	768			
184.05	1,118	850			
184.10	1,118	934			
184.15	1,118	1,021			
184.20	1,118	1,109			
184.25	1,118	1,200			
184.30	1,118	1,292			
184.35	1,118	1,388			
184.40	1,118	1,486			
184.45	1,118	1,586			
184.50	1,118	1,689			
184.55	1,118	1,795			
184.60	1,118	1,904			

Low Orifice = 184.90  
Cell Bottom = 183.75

2,617 cf  
- 447 cf  
2,170 cf

Volume Below Lowest Orifice Table

**Stage-Area-Storage for Pond 201P: Bioretention W/ ISR #201**

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
182.00	0	0	184.65	1,118	2,015
182.05	0	22	184.70	1,118	2,129
182.10	0	45	184.75	1,118	2,247
182.15	0	67	184.80	1,118	2,367
182.20	0	89	184.85	1,118	2,490
182.25	<b>1,118</b>	112	184.90	1,118	2,617
182.30	1,118	123	184.95	1,118	2,747
182.35	1,118	134	185.00	1,118	2,880
182.40	1,118	145	185.05	1,118	3,017
182.45	1,118	157	185.10	1,118	3,159
182.50	1,118	168	185.15	1,118	3,304
182.55	1,118	179	185.20	1,118	3,455
182.60	1,118	190	185.25	1,118	3,609
182.65	1,118	201	185.30	1,118	3,769
182.70	1,118	212	185.35	1,118	3,934
182.75	1,118	224	185.40	1,118	4,103
182.80	1,118	235	185.45	1,118	4,278
182.85	1,118	246	185.50	1,118	4,458
182.90	1,118	257	185.55	1,118	4,653
182.95	1,118	268	185.60	1,118	4,855
183.00	1,118	280	185.65	1,118	5,063
183.05	1,118	291	185.70	1,118	5,279
183.10	1,118	302	185.75	1,118	5,501
183.15	1,118	313	185.80	1,118	5,730
183.20	1,118	324	185.85	1,118	5,967
183.25	1,118	335	185.90	1,118	6,210
183.30	1,118	347	185.95	1,118	6,461
183.35	1,118	358	186.00	1,118	<b>6,720</b>
183.40	1,118	369			
183.45	1,118	380			
183.50	1,118	391			
183.55	1,118	402			
183.60	1,118	414			
183.65	1,118	425			
183.70	1,118	436			
183.75	1,118	447			
183.80	1,118	505			
183.85	1,118	565			
183.90	1,118	630			
183.95	1,118	697			
184.00	1,118	768			
184.05	1,118	850			
184.10	1,118	934			
184.15	1,118	1,021			
184.20	1,118	1,109			
184.25	1,118	1,200			
184.30	1,118	1,292			
184.35	1,118	1,388			
184.40	1,118	1,486			
184.45	1,118	1,586			
184.50	1,118	1,689			
184.55	1,118	1,795			
<b>184.60</b>	<b>1,118</b>	<b>1,904</b>			

WQV = 1,899 cf  
Elev 184.60 = 1,904 cf

WQV Storage Table

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

**Stage-Discharge for Pond 201P: Bioretention W/ ISR #201**

Elevation (feet)	Discharge (cfs)	Primary (cfs)	Secondary (cfs)	Elevation (feet)	Discharge (cfs)	Primary (cfs)	Secondary (cfs)
182.00	0.00	0.00	0.00	184.65	0.02	0.02	0.00
182.05	0.00	0.00	0.00	184.70	0.02	0.02	0.00
182.10	0.00	0.00	0.00	184.75	0.02	0.02	0.00
182.15	0.00	0.00	0.00	184.80	0.02	0.02	0.00
182.20	0.00	0.00	0.00	184.85	0.02	0.02	0.00
182.25	0.01	0.01	0.00	184.90	0.02	0.02	0.00
182.30	0.01	0.01	0.00	184.95	0.04	0.02	0.02
182.35	0.01	0.01	0.00	185.00	0.07	0.02	0.05
182.40	0.01	0.01	0.00	185.05	0.12	0.02	0.09
182.45	0.01	0.01	0.00	185.10	0.17	0.02	0.14
182.50	0.01	0.01	0.00	185.15	0.22	0.02	0.20
182.55	0.01	0.01	0.00	185.20	0.29	0.02	0.26
182.60	0.01	0.01	0.00	185.25	0.36	0.02	0.33
182.65	0.01	0.01	0.00	185.30	0.43	0.02	0.41
182.70	0.01	0.01	0.00	185.35	0.51	0.02	0.48
182.75	0.01	0.01	0.00	185.40	0.59	0.02	0.57
182.80	0.01	0.01	0.00	185.45	0.68	0.02	0.65
182.85	0.01	0.01	0.00	185.50	0.77	0.02	0.75
182.90	0.01	0.01	0.00	185.55	0.87	0.02	0.84
182.95	0.01	0.01	0.00	185.60	0.96	0.02	0.94
183.00	0.01	0.01	0.00	185.65	1.07	0.02	1.04
183.05	0.01	0.01	0.00	185.70	1.17	0.02	1.15
183.10	0.01	0.01	0.00	185.75	1.28	0.02	1.25
183.15	0.01	0.01	0.00	185.80	1.83	0.02	1.80
183.20	0.01	0.01	0.00	185.85	2.75	0.03	2.72
183.25	0.01	0.01	0.00	185.90	3.91	0.03	3.88
183.30	0.01	0.01	0.00	185.95	5.27	0.03	5.24
183.35	0.01	0.01	0.00	186.00	<b>6.79</b>	<b>0.03</b>	<b>6.77</b>
183.40	0.02	0.02	0.00				
183.45	0.02	0.02	0.00				
183.50	0.02	0.02	0.00				
183.55	0.02	0.02	0.00				
183.60	0.02	0.02	0.00				
183.65	0.02	0.02	0.00				
183.70	0.02	0.02	0.00				
183.75	0.02	0.02	0.00				
183.80	0.02	0.02	0.00				
183.85	0.02	0.02	0.00				
183.90	0.02	0.02	0.00				
183.95	0.02	0.02	0.00				
184.00	0.02	0.02	0.00				
184.05	0.02	0.02	0.00				
184.10	0.02	0.02	0.00				
184.15	0.02	0.02	0.00				
184.20	0.02	0.02	0.00				
184.25	0.02	0.02	0.00				
184.30	0.02	0.02	0.00				
184.35	0.02	0.02	0.00				
184.40	0.02	0.02	0.00				
184.45	0.02	0.02	0.00				
184.50	0.02	0.02	0.00				
184.55	0.02	0.02	0.00				
184.60	0.02	0.02	0.00				

WQV Discharge Table

184.60 =  
0.02CFS



## BIORETENTION SYSTEM WITH INTERNAL STORAGE RESERVOIR (UNH Stormwater Center Specification)

**Type/Node Name:** Bioretention W/ ISR #202 (Pond 202P)

Enter the node name in the drainage analysis if applicable.

2.35	ac	A = Area draining to the practice	
1.10	ac	A <sub>i</sub> = Impervious area draining to the practice	
0.47	decimal	I = Percent impervious area draining to the practice, in decimal form	
0.47	unitless	R <sub>v</sub> = Runoff coefficient = 0.05 + (0.9 x I)	
1.10	ac-in	WQV = 1" x R <sub>v</sub> x A	
4,005	cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
400	cf	10% x WQV (check calc for sediment forebay)	
1,001	cf	25% x WQV (check calc for water stored in saturated zone)	
<b>Sediment Forebay</b>		Method of Pretreatment	
903	cf	If pretrt is sed forebay: V <sub>SED</sub> (sediment forebay volume)	≥ 10%WQV
4,740	cf	Volume below lowest orifice <sup>1</sup>	≥ 100%WQV
1,288	cf	Water stored in voids of saturated zone	≥ 26%WQV
0.09	cfs	2Q <sub>avg</sub> = 2* WQV / 24 hrs * (1hr / 3600 sec) <sup>2</sup>	
176.61	ft	E <sub>WQV</sub> = Elevation of WQV (attach stage-storage table)	
0.06	cfs	Q <sub>WQV</sub> = Discharge at the E <sub>WQV</sub> (attach stage-discharge table)	< 2Q <sub>WQV</sub>
37.08	hours	T <sub>ED</sub> = Drawdown time of extended detention = 2WQV/Q <sub>WQV</sub>	≥ 24-hrs
18.00	in	Depth of Filter Media	≥ 18"
3.00	:1	Pond side slopes	≥ 3:1
Angle Grate		What mechanism is proposed to prevent the outlet structure from clogging (applicable for orifices/weirs with a dimension of ≤6")?	
179.80	ft	Peak elevation of the 50-year storm event (E <sub>50</sub> )	
180.50	ft	Berm elevation of the pond	
YES		E <sub>50</sub> ≤ the berm elevation?	← yes

1. Volume stored above the wetland soil and below the high flow by-pass.

**Designer's Notes:**

26% WQV = 1,041 CF

1,981 SF Bottom Pond, Stone = 40% Voids, 24" ISR Stone Base

Min ISR height @ Liner Low Point = 1.25'

1.25FT\*1,981 SF= 2,476 CF \* 40% Voids = 991 CF

Triangle of ISR remaining between liner low and high point

Max ISR height above triangle = 0.75'

0.5\*0.75FT\*1,981 SF= 743 CF \* 40% Voids = 297 CF

991 CF + 297 CF = 1,288 CF Total ISR Storage

**Summary for Pond 202P: Bioretention W/ ISR #202**

Inflow Area = 2.354 ac, 46.52% Impervious, Inflow Depth > 4.77" for 50YR-24HR event  
 Inflow = 10.32 cfs @ 12.11 hrs, Volume= 0.936 af  
 Outflow = 1.19 cfs @ 13.02 hrs, Volume= 0.709 af, Atten= 88%, Lag= 54.6 min  
 Primary = 0.09 cfs @ 12.64 hrs, Volume= 0.112 af  
 Secondary = 1.10 cfs @ 13.02 hrs, Volume= 0.597 af  
 Tertiary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 179.80' @ 13.02 hrs Surf.Area= 1,981 sf Storage= 22,760 cf  
 Flood Elev= 180.50' Surf.Area= 1,981 sf Storage= 28,868 cf

Plug-Flow detention time= 312.0 min calculated for 0.709 af (76% of inflow)  
 Center-of-Mass det. time= 225.2 min ( 1,014.8 - 789.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	173.75'	198 cf	<b>Stone (Irregular)</b> Listed below (Recalc) -Impervious 495 cf Overall x 40.0% Voids
#2	174.00'	594 cf	<b>Bio Media (Irregular)</b> Listed below (Recalc) 2,972 cf Overall x 20.0% Voids
#3	175.50'	903 cf	<b>Sediment Forebay (Irregular)</b> Listed below (Recalc) -Impervious
#4	175.50'	6,102 cf	<b>Cell (Irregular)</b> Listed below (Recalc) -Impervious
#5	177.50'	21,071 cf	<b>Open Water Storage (Irregular)</b> Listed below (Recalc) -Impervious
		28,868 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
173.75	1,981	351.5	0	0	1,981
174.00	1,981	351.5	495	495	2,069

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
174.00	1,981	351.5	0	0	1,981
175.50	1,981	351.5	2,972	2,972	2,508

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
175.50	243	61.9	0	0	243
176.00	337	70.7	144	144	342
177.00	563	89.6	445	590	596
177.50	693	98.2	313	903	732

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
175.50	1,981	351.5	0	0	1,981
176.00	2,509	360.4	1,120	1,120	2,516
177.00	3,604	379.0	3,040	4,160	3,671
177.50	4,170	387.1	1,942	6,102	4,201

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
177.50	4,925	448.8	0	0	4,925
178.00	5,605	458.3	2,631	2,631	5,647
179.00	7,008	477.1	6,293	8,924	7,123
180.00	8,468	496.0	7,726	16,651	8,665
180.50	9,219	505.4	4,420	21,071	9,455

Device	Routing	Invert	Outlet Devices
#1	Primary	173.75'	<b>6.0" Round 6" HDPE N-12</b> L= 30.0' Ke= 0.500 Inlet / Outlet Invert= 173.75' / 173.50' S= 0.0083 ' / Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#2	Secondary	173.75'	<b>15.0" Round 15" HDPE N-12</b> L= 30.0' Ke= 0.500 Inlet / Outlet Invert= 173.75' / 173.50' S= 0.0083 ' / Cc= 0.900 n= 0.012, Flow Area= 1.23 sf
#3	Device 1	173.75'	<b>1.2" Vert. 1.25" Orifice</b> C= 0.600
#4	Device 3	174.00'	<b>10.000 in/hr Bio Media over Surface area</b>
#5	Device 2	177.00'	<b>4.0" Vert. 4" Orifice</b> C= 0.600
#6	Device 2	179.75'	<b>48.0" Horiz. 48" Outlet Structure</b> C= 0.600 Limited to weir flow at low heads
#7	Tertiary	180.00'	<b>10.0' long x 8.5' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.45 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65 2.65 2.65 2.66 2.67 2.69 2.71

**Primary OutFlow** Max=0.09 cfs @ 12.64 hrs HW=179.71' TW=173.86' (Dynamic Tailwater)

↑ **1=6" HDPE N-12** (Passes 0.09 cfs of 2.01 cfs potential flow)

↑ **3=1.25" Orifice** (Orifice Controls 0.09 cfs @ 11.64 fps)

↑ **4=Bio Media** (Passes 0.09 cfs of 0.46 cfs potential flow)

**Secondary OutFlow** Max=1.10 cfs @ 13.02 hrs HW=179.80' TW=174.34' (Dynamic Tailwater)

↑ **2=15" HDPE N-12** (Passes 1.10 cfs of 13.76 cfs potential flow)

↑ **5=4" Orifice** (Orifice Controls 0.68 cfs @ 7.81 fps)

↑ **6=48" Outlet Structure** (Weir Controls 0.42 cfs @ 0.71 fps)

**Tertiary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=173.75' TW=172.50' (Dynamic Tailwater)

↑ **7=Spillway** ( Controls 0.00 cfs)

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

**Stage-Area-Storage for Pond 202P: Bioretention W/ ISR #202**

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
173.75	0	0	179.05	1,981	17,073
173.85	0	79	179.15	1,981	17,788
173.95	<b>0</b>	158	179.25	1,981	18,517
174.05	<b>1,981</b>	218	179.35	1,981	19,260
174.15	1,981	258	179.45	1,981	20,018
174.25	1,981	297	179.55	1,981	20,790
174.35	1,981	337	179.65	1,981	21,577
174.45	1,981	376	179.75	1,981	22,378
174.55	1,981	416	179.85	1,981	23,195
174.65	1,981	456	179.95	1,981	24,026
174.75	1,981	495	180.05	1,981	24,873
174.85	1,981	535	180.15	1,981	25,735
174.95	1,981	574	180.25	1,981	26,611
175.05	1,981	614	180.35	1,981	27,502
175.15	1,981	654	180.45	1,981	<b>28,409</b>
175.25	1,981	693			
175.35	1,981	733			
175.45	1,981	773			
175.55	1,981	905			
175.65	1,981	1,139			
175.75	1,981	1,386			
175.85	1,981	1,644			
175.95	1,981	1,916			
176.05	1,981	2,200			
176.15	1,981	2,497			
176.25	1,981	2,806			
176.35	1,981	3,128			
176.45	1,981	3,462			
176.55	1,981	3,810			
176.65	1,981	4,170			
176.75	1,981	4,545			
176.85	1,981	4,933			
176.95	1,981	5,335			
177.05	1,981	5,752			
177.15	1,981	6,182			
177.25	1,981	6,626			
177.35	1,981	7,084			
177.45	1,981	7,556			
177.55	1,981	8,045			
177.65	1,981	8,551			
177.75	1,981	9,070			
177.85	1,981	9,603			
177.95	1,981	10,149			
178.05	1,981	10,710			
178.15	1,981	11,284			
178.25	1,981	11,871			
178.35	1,981	12,472			
178.45	1,981	13,087			
178.55	1,981	13,715			
178.65	1,981	14,358			
178.75	1,981	15,015			
178.85	1,981	15,687			
178.95	1,981	16,373			

Low Orifice = 177.00  
Cell Bottom = 175.50

5,542 cf
- 802 cf
4,740 cf

Volume Below Lowest Orifice Table



**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

**Stage-Area-Storage for Pond 202P: Bioretention W/ ISR #202**

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
173.75	0	0	179.05	1,981	17,073
173.85	0	79	179.15	1,981	17,788
173.95	<b>0</b>	158	179.25	1,981	18,517
174.05	<b>1,981</b>	218	179.35	1,981	19,260
174.15	1,981	258	179.45	1,981	20,018
174.25	1,981	297	179.55	1,981	20,790
174.35	1,981	337	179.65	1,981	21,577
174.45	1,981	376	179.75	1,981	22,378
174.55	1,981	416	179.85	1,981	23,195
174.65	1,981	456	179.95	1,981	24,026
174.75	1,981	495	180.05	1,981	24,873
174.85	1,981	535	180.15	1,981	25,735
174.95	1,981	574	180.25	1,981	26,611
175.05	1,981	614	180.35	1,981	27,502
175.15	1,981	654	180.45	1,981	<b>28,409</b>
175.25	1,981	693			
175.35	1,981	733			
175.45	1,981	773			
175.55	1,981	905			
175.65	1,981	1,139			
175.75	1,981	1,386			
175.85	1,981	1,644			
175.95	1,981	1,916			
176.05	1,981	2,200			
176.15	1,981	2,497			
176.25	1,981	2,806			
176.35	1,981	3,128			
176.45	1,981	3,462			
176.55	1,981	3,810			
176.65	1,981	4,170			
176.75	1,981	4,545			
176.85	1,981	4,933			
176.95	1,981	5,335			
177.05	1,981	5,752			
177.15	1,981	6,182			
177.25	1,981	6,626			
177.35	1,981	7,084			
177.45	1,981	7,556			
177.55	1,981	8,045			
177.65	1,981	8,551			
177.75	1,981	9,070			
177.85	1,981	9,603			
177.95	1,981	10,149			
178.05	1,981	10,710			
178.15	1,981	11,284			
178.25	1,981	11,871			
178.35	1,981	12,472			
178.45	1,981	13,087			
178.55	1,981	13,715			
178.65	1,981	14,358			
178.75	1,981	15,015			
178.85	1,981	15,687			
178.95	1,981	16,373			

WQV Storage Table

WQV = 4,005 cf  
Elev. 176.61 = 4,024 cf

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

**Stage-Discharge for Pond 202P: Bioretention W/ ISR #202**

Elevation (feet)	Discharge (cfs)	Primary (cfs)	Secondary (cfs)	Tertiary (cfs)	
173.75	0.00	0.00	0.00	0.00	
173.95	0.00	0.00	0.00	0.00	
174.15	0.02	0.02	0.00	0.00	
174.35	0.03	0.03	0.00	0.00	
174.55	0.03	0.03	0.00	0.00	
174.75	0.04	0.04	0.00	0.00	
174.95	0.04	0.04	0.00	0.00	
175.15	0.04	0.04	0.00	0.00	
175.35	0.05	0.05	0.00	0.00	
175.55	0.05	0.05	0.00	0.00	
175.75	0.05	0.05	0.00	0.00	
175.95	0.06	0.06	0.00	0.00	
176.15	0.06	0.06	0.00	0.00	
176.35	0.06	0.06	0.00	0.00	
176.55	0.06	0.06	0.00	0.00	
176.75	0.06	0.06	0.00	0.00	176.61 = 0.06CFS
176.95	0.07	0.07	0.00	0.00	
177.15	0.12	0.07	0.05	0.00	
177.35	0.25	0.07	0.18	0.00	
177.55	0.33	0.07	0.26	0.00	
177.75	0.40	0.08	0.32	0.00	
177.95	0.45	0.08	0.37	0.00	
178.15	0.50	0.08	0.42	0.00	
178.35	0.54	0.08	0.46	0.00	
178.55	0.58	0.08	0.49	0.00	
178.75	0.61	0.08	0.53	0.00	
178.95	0.65	0.09	0.56	0.00	
179.15	0.68	0.09	0.59	0.00	
179.35	0.71	0.09	0.62	0.00	
179.55	0.74	0.09	0.65	0.00	
179.75	0.77	0.09	0.68	0.00	
179.95	4.47	0.09	4.38	0.00	
180.15	12.64	0.10	11.12	1.42	
180.35	<b>19.75</b>	<b>0.10</b>	<b>14.44</b>	<b>5.21</b>	

WQV Discharge Table



## INFILTRATION PRACTICE CRITERIA (Env-Wq 1508.06)

**Type/Node Name:** Infiltration Pond #203 (203P)

Enter the type of infiltration practice (e.g., basin, trench) and the node name in the drainage analysis, if applicable.

<b>Yes</b>		Have you reviewed Env-Wq 1508.06(a) to ensure that infiltration is allowed?	<b>← yes</b>
0.37	ac	A = Area draining to the practice	
-	ac	A <sub>i</sub> = Impervious area draining to the practice	
-	decimal	I = Percent impervious area draining to the practice, in decimal form	
0.05	unitless	R <sub>v</sub> = Runoff coefficient = 0.05 + (0.9 x I)	
0.02	ac-in	WQV = 1" x R <sub>v</sub> x A	
67	cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
17	cf	25% x WQV (check calc for sediment forebay volume)	
Method of pretreatment? (not required for clean or roof runoff)			
	cf	V <sub>SED</sub> = Sediment forebay volume, if used for pretreatment	<b>≥ 25%WQV</b>
4,586	cf	V = Volume <sup>1</sup> (attach a stage-storage table)	<b>≥ WQV</b>
1,574	sf	A <sub>SA</sub> = Surface area of the bottom of the pond	
3.00	iph	K <sub>sat</sub> <sub>DESIGN</sub> = Design infiltration rate <sup>2</sup>	
0.2	hours	I <sub>DRAIN</sub> = Drain time = V / (A <sub>SA</sub> * I <sub>DESIGN</sub> )	<b>≤ 72-hrs</b>
172.50	feet	E <sub>BTM</sub> = Elevation of the bottom of the basin	
170.21	feet	E <sub>SHWT</sub> = Elevation of SHWT (if none found, enter the lowest elevation of the test pit)	
168.17	feet	E <sub>ROCK</sub> = Elevation of bedrock (if none found, enter the lowest elevation of the test pit)	
2.29	feet	D <sub>SHWT</sub> = Separation from SHWT	<b>≥ *<sup>3</sup></b>
4.3	feet	D <sub>ROCK</sub> = Separation from bedrock	<b>≥ *<sup>3</sup></b>
	ft	D <sub>amend</sub> = Depth of amended soil, if applicable due high infiltration rate	<b>≥ 24"</b>
	ft	D <sub>T</sub> = Depth of trench, if trench proposed	<b>4 - 10 ft</b>
	Yes/No	If a trench or underground system is proposed, has observation well been provided?	<b>← yes</b>
		If a trench is proposed, does material meet Env-Wq 1508.06(k)(2) requirements. <sup>4</sup>	<b>← yes</b>
Yes	Yes/No	If a basin is proposed, Is the perimeter curvilinear, and basin floor flat?	<b>← yes</b>
3.0	:1	If a basin is proposed, pond side slopes.	<b>≥ 3:1</b>
174.56	ft	Peak elevation of the 10-year storm event (infiltration can be used in analysis)	
174.61	ft	Peak elevation of the 50-year storm event (infiltration can be used in analysis)	
175.00	ft	Elevation of the top of the practice (if a basin, this is the elevation of the berm)	
YES		10 peak elevation ≤ Elevation of the top of the trench? <sup>5</sup>	<b>← yes</b>
YES		If a basin is proposed, 50-year peak elevation ≤ Elevation of berm?	<b>← yes</b>

1. Volume below the lowest invert of the outlet structure and excludes forebay volume
2. K<sub>sat</sub><sub>DESIGN</sub> includes a factor of safety. See Env-Wq 1504.14 for requirements for determining the infiltr. rate
3. 1' separation if treatment not required; 4' for treatment in GPAs & WSIPAs; & 3' in all other areas.
4. Clean, washed well graded diameter of 1.5 to 3 inches above the in-situ soil.
5. If 50-year peak elevation exceeds top of trench, the overflow must be routed in HydroCAD as secondary discharge.

**Designer's Notes:** Only land area contributing runoff directly to Pond #203 is considered for WQV calculation.

Runoff treated by Pond #202 is not considered.

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 10YR-24HR Rainfall=4.65"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

**Summary for Pond 203P: Infiltration Pond #203**

Inflow Area = 2.722 ac, 40.23% Impervious, Inflow Depth > 1.87" for 10YR-24HR event  
 Inflow = 0.68 cfs @ 12.33 hrs, Volume= 0.425 af  
 Outflow = 0.53 cfs @ 15.54 hrs, Volume= 0.326 af, Atten= 22%, Lag= 192.8 min  
 Discarded = 0.21 cfs @ 15.54 hrs, Volume= 0.221 af  
 Primary = 0.32 cfs @ 15.54 hrs, Volume= 0.105 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 174.56' @ 15.54 hrs Surf.Area= 3,042 sf Storage= 4,756 cf  
 Flood Elev= 175.00' Surf.Area= 3,385 sf Storage= 6,181 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 67.4 min ( 1,045.0 - 977.6 )

Volume	Invert	Avail.Storage	Storage Description		
#1	172.50'	6,181 cf	<b>Open Water Storage (Irregular)</b> Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
172.50	1,574	204.2	0	0	1,574
173.00	1,967	225.0	883	883	2,292
174.00	2,638	239.5	2,294	3,178	2,877
175.00	3,385	258.5	3,004	6,181	3,670

Device	Routing	Invert	Outlet Devices										
#1	Discarded	172.50'	<b>3.000 in/hr Infiltration over Surface area</b>										
#2	Primary	174.50'	<b>10.0' long x 7.0' breadth Spillway</b>										
			Head (feet)	0.20	0.40	0.60	0.80	1.00	1.20	1.40	1.60	1.80	2.00
				2.50	3.00	3.50	4.00	4.50	5.00	5.50			
			Coef. (English)	2.40	2.52	2.70	2.68	2.68	2.67	2.66	2.65	2.65	
				2.65	2.66	2.65	2.66	2.68	2.70	2.73	2.78		

**Discarded OutFlow** Max=0.21 cfs @ 15.54 hrs HW=174.56' (Free Discharge)

↑**1=Infiltration** (Exfiltration Controls 0.21 cfs)

**Primary OutFlow** Max=0.32 cfs @ 15.54 hrs HW=174.56' TW=0.00' (Dynamic Tailwater)

↑**2=Spillway** (Weir Controls 0.32 cfs @ 0.57 fps)

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 50YR-24HR Rainfall=7.02"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

**Summary for Pond 203P: Infiltration Pond #203**

Inflow Area = 2.722 ac, 40.23% Impervious, Inflow Depth > 3.45" for 50YR-24HR event  
 Inflow = 1.46 cfs @ 12.17 hrs, Volume= 0.783 af  
 Outflow = 1.12 cfs @ 13.42 hrs, Volume= 0.674 af, Atten= 24%, Lag= 74.9 min  
 Discarded = 0.21 cfs @ 13.42 hrs, Volume= 0.241 af  
 Primary = 0.90 cfs @ 13.42 hrs, Volume= 0.433 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs  
 Peak Elev= 174.61' @ 13.42 hrs Surf.Area= 3,084 sf Storage= 4,928 cf  
 Flood Elev= 175.00' Surf.Area= 3,385 sf Storage= 6,181 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 41.9 min ( 1,042.2 - 1,000.3 )

Volume	Invert	Avail.Storage	Storage Description			
#1	172.50'	6,181 cf	<b>Open Water Storage (Irregular)</b> Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
172.50	1,574	204.2	0	0	1,574	
173.00	1,967	225.0	883	883	2,292	
174.00	2,638	239.5	2,294	3,178	2,877	
175.00	3,385	258.5	3,004	6,181	3,670	

Device	Routing	Invert	Outlet Devices											
#1	Discarded	172.50'	<b>3.000 in/hr Infiltration over Surface area</b>											
#2	Primary	174.50'	<b>10.0' long x 7.0' breadth Spillway</b>											
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00											
			2.50 3.00 3.50 4.00 4.50 5.00 5.50											
			Coef. (English) 2.40 2.52 2.70 2.68 2.68 2.67 2.66 2.65 2.65											
			2.65 2.66 2.65 2.66 2.68 2.70 2.73 2.78											

**Discarded OutFlow** Max=0.21 cfs @ 13.42 hrs HW=174.61' (Free Discharge)

↑**1=Infiltration** (Exfiltration Controls 0.21 cfs)

**Primary OutFlow** Max=0.90 cfs @ 13.42 hrs HW=174.61' TW=0.00' (Dynamic Tailwater)

↑**2=Spillway** (Weir Controls 0.90 cfs @ 0.80 fps)

**23-017 Pro Analysis Ex TCAM Site Mods**

Type III 24-hr 25YR-24HR Rainfall=5.87"

Prepared by Berry Surveying & Engineering

Printed 4/17/2024

HydroCAD® 10.00-25 s/n 10221 © 2019 HydroCAD Software Solutions LLC

---

**Stage-Area-Storage for Pond 203P: Infiltration Pond #203**

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
172.50	1,574	0
172.55	1,611	80
172.60	1,649	161
172.65	1,687	245
172.70	1,726	330
172.75	1,765	417
172.80	1,805	506
172.85	1,845	598
172.90	1,885	691
172.95	1,926	786
173.00	1,967	883
173.05	1,998	983
173.10	2,030	1,083
173.15	2,061	1,186
173.20	2,093	1,289
173.25	2,126	1,395
173.30	2,158	1,502
173.35	2,191	1,611
173.40	2,224	1,721
173.45	2,257	1,833
173.50	2,290	1,947
173.55	2,324	2,062
173.60	2,358	2,179
173.65	2,392	2,298
173.70	2,426	2,418
173.75	2,461	2,540
173.80	2,496	2,664
173.85	2,531	2,790
173.90	2,566	2,918
173.95	2,602	3,047
174.00	2,638	3,178
174.05	2,673	3,311
174.10	2,709	3,445
174.15	2,744	3,581
174.20	2,780	3,719
174.25	2,816	3,859
174.30	2,852	4,001
174.35	2,889	4,145
174.40	2,926	4,290
174.45	2,963	4,437
174.50	3,000	4,586
174.55	3,037	4,737
174.60	3,075	4,890
174.65	3,113	5,045
174.70	3,151	5,201
174.75	3,190	5,360
174.80	3,228	5,520
174.85	3,267	5,683
174.90	3,306	5,847
174.95	3,345	6,013
175.00	3,385	6,181

Total Storage Capacity = 4,586 CF



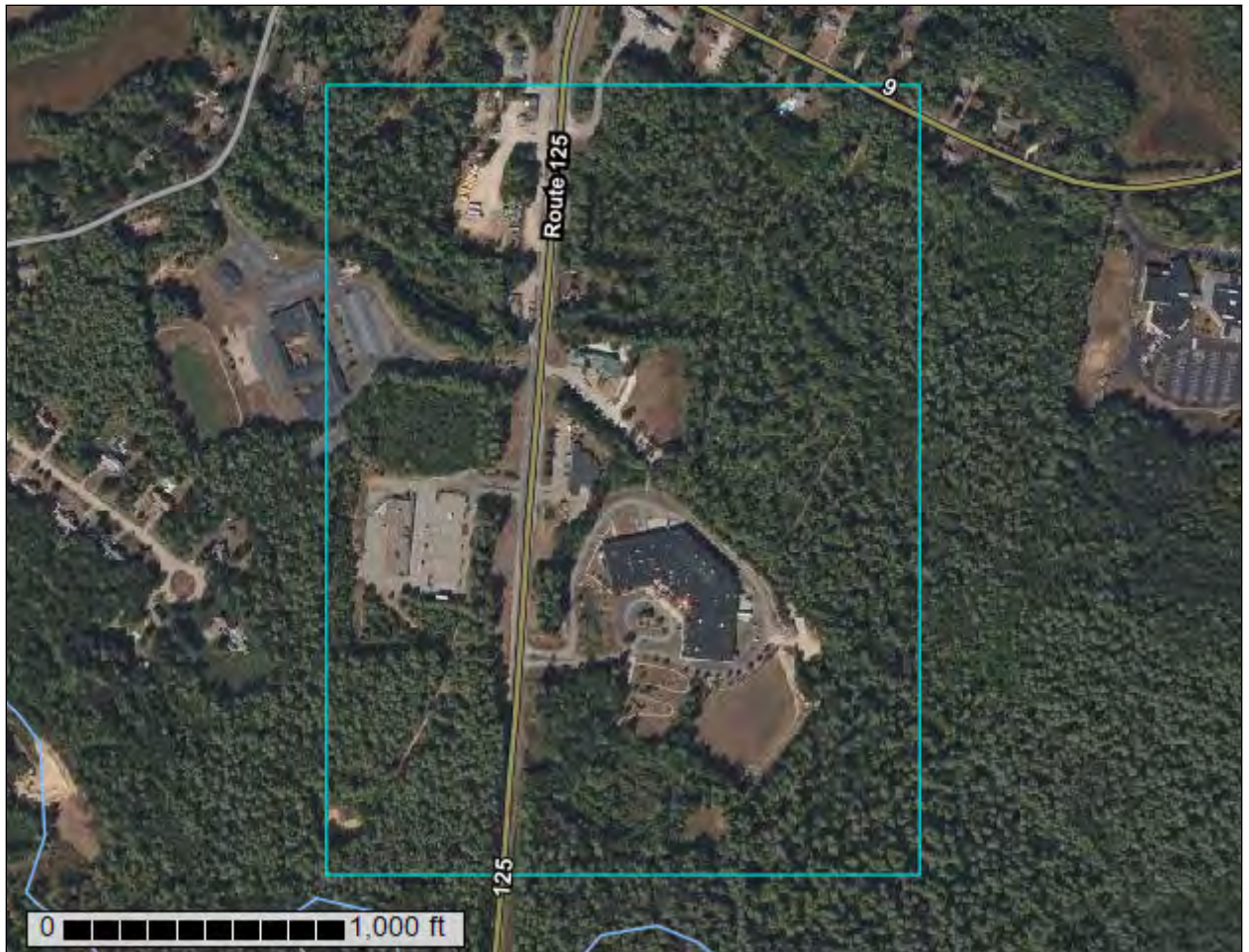
United States  
Department of  
Agriculture

**NRCS**

Natural  
Resources  
Conservation  
Service

A product of the National  
Cooperative Soil Survey,  
a joint effort of the United  
States Department of  
Agriculture and other  
Federal agencies, State  
agencies including the  
Agricultural Experiment  
Stations, and local  
participants

# Custom Soil Resource Report for Strafford County, New Hampshire



# Preface

---

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist ([http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2\\_053951](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951)).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require



alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

# Contents

---

<b>Preface</b> .....	2
<b>How Soil Surveys Are Made</b> .....	5
<b>Soil Map</b> .....	8
Soil Map.....	9
Legend.....	10
Map Unit Legend.....	11
Map Unit Descriptions.....	11
Strafford County, New Hampshire.....	13
Gv—Gravel and borrow pits.....	13
HaA—Hinckley loamy sand, 0 to 3 percent slopes.....	13
HaB—Hinckley loamy sand, 3 to 8 percent slopes.....	15
HaC—Hinckley loamy sand, 8 to 15 percent slopes.....	16
Sb—Saugatuck loamy sand.....	18
WdA—Windsor loamy sand, 0 to 3 percent slopes.....	19
<b>References</b> .....	21

# How Soil Surveys Are Made

---

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

## Custom Soil Resource Report

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

## Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

# Soil Map

---

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

# Custom Soil Resource Report Soil Map



Soil Map may not be valid at this scale.

Map Scale: 1:5,020 if printed on A portrait (8.5" x 11") sheet.


0 50 100 200 300 Meters

0 200 400 800 1200 Feet

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84

### MAP LEGEND

**Area of Interest (AOI)**

 Area of Interest (AOI)




















**Soils**







 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

**Special Point Features**






-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features


**Water Features**

 Streams and Canals

**Transportation**

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

**Background**

 Aerial Photography

### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Strafford County, New Hampshire  
 Survey Area Data: Version 24, Aug 22, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 19, 2020—Sep 20, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Gv	Gravel and borrow pits	0.1	0.1%
HaA	Hinckley loamy sand, 0 to 3 percent slopes	45.4	33.1%
HaB	Hinckley loamy sand, 3 to 8 percent slopes	10.3	7.5%
HaC	Hinckley loamy sand, 8 to 15 percent slopes	3.8	2.8%
Sb	Saugatuck loamy sand	7.6	5.6%
WdA	Windsor loamy sand, 0 to 3 percent slopes	69.9	51.0%
<b>Totals for Area of Interest</b>		<b>137.2</b>	<b>100.0%</b>

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it

## Custom Soil Resource Report

was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## **Strafford County, New Hampshire**

### **Gv—Gravel and borrow pits**

#### **Map Unit Setting**

*National map unit symbol:* 9d7c  
*Mean annual precipitation:* 36 to 71 inches  
*Mean annual air temperature:* 39 to 55 degrees F  
*Frost-free period:* 140 to 240 days  
*Farmland classification:* Not prime farmland

#### **Map Unit Composition**

*Gravel and borrow pits:* 100 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### **Description of Gravel And Borrow Pits**

##### **Typical profile**

*H1 - 0 to 6 inches:* extremely gravelly sand  
*H2 - 6 to 60 inches:* extremely gravelly sand

##### **Interpretive groups**

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 8s  
*Hydric soil rating:* Unranked

### **HaA—Hinckley loamy sand, 0 to 3 percent slopes**

#### **Map Unit Setting**

*National map unit symbol:* 2svm7  
*Elevation:* 0 to 1,420 feet  
*Mean annual precipitation:* 36 to 71 inches  
*Mean annual air temperature:* 39 to 55 degrees F  
*Frost-free period:* 140 to 240 days  
*Farmland classification:* Not prime farmland

#### **Map Unit Composition**

*Hinckley and similar soils:* 85 percent  
*Minor components:* 15 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### **Description of Hinckley**

##### **Setting**

*Landform:* Outwash deltas, kame terraces, outwash plains, outwash terraces  
*Landform position (three-dimensional):* Tread  
*Down-slope shape:* Concave, convex, linear  
*Across-slope shape:* Convex, linear, concave  
*Parent material:* Sandy and gravelly glaciofluvial deposits derived from gneiss and/or granite and/or schist

## Custom Soil Resource Report

### Typical profile

*Oe - 0 to 1 inches:* moderately decomposed plant material  
*A - 1 to 8 inches:* loamy sand  
*Bw1 - 8 to 11 inches:* gravelly loamy sand  
*Bw2 - 11 to 16 inches:* gravelly loamy sand  
*BC - 16 to 19 inches:* very gravelly loamy sand  
*C - 19 to 65 inches:* very gravelly sand

### Properties and qualities

*Slope:* 0 to 3 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Excessively drained  
*Runoff class:* Negligible  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to very high (1.42 to 99.90 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Maximum salinity:* Nonsaline (0.0 to 1.9 mmhos/cm)  
*Available water supply, 0 to 60 inches:* Low (about 3.1 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 3s  
*Hydrologic Soil Group:* A  
*Ecological site:* F144AY022MA - Dry Outwash  
*Hydric soil rating:* No

### Minor Components

#### Windsor

*Percent of map unit:* 5 percent  
*Landform:* Outwash terraces, kame terraces, outwash deltas  
*Landform position (three-dimensional):* Tread  
*Down-slope shape:* Concave, convex, linear  
*Across-slope shape:* Convex, linear, concave  
*Hydric soil rating:* No

#### Sudbury

*Percent of map unit:* 5 percent  
*Landform:* Kame terraces, outwash terraces, outwash deltas  
*Landform position (three-dimensional):* Tread  
*Down-slope shape:* Concave, convex, linear  
*Across-slope shape:* Convex, linear, concave  
*Hydric soil rating:* No

#### Merrimac

*Percent of map unit:* 5 percent  
*Landform:* Kame terraces, outwash terraces, outwash deltas  
*Landform position (three-dimensional):* Tread  
*Down-slope shape:* Concave, convex, linear  
*Across-slope shape:* Convex, linear, concave  
*Hydric soil rating:* No

## HaB—Hinckley loamy sand, 3 to 8 percent slopes

### Map Unit Setting

*National map unit symbol:* 2svm8  
*Elevation:* 0 to 1,430 feet  
*Mean annual precipitation:* 36 to 53 inches  
*Mean annual air temperature:* 39 to 55 degrees F  
*Frost-free period:* 140 to 250 days  
*Farmland classification:* Not prime farmland

### Map Unit Composition

*Hinckley and similar soils:* 85 percent  
*Minor components:* 15 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### Description of Hinckley

#### Setting

*Landform:* Outwash plains, eskers, moraines, kame terraces, kames, outwash terraces, outwash deltas  
*Landform position (two-dimensional):* Summit, shoulder, backslope, footslope  
*Landform position (three-dimensional):* Nose slope, side slope, base slope, crest, riser, tread  
*Down-slope shape:* Concave, convex, linear  
*Across-slope shape:* Convex, linear, concave  
*Parent material:* Sandy and gravelly glaciofluvial deposits derived from gneiss and/or granite and/or schist

#### Typical profile

*Oe - 0 to 1 inches:* moderately decomposed plant material  
*A - 1 to 8 inches:* loamy sand  
*Bw1 - 8 to 11 inches:* gravelly loamy sand  
*Bw2 - 11 to 16 inches:* gravelly loamy sand  
*BC - 16 to 19 inches:* very gravelly loamy sand  
*C - 19 to 65 inches:* very gravelly sand

#### Properties and qualities

*Slope:* 3 to 8 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Excessively drained  
*Runoff class:* Very low  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to very high (1.42 to 99.90 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Maximum salinity:* Nonsaline (0.0 to 1.9 mmhos/cm)  
*Available water supply, 0 to 60 inches:* Very low (about 3.0 inches)

**Interpretive groups**

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 3s

*Hydrologic Soil Group:* A

*Ecological site:* F144AY022MA - Dry Outwash

*Hydric soil rating:* No

**Minor Components**

**Windsor**

*Percent of map unit:* 8 percent

*Landform:* Kame terraces, outwash plains, kames, eskers, moraines, outwash terraces, outwash deltas

*Landform position (two-dimensional):* Summit, shoulder, backslope, footslope

*Landform position (three-dimensional):* Nose slope, side slope, base slope, crest, riser, tread

*Down-slope shape:* Concave, convex, linear

*Across-slope shape:* Convex, linear, concave

*Hydric soil rating:* No

**Sudbury**

*Percent of map unit:* 5 percent

*Landform:* Kame terraces, outwash plains, moraines, outwash terraces, outwash deltas

*Landform position (two-dimensional):* Backslope, footslope

*Landform position (three-dimensional):* Head slope, side slope, base slope, tread

*Down-slope shape:* Concave, linear

*Across-slope shape:* Concave, linear

*Hydric soil rating:* No

**Agawam**

*Percent of map unit:* 2 percent

*Landform:* Kame terraces, outwash plains, kames, eskers, moraines, outwash terraces, outwash deltas

*Landform position (two-dimensional):* Summit, shoulder, backslope, footslope

*Landform position (three-dimensional):* Nose slope, side slope, base slope, crest, riser, tread

*Down-slope shape:* Concave, convex, linear

*Across-slope shape:* Convex, linear, concave

*Hydric soil rating:* No

**HaC—Hinckley loamy sand, 8 to 15 percent slopes**

**Map Unit Setting**

*National map unit symbol:* 2svm9

*Elevation:* 0 to 1,480 feet

*Mean annual precipitation:* 36 to 71 inches

*Mean annual air temperature:* 39 to 55 degrees F

*Frost-free period:* 140 to 240 days

*Farmland classification:* Not prime farmland

### Map Unit Composition

*Hinckley and similar soils:* 85 percent

*Minor components:* 15 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

### Description of Hinckley

#### Setting

*Landform:* Kame terraces, outwash plains, kames, eskers, moraines, outwash terraces, outwash deltas

*Landform position (two-dimensional):* Shoulder, backslope, footslope, toeslope

*Landform position (three-dimensional):* Head slope, nose slope, side slope, crest, riser

*Down-slope shape:* Concave, convex, linear

*Across-slope shape:* Convex, linear, concave

*Parent material:* Sandy and gravelly glaciofluvial deposits derived from gneiss and/or granite and/or schist

#### Typical profile

*Oe - 0 to 1 inches:* moderately decomposed plant material

*A - 1 to 8 inches:* loamy sand

*Bw1 - 8 to 11 inches:* gravelly loamy sand

*Bw2 - 11 to 16 inches:* gravelly loamy sand

*BC - 16 to 19 inches:* very gravelly loamy sand

*C - 19 to 65 inches:* very gravelly sand

#### Properties and qualities

*Slope:* 8 to 15 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Excessively drained

*Runoff class:* Very low

*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to very high (1.42 to 99.90 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Maximum salinity:* Nonsaline (0.0 to 1.9 mmhos/cm)

*Available water supply, 0 to 60 inches:* Low (about 3.1 inches)

#### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 4e

*Hydrologic Soil Group:* A

*Ecological site:* F144AY022MA - Dry Outwash

*Hydric soil rating:* No

### Minor Components

#### Merrimac

*Percent of map unit:* 5 percent

*Landform:* Eskers, moraines, outwash terraces, outwash plains, kames

*Landform position (two-dimensional):* Shoulder, backslope, footslope, toeslope

*Landform position (three-dimensional):* Head slope, nose slope, side slope, crest, riser

*Down-slope shape:* Convex

*Across-slope shape:* Convex

## Custom Soil Resource Report

*Hydric soil rating:* No

### **Sudbury**

*Percent of map unit:* 5 percent

*Landform:* Outwash terraces, kame terraces, outwash plains, moraines, outwash deltas

*Landform position (two-dimensional):* Backslope, footslope

*Landform position (three-dimensional):* Base slope, tread

*Down-slope shape:* Concave, linear

*Across-slope shape:* Concave, linear

*Hydric soil rating:* No

### **Windsor**

*Percent of map unit:* 5 percent

*Landform:* Kame terraces, outwash plains, outwash terraces, outwash deltas, kames, eskers, moraines

*Landform position (two-dimensional):* Shoulder, backslope, footslope, toeslope

*Landform position (three-dimensional):* Head slope, nose slope, side slope, crest, riser

*Down-slope shape:* Concave, convex, linear

*Across-slope shape:* Convex, linear, concave

*Hydric soil rating:* No

## **Sb—Saugatuck loamy sand**

### **Map Unit Setting**

*National map unit symbol:* 9d8r

*Elevation:* 300 to 1,000 feet

*Mean annual precipitation:* 27 to 71 inches

*Mean annual air temperature:* 39 to 55 degrees F

*Frost-free period:* 125 to 240 days

*Farmland classification:* Not prime farmland

### **Map Unit Composition**

*Saugatuck and similar soils:* 85 percent

*Minor components:* 15 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

### **Description of Saugatuck**

#### **Setting**

*Landform:* Outwash terraces

*Parent material:* Outwash

#### **Typical profile**

*H1 - 0 to 4 inches:* loamy sand

*H2 - 4 to 7 inches:* sand

*H3 - 7 to 26 inches:* loamy sand

*H4 - 26 to 42 inches:* sand



## Custom Soil Resource Report

### Properties and qualities

*Slope:* 0 to 3 percent  
*Depth to restrictive feature:* 10 to 16 inches to undefined  
*Drainage class:* Poorly drained  
*Runoff class:* High  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to high  
(0.60 to 2.00 in/hr)  
*Depth to water table:* About 0 to 12 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water supply, 0 to 60 inches:* Very low (about 1.1 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 4w  
*Hydrologic Soil Group:* B/D  
*Ecological site:* F144AY028MA - Wet Outwash  
*Hydric soil rating:* Yes

### Minor Components

#### Not named wet

*Percent of map unit:* 15 percent  
*Landform:* Outwash terraces  
*Hydric soil rating:* Yes

## WdA—Windsor loamy sand, 0 to 3 percent slopes

### Map Unit Setting

*National map unit symbol:* 2svkg  
*Elevation:* 0 to 990 feet  
*Mean annual precipitation:* 36 to 71 inches  
*Mean annual air temperature:* 39 to 55 degrees F  
*Frost-free period:* 140 to 240 days  
*Farmland classification:* Farmland of local importance

### Map Unit Composition

*Windsor, loamy sand, and similar soils:* 85 percent  
*Minor components:* 15 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### Description of Windsor, Loamy Sand

#### Setting

*Landform:* Dunes, deltas, outwash terraces, outwash plains  
*Landform position (three-dimensional):* Tread, riser  
*Down-slope shape:* Convex, linear  
*Across-slope shape:* Convex, linear

## Custom Soil Resource Report

*Parent material:* Loose sandy glaciofluvial deposits derived from granite and/or loose sandy glaciofluvial deposits derived from schist and/or loose sandy glaciofluvial deposits derived from gneiss

### Typical profile

*O - 0 to 1 inches:* moderately decomposed plant material  
*A - 1 to 3 inches:* loamy sand  
*Bw - 3 to 25 inches:* loamy sand  
*C - 25 to 65 inches:* sand

### Properties and qualities

*Slope:* 0 to 3 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Excessively drained  
*Runoff class:* Low  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately high to very high (1.42 to 99.90 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Maximum salinity:* Nonsaline (0.0 to 1.9 mmhos/cm)  
*Available water supply, 0 to 60 inches:* Low (about 3.6 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 2s  
*Hydrologic Soil Group:* A  
*Ecological site:* F144AY022MA - Dry Outwash  
*Hydric soil rating:* No

### Minor Components

#### Deerfield, loamy sand

*Percent of map unit:* 10 percent  
*Landform:* Outwash plains, terraces, deltas  
*Landform position (two-dimensional):* Footslope  
*Landform position (three-dimensional):* Tread, talf  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Hydric soil rating:* No

#### Hinckley, loamy sand

*Percent of map unit:* 5 percent  
*Landform:* Outwash plains, eskers, kames, deltas  
*Landform position (two-dimensional):* Summit, shoulder, backslope  
*Landform position (three-dimensional):* Head slope, nose slope, side slope, crest, rise  
*Down-slope shape:* Convex  
*Across-slope shape:* Linear, convex  
*Hydric soil rating:* No

# References

---

- American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.
- American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.
- Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.
- Federal Register. July 13, 1994. Changes in hydric soils of the United States.
- Federal Register. September 18, 2002. Hydric soils of the United States.
- Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.
- National Research Council. 1995. Wetlands: Characteristics and boundaries.
- Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. [http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2\\_054262](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_054262)
- Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. [http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2\\_053577](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053577)
- Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. [http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2\\_053580](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053580)
- Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.
- United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.
- United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. [http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/home/?cid=nrcs142p2\\_053374](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/home/?cid=nrcs142p2_053374)
- United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. <http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/landuse/rangepasture/?cid=stelprdb1043084>

## Custom Soil Resource Report

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. [http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2\\_054242](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242)

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. [http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2\\_053624](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624)

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. [http://www.nrcs.usda.gov/Internet/FSE\\_DOCUMENTS/nrcs142p2\\_052290.pdf](http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf)

**John P. Hayes III CSS, CWS,  
7 Limestone Way  
North Hampton, NH 03862  
603-205-4396  
johnphayes@comcast.net**

**1/24/24**

**Christopher Berry  
Berry Surveying and Engineering  
335 Second Crown Point Road  
Barrington NH 03825**

**Job # 23-006**

**Site Specific Soil Survey 1/24/23  
Map 238 Lot 44  
Calef Highway Barrington, NH**

Dear Chris,

This letter report presents the findings of a Site Specific Soil Survey conducted on the referenced properties by John P. Hayes III on January 23, 2024. The soil survey was conducted in accordance with the New Hampshire Supplement of the Site-Specific Soil Mapping Standard For New Hampshire and Vermont, Special Publication # 3, Version 7.0 July 2021, published by the Society of Soil Scientist of Northern New England. Soil series information was also taken from the Soil Survey of Strafford County New Hampshire issued March 1973.

The parcel is located on the southeast side of Route 125, northeast of Providence road, and northwest of Mallego Brook, in Barrington, NH. Lot 44 is approximately 28 acres in size. Only a portion of Lot 44, in the northeast section, around the present structure, was mapped. The plans used for these soil maps are a 40 scale plan, where 1 inch equals 40 feet, with two foot contours.

The purpose of the soil survey is to provide the client with soils information for urban and suburban or rural land planning. Soil characteristics on the property were evaluated through observation of numerous test pits, and hand auger probes conducted throughout the property. Slope phases were determined with the use of the topography provided on the plan. The Site-specific Soil Map Units identified are taken from the New Hampshire State-Wide Numerical Soils Legend, Issue #10 January 2011, and are briefly described below. Official Series Descriptions (OSD) for each of these soil series are enclosed with this report. The soil map units comply with the Range In Characteristics described in the OSD. Any limiting inclusions on the site, do not exceed 15 percent of any of the soil map units. Dissimilar inclusions, if any, will be noted in the report. Limits of the Site Specific mapping units are highlighted on the plan. The Hydrological Soil Groups for each of the soil series was determined using SSSNNE Publication No. 5 Ksat Values for New Hampshire Soils September 2009. Limits of the Site Specific mapping units are highlighted on the plan.

The Hydrological Soil Groups for each of the soil series was determined using SSSNNE Publication No. 5 Ksat Values for New Hampshire Soils September 2009. Limits of the Site Specific mapping units are highlighted on the plan.

Portions of the soil map with the map unit denominator of P and VP contain poorly drained soils, and very poorly drained soils respectively. Portions of the soil map with the map unit 400, and 299, contain disturbed soils that have been excavated and/or regraded. They are well drained, to moderately well drained, and are sandy in texture. Portions of the soil map with the map unit 900, contain disturbed soils that have been excavated down to, or near the water table, and are poorly drained. These soils are also sandy in texture. A Disturbed Soil Mapping Unit Supplement for New Hampshire DES AoT Site Specific Soil Maps is also included. This supplement explains the additional information given about each disturbed soil map units that are present on the site.

MAP UNIT #	SOIL TAXANOMI C NAME	SLOPES	HYDRO LOGIC SOIL GROUP	DESCRIPTION
26	Windsor	C	A	The Windsor series consists of very deep, excessively drained soils formed in sandy outwash or eolian deposits. These soils are located in the northeastern portion of the property. The soil texture is loamy sand over sand. These soils are deep to bedrock. Saturated hydraulic conductivity is high or very high. Some inclusions of moderately drained Deerfield soils may be present, but are less than 10 percent of the mapped area. Estimated seasonal high water tables in these soils range from 38 to 60 inches.
<u>34</u> P	Wareham	B	C	The Wareham series consists of very deep, poorly and drained sandy soils formed in outwash on plains, deltas, and terraces. These soils are located in the wetland areas in the southwestern portion of the property. The soil texture is loamy coarse sand over coarse sand. These soils are deep to bedrock. Permeability is rapid throughout these soils. Some inclusions of somewhat poorly drained Deerfield Variant soils may be present, but are less than 10 percent of the mapped area. Estimated seasonal high water tables in these soils range from 0 to 10 inches.
299 (cbadb)	Udorthents (smoothed)	A	B	The Udorthents smoothed map unit represents soils that have been cut and filled and made into level areas. The soil material typically comes from the soils in the immediate surrounding areas. These soils are located in the southeastern portion of the property. These disturbed soils are mostly derived from the Windsor and/or Deerfield soil series, but also some fill material and concrete was found in the soil profile. The soil textures include loamy sand over coarse sand, and over stratified sand and fine sand. These soils are well drained, and are deep to bedrock. Saturated hydraulic conductivity is high or very high. Estimated seasonal high water tables in these soils are over 40 inches.

MAP UNIT #	SOIL TAXANOMI C NAME	SLOPES	HYDRO LOGIC SOIL GROUP	DESCRIPTION
313	Deerfield	A B C D E	B	The Deerfield series consists of very deep, moderately well drained soils formed in glaciofluvial deposits. These soils are located throughout the property. The soil texture is loamy sand over sand. These soils are deep to bedrock. Saturated hydraulic conductivity is high or very high. Some inclusions of excessively well drained Windsor, and somewhat poorly drained Deerfield Variant, soils may be present, but are less than 10 percent of the mapped area. Estimated seasonal high water tables in these soils range from 15 to 38 inches.
400 (c/dbadb)	Udorthents (sandy or gravelly) (moderately well drained)	A C D E	B	Udorthents are disturbed soils that have been excavated and/or regraded, and are sandy or gravelly in texture. These soils are located mostly in the northeast portion of the property. There is also an area of this disturbed map unit in the central portion of the lot. These disturbed soils are mostly derived from the Windsor and/or Deerfield soil series. The soil textures are loamy sand over stratified sand. These soils are moderately well drained, and are deep to bedrock. Saturated hydraulic conductivity is high or very high. Estimated seasonal high water tables in these soils range from 15 to 50 inches.
<u>546</u> P	Walpole	A	C	The Walpole Series consists of very deep, poorly drained sandy soils formed in outwash and stratified drift. A small area of these soils are located in the wetland, in the south central portion of the property. The soil texture is loamy sand over sandy loam over gravelly sand. Saturated hydraulic conductivity is moderately high or high in the surface layer and subsoil, and high or very high in the substratum. Some inclusions of the very poorly drained Scarboro soil series, and the somewhat poorly drained component of the Sudbury soils may be present, but are less than 10 percent of the mapped area. Estimated seasonal high water tables in these soils range from 0 to 10 inches.



MAP UNIT #	SOIL TAXANOMIC NAME	SLOPES	HYDRO LOGIC SOIL GROUP	DESCRIPTION
<b>900 P (fbadd)</b>	<b>Endoaquents (sandy or gravelly)</b>	<b>A</b>	<b>D</b>	Endoaquents represents areas of disturbed soils where the soil material was excavated down to, or near, the water table, and are sandy or gravelly in texture. These soils are located in the northeastern portion of the property in a manmade detention basin. These disturbed soils appear to be the lower horizons of the Windsor or Deerfield soil series. These soils are deep to bedrock. Saturated hydraulic conductivity is mineral portion is high or very high. Estimated seasonal high water tables in these soils range from 0 to 10 inches

### Slope Phases

#### Alpha Slope Symbol

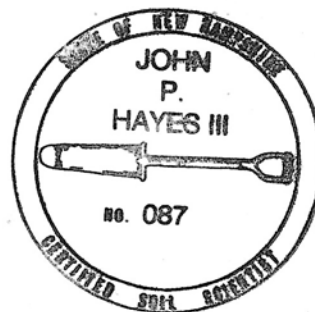
#### Range

<b>A</b>	<b>0 – 3%</b>
<b>B</b>	<b>3 – 8%</b>
<b>C</b>	<b>8 – 15%</b>
<b>D</b>	<b>15 – 25%</b>
<b>E</b>	<b>25 – 50%</b>
<b>F</b>	<b>&gt; 50%</b>

I trust that this Soil Survey and report meet your current planning needs. Please do not hesitate to contact me if you have any questions.

Sincerely:

*John P. Hayes III*



John P. Hayes III CSS, CWS

# **Disturbed Soil Mapping Unit Supplement for New Hampshire DES AoT Site Specific Soil Maps**

## **Introduction**

The NRCS NH State-Wide Legend, as amended, contains a number of distinct map units used for identifying areas of soils altered or disturbed by human influence. However, in preparing the required Site Specific Soils Maps for compliance with NH Department of Environmental Services Alteration of Terrain (AoT) rules, additional information is often needed and desired. This supplement provides a means to supply the user a more detailed soil mapping unit description to meet this need.

## **Purpose**

To provide soil scientists with additional soil mapping tools for disturbed sites and miscellaneous areas to enhance site specific soil maps and interpretations to reflect new requirements under the revised NH Alteration of Terrain regulations. This supplement is intended to allow the creation of soil maps with mapping units that can be expanded beyond those of the NRCS NH State-Wide Numerical Legend and the standards of the National Cooperative Soil Survey for disturbed units in order to provide specific information useful in preparation of site specific soils maps and reports to comply with NHDES Env-Wq 1500-Alteration of Terrain.

Note that the disturbed soil supplement has been created by SSSNNE and is not a product of the NRCS or the National Cooperative Soil Survey. Additionally, the supplemental legend can only be used in conjunction with the Site Specific Soil Mapping standards and cannot be used to create a stand-alone soils map.

For the purposes of this supplement, the definition of disturbed land, including excavate and fill, is as defined by RSA 485-A: 6, VIII; RSA 485-A: 17, and NHDES Env-Wq 1500.

## Map Notation

Notation on the Site Specific Soil Map completed to comply with the NH AoT rules should include the following disclaimer:

### Site-Specific Soil Map

1. This detailed Site-Specific Soil Map conforms to the standards of SSSNNE Publication No. 3, as amended, "Site-Specific Soil Mapping Standards for NH and VT".
2. This map has been prepared to comply with soil mapping requirements of RSA 485 A: 17 and NHDES Env-Wq 1500, Alteration of Terrain.
3. See accompanying narrative report for methodology, map symbol legend, and interpretations.

## Map Symbol Denominators for Disturbed Unit Supplements

The map symbols for Site-Specific Soil Mapping of disturbed soils in New Hampshire is a two part symbol with parts separated by a forward slash (/).

The first part consists of the USDA-NRCS Disturbed Map Unit symbol from the NH State-Wide Numerical Soil Legend. The map symbol is composed of 1 to 3 digits followed by a capital letter designating slope.

The second part consists of symbols of the SSSNNE NH Disturbed Soil Supplement to the Site Specific Soil Survey Standards, as detailed below. The disturbed map symbol is composed of 5 lower case letters.

Thus a Site Specific map symbol for a map prepared for an AoT application would be formatted as follows:

*400A/aaaaa*

These SSSNNE NH Disturbed Soil Supplemental symbols can only be used in conjunction with the USDA-NRCS Disturbed Map Unit symbols for the NH Statewide Numerical Soil Legend.

## Supplemental Symbols

The five components of the Disturbed Soil Mapping Unit Supplement are as follows:

### Symbol 1: Drainage Class

- a-Excessively Drained
- b-Somewhat Excessively Drained
- c-Well Drained
- d-Moderately Well Drained
- e-Somewhat Poorly Drained
- f-Poorly Drained
- g-Very Poorly Drained
- h-Not Determined

### Symbol 2: Parent Material (of naturally formed soil only, if present)

- a-No natural soil within 60"
- b-Glaciofluvial Deposits (outwash/terraces of sand or sand and gravel)
- c-Glacial Till Material (active ice)
- d-Glaciolacustrine very fine sand and silt deposits (glacial lakes)
- e-Loamy/sandy over Silt/Clay deposits
- f-Marine Silt and Clay deposits (ocean waters)
- g-Alluvial Deposits (floodplains)
- h-Organic Materials-Fresh water Bogs, etc
- i- Organic Materials-Tidal Marsh

### Symbol 3: Restrictive/Impervious Layers

- a-None
- b-Bouldery surface with more than 15% of the surface covered with boulders
- c-Mineral restrictive layer(s) are present in the soil profile less than 40 inches below the soil surface such as hard pan, platy structure or clayey texture with consistence of at least firm ( i.e. more than 20 newtons). For other examples of soil characteristics that qualify for restrictive layers, see "Soil Manual for Site evaluations in NH" 2<sup>nd</sup> Ed., (page 3-17, figure 3-14)
- d-Bedrock in the soil profile; 0-20 inches
- e-Bedrock in the soil profile; 20-60 inches
- f-Areas where depth to bedrock is so variable that a single soil type cannot be applied, will be mapped as a complex of soil types
- g-Subject to Flooding
- h-Man-made impervious surface including pavement, concrete, or built-up surfaces (i.e. buildings) with no morphological restrictive layer within control section

**Symbol 4: Estimated Ksat\* (most limiting layer excluding symbol 3h above).**

a- High.

b-Moderate

c-Low

d-Not determined

\*See "Guidelines for Ksat Class Placement" in Chapter 3 of the Soil Survey Manual, USDA

**Symbol 5: Hydrologic Soil Group\***

a-Group A

b-Group B

c-Group C

d-Group D

e-Not determined

\*excluding man-made surface impervious/restrictive layers

## Disturbed Map Units

This edition of the New Hampshire State-Wide Numerical Soil Legend contains eleven distinct map units used for identifying areas of soils altered or disturbed by human influence and the addition of one naturally formed map unit. These map units were designed for the Order 2 and Order 3 levels of mapping intensity, but can be used in Order 1 mapping if appropriate.

The definition of disturbed map units is intentionally brief and vague. Classification at the Great Group level allows for a wide range in soil properties and behavioral characteristics. The variability in soil properties typically requires on-site investigations before any interpretation can be developed. The map unit descriptions are intended to provide guidance in differentiating map units. The author of the soil map is expected to provide additional information to reflect the nature of the disturbed areas within the survey area.

### I. Excavated land

#### **300 Udipsamments**

This map unit is characterized by soil textures of loamy fine sand to sand and gravel throughout the entire particle-size class control section (25 - 100 cm or 10 - 40 inches). Saturated hydraulic conductivity ( $K_{sat}$ ) is high or very high. Drainage class ranges from excessively drained to well drained. The Hydrologic Soil Group (HSG) is A. Typical sand pit.

#### **350 Udipsamments, wet substratum**

This map unit is characterized by soil textures of loamy fine sand to sand and gravel throughout the entire particle-size class control section (25 - 100 cm or 10 - 40 inches). Saturated hydraulic conductivity ( $K_{sat}$ ) is high or very high. Drainage class ranges from moderately well drained to somewhat poorly drained.

#### **400 Udorthents, sandy or gravelly**

This map unit typically includes the following concepts: 1) very gravelly (> 35%) sand or very gravelly loamy sand; Or 2) sand or loamy sand textures that may have lenses of loamy very fine sand or finer somewhere in the particle-size class control section (25 - 100 cm or 10 - 40"). Saturated hydraulic conductivity ( $K_{sat}$ ) is high or very high. Drainage class ranges from excessively drained to somewhat poorly drained. Typical gravel pit.

## Disturbed Map Units (continued)

### **500 Udorthents, loamy**

This map unit is characterized typically by soil textures that are sandy loam, loam, or silt loam within the particle size control section (25 – 100cm or 10 – 40”). Saturated hydraulic conductivity ( $K_{sat}$ ) is low through high. Drainage class ranges from well drained to somewhat poorly drained. These areas typically represent excavated glacial till or perhaps areas where sand and gravel was excavated down to the loamy underlying material.

### **550 Udorthents, Bedrock substratum**

This map unit is characterized by soil textures of sandy loam, loam, or silt loam within the particle-size class control section (25 - 100 cm or 10 - 40 inches). These areas typically represent excavated soil materials where the range in depth to bedrock is 10 - 60 inches (25 - 152 cm). Saturated hydraulic conductivity ( $K_{sat}$ ) is low through high. Drainage class ranges from somewhat excessively drained to somewhat poorly drained.

### **600 Endoaquents, loamy**

This map unit represents areas where soil material was excavated down to, or near the water table. Soil material is typically sandy loam, loam or silt loam within the particle-size class control section (25 - 100 cm or 10 - 40 inches). Saturated hydraulic conductivity ( $K_{sat}$ ) is low through high. Drainage class is poorly or very poorly drained. The Hydrologic Soil Group (HSG) is D.

### **900 Endoaquents, sandy or gravelly**

This map unit represents areas where soil material was excavated down to / near the water table. This map unit is characterized typically by soil textures of: 1) very gravelly (> 35% gravel) sand or very gravelly loamy sand or; 2) sand or loamy sand textures that may have lenses of loamy very fine sand or finer somewhere in the particle-size class control section (25 - 100 cm or 10 - 40”). Saturated hydraulic conductivity ( $K_{sat}$ ) is high or very high. Drainage class is poorly or very poorly drained. The Hydrologic Soil Group (HSG) is D. Typical gravel pit dug down to or close to the water table.

## **Disturbed Map Units** (continued)

### **II. Filled land**

#### **100 Udorthents, wet substratum**

This map unit represents areas that have been filled and leveled over what were originally hydric soils.

#### **199 Dumps, bark chips, and organic material**

This map unit consists of man-made deposits of bark, wood chips, sawdust, paper mill sludge, cinders, waste paper, ashes, and other similar refuse from the operation of paper mills and sawmills.

#### **200 Udorthents, refuse substratum**

This map unit represents alternating layers of soil and refuse such as in sanitary landfills. Closed landfills typically have 2 feet of loamy material capping the area.

#### **299 Udorthents, smoothed**

This map unit represents areas that have been cut and filled to create a large level or nearly level area. Soil material making up the map units typically came from the immediate area. School athletic fields are an example (unless they were created on hydric soils – see Map Unit 100).

### **III. Bottom Land**

#### **7 Fluvaquents**

This map unit represents areas of various kinds of soil materials on the bottom lands of streams and rivers. The soil material ranges in texture from silt loam to sand and gravel within the particle-size class control section (25 - 100 cm or 10 - 40 inches). Drainage class is poorly or very poorly drained. The Hydrologic Soil Group (HSG) is D.



**John P. Hayes III CSS, CWS**  
**7 Limestone Way**  
**North Hampton, NH 03862**  
**Phone:603-205-4396**  
**johnphayes@comcast.net**

**Job# 23-006**

**Test Pit Logs 1/22/24**  
**Map 238 Lot 44-1**  
**607 Calef Highway Barrington NH**

**Test Pit 206**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-4	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
4-14	10YR 5/6 Yellowish Brown	Loamy Sand	Granular	Friable
14-20	2.5Y 6/4 Light Yellowish Brown	Loamy Sand	Granular	Friable
20-28	2.5Y 6/3 Light Yellowish Brown	Sand with Redoximorphic features present	Single Grain	Loose
28-62	2.5YR 4/4 Reddish Brown	Gravelly Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: 20 in. Restrictive Layer : None Observed H2O: 32 in. Refusal: None**

**Test Pit 207**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-4	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
4-16	7.5YR 5/6 Strong Brown	Loamy Sand	Granular	Friable
16-30	10YR 5/6 Yellowish Brown	Loamy Sand	Granular	Friable
30-40	10YR 6/4 Light Yellowish Brown	Sand	Single Grain	Loose
40-65	2.5Y 6/3 Light Yellowish Brown	Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: 40 in. Restrictive Layer : None Observed H2O: None Refusal: None**

**John P. Hayes III CSS, CWS**  
**7 Limestone Way**  
**North Hampton, NH 03862**  
**Phone:603-205-4396**  
**johnphayes@comcast.net**

**Job# 23-006**

**Test Pit Logs 1/22/24**  
**Map 238 Lot 44-1**  
**607 Calef Highway Barrington NH**

**Test Pit 208**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-10	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
10-18	10YR 5/6 Yellowish Brown	Loamy Sand	Granular	Friable
18-36	2.5Y 6/4 Light Yellowish Brown	Loamy Sand	Granular	Friable
36-65	2.5YR 4/4 Reddish Brown	Gravelly Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: 36 in. Restrictive Layer : None Observed H2O: None Refusal: None**

**Test Pit 209**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-10	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
10-18	10YR 5/6 Yellowish Brown	Loamy Sand	Granular	Friable
18-34	2.5Y 6/4 Light Yellowish Brown	Loamy Sand	Granular	Friable
34-65	2.5YR 4/4 Reddish Brown	Gravelly Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: 34 in. Restrictive Layer : None Observed H2O: None Refusal: None**

**John P. Hayes III CSS, CWS**  
**7 Limestone Way**  
**North Hampton, NH 03862**  
**Phone:603-205-4396**  
**johnphayes@comcast.net**

**Job# 23-006**

**Test Pit Logs 1/22/24**  
**Map 238 Lot 44-1**  
**607 Calef Highway Barrington NH**

**Test Pit 210**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistence
0-8	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
8-14	7.5YR 5/6 Strong Brown	Loamy Sand	Granular	Friable
14-24	10YR 5/6 Yellowish Brown	Loamy Sand	Granular	Friable
24-32	10YR 6/4 Light Yellowish Brown	Sand	Single Grain	Loose
32-64	2.5Y 6/3 Light Yellowish Brown	Gravelly Sand with Redoximorphic features present	Single Grain	Loose

**ESHW: 32 in. Restrictive Layer : None Observed H2O: None Refusal: None**

**Test Pit 211**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistence
0-8	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
8-14	10YR 5/6 Yellowish Brown	Loamy Sand	Granular	Friable
14-22	10YR 6/4 Light Yellowish Brown	Loamy Sand	Granular	Friable
22-60	2.5Y 6/3 Light Yellowish Brown	Sand with Redoximorphic features present	Single Grain	Loose

**ESHW: 22 in. Restrictive Layer : None Observed H2O: 32 in. Refusal: None**

**John P. Hayes III CSS, CWS**  
**7 Limestone Way**  
**North Hampton, NH 03862**  
**Phone:603-205-4396**  
**johnphayes@comcast.net**

**Job# 23-006**

**Test Pit Logs 1/22/24**  
**Map 238 Lot 44-1**  
**607 Calef Highway Barrington NH**

**Test Pit 212**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-8	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
8-16	7.5YR 5/6 Strong Brown	Loamy Sand	Granular	Friable
16-28	10YR 5/6 Yellowish Brown	Loamy Sand	Granular	Friable
26-38	10YR 6/4 Light Yellowish Brown	Sand	Single Grain	Loose
38-60	2.5Y 6/3 Light Yellowish Brown	Gravelly Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: 38 in. Restrictive Layer : None Observed H2O: None Refusal: None**

**Test Pit 213**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-10	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
10-24	10 YR 5/4 Yellowish Brown	Gravelly Sand	Single Grain	Loose
24-42	10YR 3/1 Very Dark Gray	Loamy Fine Sand	Massive	Friable
42-52	2.5Y 6/3 Light Yellowish Brown	Gravelly Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: None to 52 in. Restrictive Layer : None Observed H2O: None Refusal: None**

**Note: 0 to 24 inches is fill material. Some concrete in fill material 10 to 24 in.**

**John P. Hayes III CSS, CWS**  
**7 Limestone Way**  
**North Hampton, NH 03862**  
**Phone:603-205-4396**  
**johnphayes@comcast.net**

**Job# 23-006**

**Test Pit Logs 1/22/24**  
**Map 238 Lot 44-1**

**Test Pit 214**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistence
0-8	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
8-28	10YR 6/4 Light Yellowish Brown	Loamy Sand	Granular	Friable
28-36	10YR 3/1 Very Dark Gray	Loamy Fine Sand	Granular	Friable
36-50	2.5Y 6/3 Light Yellowish Brown	Sand and Fine Sand with Redoximorphic features present	Massive	Friable

**ESHW: None to 36 in. Restrictive Layer : None Observed H2O: None Refusal: None**

**John P. Hayes III CSS, CWS**  
**7 Limestone Way**  
**North Hampton, NH 03862**  
**Phone:603-205-4396**  
**johnphayes@comcast.net**

**Job# 23-006**

**Test Pit Logs 3/7/24**  
**Map 238 Lot 44-1**

**Test Pit 301**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-8	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
8-18	10YR 5/6 Yellowish Brown	Gravelly Loamy Sand	Granular	Friable
18-30	10YR 6/4 Light Yellowish Brown	Gravelly Loamy Sand	Granular	Friable
30-44	10YR 6/4 Light Yellowish Brown	Sand	Single Grain	Loose
44-52	2.5Y 5/3 Light Yellowish Brown	Gravelly Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: 44 in. Restrictive Layer : None Observed H2O: None Refusal: None**

**Test Pit 302**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-8	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
8-18	10YR 5/6 Yellowish Brown	Gravelly Loamy Sand	Granular	Friable
18-32	10YR 6/4 Light Yellowish Brown	Gravelly Loamy Sand	Granular	Friable
32-46	10YR 6/4 Light Yellowish Brown	Sand	Single Grain	Loose
46-52	2.5Y 5/3 Light Yellowish Brown	Gravelly Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: 46 in. Restrictive Layer : None Observed H2O: None Refusal: None**

**John P. Hayes III CSS, CWS**  
**7 Limestone Way**  
**North Hampton, NH 03862**  
**Phone:603-205-4396**  
**johnphayes@comcast.net**

**Job# 23-006**

**Test Pit Logs 3/7/24**  
**Map 238 Lot 44-1**

**Test Pit 303**

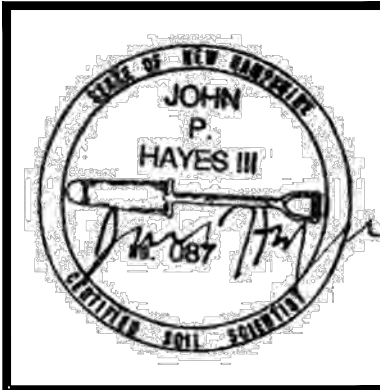
Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-8	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
8-14	7.5YR 5/6 Strong Brown	Loamy Sand	Granular	Friable
14-26	10YR 5/6 Yellowish Brown	Loamy Sand	Granular	Friable
26-32	10YR 6/4 Light Yellowish Brown	Sand	Single Grain	Loose
32-52	2.5Y 6/3 Light Yellowish Brown	Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: 32 in. Restrictive Layer : None Observed H2O: None Refusal: None**

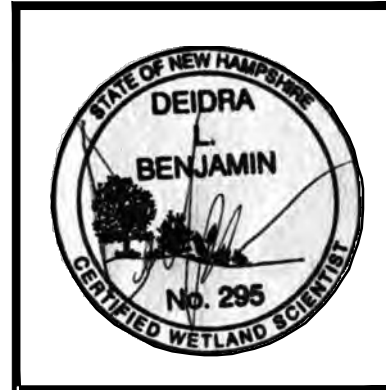
**Test Pit 304**

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-6	10YR 3/2 Dark Grayish Brown	Loamy Sand	Granular	Friable
6-14	7.5YR 5/6 Strong Brown	Loamy Sand	Granular	Friable
14-28	10YR 5/6 Yellowish Brown	Loamy Sand	Granular	Friable
28-42	10 YR 6/4 Yellowish Brown	Sand	Single Grain	Loose
42-52	2.5Y 6/3 Light Yellowish Brown	Gravelly Sand with Redoximorphic features present	Single Grain	Loose

**ESHWT: 42 in. Restrictive Layer : None Observed H2O: None Refusal: None**



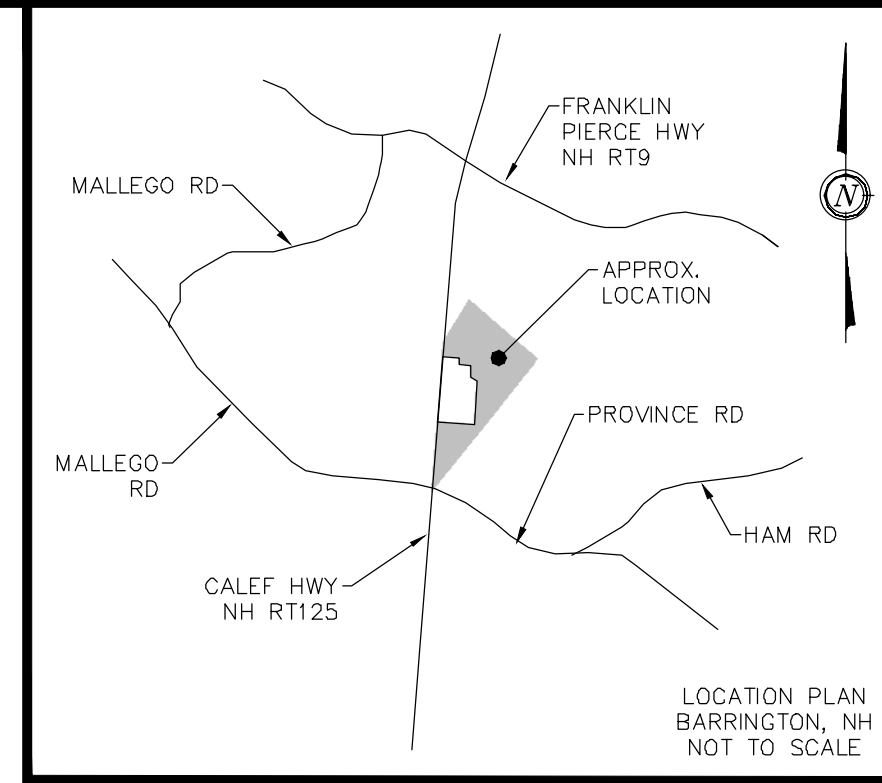
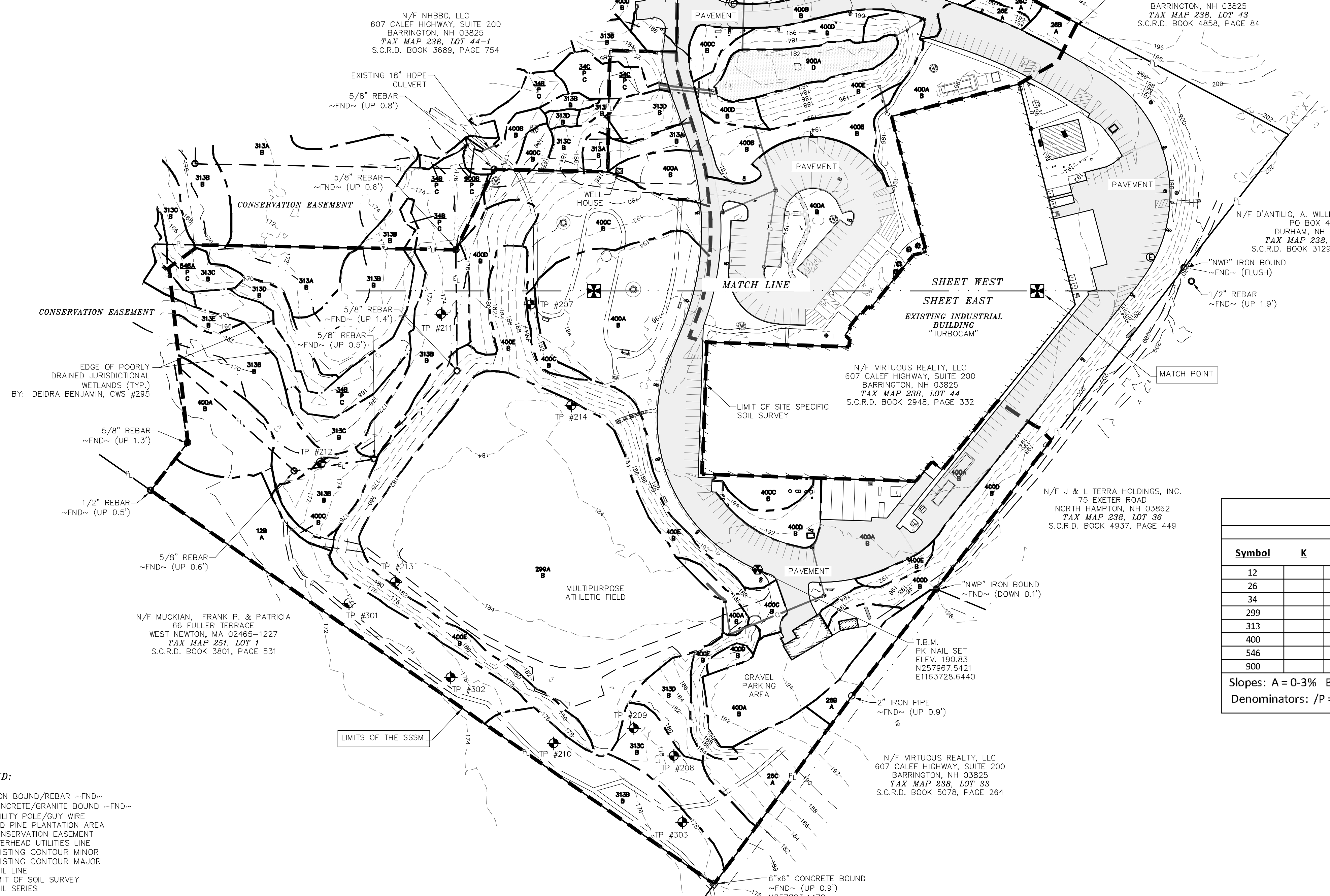
JOHN P. HAYES, III CSS #87



DEIDRA BENJAMIN, CWS #295

WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS, IN APRIL OF 2023 AND JANUARY OF 2024 UTILIZING THE FOLLOWING STANDARDS:

- 1) FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0. 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.). UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL COMMITTEE FOR HYDRIC SOILS.
- 2) FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, VERSION 3. APRIL 2004. NEWPPCC WETLANDS WORKGROUP. WILMINGTON, MA 01887.
- 3) NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, VERSION 2.1.0 (HTTP://WETLAND\_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD REGIONS RESEARCH AND ENGINEERING LABORATORY, HANOVER, NH, AND BONAP, CHAPEN HILL.
- 4) STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST, LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER, 2014. THE NATIONAL WETLAND PLANT LIST; 2014 UPDATE OF WETLAND RATINGS. PHYTONEURON 2014-41:1-42.
- 5) CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH PROGRAM TECHNICAL REPORT Y-87-1.
- 6) REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTHCENTRAL AND NORTHEAST REGION, JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS. ENVIRONMENTAL LABORATORY ERDC/EL TR-12-1.
- 7) CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES, DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. US DEPARTMENT OF THE INTERIOR. FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.



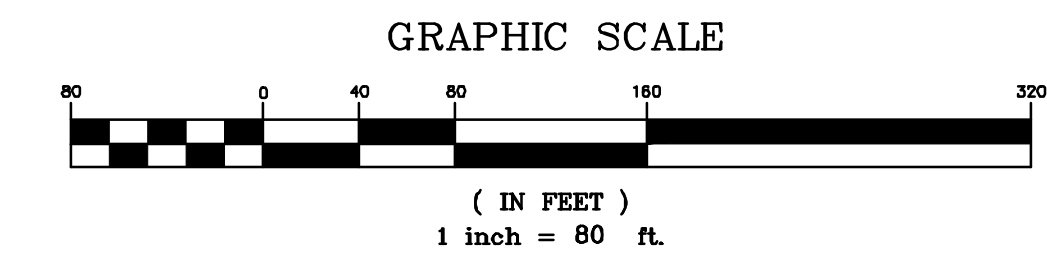
- NOTES:**
- 1.) OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 1A.) APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 2.) TAX MAP 238, LOT 44
  - 3.) LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
  - 4.) S.C.R.D. BOOK 2948, PAGE 332
  - 5.) I HEREBY CERTIFY THAT, TO THE BEST OF MY KNOWLEDGE & BELIEF, THIS PARCEL DOES NOT FALL WITHIN THE FLOOD PLAIN. FLOOD HAZARD REF.: FEMA, COMMUNITY# - 330178, MAP# - 33017C0305E, DATED: SEPT. 30, 2015.
  - 6.) VERTICAL DATUM BASED ON NAVD88 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
  - 7.) THE BOUNDARY LINES SHOWN ON THIS PLAN ARE THE RESULT OF A CLOSED TRAVERSE PERFORMED BY THIS OFFICE IN APRIL OF 2023, WITH AN ERROR OF CLOSURE OF BETTER THAN 1 PART IN 10,000.
  - 8.) THE INTENT OF THIS PLAN IS TO SHOW THE OVERVIEW OF THE SITE SPECIFIC SOIL SURVEY OF TAX MAP 238, LOT 44 AS OF THE DATE OF THE SURVEY (APRIL 2023 & JANUARY 2024).

**SOILS LEGEND**

Site Specific Soils Map 238 Lot 44

Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoaquents (Sandy or Gravelly)	D

Slopes: A=0-3% B=3-8% C=8-15% D=15-25% E=25-50% F=50%+  
Denominators: /P = Poorly Drained /VP = Very Poorly Drained /Rk = Rocky



FOR PLANNING BOARD APPROVAL PURPOSES:

THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

REVISION	DATE	DESCRIPTION

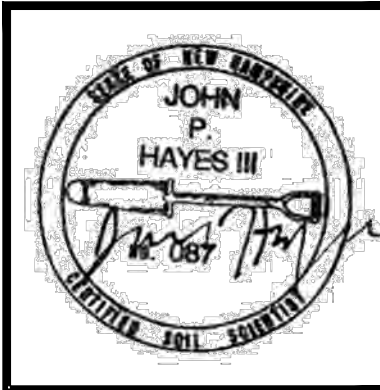
SITE SPECIFIC SOILS MAP - OVERVIEW

FOR TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

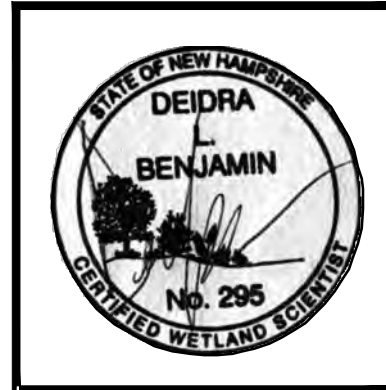
**BERRY SURVEYING & ENGINEERING**  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 80 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017

SIGNATURE





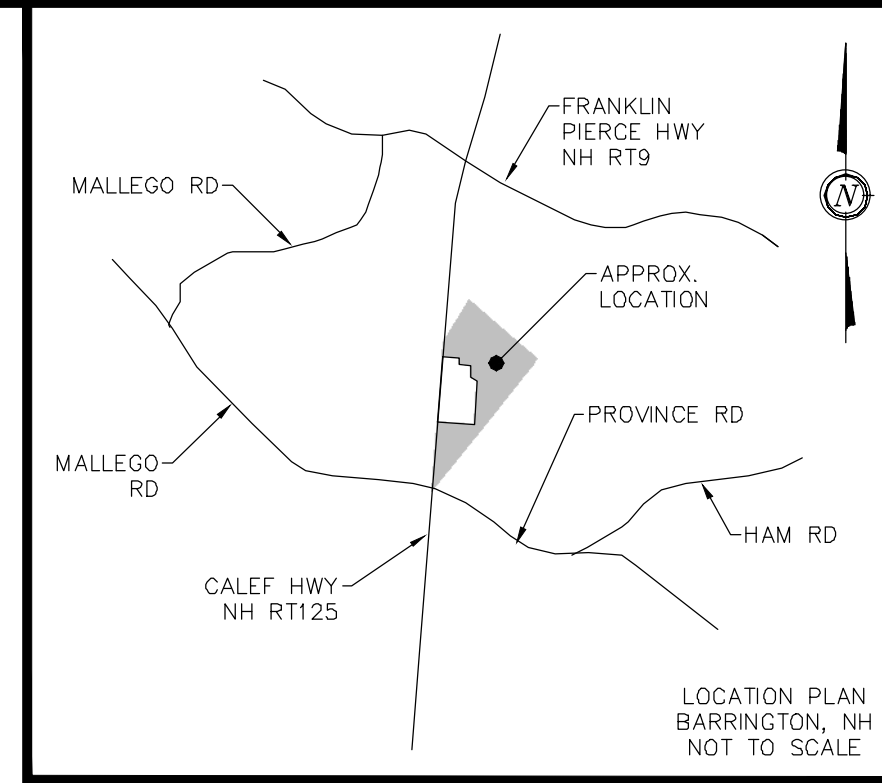
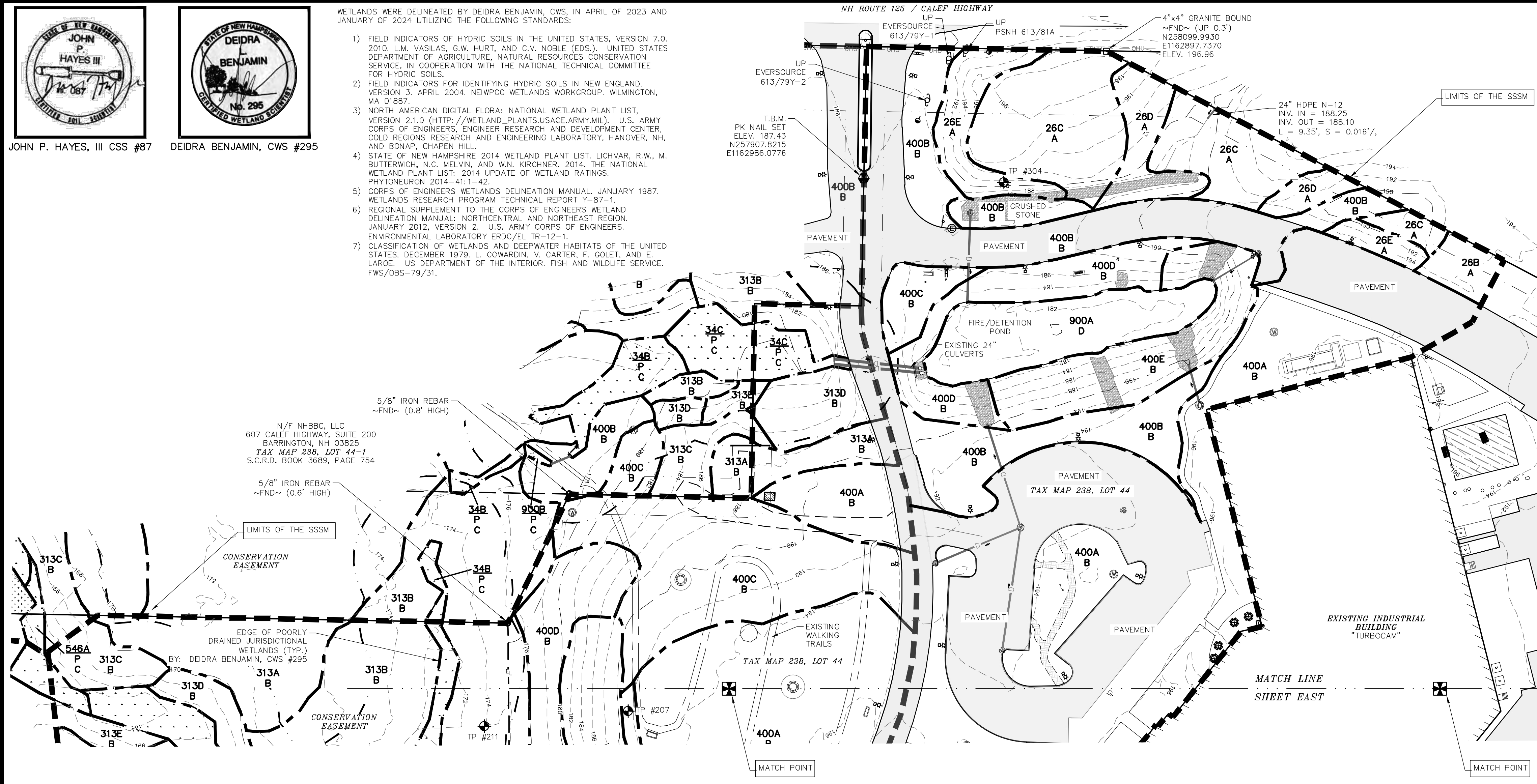
JOHN P. HAYES, III CSS #87



DEIDRA BENJAMIN, CWS #295

WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS, IN APRIL OF 2023 AND JANUARY OF 2024 UTILIZING THE FOLLOWING STANDARDS:

- 1) FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0. 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.). UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL COMMITTEE FOR HYDRIC SOILS.
- 2) FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND. VERSION 3. APRIL 2004. NEIWPCC WETLANDS WORKGROUP. WILMINGTON, MA. 01887.
- 3) NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, VERSION 2.1.0 (HTTP://WETLAND\_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD REGIONS RESEARCH AND ENGINEERING LABORATORY, HANOVER, NH, AND BONAP, CHAPEN HILL.
- 4) STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST, LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER, 2014. THE NATIONAL WETLAND PLANT LIST; 2014 UPDATE OF WETLAND RATINGS. PHYTONEURON 2014-41:1-42.
- 5) CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH PROGRAM TECHNICAL REPORT Y-87-1.
- 6) REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTHCENTRAL AND NORTHEAST REGION. JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS. ENVIRONMENTAL LABORATORY ERDC/EL TR-12-1.
- 7) CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES. DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. US DEPARTMENT OF THE INTERIOR. FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.

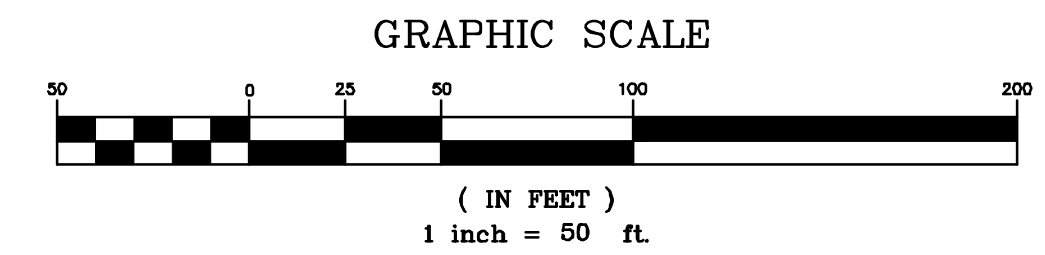


- NOTES:**
- 1.) OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 1A.) APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 2.) TAX MAP 238, LOT 44
  - 3.) LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
  - 4.) S.C.R.D. BOOK 2948, PAGE 332
  - 5.) I HEREBY CERTIFY THAT, TO THE BEST OF MY KNOWLEDGE & BELIEF, THIS PARCEL DOES NOT FALL WITHIN THE FLOOD PLAIN. FLOOD HAZARD REF.: FEMA, COMMUNITY# -330178, MAP# - 33017C0305E, DATED: SEPT. 30, 2015.
  - 6.) VERTICAL DATUM BASED ON NAVD88 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
  - 7.) THE BOUNDARY LINES SHOWN ON THIS PLAN ARE THE RESULT OF A CLOSED TRAVERSE PERFORMED BY THIS OFFICE IN APRIL OF 2023, WITH AN ERROR OF CLOSURE OF BETTER THAN 1 PART IN 10,000.
  - 8.) THE INTENT OF THIS PLAN IS TO SHOW DETAIL OF THE WEST SIDE OF THE SITE SPECIFIC SOIL SURVEY OF TAX MAP 238, LOT 44 AS OF THE DATE OF THE SURVEY (APRIL 2023 & JANUARY 2024).

- LEGEND:**
- IRON BOUND/REBAR ~FND~
  - CONCRETE/GRANITE BOUND ~FND~
  - UTILITY POLE/GUY WIRE
  - RED PINE PLANTATION AREA
  - CONSERVATION EASEMENT
  - OVERHEAD UTILITIES LINE
  - EXISTING CONTOUR MINOR
  - EXISTING CONTOUR MAJOR
  - SOIL LINE
  - LIMIT OF SOIL SURVEY
  - SOIL SERIES
  - 448A
  - TEST HOLE
  - BENCHMARK
  - S.C.R.D. TYP. FND
  - STRAFFORD COUNTY REGISTRY OF DEEDS TYPICAL FOUND
  - POORLY DRAINED JURISDICTIONAL WETLAND

SOILS LEGEND				
Site Specific Soils Map 238 Lot 44				
Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoaquents (Sandy or Gravelly)	D

Slopes: A = 0-3% B = 3-8% C = 8-15% D = 15-25% E = 25-50% F = 50%+  
Denominators: /P = Poorly Drained /VP = Very Poorly Drained /RK = Rocky



FOR PLANNING BOARD APPROVAL PURPOSES:

THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

REVISION	DATE	DESCRIPTION

SITE SPECIFIC SOILS MAP - WEST

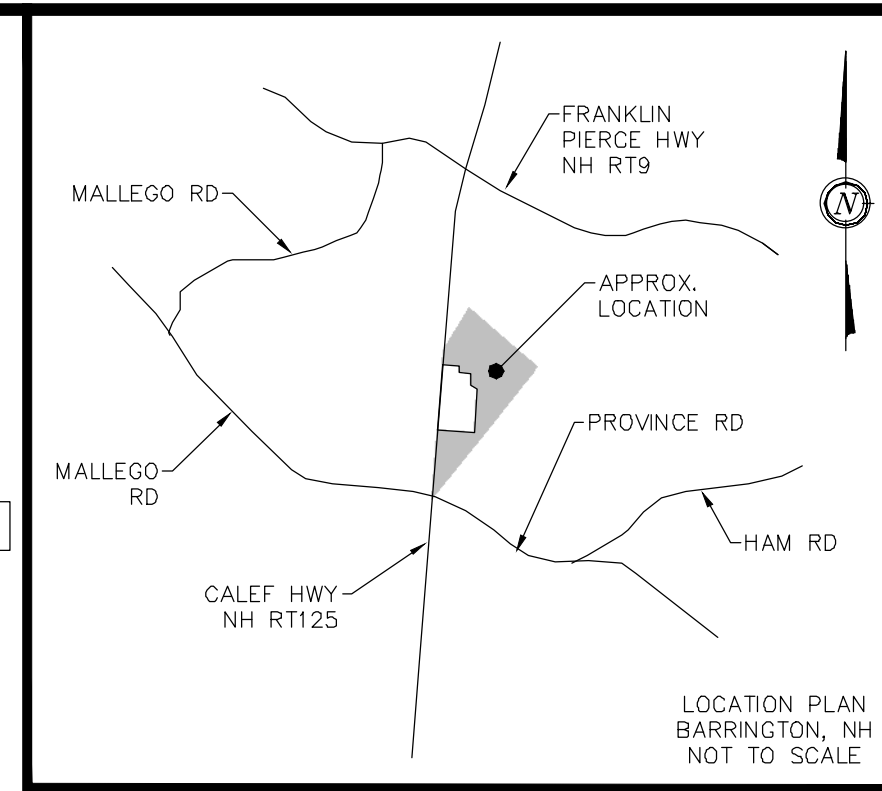
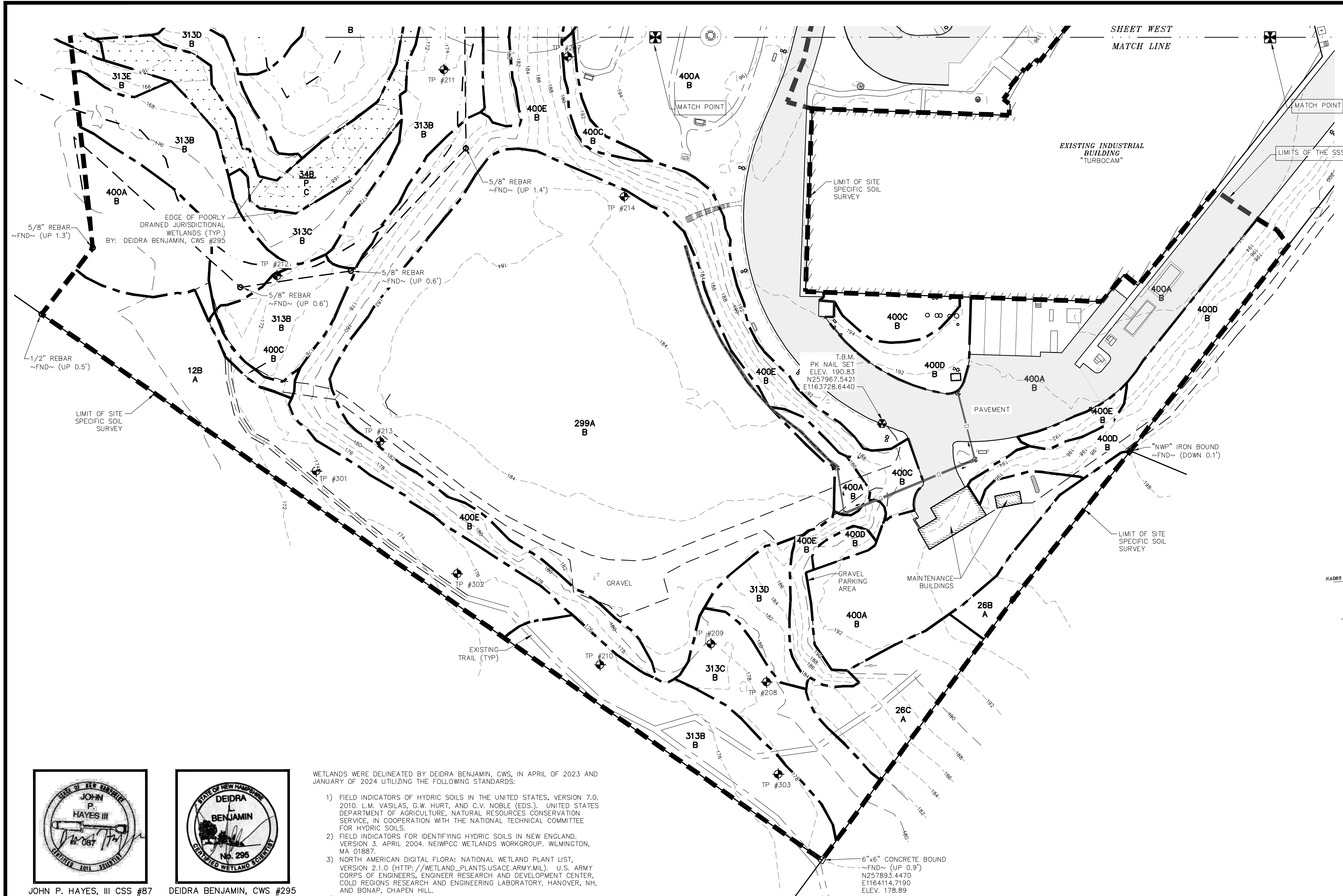
FOR TURBOCAM, INC.  
LAND OF VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

**BERRY SURVEYING & ENGINEERING**  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 50 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017

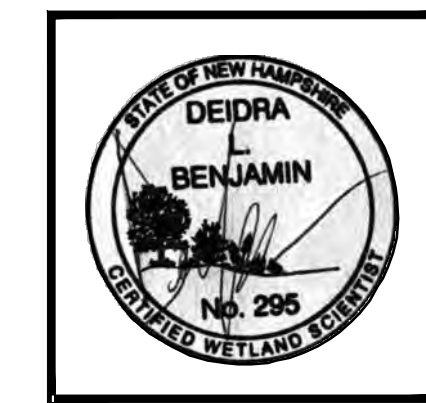
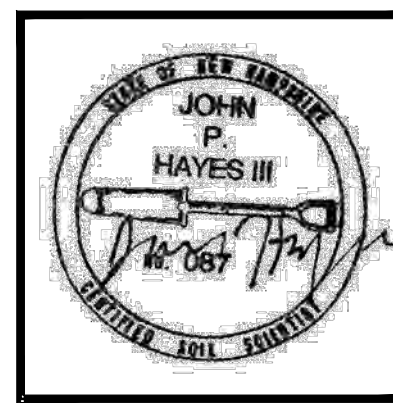
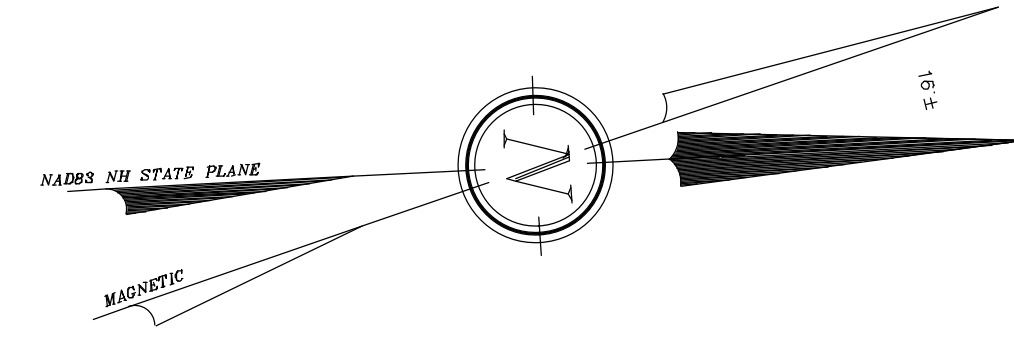
FOR PLANNING BOARD APPROVAL PURPOSES:

THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

SHEET 9 OF 43



- NOTES:**
- 1.) OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 1A.) APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 2.) TAX MAP 238, LOT 44
  - 3.) LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
  - 4.) S.C.R.D. BOOK 2948, PAGE 332
  - 5.) I HEREBY CERTIFY THAT, TO THE BEST OF MY KNOWLEDGE & BELIEF, THIS PARCEL DOES NOT FALL WITHIN THE FLOOD PLAIN FLOOD HAZARD REF.: FEMA, COMMUNITY# -330178, MAP# - 33017C0305E, DATED: SEPT. 30, 2015.
  - 6.) VERTICAL DATUM BASED ON NAVD88 ELEVATIONS. HORIZONTAL COORDINATES BASED ON NAD83. COORDINATES GATHERED USING CARLSON BRX7 SURVEY GRADE GPS RECEIVERS.
  - 7.) THE BOUNDARY LINES SHOWN ON THIS PLAN ARE THE RESULT OF A CLOSED TRAVERSE PERFORMED BY THIS OFFICE IN APRIL OF 2023, WITH AN ERROR OF CLOSURE OF BETTER THAN 1 PART IN 10,000.
  - 8.) THE INTENT OF THIS PLAN IS TO SHOW DETAIL OF THE EAST SIDE OF THE SITE SPECIFIC SOIL SURVEY OF TAX MAP 238, LOT 44 AS OF THE DATE OF THE SURVEY (APRIL 2023 & JANUARY 2024).



JOHN P. HAYES, III CWS #87 DEIDRA BENJAMIN, CWS #295

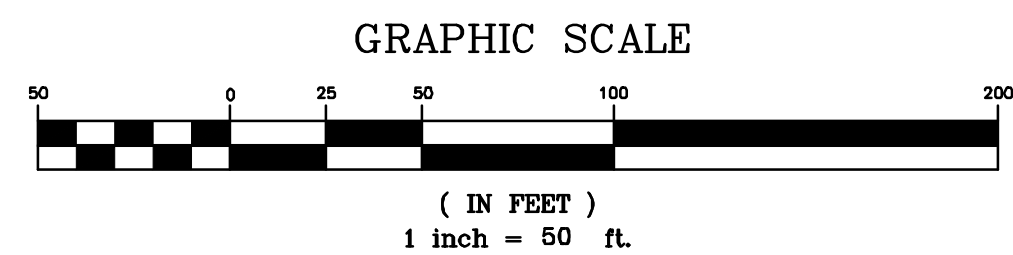
WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS, IN APRIL OF 2023 AND JANUARY OF 2024 UTILIZING THE FOLLOWING STANDARDS:

- 1) FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0. 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.). UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL COMMITTEE FOR HYDRIC SOILS.
- 2) FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND, VERSION 3. APRIL 2004. NEIWPCC WETLANDS WORKGROUP. WILMINGTON, MA 01887.
- 3) NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, VERSION 2.1.0 (HTTP://WETLAND\_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD REGIONS RESEARCH AND ENGINEERING LABORATORY, HANOVER, NH, AND BONAP, CHAPEN HILL.
- 4) STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST. LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER. 2014. THE NATIONAL WETLAND PLANT LIST: 2014 UPDATE OF WETLAND RATINGS. PHYTONEURON 2014-41:1-42.
- 5) CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH PROGRAM TECHNICAL REPORT Y-87-1.
- 6) REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTHCENTRAL AND NORTHEAST REGION. JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS. ENVIRONMENTAL LABORATORY ERDC/EL TR-12-1.
- 7) CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES. DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. US DEPARTMENT OF THE INTERIOR. FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.

SOILS LEGEND				
Site Specific Soils Map 238 Lot 44				
Symbol	K	Slopes	Taxonomic Soil Name	Hydrologic Soil Group
12		B	Hinckley	A
26		A,B,C,D,E	Windsor	A
34		B	Wareham	C
299		A	Udorthents (Smoothed)	B
313		A,B,C,D,E	Deerfield	B
400		A,C,D,E	Udorthents	B
546		A	Walpole	C
900		A	Endoaquents (Sandy or Gravelly)	D

Slopes: A = 0-3% B = 3-8% C = 8-15% D = 15-25% E = 25- 50% F = 50%+  
Denominators: /P = Poorly Drained /VP = Very Poorly Drained /Rk = Rocky

- LEGEND:**
- IRON BOUND/REBAR ~FND~
  - CONCRETE/GRANITE BOUND ~FND~
  - UTILITY POLE/GUY WIRE
  - RED PINE PLANTATION AREA
  - CONSERVATION EASEMENT
  - OVERHEAD UTILITIES LINE
  - EXISTING CONTOUR MINOR
  - EXISTING CONTOUR MAJOR
  - SOIL LINE
  - LIMIT OF SOIL SURVEY
  - SOIL SERIES
  - 448A
  - ⊕ TEST HOLE
  - ⊕ BENCHMARK
  - S.C.R.D. STRAFFORD COUNTY REGISTRY OF DEEDS
  - TYP. TYPICAL
  - FND FOUND
  - POORLY DRAINED JURISDICTIONAL WETLAND



FOR PLANNING BOARD APPROVAL PURPOSES:

THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

REVISION	DATE	DESCRIPTION

SITE SPECIFIC SOILS MAP - EAST

FOR TURBOCAM, INC. LAND OF VIRTUOUS REALTY, LLC NH ROUTE 125/CALEF HIGHWAY BARRINGTON, N.H. TAX MAP 238, LOT 44

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 50 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017

**TEST PIT DATA:**

DATE : 1/22/24

**TEST PIT #207**

0-4 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 4-16 7.5YR 5/6 STRONG BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 16-30 10YR 5/6 YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 30-40 10YR 6/4 LIGHT YELLOWISH BROWN, SAND, SINGLE GRAIN, LOOSE  
 40-65 2.5Y 6/3 LIGHT YELLOWISH BROWN, SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 40"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A"  
 TERMINATED @ 65"  
 REFUSAL @ N/A

**TEST PIT #208**

0-10 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 10-18 10YR 5/6 YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 18-36 2.5Y 6/4 LIGHT YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 36-65 2.5YR 4/4 REDDISH BROWN, GRAVELLY SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 36"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 65"  
 REFUSAL @ N/A

**TEST PIT #209**

0-10 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 10-18 10YR 5/6 YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 18-34 2.5Y 6/4 LIGHT YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 34-65 2.5YR 4/4 REDDISH BROWN, GRAVELLY SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 34"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 65"  
 REFUSAL @ N/A

**TEST PIT #210**

0-8 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 8-14 7.5YR 5/6 STRONG BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 14-24 10YR 5/6 YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 24-32 10YR 6/4 LIGHT YELLOWISH BROWN, SAND, SINGLE GRAIN, LOOSE  
 32-64 2.5Y 6/3 LIGHT YELLOWISH BROWN, GRAVELLY SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 32"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 64"  
 REFUSAL @ N/A

**TEST PIT #211**

0-8 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 8-14 10YR 5/6 YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 14-22 10YR 6/4 LIGHT YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 22-60 2.5Y 6/3 LIGHT YELLOWISH BROWN, SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 22"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ 32"  
 TERMINATED @ 60"  
 REFUSAL @ N/A

**TEST PIT #212**

0-8 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 8-16 7.5YR 5/6 STRONG BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 16-28 10YR 5/6 YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 28-38 10YR 6/4 LIGHT YELLOWISH BROWN, SAND, SINGLE GRAIN, LOOSE  
 38-60 2.5Y 6/3 LIGHT YELLOWISH BROWN, GRAVELLY SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 38"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 60"  
 REFUSAL @ N/A

**TEST PIT #213**

0-10 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 10-24 10 YR 5/4 YELLOWISH BROWN, GRAVELLY SAND, SINGLE GRAIN, LOOSE  
 24-42 10YR 3/1 VERY DARK GRAY, LOAMY FINE SAND, MASSIVE, FRIABLE  
 42-52 2.5Y 6/3 LIGHT YELLOWISH BROWN, GRAVELLY SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 42"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 52"  
 REFUSAL @ N/A  
 NOTE: 0-24" IS FILL MATERIAL. SOME CONCRETE IN FILL MATERIAL 10-24"

**TEST PIT #214**

0-8 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 8-28 10YR 6/4 LIGHT YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 28-36 10YR 3/1 VERY DARK GRAY, LOAMY FINE SAND, GRANULAR, FRIABLE  
 36-50 2.5Y 6/3 LIGHT YELLOWISH BROWN, SAND AND FINE SAND WITH REDOX. FEAT. PRESENT, MASSIVE, FRIABLE  
 E.S.H.W.T. @ 36"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A"  
 TERMINATED @ 50"  
 REFUSAL @ N/A

**TEST PIT DATA:**

DATE : 3/7/24

**TEST PIT #301**

0-8 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 8-18 10YR 5/6 YELLOWISH BROWN, GRAVELLY LOAMY SAND, GRANULAR, FRIABLE  
 18-30 10YR 6/4 LIGHT YELLOWISH BROWN, GRAVELLY LOAMY SAND, GRANULAR, FRIABLE  
 30-44 10YR 6/4 LIGHT YELLOWISH BROWN, SAND, SINGLE GRAIN, LOOSE  
 44-52 2.5YR 5/3 LIGHT YELLOWISH BROWN, GRAVELLY SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 44"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 52"  
 REFUSAL @ N/A

**TEST PIT #302**

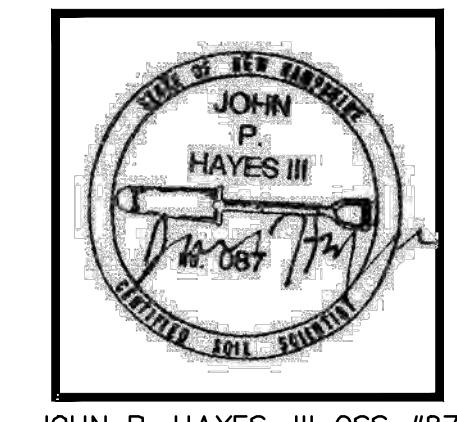
0-8 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 8-18 10YR 5/6 YELLOWISH BROWN, GRAVELLY LOAMY SAND, GRANULAR, FRIABLE  
 18-32 10YR 6/4 LIGHT YELLOWISH BROWN, GRAVELLY LOAMY SAND, GRANULAR, FRIABLE  
 32-46 10YR 6/4 LIGHT YELLOWISH BROWN, SAND, SINGLE GRAIN, LOOSE  
 46-52 2.5YR 5/3 LIGHT YELLOWISH BROWN, GRAVELLY SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 46"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 52"  
 REFUSAL @ N/A

**TEST PIT #303**

0-8 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 8-14 7.5YR 5/6 STRONG BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 14-26 10YR 5/6 YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 26-32 10YR 6/4 LIGHT YELLOWISH BROWN, SAND, SINGLE GRAIN, LOOSE  
 32-52 2.5YR 6/3 LIGHT YELLOWISH BROWN, SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 32"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 52"  
 REFUSAL @ N/A

**TEST PIT #304**

0-6 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 6-14 7.5YR 5/6 STRONG BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 14-28 10YR 5/6 YELLOWISH BROWN, LOAMY SAND, GRANULAR, FRIABLE  
 28-42 10YR 6/4 LIGHT YELLOWISH BROWN, SAND, SINGLE GRAIN, LOOSE  
 42-52 2.5YR 6/3 LIGHT YELLOWISH BROWN, SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE  
 E.S.H.W.T. @ 42"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 52"  
 REFUSAL @ N/A



JOHN P. HAYES, III CSS #87

FOR PLANNING BOARD APPROVAL PURPOSES:

THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

REVISION	DATE	DESCRIPTION

TEST PIT DATA

FOR TUREOCAM, INC.  
 LAND OF VIRTUOUS REALTY, LLC  
 NH ROUTE 125/CALEF HIGHWAY  
 BARRINGTON, N.H.  
 TAX MAP 286, LOT 44

BERRY SURVEYING & ENGINEERING  
 335 SECOND CROWN POINT ROAD  
 BARRINGTON, NH 03825 (603)332-2863  
 SCALE : NONE  
 DATE : APRIL 17, 2024  
 FILE NO. : DB 2023 - 017

STATE OF NEW HAMPSHIRE  
 JOHN P. HAYES III  
 LICENSED PROFESSIONAL ENGINEER  
 NO. 805  
 KENNETH A. BERRY  
 SIGNATURE



## **BERRY SURVEYING & ENGINEERING**

335 Second Crown Point Road

Barrington, NH 03825

Phone: (603) 332-2863

Fax: (603) 335-4623

www.BerrySurveying.Com

# **Stormwater System Management: Inspection and Maintenance Manual**

**607 Calef Highway  
Barrington, NH  
Tax Map 238, Lot 44**

Prepared for

TURBOCAM, INC.  
607 Calef Highway Suite 200  
Barrington, NH 03825

Land of

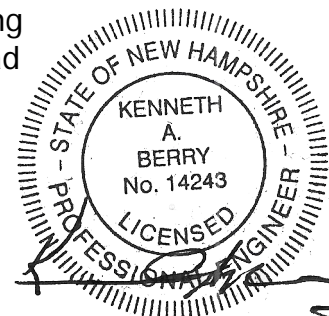
Virtuous Realty, LLC  
607 Calef Highway Suite 200  
Barrington, NH 03825

Prepared By

Berry Surveying & Engineering  
335 Second Crown Point Road  
Barrington, NH 03825  
603-332-2863

File Number  
DB2023-017

February 5, 2024  
Revised: April 17, 2024



## Table of Contents

Introduction w/ Practice Inventory .....	Page 2
Catch Basins.....	Page 4
Conveyance & Treatment Swales w/ Check Dams.....	Page 4
Sediment Forebays.....	Page 5
Bioretention W/ ISR .....	Page 5
Detention Basins .....	Page 6
Infiltration Basins.....	Page 7
Outlet Protection & Level Spreader .....	Page 8
Stabilization for Long Term Cover.....	Page 8
Control of Invasive Species .....	Page 10
Snow Removal and Winter Maintenance.....	Page 10
Annual Report.....	Page 11
Inspection & Maintenance Manual Checklist.....	Page 12
Inspection & Maintenance Log Sheet .....	Page 15
Deicing Log Sheet .....	Page 16
Owner Certification .....	Page 17
Inspection & Maintenance Plan	Attached – 2 Pages
Stormwater Practice Design Plans	Attached – 6 Pages
Control of Invasive Plants, NH Department of Agriculture	Attached – 4 Pages
NHDES Green SnoPro Utilization Chart	Attached – 1 Page

## Introduction

The Best Management Practices (BMP) described in this manual are specified in more detail within the plan set giving design details and specifications. The New Hampshire Stormwater Manual, Volume 2, Post-Construction Best Management Practices Selection & Design (December 2008, NHDES & US EPA) is included by reference to this manual. Additional details, construction specifications, and example drawings are provided within this reference. (<http://des.nh.gov/organization/divisions/water/stormwater/>)

The BMP's are covered below in the general order in which the storm water flows. Each BMP has a description and maintenance consideration listed. A Check List table is provided after the narrative to summarize the maintenance responsibilities and schedule. A Log Form is also provided for the owners use.

For details regarding the design of the Storm Water System see also Drainage Analysis & Sediment and Erosion Control Plan also published by Berry Surveying & Engineering originally dated April 17, 2024, as revised. See also plan set completed for TURBOCAM, INC. originally dated April 17, 2024, as revised.

Andrew Knapp, Director of EHS, Facilities & Maintenance, or his successor, is responsible for the Stormwater System Operation and Maintenance. A significant step in this responsibility is the Inspection and Maintenance of each component of the system. Ongoing, semi-annual, and annual inspection and maintenance requirement are documented below and must be taken seriously. Failure of any component of the system can result in surface water run-off ponding and/or freezing in the roadway and parking lots, leaving the developed site untreated, and/or causing violations to issued permits. The owner must maintain, and have available, plans of the Stormwater System in order to properly inspect and maintain the system. (Reduced copies attached.) The Director of EHS, Facilities & Maintenance, Andrew Knapp, or his successor / operator, is responsible to ensure that any subsequent owner or subcontractor has copies of the Log Form and Annual Report records and fully understands the responsibilities of this plan. The grantor owner will ensure this document is provided to the grantee owner by duplicating the Ownership Responsibility Sheet which is found toward the back of this document, which will be maintained with the Inspection & Maintenance Logs, provided to the Town of Barrington, Planning Department, with the Annual Report.

The operator of Tax Map 238, Lot 44, TURBOCAM, INC., and owner of the property, Virtuous Realty, LLC, are proposing the improvement of the parcel with two parking areas and an outdoor function area. Surface water runoff is being managed and treated by two Bioretention W/ ISRs, an infiltration pond, and a detention pond.

The following drainage features will all require periodic inspections and maintenance based on this manual:

Catch Basins #41-#47 (Ponds #C41-#C47)

Drain Manholes #51-#53 (Ponds #D51-#D53)

Inlet Sump (Pond #50)

Conveyance Swales and Roadside Ditches

Bioretention W/ ISR #201 – P-201 w/ Outlet Structures and Matted Spillway

Bioretention W/ ISR #202 – P-202 w/ Outlet Structures and Matted Spillway

Infiltration Pond #203 – P-203 w/ Matted Spillway

Detention Pond #204 – P-204 w/ Outlet Structure and Matted Spillway

Outlet Protection and Level Spreaders

## **Catch Basins (Without Sumps) & Drain Manholes**

Description: Catch Basins are used throughout the site to capture and, along with culvert pipes and manhole, route surface water runoff to stormwater treatment and detention infrastructure. During construction the catch basins will be protected by inlet protection per the approved construction plans. The practice of street sweeping on a bi-annual basis will help reduce maintenance of these catch basins and culvert pipes.

Note: Deep sump catch basins are not allowed to be used on this proposed development due to wildlife concerns and any manufacturer sump resulting in a catch basin must be filled with washed crushed stone. Sediment should be trapped in the sediment forebays but is also a concern in earlier structures. See construction details for specifications of these conveyance practices.

Maintenance Considerations: Sediment must be removed from Catch Basins and Manholes on a regular basis, at least twice a year and more often if post-winter maintenance and street sweeping is not conducted. Inspections should be conducted periodically. At a minimum they should be cleaned after snow-melt and after leaf-drop. Disposal of all material, sediment, and debris must be done in accordance with state and federal regulations. Culvert pipes will be inspected to ensure that surface water runoff is capable of leaving the structures. Drain manholes will be inspected to make sure there is not sediment build-up or blockages.

## **Conveyance Swale**

Description: Conveyance swales are stabilized channels designed to convey runoff at non-erosive velocities. They may be stabilized using vegetation, riprap, or a combination, or with an alternative lining designed to accommodate design flows while protecting the integrity of the sides and bottom of the channel. Conveyance channels may provide incidental water quality benefits, but are not specifically designed to provide treatment. Conveyance swales are not considered a Treatment or Pretreatment Practice under the AoT regulations, unless they are also designed to meet the requirements of an acceptable Treatment/Pretreatment Practice as described elsewhere in this Chapter. See SWM Volume 2, 4-6.3 Conveyance Practices, Conveyance Swale, page 166.

Maintenance Considerations: Grassed channels should be inspected periodically (at least annually) for sediment accumulation, erosion, and condition of surface lining (vegetation or riprap). Repairs, including stone or vegetation replacement, should be made based on this inspection. Remove sediment and debris annually, or more frequently as warranted by inspection. Mow vegetated channels based on frequency specified by design. Mowing at least once per year is required to control establishment of woody vegetation. It is recommended to cut grass no shorter than 4 inches.



## **Sediment Forebay**

Description: A sediment forebay is an impoundment, basin, or other storage structure designed to dissipate the energy of incoming runoff and allow for initial settling of coarse sediments. Forebays are used for pretreatment of runoff prior to discharge into the primary water quality treatment BMP. In some cases, forebays may be constructed as separate structures but often, they are integrated into the design of larger stormwater management structures. See SWM Volume 2, 4-4.1 Pre-treatment Practices, Sediment Forebay, page 140.

Maintenance Considerations: Forebays help reduce the sediment load to downstream BMPs, and will therefore require more frequent cleaning. Inspect at least annually; Conduct periodic mowing of embankments (generally two times per year) to control growth of woody vegetation on embankments; Remove debris from outlet structures at least once annually; Remove and dispose of accumulated sediment based on inspection; Install and maintain a staff gage or other measuring device, to indicate depth of sediment accumulation and level at which clean-out is required. Preserving the drainage between the Sediment Forebay and the stormwater BMP by inspecting and maintaining the connecting drainage pipes and perforations should be completed semi annually or as required to ensure the forebay is dry.

## **Bioretention W/ Internal Storage Reservoir (ISR)**

Description: A practice that provides temporary storage of runoff for filtering through an engineered soil media, augmented for enhanced phosphorus removal, followed by detention and denitrification in a subsurface internal storage reservoir (ISR) comprised of gravel. Runoff flows are routed through filter media and directed to the underlying ISR via an impermeable membrane for temporary storage. An elevated outlet control at the top of the ISR is designed to provide a retention time of at least 24 hours in the system to allow for sufficient time for denitrification and nitrogen reduction to occur prior to discharge. The design storage capacity for using the cumulative performance curves is comprised of void spaces in the filter media, temporary ponding at the surface of the practice and the void spaces in the gravel ISR. The volume of the ISR will exceed 26% of the Water Quality Volume (WQV). Reference: 2017 NH Small MS4 General Permit, Appendix F Attachment 3, and UNH Stormwater Center, "UNH Stormwater Center Hybrid Bioretention Template" (2020). *UNH Stormwater Center*. 73.  
<https://scholars.unh.edu/stormwater/73>

Maintenance Considerations: The outlet to the Internal Storage Reservoir consists of a 1.25" or 1.5" orifice in a threaded end-cap after the goose-neck pipe within the concrete outlet structure. The inlet manifold and threaded pipe outlet manifold system is designed so that the ISR, or anaerobic reservoir can be completely drained and the sump of the outlet structure pumped dry. The orifice requires periodic inspection, initially on a semi-annual basis. This time increment may need to be adjusted based on

the experience on the maintenance of the device. The draining of the ISR would only be accomplished if issues developed.

The enhanced bio-media will require additional material rototilled into the top 10-inches to foot of the rain garden after a period of approximately 20 years. The timing of this maintenance period is a factor of the methodology applied during construction and will need to be evaluated as the rain gardens age.

Rain Gardens should be inspected at least twice annually and following any rainfall event exceeding 2.5 inches in a twenty-four hour period. Maintenance rehabilitation will be conducted as warranted by each inspection. Trash and debris will be removed at each inspection.

On an annual basis the infiltration capabilities need to be confirmed by evaluation the drawdown time. If the bioretention system does not drain within 72-hours following a rainfall event, a qualified professional will assess the condition of the rain garden to determine measures required to restore the infiltration function. This is normally the direct result of sediment accumulation which will be removed to restore the filter media ratio.

Proposed side slopes of 2:1 will be maintained with a weedwhacker, with vegetation being removed from the BMP with each maintenance application.

## **Detention Basins**

Description: A detention basin is an impoundment designed to temporarily store runoff and release it at a controlled rate, reducing the intensity of peak flows during storm events. Conventional detention basins are typically designed to control peak runoff rates under a range of storm conditions, and can be used to control discharges as required under the AoT Regulations and other requirements, including, but not necessarily limited to: Storage and peak rate control to meet Channel Protection Requirements (see Section 2-17); Storage and peak rate control to meet Peak Runoff Control Requirements (see Section 2-18) (10-year and 50-year frequency, 24-hour storm events); Storage and peak rate control to prevent flood impacts within the 100-year flood plain; Storage and peak rate control to meet other regulatory requirements, including local permitting standards.

Detention basins may consist of surface basins (pond-type structures) or subsurface basins (enclosed structures located below ground. Surface basins should be designed with an emergency spillway or bypass meeting applicable dam safety standards (Env-Wr 100 - 700: Dam Safety Rules). Subsurface basins should also be designed to safely bypass flows exceeding the engineered capacity of the structure. Detention basins may be combined with treatment BMPs discussed in this guidance document, to provide for other stormwater management objectives. For example, a stormwater pond may be

designed to provide treatment as well as detention. However, a detention basin is not by itself considered a "Treatment Practice" under the AoT Regulations. See SWM Volume 2, 4-6.1 Conveyance Practices, Detention Basins, page 156.

Maintenance Considerations: The bottoms, interior and exterior side slopes, and crest of earthen detention basins should be mowed, and the vegetation maintained in healthy condition, as appropriate to the function of the facility and type of vegetation. Vegetated embankments that serve as "berms" or "dams" that impound water should be mowed at least once annually to prevent the establishment of woody vegetation.

### **In-Ground Infiltration Basin**

Description: Infiltration basins are impoundments designed to temporarily store runoff, allowing all or a portion of the water to infiltrate into the ground. An infiltration basin is designed to completely drain between storm events. An infiltration basin is specifically designed to retain and infiltrate the entire Water Quality Volume. Some infiltration basins may infiltrate additional volumes during larger storm events, but many will be designed to release stormwater exceeding the water quality volume from the larger storms. In a properly sited and designed infiltration basin, water quality treatment is provided by runoff pollutants binding to soil particles beneath the basin as water percolates into the subsurface. Biological and chemical processes occurring in the soil also contribute to the breakdown of pollutants. Infiltrated water is used by plants to support growth or it is recharged to the underlying groundwater. As with all impoundment BMPs, surface infiltration basins should be designed with an outlet structure to pass peak flows during a range of storm events, as well as with an emergency spillway to pass peak flows around the embankment during extreme storm events that exceed the combined infiltration capacity and outlet structure capacity of the facility. See SWM Volume 2, 4-3.3b, Treatment Practices, In-Ground Infiltration Basin, page 88.

Maintenance Considerations: Removal of debris from inlet and outlet structures. Removal of accumulated sediment. Inspection and repair of outlet structures and appurtenances. Inspection of infiltration components at least twice annually, and following any rainfall event exceeding 2.5 inches in a 24 hour period, with maintenance or rehabilitation conducted as warranted by such inspection. Inspection of pretreatment measures at least twice annually, and removal of accumulated sediment as warranted by inspection, but no less than once annually. If an infiltration system does not drain within 72-hours following a rainfall event, then a qualified professional should assess the condition of the facility to determine measures required to restore infiltration function, including but not limited to removal of accumulated sediments or reconstruction of the infiltration trench.

## Stone Berm Level Spreader

Description: A stone berm level spreader is an outlet structure constructed at zero percent grade across a slope used to convert concentrated flow to “sheet flow.” It disperses or “spreads” flow thinly over a receiving area, usually consisting of undisturbed, vegetated ground. The conversion of concentrated flow to shallow, sheet flow allows runoff to be discharged at non-erosive velocities onto natural ground. To stabilize the spreader outlet, a stone berm is provided to dissipate flow energy, and help disperse flows along the length of the spreader. Level spreaders are not designed to remove pollutants from stormwater; however, some suspended sediment and associated phosphorous, nitrogen, metals and hydrocarbons will settle out of the runoff through settlement, filtration, infiltration, absorption, decomposition and volatilization. See SWM Volume 2, 4-6.6 Conveyance Practices, Stone Berm Level Spreader, page 162.

Maintenance Considerations: Inspect at least once annually for accumulation of sediment and debris and for signs of erosion within approach channel, spreader channel or down-slope of the spreader. Remove debris whenever observed during inspection. Remove sediment when accumulation exceeds 25% of spreader channel depth. Mow as required by landscaping design. At a minimum, mow annually to control woody vegetation within the spreader. Snow should not be stored within or down-slope of the level spreader or its approach channel. Repair any erosion and re-grade or replace stone berm material, as warranted by inspection. Reconstruct the spreader if down-slope channelization indicates that the spreader is not level or that discharge has become concentrated, and corrections cannot be made through minor re-grading.

## Stabilization for Long Term Cover

### Vegetated Stabilization – Original Planting

All areas that are disturbed during construction will be stabilized with vegetated material within 30 days of breaking ground. Construction will be managed in such a manner that erosion is prevented and that no abutter’s property will be subjected to any siltation, unless otherwise permitted. All areas to be planted with grass for long-term cover will follow the specification and on Sheet E-102 using seeding mixture C, as follows:

Mixture	Pounds per Acre	Pounds per 1,000 Sq. Ft.
Tall Fescue	24	0.55
Creeping Red Fescue	24	0.55
Total	48	1.10

## Conservation Mix

Virginia Wild Rye	Native	FACW-
Little Bluestem	Native	FACU
Big Bluestem	Native	FAC
Red Fescue	Native	FACU
Switch Grass	Native	FAC
Partridge Pea	Native	FACU
Showy Tick Trefoil	Native	FAC
Butterfly Milkweed	Native	NI
Beggar Ticks	Native	FACW
Purple Joe Pye Weed	Native	FAC
Black Eyed Susan	Native	FACU-
<b>Total</b>	<b>25</b>	<b>0.57</b>

Conservation Mix to be provided by New England Wetland Plants, Inc., Amherst, MA as outline in their New England Conservation / Wildlife Mix or approved equal. Mix to be applied at a rate of 25 lbs. per acre or one-lb. per 1750 square feet. Ratio of seed is proprietary and substitutions are not allowed.

Conservation Mix will used to stabilize all 2:1 slopes and all land area disturbed within the wetland buffer.

Stormwater BMP Mix: The grass that is planted within a stormwater BMP will be a mix designed for both inundation and dry conditions such as Ernst Seeds, Retention Basin Floor Mix ERNMX-126.

Maintenance Considerations: Permanent seeded areas for long-term cover will be inspected on a periodic basis looking for signs of growth loss or erosion. Any areas found to be damaged will be repaired and replanted to reestablish the growth. The grass should be mowed at least twice per year and any dead material removed. Any woody growth that becomes established will need to be cut and removed.

Long-term maintenance of the land cover is critical and must be maintained at least 85% grass / vegetation coverage, must be inspected for concentrated flow, rills, and channels; and must be repaired as necessary to prevent erosion.

## Control of Invasive Plants

During maintenance activities, check for the presence of invasive plants and remove in a safe manner as described on the following pages. They should be controlled as described on the following pages.

Invasive plants are introduced, alien, or non-native plants, which have been moved by people from their native habitat to a new area. Some exotic plants are imported for human use such as landscaping, erosion control, or food crops. They also can arrive as "hitchhikers" among shipments of other plants, seeds, packing materials, or fresh produce. Some exotic plants become invasive and cause harm by:

- becoming weedy and overgrown;
- killing established shade trees;
- obstructing pipes and drainage systems;
- forming dense beds in water;
- lowering water levels in lakes, streams, and wetlands;
- destroying natural communities;
- promoting erosion on stream banks and hillsides; and
- resisting control except by hazardous chemical.

## Snow Removal & Winter Maintenance

Description: Drainage and stormwater systems need to be maintained during the winter months so that surface water runoff from a rain storm does not become a impounding and icing problem. Catch basins must remain viable and where sheet flow is a design factor, the edge of pavement and should need to be maintained so that runoff can leave the pavement area. Sand and salt should be used at the rate that prevents sedimentation problems or excess salt deposited but yet enough to allow for protection for pedestrians and vehicles.

Maintenance Considerations: Catch basins are required to be kept viable by removing snow that is block surface water runoff from entering the structure. The edge of pavement where surface water sheet flow is designed to leave the paved area, the edge of pavement and shoulder need to be plowed to allow runoff to leave the pavement. Snow is to piled in designated areas and removed from the site when the on-site storage locations have been exceeded. At the end of the winter season, sediment is to be swept from the paved surfaces and removed from the drainage system. (Sumps if provided, sediment forebays, swale lines.) (See catch basin and drainage pipe maintenance.) NHDES offers training (Green SnowPro Certification) for contractors and owners. <https://www.des.nh.gov/land/roads/road-salt-reduction/green-snowpro-certification> Please find attached NHDES Green SnoPro Utilization Chart which is required to be used.

## Annual Report

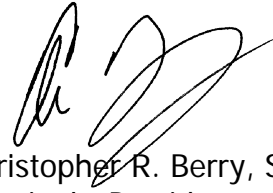
Description: The owner is responsible to keep an **I & M Activity Log** that documents inspection, maintenance and repairs to the storm water management system, and a **Deicing Log** to track the amount and type of deicing material applied to the site. The original owner is responsible to ensure that any subsequent owner (s) have copies of the Stormwater System Operation: Inspection & Maintenance Manual, copies of past logs and check lists. This includes any owner association for potential condominium conversion of the property. The Annual Report will be prepared and submitted to the Town of Barrington, Planning Department with copies of both logs and check lists no later than December 15<sup>th</sup> of each year and made available to NHDES upon request. Upon an ownership change, the Annual Report will include the Transfer of Ownership Responsibility Forms duplicated from the form found below.

The plans that accompany this manual include a plan sheet, "Inspection & Maintenance Plan" and copies of the Stormwater Treatment Design Sheets. The owner will also maintain a complete set of the approved original design plans.

Respectfully  
BERRY SURVEYING & ENGINEERING



Kenneth A. Berry, PE, LLS  
CPSWQ, CPESC, CESSWI  
Principal, VP – Technical Operations



Christopher R. Berry, SIT  
Principal - President  
Design Engineer



Kevin R. Poulin, PE  
Design Engineer

**STORMWATER SYSTEM OPERATIONS: INSPECTION & MAINTENANCE MANUAL**

**Inspection & Maintenance Manual Checklist**

Calef Highway, Barrington, NH, Tax Map 238, Lot 44  
 TURBOCAM, INC.  
 607 Calef Highway Suite 200  
 Barrington, NH 03825

<input checked="" type="checkbox"/>	Date	BMP / System	Minimum Inspection Frequency	Minimum Inspection Requirements	Maintenance / Cleanout Threshold
		Pavement Sweeping	Three Times Per Year	Clean Pavement	Pavement areas will be swept and sedimentation removed so the surface is clean
		Litter/Trash Removal	Routinely	Inspect dumpsters, outdoor waste receptacles area, and yard areas.	Parcel will be free of litter/trash.
		Deicing Agents	N/A	N/A	Use salt as the primary agent for roadway safety during winter.
		Invasive Species	Two times per year.	Inspect for Invasive Species	Remove and dispose invasive species.
		<b>Closed Drainage System:</b>			
		Drainage Pipes	1 time per 2 years	Check for sediment accumulation & clogging.	Less than 2" sediment depth
		Catch Basins & Drain Manholes	2 times per year	Check for sediment accumulation & clogging.	Any accumulated Sediment or debris.



☑	Date	BMP / System	Minimum Inspection Frequency	Minimum Inspection Requirements	Maintenance / Cleanout Threshold
		Bioretention W/ ISRs, & Infiltration Ponds	2 times per year	Check for sediment and debris accumulation buildup.	Remove sediment & debris when required. Remove Invasive Species
		Bioretention W/ ISR and system clean-outs.	Annually	72-Hour drawdown time evaluation and vegetation evaluation. Underdrain flushing.	Remove dead & diseased vegetation along with all debris, take corrective measures of filtration media if required. Flush underdrain clean-outs with a hose. Weed whacker required for 2:1 slopes. All weed whacked vegetation to be removed
		Infiltration Ponds	Annually after a storm event of greater than 2.5-inches	Evaluate the drawdown of the Infiltration Basin systems to ensure that through infiltration the system is completely drained in 72 hours.	Ensure sediment is not entering the Infiltration Basin.
		Riprap Outlet Protection	Annually	Check for sediment buildup and structure damage.	Remove excess sediment and repair damage.
		Winter Maintenance	Ongoing	Remove snow as directed.	Ongoing
		Post Winter Maintenance	Annually	Remove excess sand, gross solids, and repair vegetation and plantings	Parcel will be free of excess sand, litter/trash.
		Annual Report	1 time per year	Submit Annual Report to Barrington Planning Dept. and kept on file by the owner.	Report to be submitted on or before December 15th each year.

Inspection Check List: Page 3

Catch Basins #41-#47 (Ponds #C41-#C47)

Drain Manholes #51-#53 (Ponds #D51-#D53)

Inlet Sump (Pond #50)

Conveyance Swales and Roadside Ditches

Bioretention W/ ISR #201 – P-201 w/ Outlet Structures and Matted Spillway

Bioretention W/ ISR #202 – P-202 w/ Outlet Structures and Matted Spillway

Infiltration Pond #203 – P-203 w/ Matted Spillway

Detention Pond #204 – P-204 w/ Outlet Structure and Matted Spillway

Outlet Protection and Level Spreaders





**STORMWATER SYSTEM OPERATION & MAINTENANCE PLAN CERTIFICATION**

Owner	Responsibility
Name: TURBOCAM, INC. / Virtuous Realty, LLC Owner Andrew Knapp, Director of EHS, Facilities & Maintenance Address: 607 Calef Highway Suite 200 Barrington, NH 03825 Telephone: 1-603-905-0203 E-mail: andy.knapp@turbocam.com	The owner is responsible for the conduct of all construction activities, and ultimate compliance with all the provisions of the Stormwater System Operation & Maintenance Plan and the implementation of the Inspection and Maintenance Manual.

607 Calef Highway, Barrington, NH, Tax Map 238, Lot 44

**OWNER CERTIFICATION**

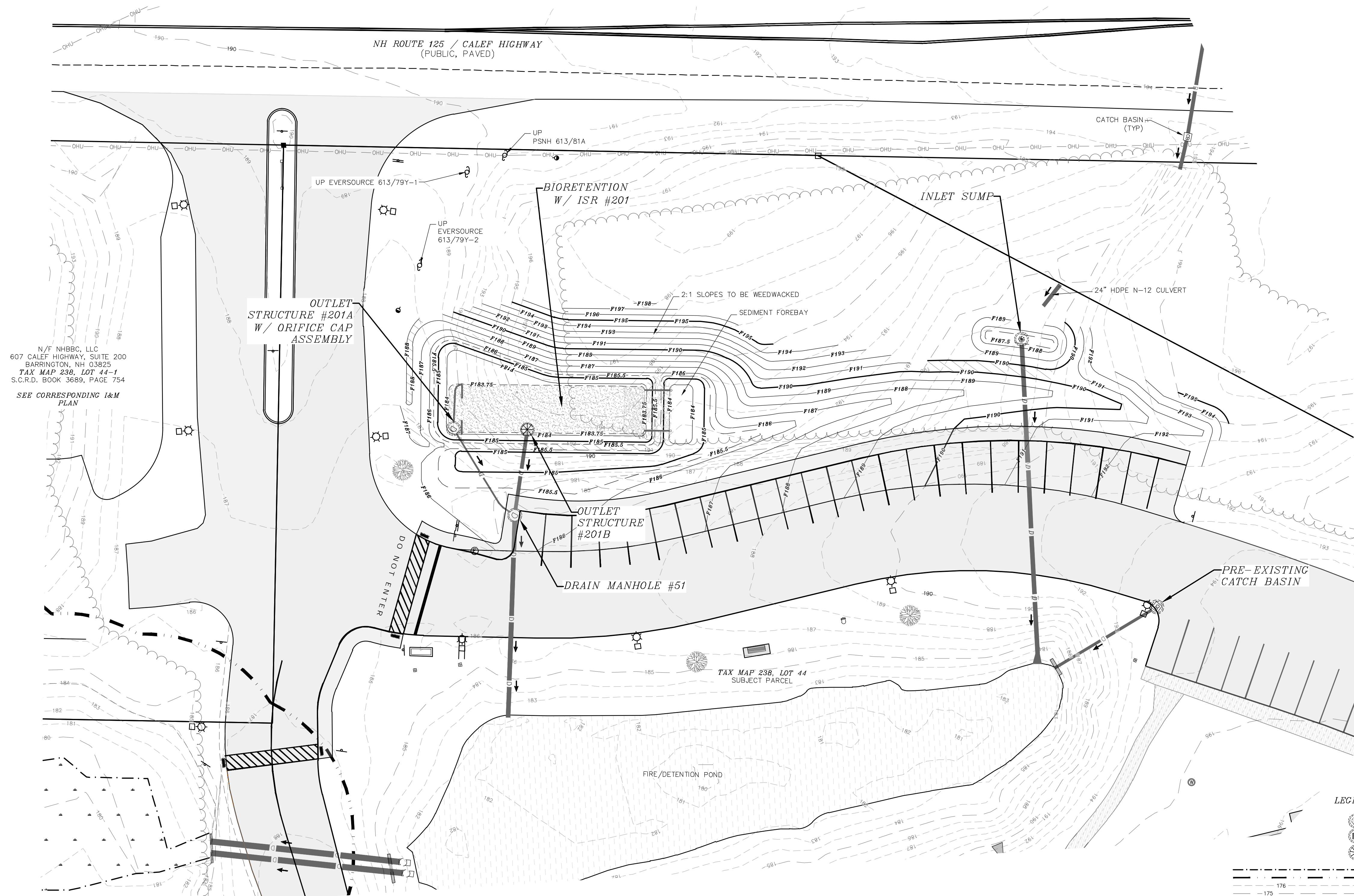
I certify under penalty of law that this document and all attachments were prepared under my direction and supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Signed: \_\_\_\_\_ Date: \_\_\_\_\_

Printed Name:

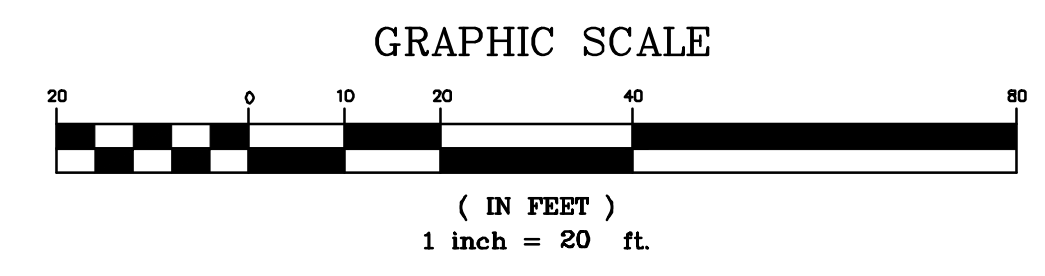
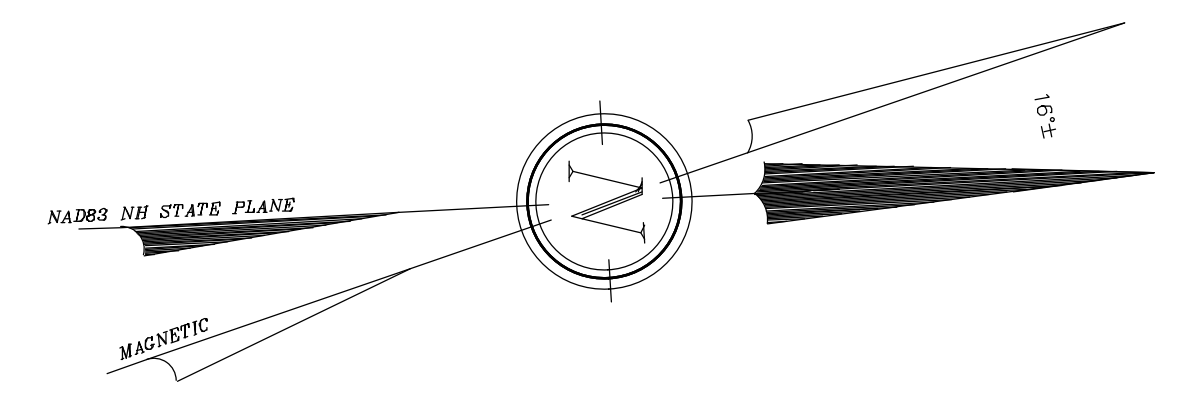
Representing:

- NOTES:**
- 1.) OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 1A.) APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
  - 2.) TAX MAP 238, LOT 44
  - 3.) LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
  - 4.) S.C.R.D. BOOK 2948, PAGE 332
  - 5.) THE INTENT OF THIS PLAN IS TO DEMONSTRATE THE DRAINAGE INFRASTRUCTURE THAT REQUIRES PERIODIC INSPECTION AND MAINTENANCE. IT IS TO ACCOMPANY A DOCUMENT ENTITLED "STORMWATER SYSTEM MANAGEMENT: INSPECTION AND MAINTENANCE MANUAL" PUBLISHED BY BERRY SURVEYING & ENGINEERING ON THE SAME DATE.
  - 6.) 11x17" PLANS ARE TWICE THE PUBLISHED SCALE.



N/F NHBBC, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825  
TAX MAP 238, LOT 44-1  
S.C.R.D. BOOK 3689, PAGE 754  
SEE CORRESPONDING I&M  
PLAN

- LEGEND:**
- PROPOSED DRAIN MANHOLE W/ STRUCTURE
  - PROPOSED CATCH BASIN W/ STRUCTURE
  - PROPOSED STORMWATER BMP OUTLET STRUCTURE
  - POORLY DRAINED JURISDICTIONAL WETLAND
  - 50' WETLAND BUFFER
  - CONTOUR MINOR, EXISTING
  - CONTOUR MINOR, PROPOSED
  - CONTOUR MAJOR, EXISTING
  - CONTOUR MAJOR, PROPOSED
  - EXISTING DRAINAGE LINE
  - PROPOSED DRAINAGE LINE
  - RIP RAP
  - STORMWATER BEST MANAGEMENT PRACTICE (BMP)
  - BERM

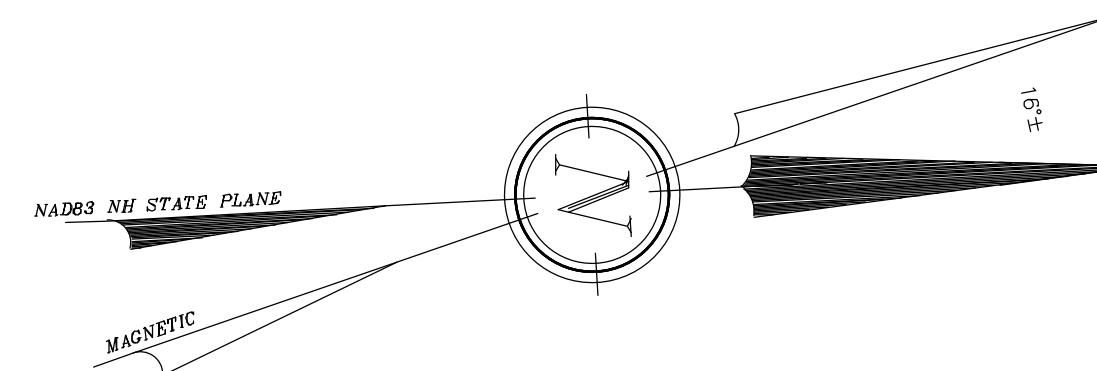


REVISION	DATE	DESCRIPTION

INSPECTION & MAINTENANCE PLAN

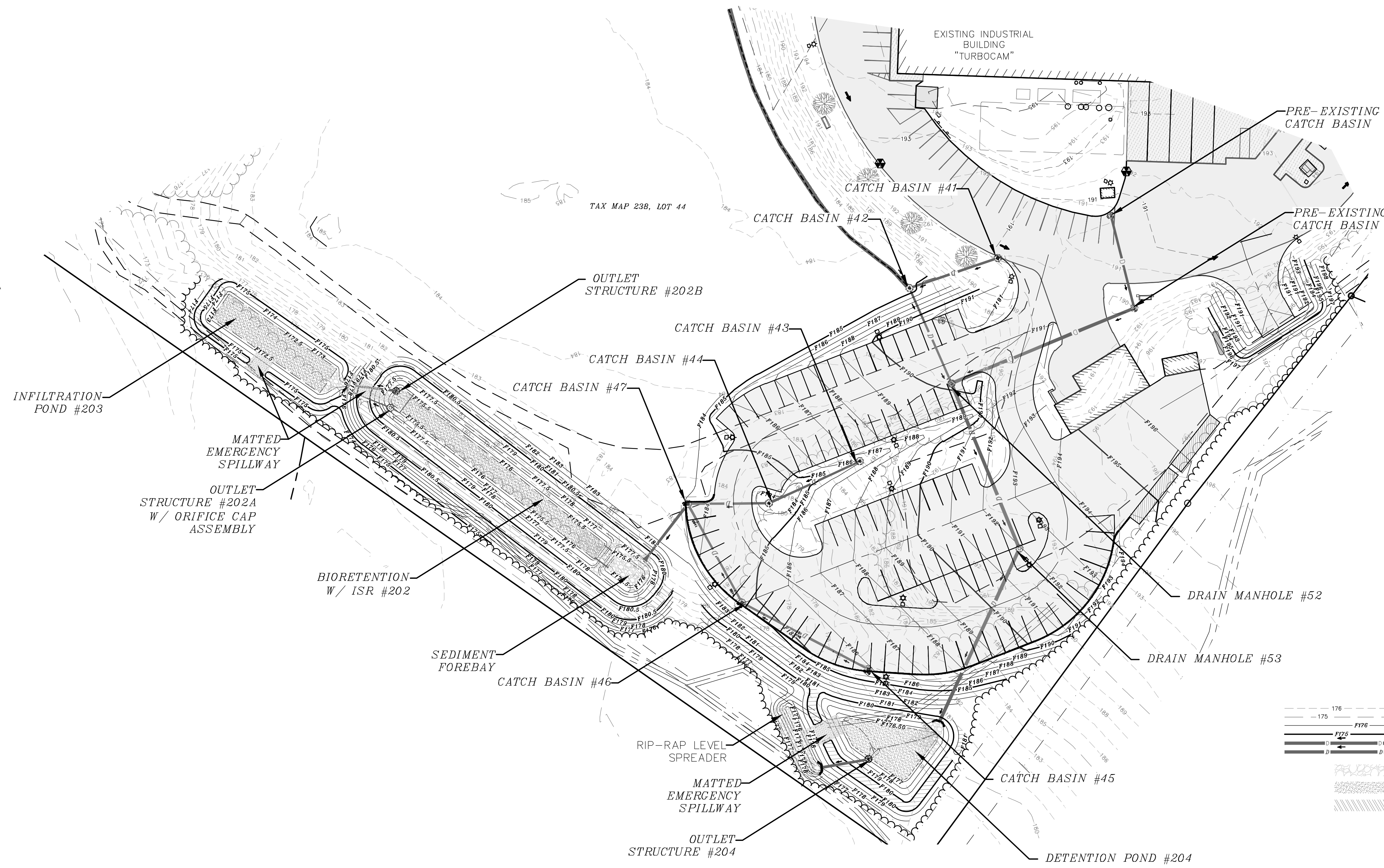
FOR  
TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

**BERRY SURVEYING & ENGINEERING**  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE : 1 IN. EQUALS 20 FT.  
DATE : APRIL 17, 2024  
FILE NO. : DB 2023 - 017



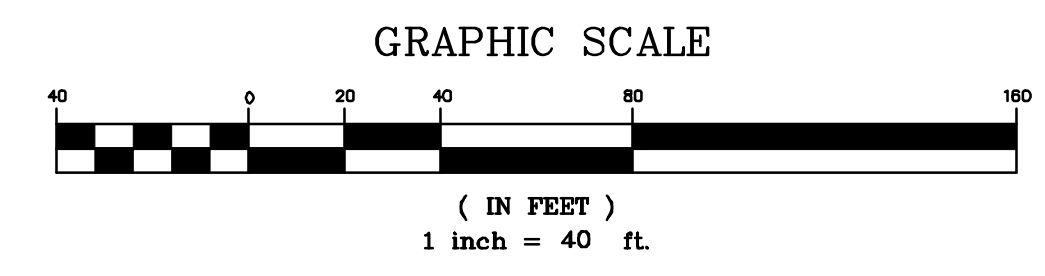
**NOTES:**

- 1.) OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- 1A.) APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- 2.) TAX MAP 238, LOT 44
- 3.) LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
- 4.) S.C.R.D. BOOK 2948, PAGE 332
- 5.) THE INTENT OF THIS PLAN IS TO DEMONSTRATE THE DRAINAGE INFRASTRUCTURE THAT REQUIRES PERIODIC INSPECTION AND MAINTENANCE. IT IS TO ACCOMPANY A DOCUMENT ENTITLED "STORMWATER SYSTEM MANAGEMENT: INSPECTION AND MAINTENANCE MANUAL" PUBLISHED BY BERRY SURVEYING & ENGINEERING ON THE SAME DATE.
- 6.) 11x17" PLANS ARE TWICE THE PUBLISHED SCALE.



**LEGEND:**

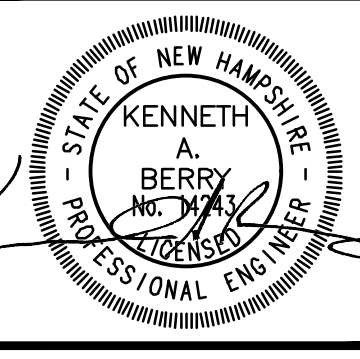
- PROPOSED DRAIN MANHOLE W/ STRUCTURE
- PROPOSED CATCH BASIN W/ STRUCTURE
- PROPOSED STORMWATER BMP OUTLET STRUCTURE
- CONTOUR MINOR, EXISTING
- CONTOUR MAJOR, EXISTING
- CONTOUR MINOR, PROPOSED
- CONTOUR MAJOR, PROPOSED
- EXISTING DRAINAGE LINE
- PROPOSED DRAINAGE LINE
- RIP RAP
- STORMWATER BEST MANAGEMENT PRACTICE (BMP)
- BERM



REVISION	DATE	DESCRIPTION

INSPECTION & MAINTENANCE PLAN  
FOR  
TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

**BERRY SURVEYING & ENGINEERING**  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE : 1 IN. EQUALS 40 FT.  
DATE : APRIL 17, 2024  
FILE NO. : DB 2023 - 017



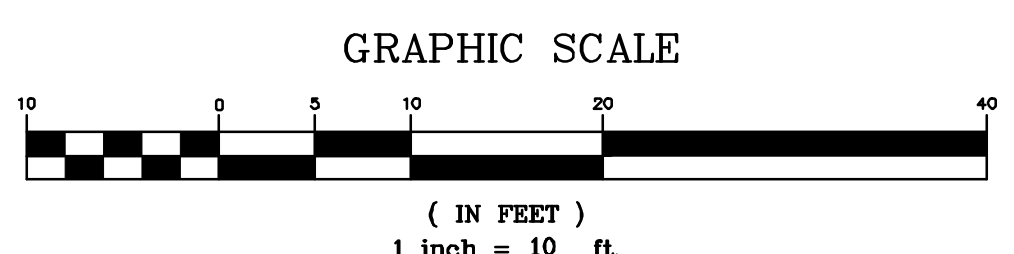
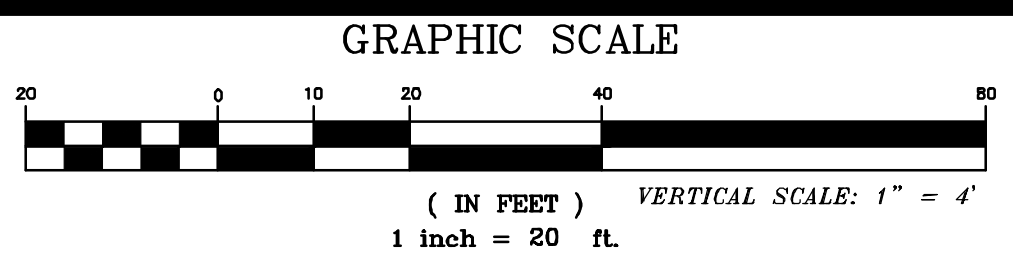
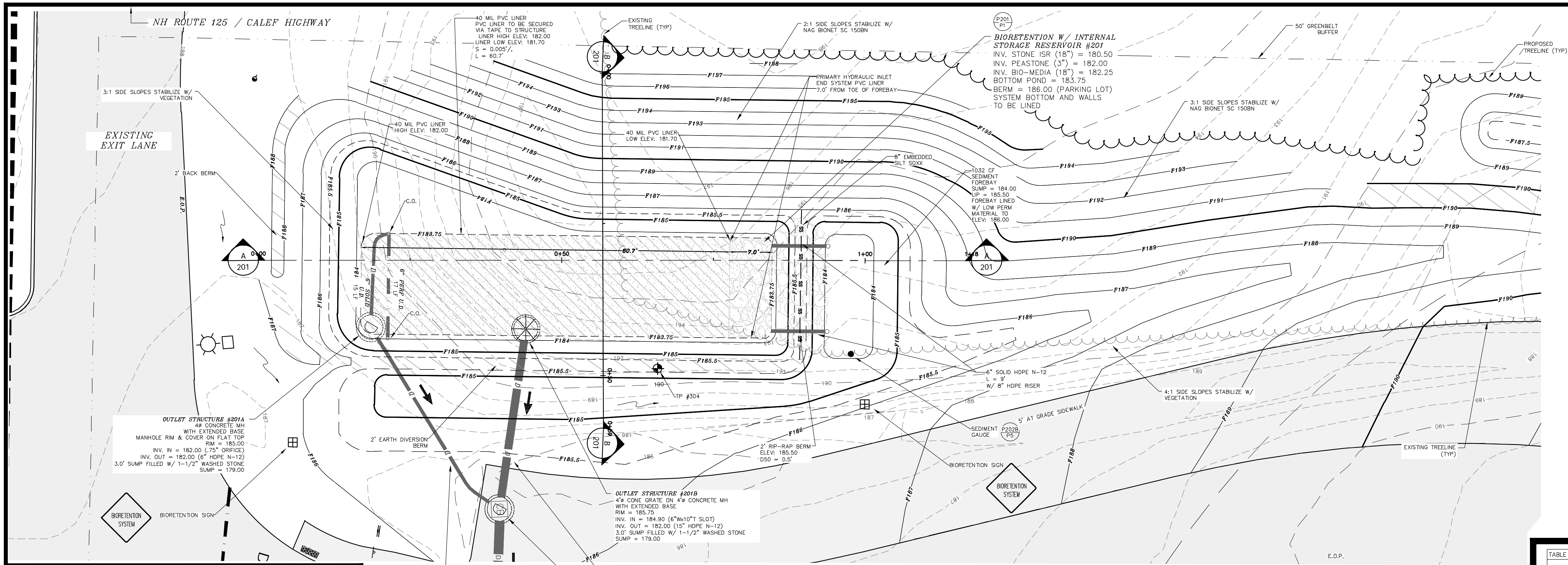
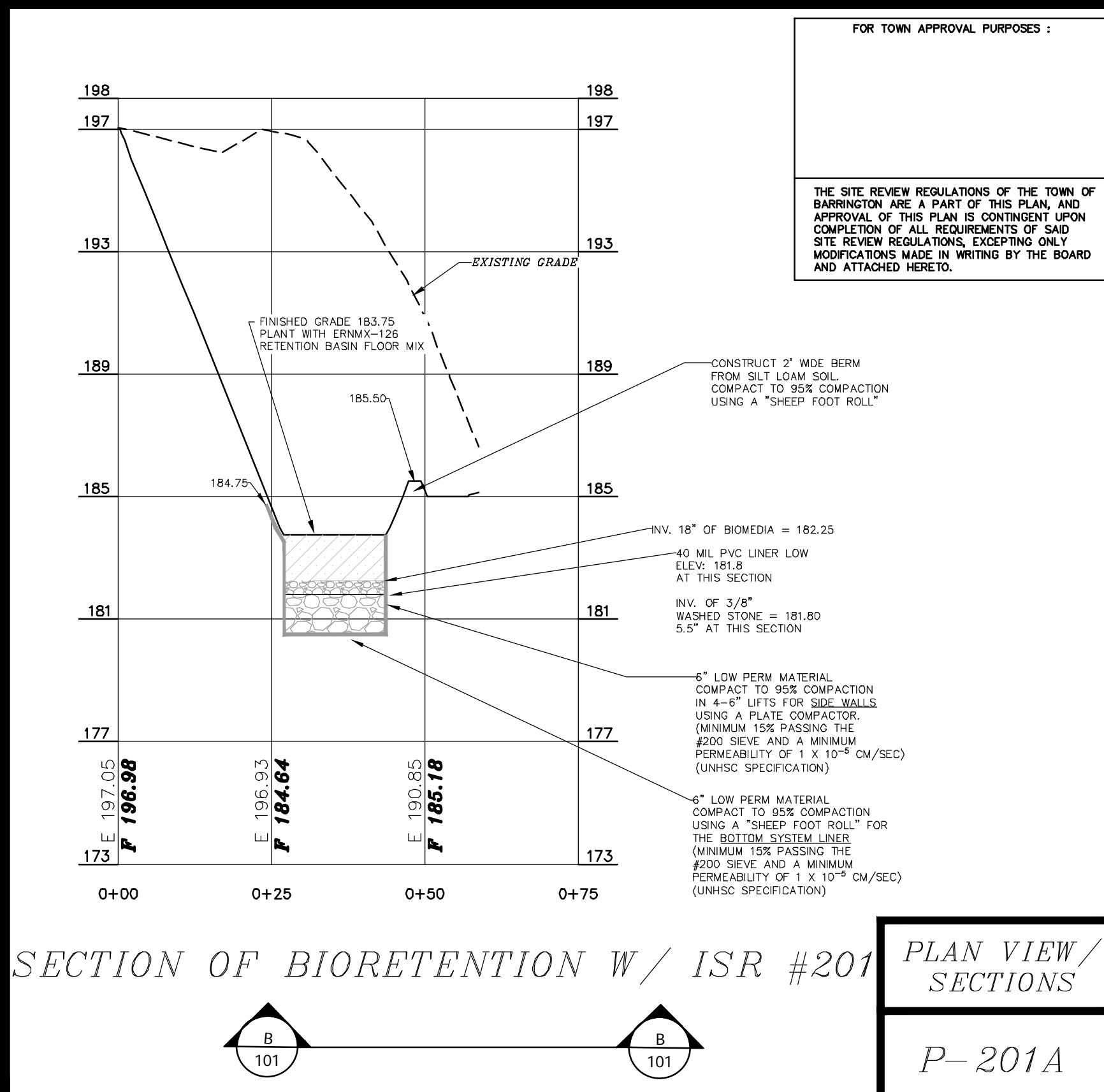
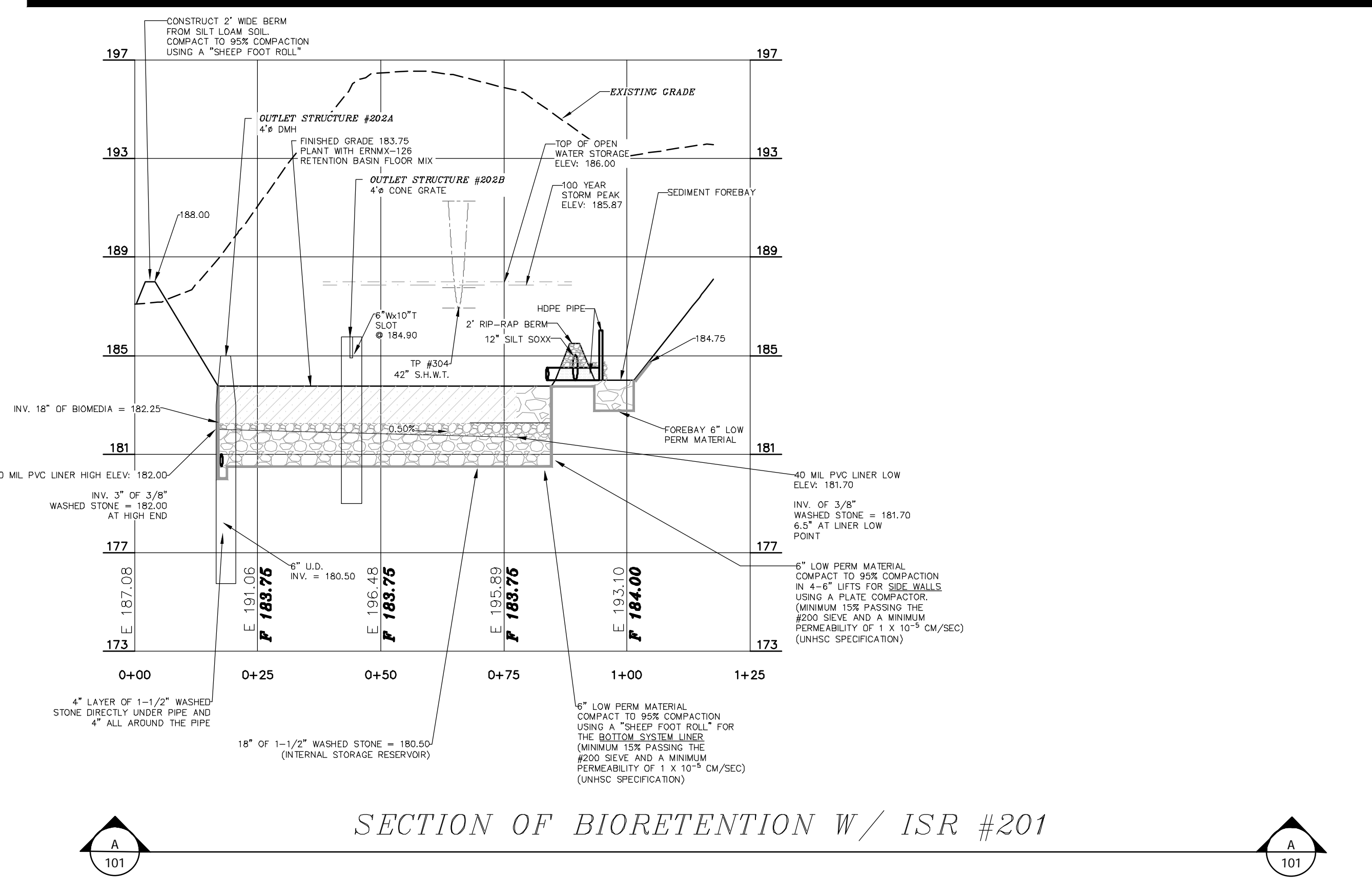


TABLE 7-24--RECOMMENDED RIP RAP GRADATION RANGES

DESIGN SIZE	0.5 FEET	6 INCHES
% OF WEIGHT SMALLER THAN THE GIVEN SIZE	SIZE OF STONE (INCHES)	SIZE OF STONE (INCHES)
100%	9	12
85%	8	11
50%	6	9
15%	2	3



REVISION	DATE	DESCRIPTION

FOR TOWN APPROVAL PURPOSES:

THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

BIORETENTION W/ ISR #201 PLAN VIEW & PROFILES

TUREOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
74X MAP 286, LOT 44

**BERRY SURVEYING & ENGINEERING**  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863

SCALE: AS SHOWN  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017

KENNETH A. BERRY  
REGISTERED PROFESSIONAL ENGINEER

**SECTION OF BIORETENTION W/ ISR #201 PLAN VIEW / SECTIONS**  
P-201A

SHEET 21 OF 43



UNH STORMWATER CENTER SPECIFICATIONS  
ACCEPTABLE PARTICLE SIZE DISTRIBUTION OF FINAL BIORETENTION SOIL MIX

MEDIA TYPE	SIEVE #	SIZE (IN)	% PASSING
COURSE SAND	4	0.187	100
MEDIUM SAND	10	0.075	95
FINE SAND	40	0.017	15-40
SILTS	200	0.003	10-20
CLAYS	<200	PAN	0-5

3/8" WASHED CRUSHED STONE\*

SIEVE SIZE	% PASSING BY WEIGHT
1/2"	100
3/8"	95 - 100
# 4	22 - 55
# 8	0 - 10

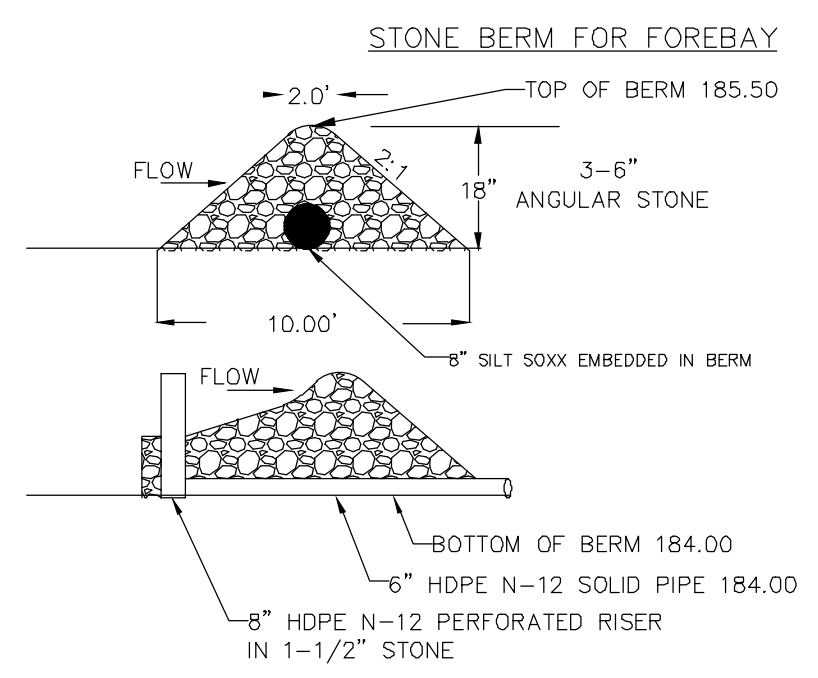
1-1/2" WASHED CRUSHED STONE\*

SIEVE SIZE	% PASSING BY WEIGHT
2"	100
1-1/2"	90 - 100
1"	20 - 55
1/2"	0 - 15
3/8"	0 - 5

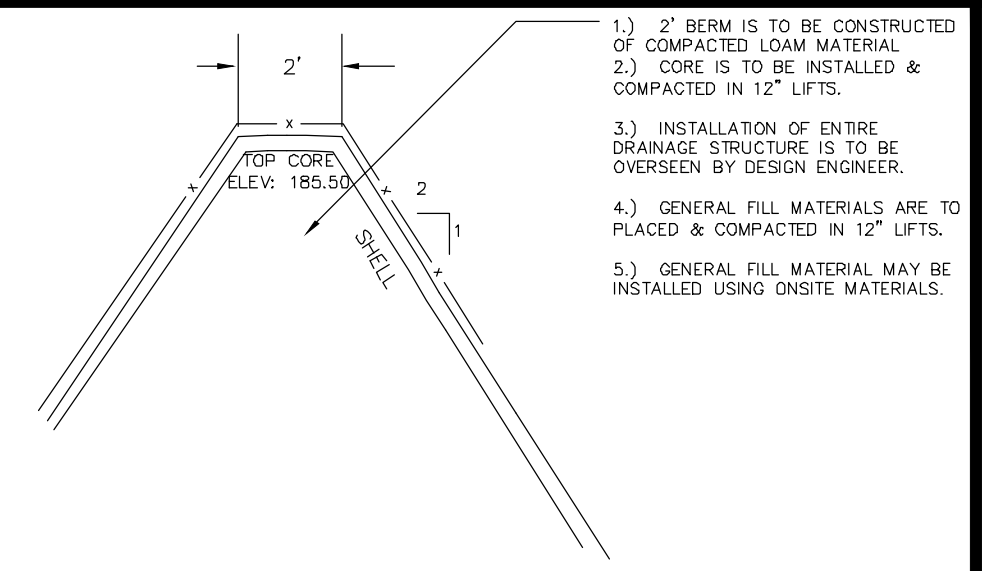
BIORETENTION SOIL MEDIA COMPONENTS  
-AMOUNTS MIXED BY TOTAL VOLUME  
\*60-85% SAND  
\*15-25% LOAM OR TOPSOIL  
\*3-8% ORGANIC MATTER  
\*5% WATER TREATMENT RESIDUALS OR 0.5% IRON FILINGS  
WATER TREATMENT RESIDUALS OR IRON FILINGS ARE REQUIRED DUE TO THE CLASS "A" RECEIVING WATERS  
REFER TO THE UNHSC STANDARD DETAIL, BIORETENTION ISR STORMWATER SYSTEM, FEBRUARY 2020 FOR FURTHER PUBLISHED GENERAL CONSTRUCTION GUIDELINES  
DESIGN REFERENCES  
1 UNH STORMWATER CENTER.  
<https://scholars.unh.edu/stormwater/73/>

**STABILIZATION NOTE:**  
SEE NOTE #6, SHEET E-102, DETAIL E-18  
SIDE SLOPES ARE TO BE STABILIZED WITHIN THREE WORKING DAY UPON COMPLETION OF FINAL GRADE.  
**CONSTRUCTION NOTE:** THE DESIGN ENGINEER IS TO BE ON SITE PRIOR TO AND DURING THE PLACEMENT OF THE RESERVOIR STONE, AND TO WITNESS THE INSTALLATION OF THE UNDER-DRAIN AND LINER. PLEASE PROVIDE 48 HOURS NOTICE PRIOR TO REQUESTED INSPECTION.  
PVC LINER TO BE 40 TO 60 MIL, PREFERABLY SEAMLESS. IF SEAMS ARE UNAVOIDABLE, THE SEAMS SHOULD BE SEALED

P1



P2 P3

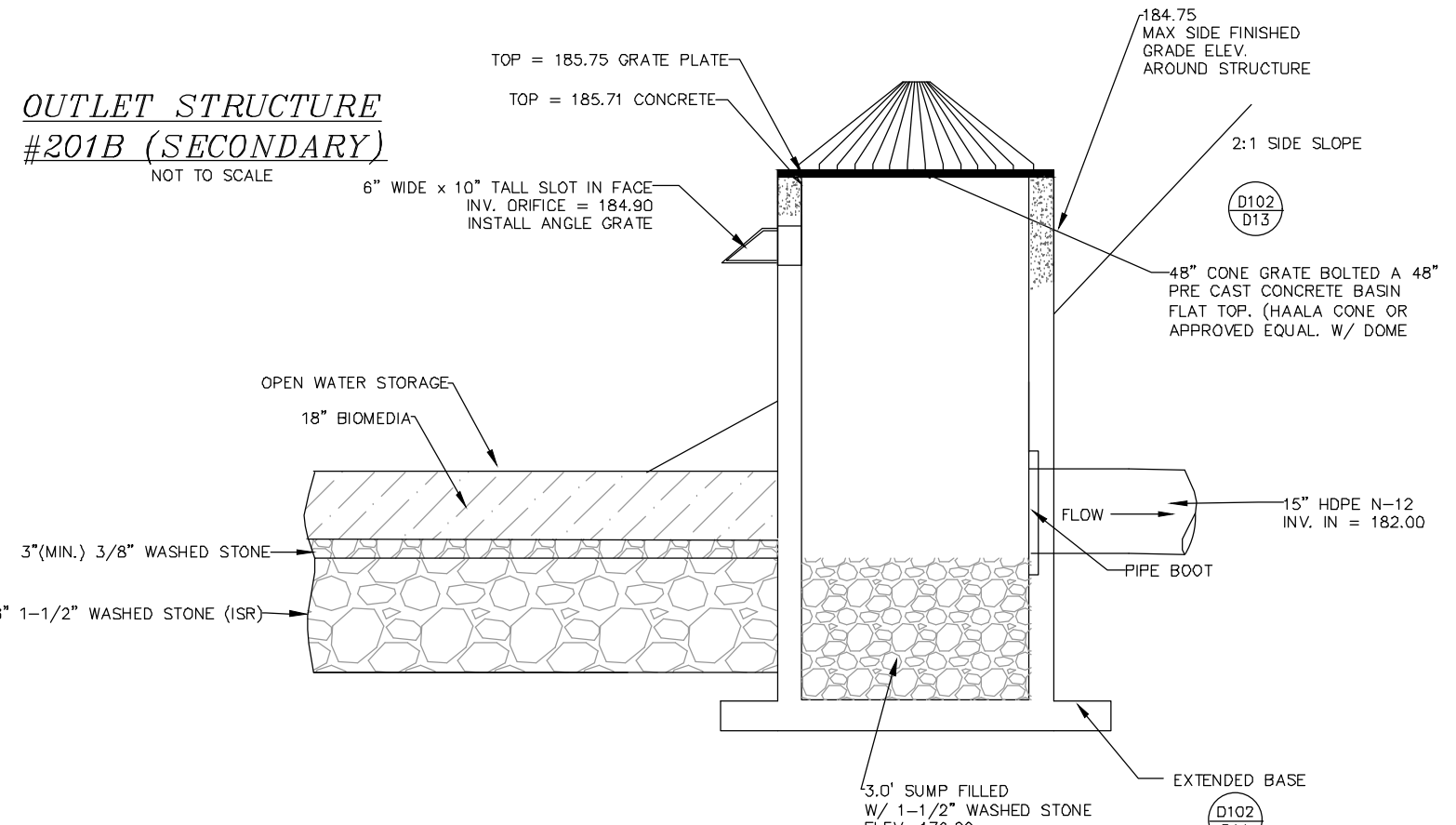
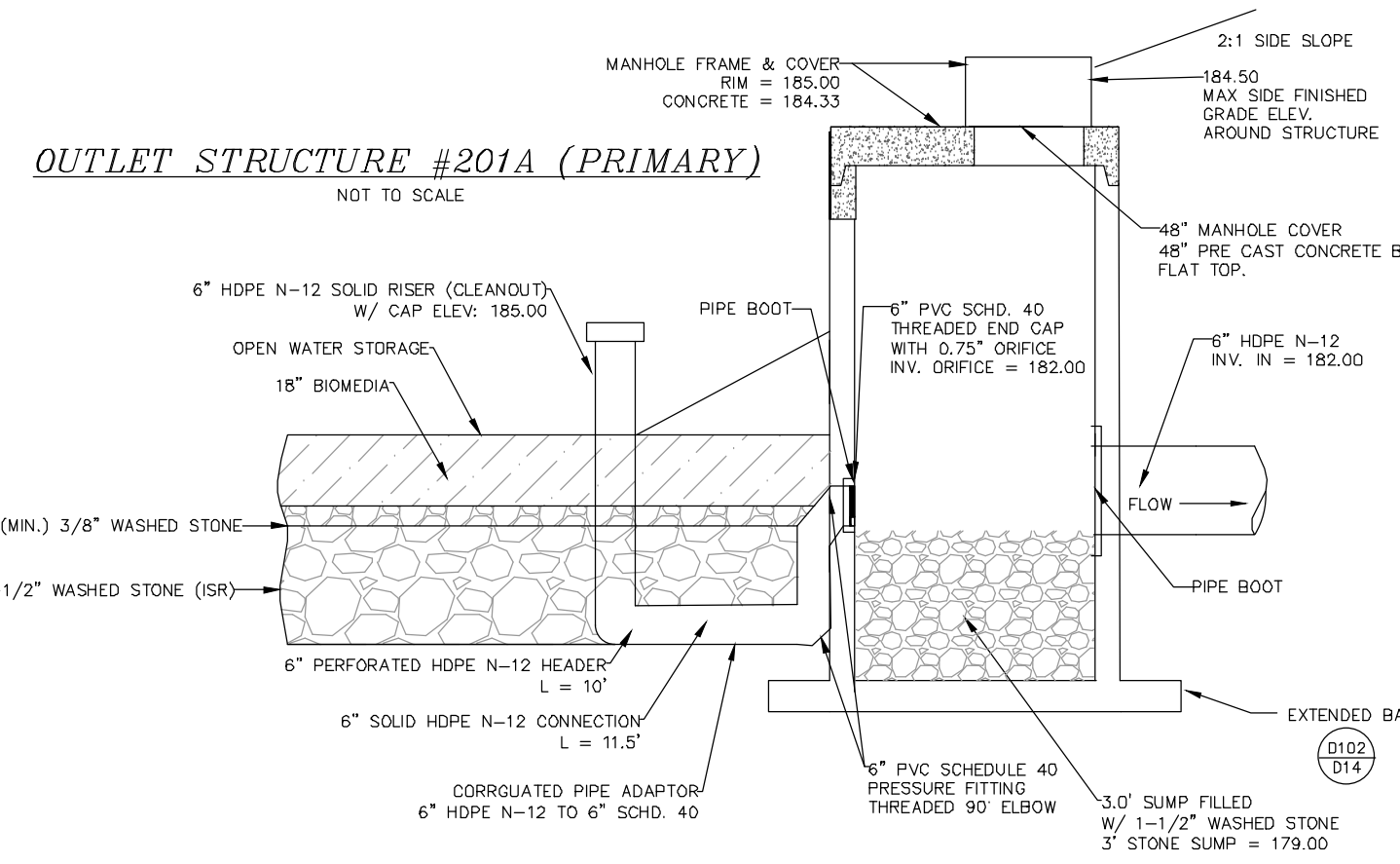
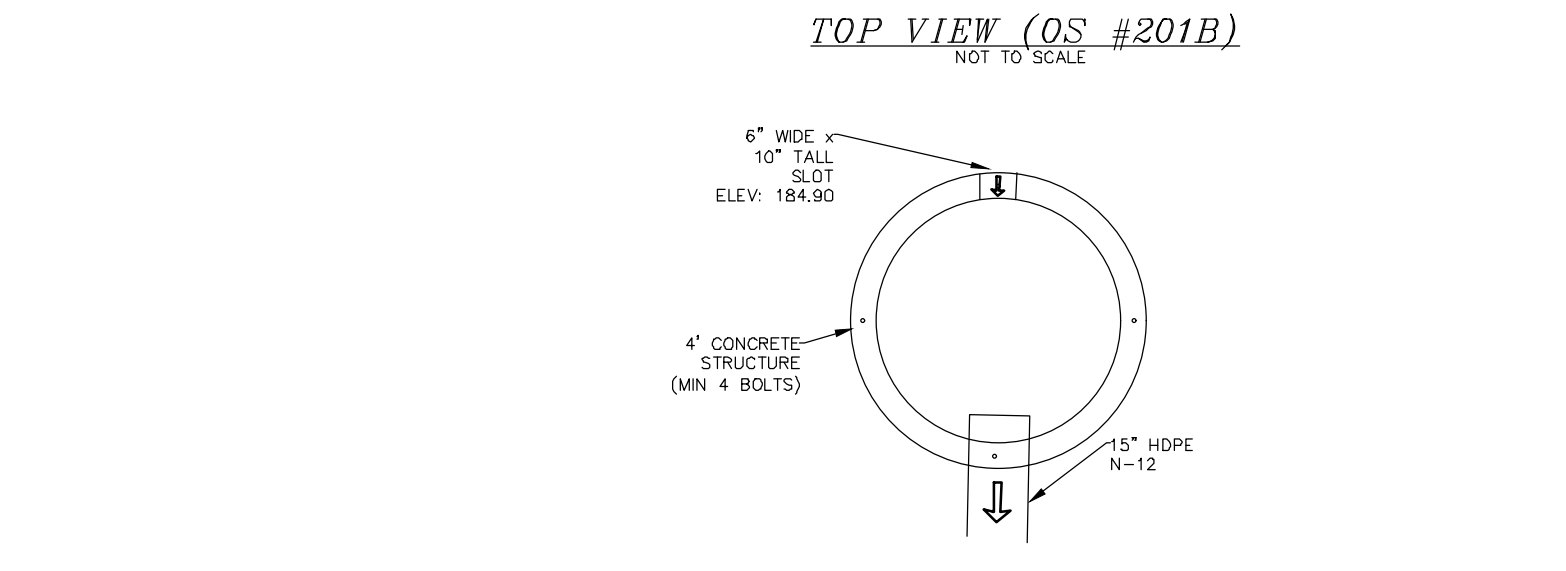
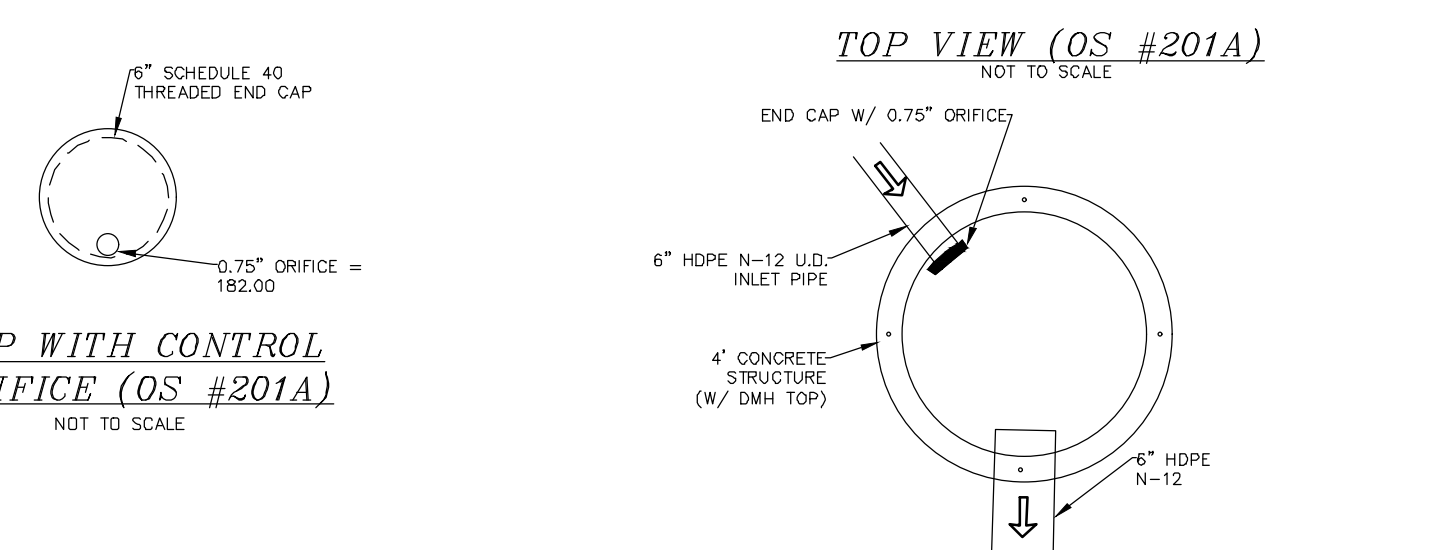


BIORETENTION W/ ISR BERM CONSTRUCTION  
NOT TO SCALE

BIORETENTION W/ ISR CONSTRUCTION SPECIFICATIONS

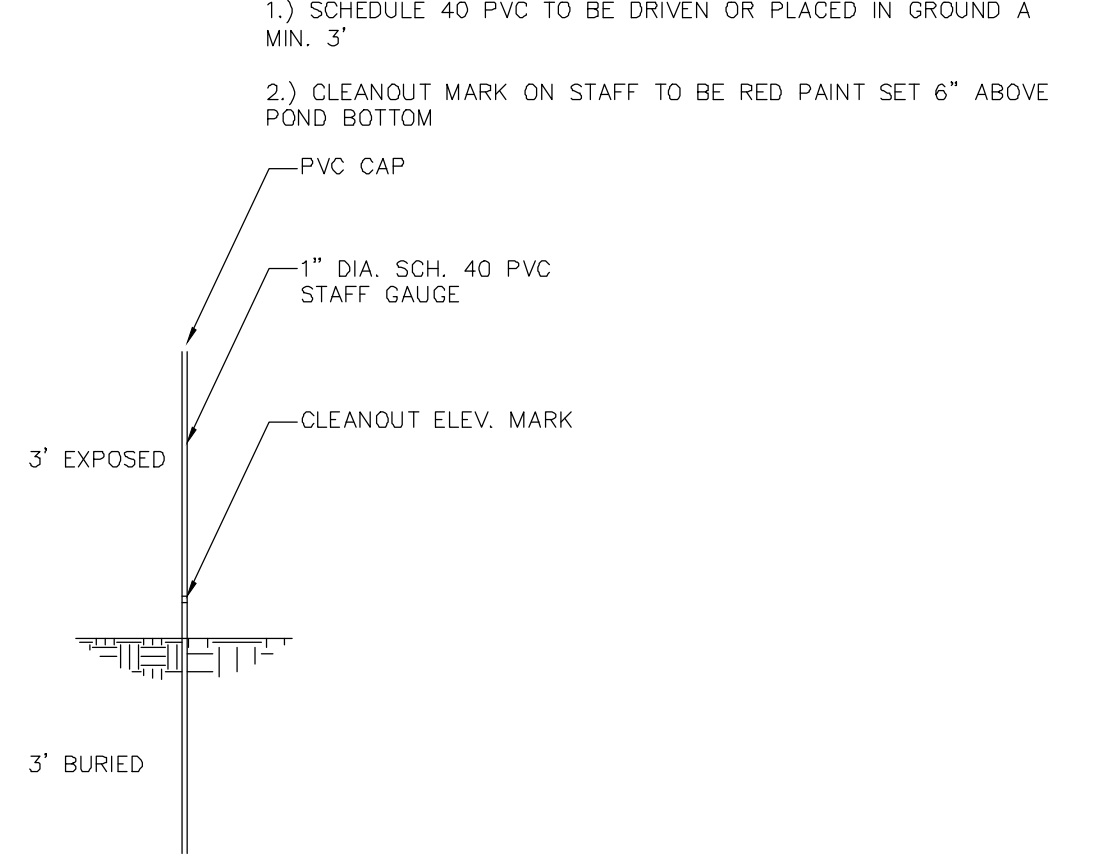
BIORETENTION W/ ISR STRUCTURES & DETAILS  
NOT TO SCALE

P4



P5

SEDIMENT FORBAY GAUGE DETAIL



FOR TOWN APPROVAL PURPOSES :  
THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE : N/A  
DATE : APRIL 17, 2024  
FILE NO. : DB 2023 - 017

FOR TOWN APPROVAL PURPOSES :  
FOR  
TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
74X MAP 286, LOT 44

STATE OF NEW HAMPSHIRE  
KENNETH A. BERRY  
No. 1111  
PROFESSIONAL ENGINEER

DETAILS  
P-201B



UNH STORMWATER CENTER SPECIFICATIONS  
ACCEPTABLE PARTICLE SIZE DISTRIBUTION OF  
FINAL BIORETENTION SOIL MIX

MEDIA TYPE	SIEVE #	SIZE (IN)	% PASSING
COURSE SAND	4	0.187	100
MEDIUM SAND	10	0.075	95
FINE SAND	40	0.017	15-40
SILTS	200	0.003	10-20
CLAYS	<200	PLAN	0-5

3/8" WASHED CRUSHED STONE\*

SIEVE SIZE	% PASSING BY WEIGHT
1/2"	100
3/8"	95 - 100
# 4	22 - 55
# 8	0 - 10

1-1/2" WASHED CRUSHED STONE\*

SIEVE SIZE	% PASSING BY WEIGHT
2"	100
1-1/2"	90 - 100
1"	20 - 55
1/2"	0 - 15
3/8"	0 - 5

**STABILIZATION NOTE:**  
SEE NOTE #6, SHEET E-102, DETAIL E-18  
SIDE SLOPES ARE TO BE STABILIZED WITHIN THREE  
WORKING DAY UPON COMPLETION OF FINAL GRADE.

**CONSTRUCTION NOTE:** THE DESIGN ENGINEER IS TO BE ON SITE PRIOR TO AND DURING THE  
PLACEMENT OF THE RESERVOIR STONE, AND TO WITNESS THE INSTALLATION OF THE UNDER-DRAIN  
AND LINER. PLEASE PROVIDE 48 HOURS NOTICE PRIOR TO REQUESTED INSPECTION.

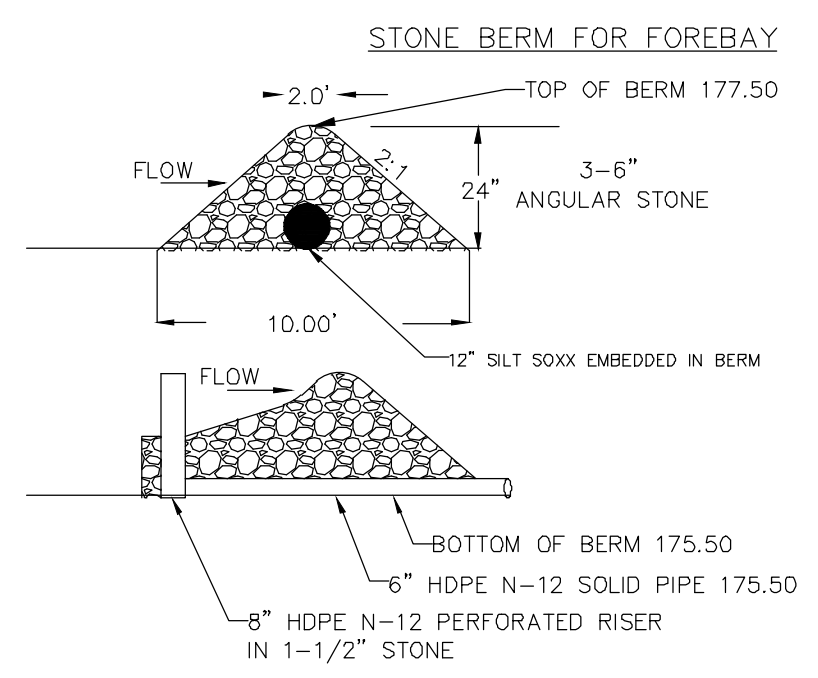
PVC LINER TO BE 40 TO 60 MIL. THE SEAMS SHALL BE SEALED

\*TESTING TO DETERMINE HYDRAULIC CONDUCTIVITY AND NEED FOR BOTTOM/SIDE SYSTEM LINER TO BE  
DETERMINED DURING CONSTRUCTION PROCESS.\*

\*EQUIVALENT TO STANDARD WASHED  
STONE - SECTION 702 OF NHDOT  
STANDARD SPECIFICATIONS

\*EQUIVALENT TO STANDARD WASHED  
STONE - SECTION 702 OF NHDOT  
STANDARD SPECIFICATIONS

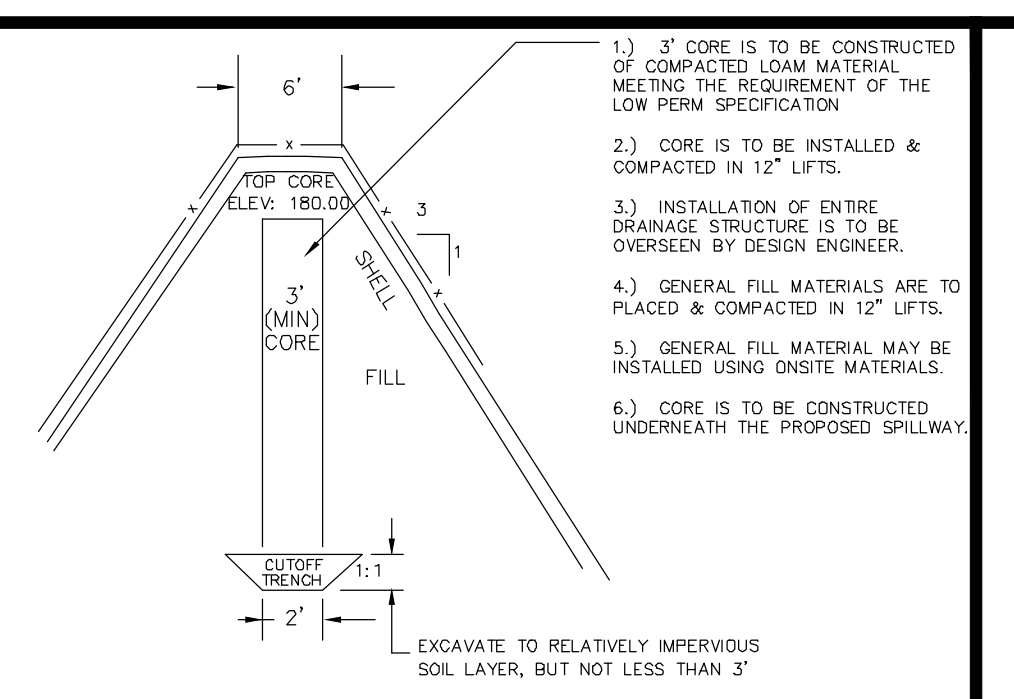
**P1**



**P2 P3**

LOW PERMEABILITY MATERIAL GRADATION

SIEVE SIZE	% PASSING BY WEIGHT
#4	95 - 100
#40	60 - 95
#100	40 - 60
#200	25 - 45



BIORETENTION SOIL MEDIA COMPONENTS  
-AMOUNTS MIXED BY TOTAL VOLUME  
\*60-85% SAND  
\*15-25% LOAM OR TOPSOIL  
\*3-8% ORGANIC MATTER  
\*5% WATER TREATMENT RESIDUALS OR 0.5% IRON FILINGS

WATER TREATMENT RESIDUALS OR IRON FILINGS ARE REQUIRED DUE TO THE CLASS "A" RECEIVING WATERS

REFER TO THE "UNHSC STANDARD DETAIL, BIORETENTION ISR STORMWATER SYSTEM", FEBRUARY 2020 FOR FURTHER PUBLISHED GENERAL CONSTRUCTION GUIDELINES

DESIGN REFERENCES

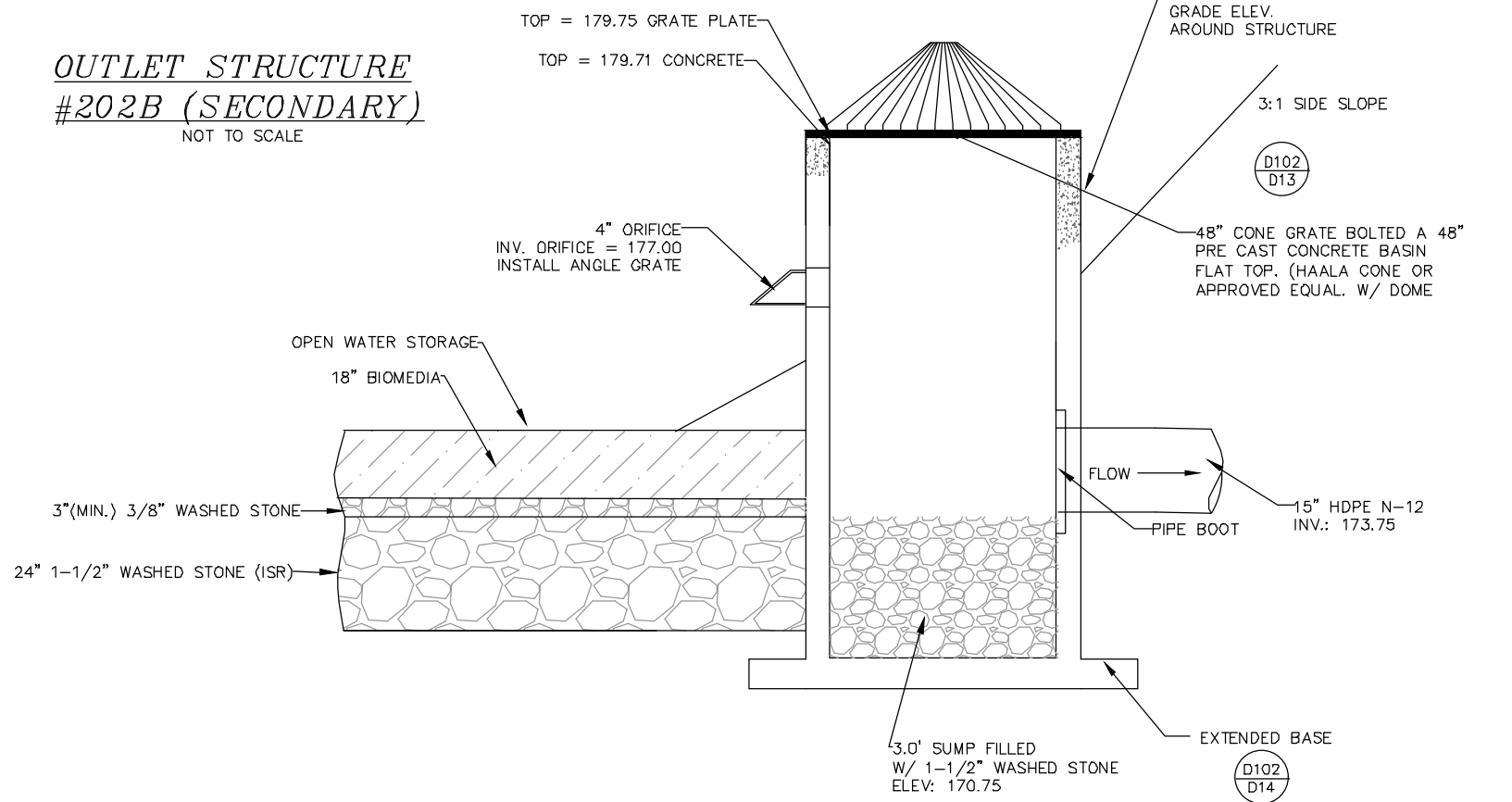
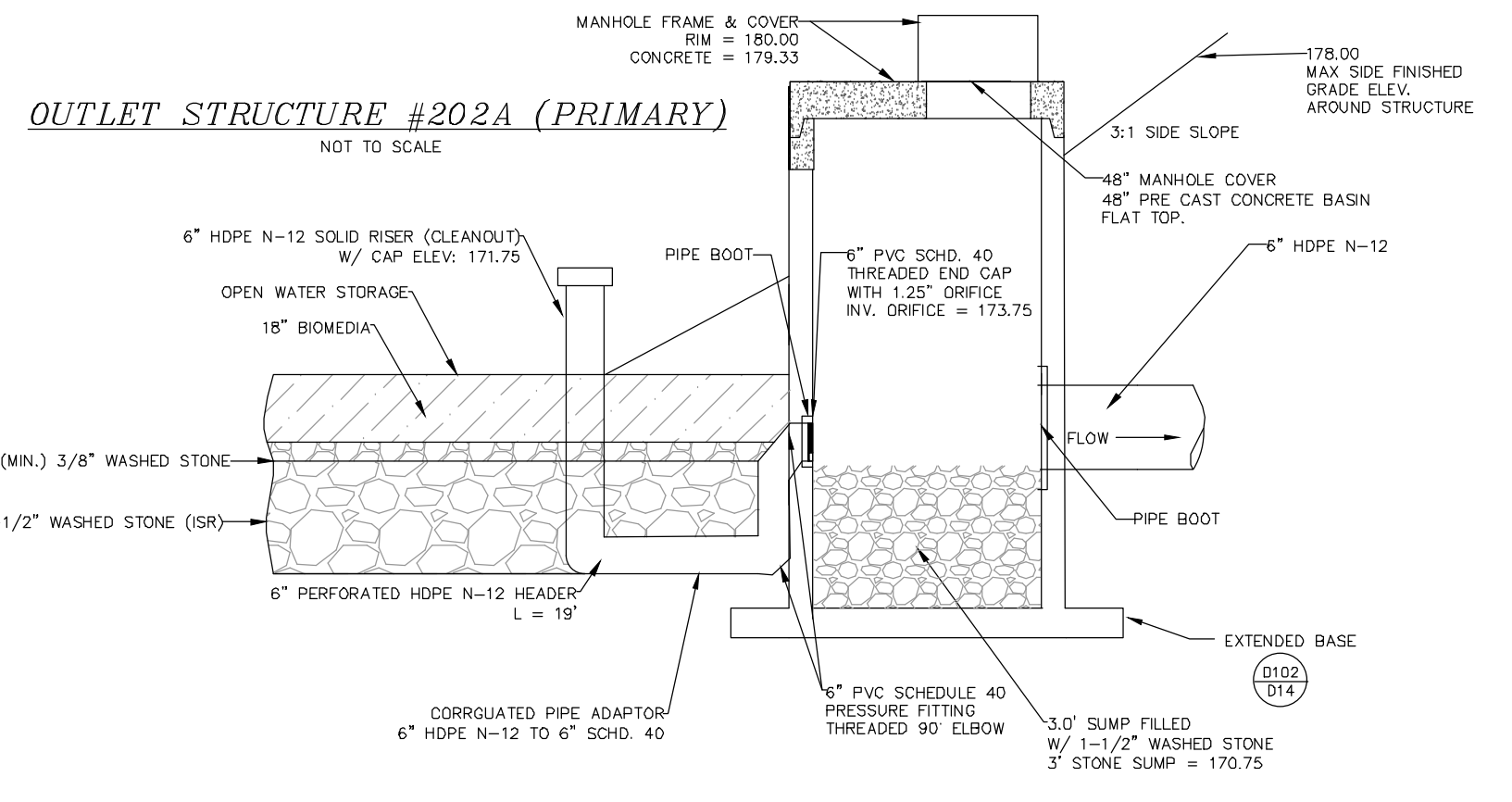
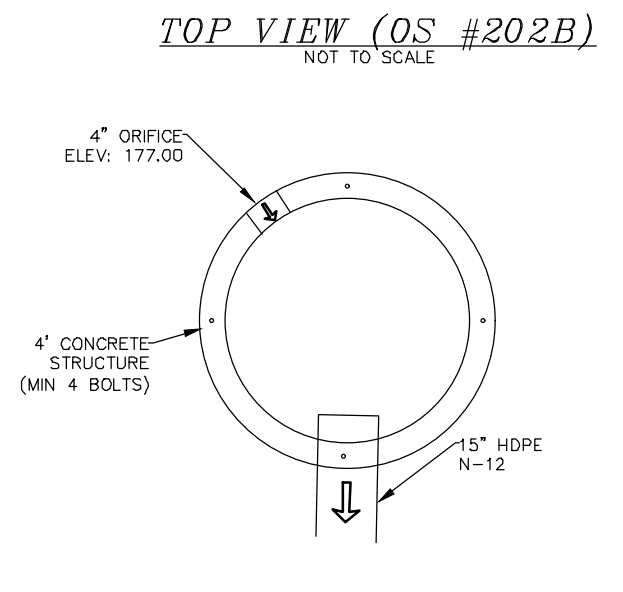
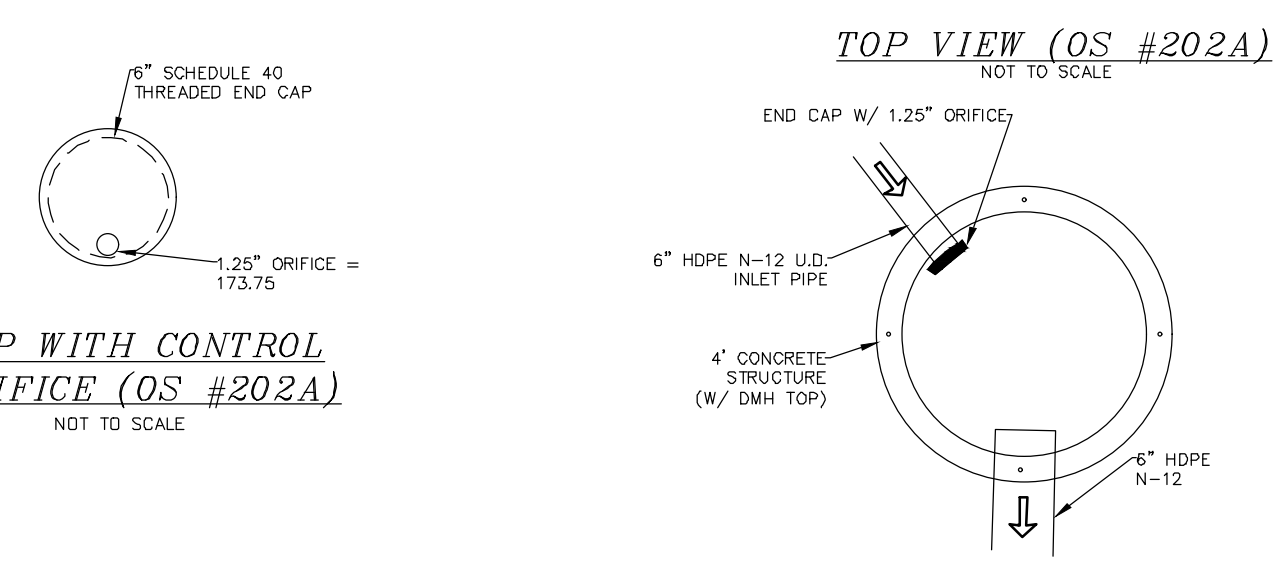
1 UNH STORMWATER CENTER.  
<https://scholars.unh.edu/stormwater/73/>

**BIORETENTION W/ ISR CONSTRUCTION SPECIFICATIONS**

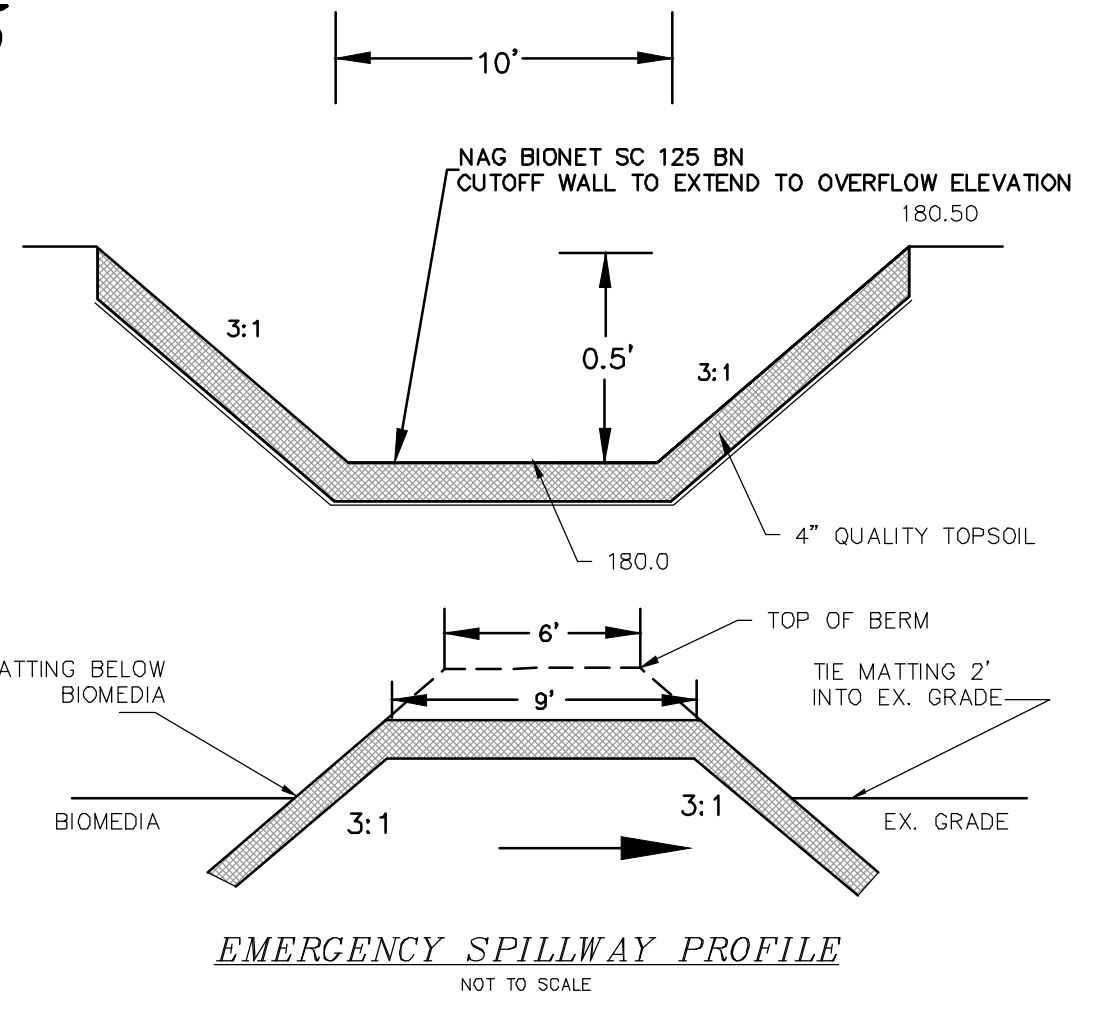
**BIORETENTION W/ ISR STRUCTURES & DETAILS**

NOT TO SCALE

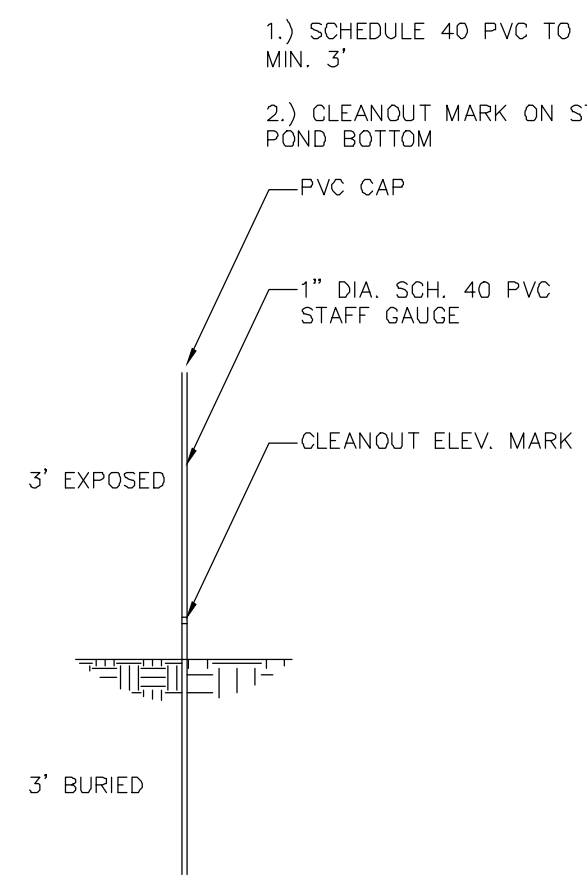
**P4**



**P5**



**SEDIMENT FORBAY GAUGE DETAIL P6**



FOR TOWN APPROVAL PURPOSES :

THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

FOR TOWN APPROVAL PURPOSES :

THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

FOR BIURETENTION W/ ISR #202 DETAILS

FOR TUREOCAM, INC. LAND OF VIRTUOUS REALTY, LLC NH ROUTE 125/CALEF HIGHWAY BARRINGTON, N.H. TAX MAP 286, LOT 44

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE : N/A  
DATE : APRIL 17, 2024  
FILE NO. : DB 2023 - 017

KENNETH A. BERRY  
REGISTERED PROFESSIONAL ENGINEER

DETAILS

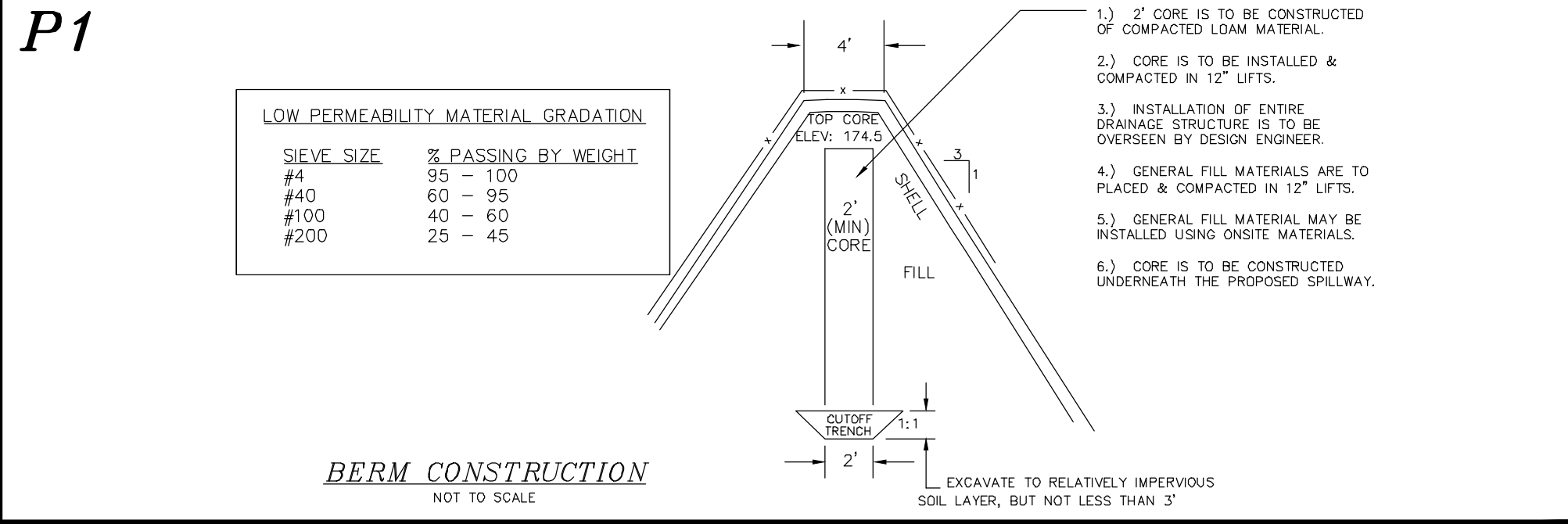
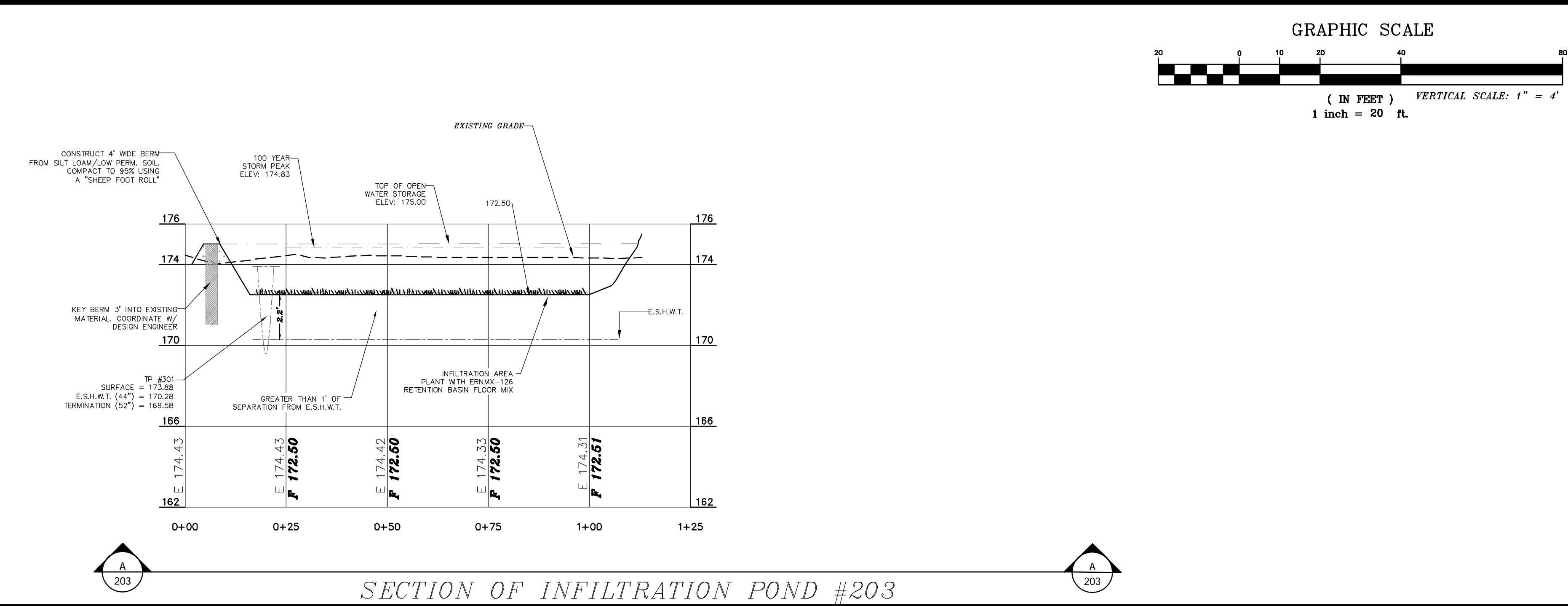
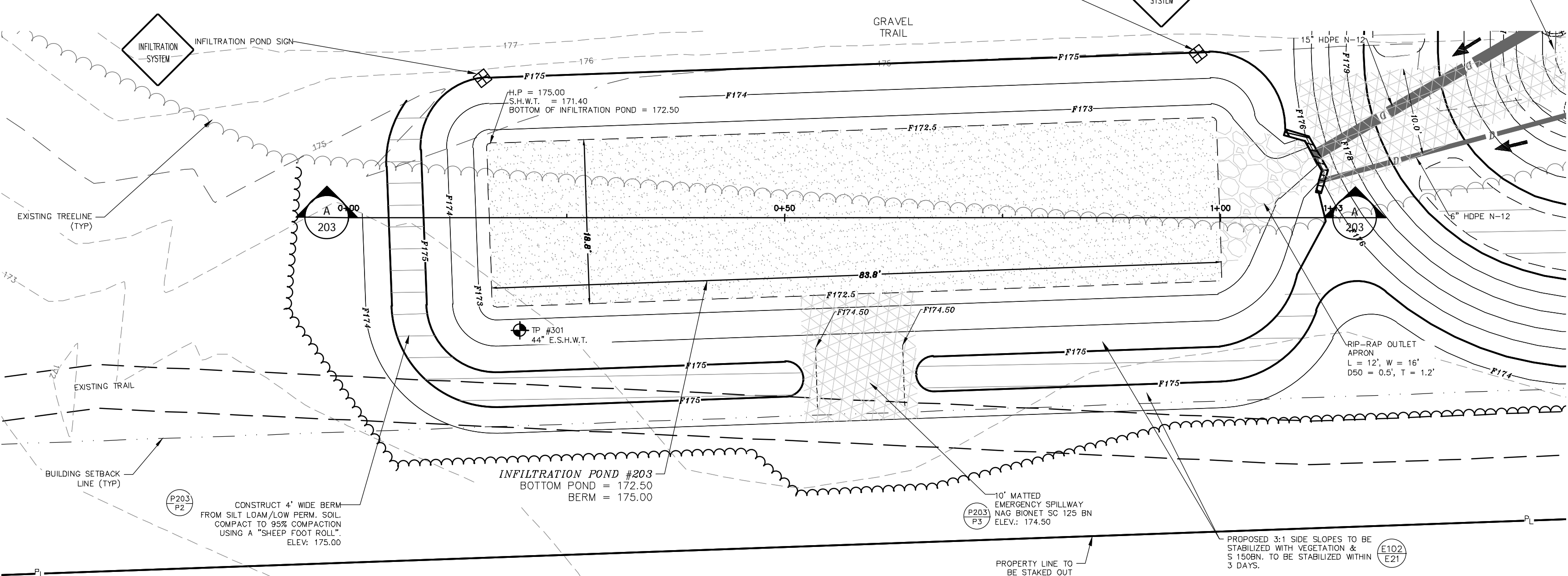
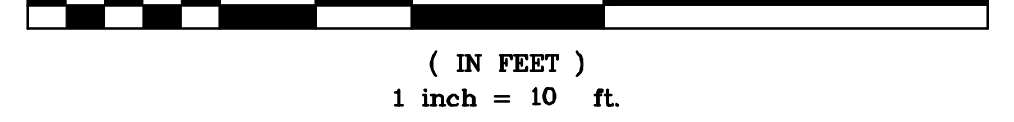
P-202B

SHEET 24 OF 43

REVISION	DATE	DESCRIPTION

PLAN VIEW  
INFILTRATION POND #203

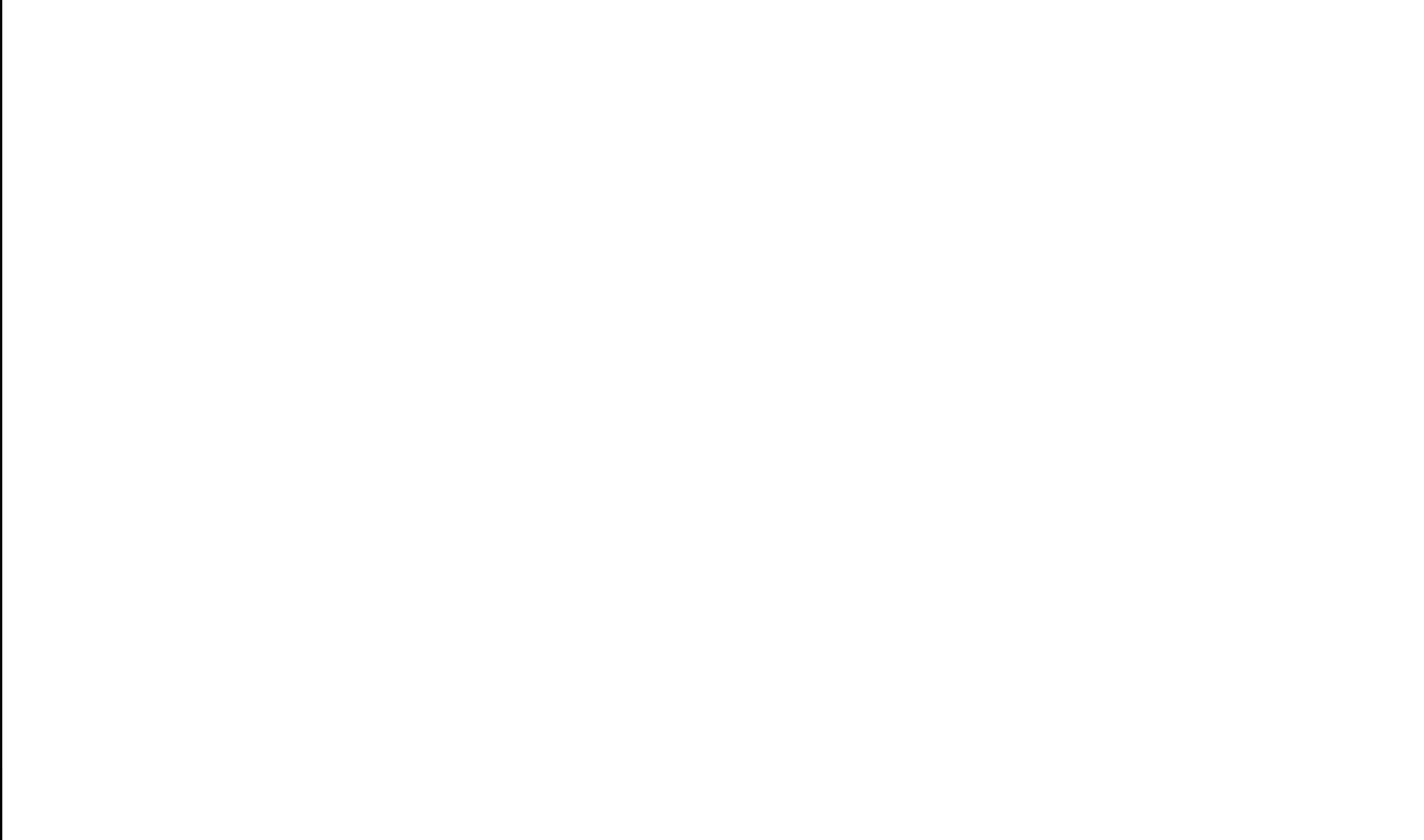
EXISTING LEGEND:	PROPOSED LEGEND:
○ IRON BOUND/IRON ROD (FND)	⊕ TEMPORARY BENCHMARK (T.B.M.)
○ IRON PIPE (FND)	DETAIL SHEET / DETAIL
□ STONE BOUND (FND)	— F176 CONTOUR MINOR, PROPOSED
○ UTILITY POLE / GUY WIRE	— F175 CONTOUR MAJOR, PROPOSED
○ SINGLE POST SIGN	— ROAD CENTERLINE
○ TEMPORARY BENCHMARK	— SHOULDER
○ WELL	— PAVEMENT SEAM
○ TESTHOLE	— DRAIN CULVERT W/ FLARED END SECTION (F.E.S.)
— PERIMETER BOUNDARY LINE	— UNDERDRAIN
— EXISTING CONSERVATION EASEMENT LINE	— GUARD RAIL
— BUILDING SETBACK LINE	— EXISTING SPOT GRADE
— NHDES PROTECTIVE WELL RADIUS	— PROPOSED SPOT GRADE
— OVERHEAD UTILITIES	— FLOW ARROW
— EXISTING CONTOUR MINOR	— RIP RAP
— EXISTING CONTOUR MAJOR	— STORMWATER BEST MANAGEMENT PRACTICE (BMP)
— POORLY DRAINED WETLAND LINE	— BERM
— VERY POORLY DRAINED WETLAND LINE	
— 50' WETLAND BUFFER LINE	
— TREE LINE	
FND FOUND	○ DRAIN MANHOLE W/ STRUCTURE
TYP TYPICAL	○ CATCH BASIN W/ STRUCTURE
S.C.R.D. STRAFFORD COUNTY	○ STORMWATER BMP OUTLET STRUCTURE
REGISTRY OF DEEDS	



**P1**

**STABILIZATION NOTE:**  
SEE NOTE #6, SHEET E-102, DETAIL E-18  
SIDE SLOPES ARE TO BE STABILIZED WITHIN THREE WORKING DAY UPON COMPLETION OF FINAL GRADE.

**INFILTRATION SURFACE PROTECTION NOTE:**  
SEE EROSION AND SEDIMENT CONTROL PLAN  
IT IS RECOMMENDED THAT THE CONTRACTOR TAKE MEASURES TO PROTECT THE INFILTRATION SURFACE FROM SEDIMENTATION CAUSED BY EROSION ON THE SIDE SLOPES



REVISION	DATE	DESCRIPTION

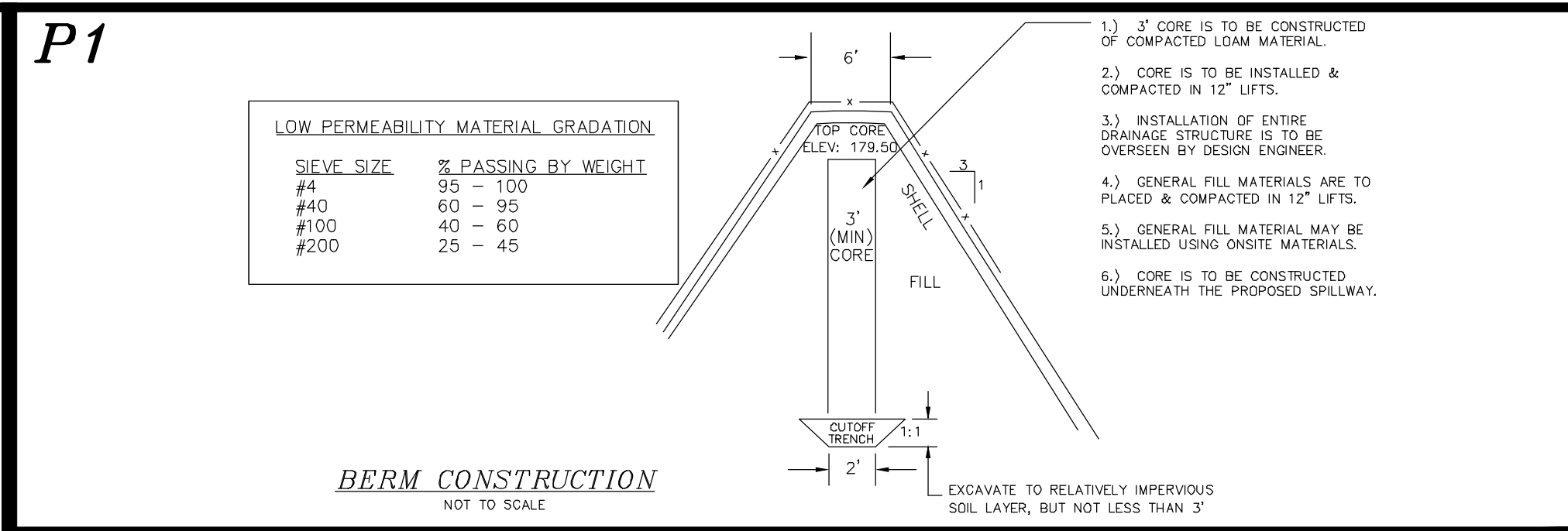
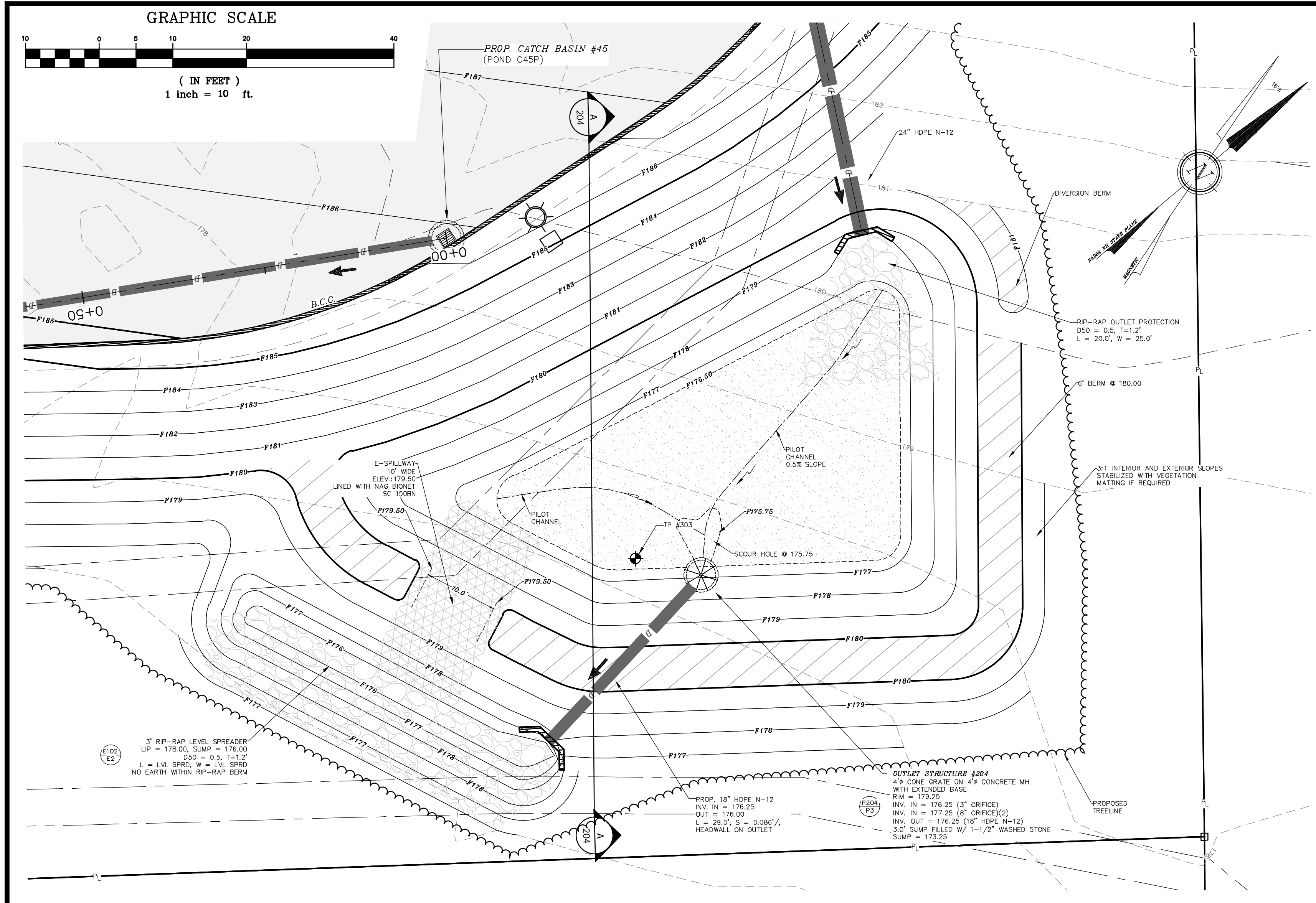
INFILTRATION POND #203  
FOR  
TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 286, LOT 44

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE : 1 IN. EQUALS 20 FT.  
DATE : APRIL 17, 2024  
FILE NO. : DB 2023 - 017

FOR TOWN APPROVAL PURPOSES :

THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.

KENNETH A. BERRY  
REGISTERED PROFESSIONAL ENGINEER



**P2**

**NOTES**

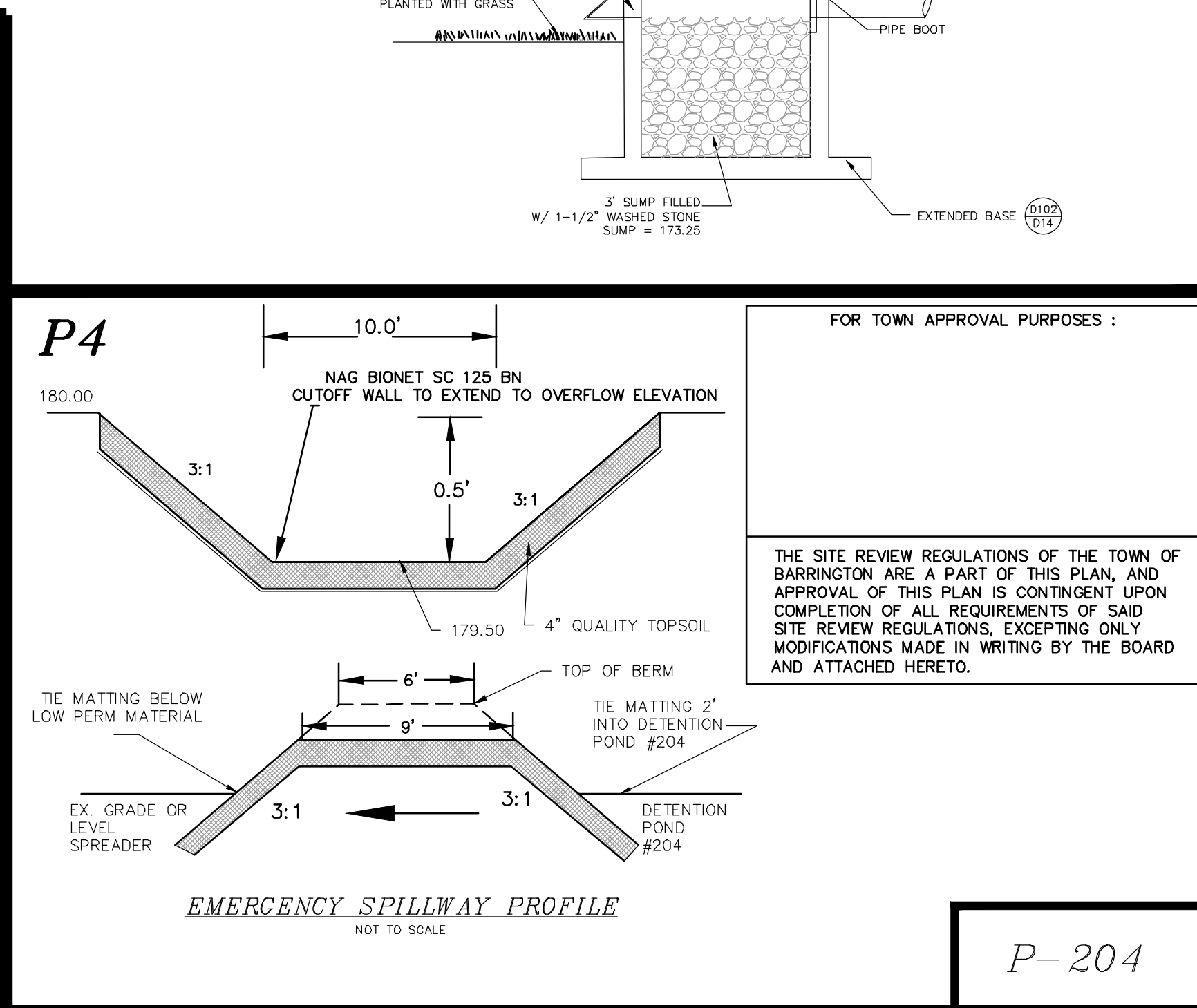
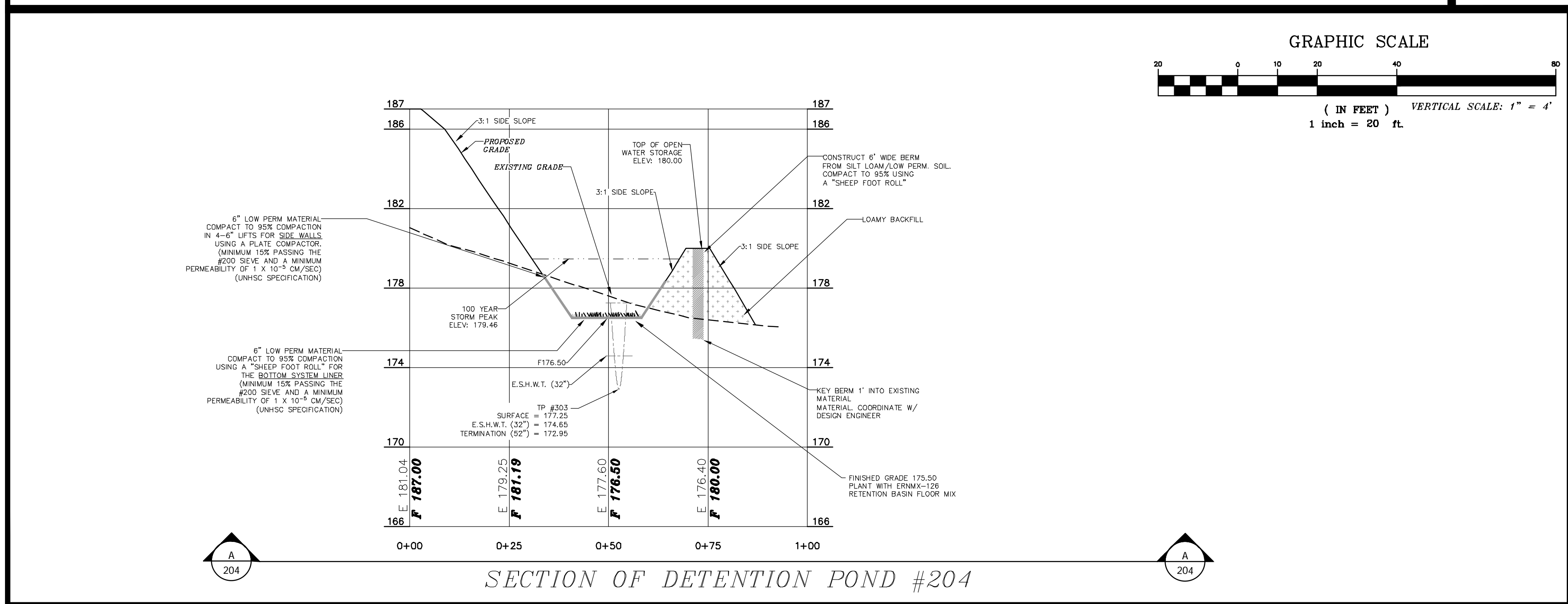
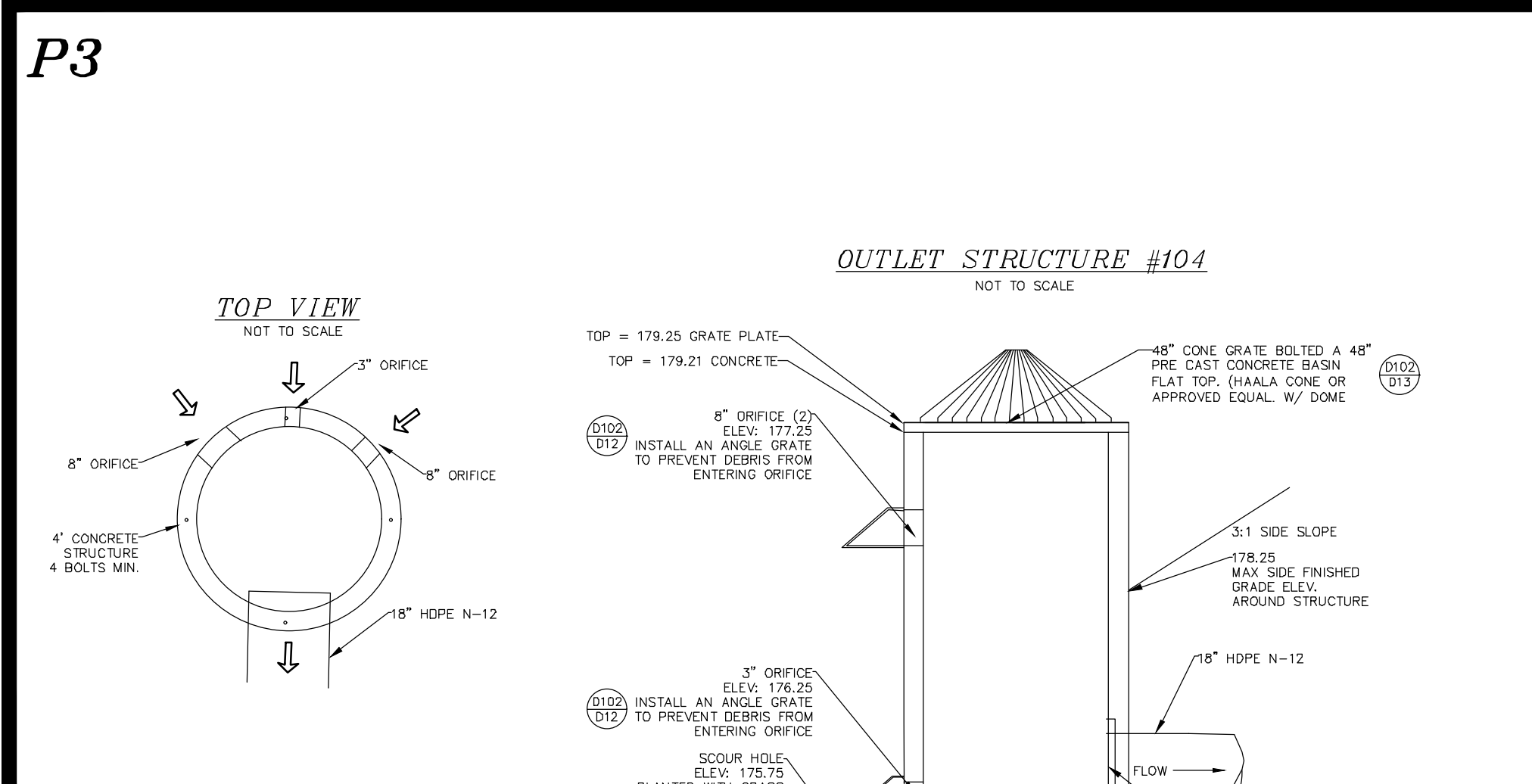
- DO NOT PLACE THE DETENTION SYSTEM INTO SERVICE UNTIL THE BMP HAS BEEN PLANTED AND ITS CONTRIBUTING AREAS HAVE BEEN FULLY STABILIZED.
- DO NOT DISCHARGE SEDIMENT-LADEN WATERS FROM CONSTRUCTION ACTIVITIES (RUNOFF WATER FROM EXCAVATIONS) TO THE DETENTION AREA DURING ANY STAGE OF CONSTRUCTION.
- DETENTION AREA TO BE PLANTED WITH ERNMX-126 RETENTION BASIN FLOOR MIX.

**MAINTENANCE REQUIREMENTS**

- SYSTEMS SHOULD BE INSPECTED AT LEAST TWICE ANNUALLY, AND FOLLOWING ANY RAINFALL EXCEEDING 0.25 INCHES IN A 24-HOUR PERIOD, WITH MAINTENANCE OR REHABILITATION CONDUCTED AS WARRANTED BY SUCH INSPECTION.
- PRETREATMENT MEASURES SHOULD BE INSPECTED AT LEAST TWICE ANNUALLY, AND CLEANED OF ACCUMULATED SEDIMENT AS WARRANTED BY INSPECTION, BUT NO LESS THAN ONCE ANNUALLY.
- VEGETATION SHOULD BE INSPECTED AT LEAST ANNUALLY, AND MAINTAINED IN HEALTHY CONDITION, INCLUDING PRUNING, REMOVAL, AND REPLACEMENT OF DEAD OR DISEASED VEGETATION, AND REMOVAL OF INVASIVE SPECIES.

**DESIGN REFERENCES**

- UNH STORMWATER CENTER
- NEW HAMPSHIRE STORMWATER MANAGEMENT MANUAL, VOLUME 2, DECEMBER 2008 AS AMENDED.



REVISION	DATE	DESCRIPTION

DETENTION POND #204

FOR TURBOCAM, INC.  
LAND OF  
VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
74X MAP 286, LOT 44

**BERRY SURVEYING & ENGINEERING**  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE: 1 IN. EQUALS 10 FT.  
DATE: APRIL 17, 2024  
FILE NO.: DB 2023 - 017

**KENNETH A. BERRY**  
REGISTERED PROFESSIONAL ENGINEER

**P-204**

SHEET 26 OF 43

# **Control of Invasive Plants**

**New Hampshire**  
**Department of Agriculture,**  
**Markets & Food**  
**Douglas Cygan**  
**603-271-3488**  
[doug.cygan@agr.nh.gov](mailto:doug.cygan@agr.nh.gov)

This guide lists garden plants and weeds which are already causing significant changes to natural areas in the Mid-Atlantic. **Measures for controlling each species are indicated by number, e.g., (3), in the text with a full explanation at the end of this article.** Click on the word [Control](#): to jump to that section. Then click your "back" button to return to the text. Following each section suggested alternative plants are given. These alternatives are native plants, well adapted and needing little care, attractive to birds and butterflies, and an important part of the food web for our indigenous species.

## INVASIVE TREES

**NORWAY MAPLE** (*Acer platanoides*) has large leaves similar to sugar maple. To easily confirm that the plant is Norway maple, break off a leaf and if it's truly Norway maple it will exude milky white sap. Fall foliage is yellow. (Exception: cultivars such as 'Crimson King,' which have red leaves in spring or summer, may have red autumn leaves.) The leaves turn color late, usually in late October after native trees have dropped their foliage. This tree suppresses growth of grass, garden plants, and forest understory beneath it, at least as far as the drip-line. Its wind-borne seeds can germinate and grow in deep shade. The presence of young Norway maples in our woodlands is increasing.

[Control](#): (1); (7), (8), (9), or (10); (11) in mid-October to early November, before the leaves turn color.

**TREE OF HEAVEN** (*Ailanthus altissima*), is incredibly tough and can grow in the poorest conditions. It produces huge quantities of wind-borne seeds, grows rapidly, and secretes a toxin that kills other plants. Its long compound leaves, with 11-25 lance-shaped leaflets, smell like peanut butter or burnt coffee when crushed. Once established, this tree cannot be removed by mechanical means alone.

[Control](#): (1) - seedlings only. Herbicide - use Garlon 3a (9) with no more than a 1" gap between cuts, or (10); plus (11) on re-growth. Or paint bottom 12" of bark with Garlon 4 Ultra (in February or March to protect surrounding plants). USE MAXIMUM STRENGTH SPECIFIED ON LABEL for all herbicide applications on Ailanthus. Glyphosate is not effective against Ailanthus.

## INVASIVE SHRUBS

**AUTUMN OLIVE** (*Eleagnus umbellata*): Formerly recommended for erosion control and wildlife value, these have proved highly invasive and diminish the overall quality of wildlife habitat.

[Control](#): (1) - up to 4" diameter trunks; (7) or (10) or bury stump. Do not mow.

**MULTIFLORA ROSE** (*Rosa multiflora*), formerly recommended for erosion control, hedges, and wildlife habitat, becomes a huge shrub that chokes out all other vegetation and is too dense for many species of birds to nest in, though a few favor it. In shade, it grows up trees like a vine. It is covered with white flowers in June. (Our native roses have fewer flowers, mostly pink.) Distinguish multiflora by its size, and by the presence of very hard, curved thorns, and a fringed edge to the leaf stalk.

[Control](#): (1) - pull seedlings, dig out larger plants at least 6" from the crown and 6" down; (4) on extensive infestations; (10) or (11). It may remain green in winter, so herbicide may *applied when other plants are dormant. For foliar application, mix Rodeo with extra sticker-spreader, or use Roundup Sure Shot Foam on small plants.*

**BUSH HONEYSUCKLES** (*Lonicera spp.*), including Belle, Amur, Morrow's, and Tatarian honeysuckle. (In our region, assume that any honeysuckle is exotic unless it is a scarlet-flowered vine). Bush honeysuckles create denser shade than native shrubs, reducing plant diversity and eliminating nest sites for many forest interior species.

Control: (2) on ornamentals; (1); on shady sites only, brush cut in early spring and again in early fall (3); (4) during the growing season; (7); or (10) late in the growing season.

**BLUNT-LEAVED PRIVET** (*Ligustrum obtusifolium*). Control: (1); (7) or (10); or trim off all flowers. Do not cut back or mow.

**BURNING BUSH, WINGED EUONYMUS** (*Euonymus alatus*), identified by wide, corky wings on the branches.

Control: (1); (7) or (10); or trim off all flowers.

**JAPANESE BARBERRY** (*Berberis thunbergii*), and all cultivars and varieties.

Control: (1); (7) or (10); or trim off all flowers.

### INVASIVE WOODY VINES

All of these vines shade out the shrubs and young trees of the forest understory, eventually killing them, and changing the open structure of the forest into a dense tangle. DO NOT PLANT NEXT TO OPEN SPACE.

**JAPANESE HONEYSUCKLE** (*Lonicera japonica*), including Hall's honeysuckle, has gold-and-white flowers with a heavenly scent and sweet nectar in June. This is probably the familiar honeysuckle of your childhood. It is a rampant grower that spirals around trees, often strangling them.

Control: (1); (3); (10); (11) in fall or early spring when native vegetation is dormant. Plan to re-treat repeatedly.

**ORIENTAL BITTERSWEET** (*Celastrus orbiculatus*) has almost completely displaced American bittersweet (*C. scandens*). The Asian plant has its flowers and bright orange seed capsules in clusters all along the stem, while the native species bears them only at the branch tips.

Control: (1); keep ornamental plants cut back, remove all fruits as soon as they open, and bag or burn fruits; to eradicate use Garlon 3a (10).

**JAPANESE KNOTWEED, MEXICAN BAMBOO** (*Polygonum cuspidatum*) can grow in shade. The stems have knotty joints, reminiscent of bamboo. It grows 6-10' tall and has large pointed oval or triangular leaves.

Control: Cut at least 3 times each growing season and/or treat with Rodeo (10) or (11). In gardens, heavy mulch or dense shade may kill it.

### INVASIVE HERBACEOUS PLANTS

**GARLIC MUSTARD** (*Alliaria petiolata*, *A. officinalis*), a white-flowered biennial with rough, scalloped leaves (kidney-, heart- or arrow-shaped), recognizable by the smell of garlic and taste of mustard when its leaves are crushed. (The odor fades by fall.)

Control: Pull before it flowers in spring (1), removing crown and roots. Tamp down soil afterwards. Once it has flowered, cut (2), being careful not to scatter seed, then bag and burn or send to the landfill. (11) may be appropriate in some settings.

**JAPANESE STILT GRASS** (*Microstegium vimineum*) can be identified by its lime-green color and a line of silvery hairs down the middle of the 2-3" long blade. It tolerates sun or dense shade and quickly invades areas left bare or disturbed by tilling or flooding. An annual grass, it builds up a large seed bank in the soil.

Control: Easily pulled in early to mid-summer (1) - be sure to pull before it goes to seed. If seeds have formed, bag and burn or send to landfill. Mowing weekly or when it has just begun to flower may prevent it from setting seed (3). Use glyphosate (11) or herbicidal soap (less effective) on large infestations. Follow up with (5) in spring.

**MILE-A-MINUTE VINE, DEVIL'S TAIL TEARTHUMB** (*Polygonum perfoliatum*), a rapidly growing annual vine with triangular leaves, barbed stems, and turquoise berries in August which are spread by birds. It quickly covers and shades out herbaceous plants.

Control: same as for stilt grass.

**SPOTTED KNAPWEED** (*Centaurea maculosa*), a biennial with thistle-like flowers.

Control: Do NOT pull (1) unless the plant is young and the ground is very soft - the tap root will break off and produce several new plants. Wear sturdy gloves. (2); (6); (10) or (11).

-----  
**CONTROL MEASURES**

- (1) PULL seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems. Use a forked spade or weed wrench for trees or shrubs.
- (2) DEADHEAD to prevent spread of seeds of invasive plants. Cut off seeds or fruits before they ripen. Bag, and burn or send to a landfill.
- (3) MOW or CUTTING at least 4 times a season to deplete plants' store of nutrients and carbohydrates, reduce seed formation, and kill or minimize spread of plants. If necessary, repeat each year.
- (4) CONTROLLED BURNING during the spring, repeated over several years, allows native vegetation to compete more effectively with the invasive species. This requires a permit. Spot treatment with glyphosate in late fall can be used to make this method more effective.
- (5) Use a CORN-BASED PRE-EMERGENCE HERBICIDE on annual weeds. This product is also an organic fertilizer, i.e., it can stimulate growth of existing plants, including weeds, so it is appropriate for lawns and gardens but may not be appropriate in woodlands.
- (6) In lawns, SPOT TREAT with BROAD-LEAF WEEDKILLER. Good lawn-care practices (test soil; use lime and fertilizer only when soil test shows a need; mow high and frequently; leave clippings on lawn) reduce weed infestations.
- (7) CUT DOWN the tree. Grind out the stump, or clip off re-growth.
- (8) GIRDLE tree: cut through the bark and growing layer (cambium) all around the trunk, about 6" above the ground. Girdling is most effective in spring when the sap is rising, and from middle to late summer when the tree is sending down food to the roots. Clip off sucker sprouts.
- (9) FRILL: Using a machete, hatchet or similar device, hack scars (several holes in larger trees) downward into the cambium layer, and squirt in glyphosate (or triclopyr if recommended in text above). Follow label directions for Injection and Frill Applications. This is most effective from middle to late summer. Clip off any sucker sprouts or treat with glyphosate.
- (10) CUT STEM / CUT STUMP WITH GLYPHOSATE (or triclopyr if specified above). Follow label directions for Cut Stump Application. Clip off sucker sprouts or paint with glyphosate. See Note on Herbicides.
- (11) FOLIAR SPRAY WITH GLYPHOSATE herbicide (see Note on Herbicides). Use a backpack or garden sprayer or mist blower, following label directions. Avoid overspray and/or dripping onto non-target plants, because glyphosate kills most plants except moss. If it rolls off waxy or grass-like foliage, use additional sticker-spreader. Deciduous trees, shrubs, and perennials move nutrients down to the roots in late summer. Glyphosate is particularly effective at this time and when plants have just gone out of flowering. Several invasive species retain their foliage after native plants have lost theirs, and resume growth earlier in spring than most natives. This allows you to treat them without harming the natives. However, the plant must be actively growing for the herbicide to work. Retreatments may be necessary the following year if suckering occurs or the plant hasn't been entirely killed.

NOTE ON HERBICIDES: It is highly recommended that small populations try to be controlled using non-chemical methods wherever feasible. However, for large infestations, and for a few plants specified above, herbicide use is essential. Apply herbicides carefully to avoid non-target plants, glyphosate is the least environmentally damaging herbicide in most cases. Add food coloring for visibility, and a soap-based sticker such as Cide-Kick. Glyphosate is ineffective on some



plants; for these, triclopyr (Garlon) may be indicated. When using herbicides, read the entire label and observe all precautions listed, including proper disposal. If in doubt, call your local Cooperative Extension Service.

Pavement Temp. (°F) and Trend (↑ ↓)	Weather Condition	Maintenance Actions	Application Rate (lbs/per 1000 sq.ft.)			
			Salt Prewetted/Pre treated with salt brine	Salt Prewetted/Pret reated with other blends	Dry salt	Winter sand
>30 ↑	Snow	Plow, treat intersections only				Not recommended
	Frz. Rain	Apply chemical				Not recommended
30 ↓	Snow	Plow and apply chemical				Not recommended
	Frz. Rain	Apply chemical				Not recommended
25 - 30 ↑	Snow	Plow and apply chemical				Not recommended
	Frz. Rain	Apply chemical				Not recommended
25 - 30 ↓	Snow	Plow and apply chemical				Not recommended
	Frz. Rain	Apply chemical				3.25
20 - 25 ↑	Snow or frz. Rain	Plow and Apply chemical				3.25 for frz. Rain
20 - 25 ↓	Snow	Plow and apply chemical				Not recommended
	Frz. Rain	Apply chemical				3.25
15 - 20 ↑	Snow	Plow and apply chemical				Not recommended
	Frz. Rain	Apply chemical				3.25
15 - 20 ↓	Snow or Frz. Rain	Plow and apply chemical				3.25 for frz. Rain
0 to 15 ↑↓	Snow	Plow, treat with blends, sand hazardous areas	Not recommended		Not recommended	5.0 and spot-treat as needed
< 0	Snow	Plow, treat with blends, sand hazardous areas	Not recommended		Not recommended	5.0 and spot-treat as needed

**Table 19. Application Rates for Deicing**

These rates & table format are based on road application guidelines (Mn Snow & Ice Control Field Handbook, Manual 2005-1). Develop your own application rates by adjusting your current rates incrementally downward toward these guidelines. Where temperature categories overlap, select the rate most applicable to your situation.

# Infiltration Feasibility Report

**607 Calef Highway  
Barrington, NH  
Tax Map 238, Lot 44**

Prepared for

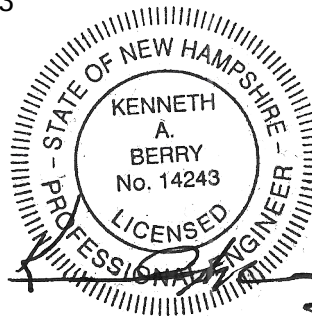
TURBOCAM, INC.  
607 Calef Highway Suite 200  
Barrington, NH 03825

Land of

Virtuous Realty LLC  
607 Calef Highway Suite 200  
Barrington, NH 03825

Prepared By

Berry Surveying & Engineering  
335 Second Crown Point Road  
Barrington, NH 03825  
603-332-2863



File Number  
DB2023-017

February 5, 2024  
Revised: April 17, 2024

## Table of Contents

1.0	Location of Practice	Page 2
2.0	Existing Topography at Location of Practice	Page 2
3.0	Test Pit Location	Page 2
4.0	Seasonal High Water Table and Bedrock Elevations	Page 3
5.0	Profile Descriptions	Page 3
6.0	Soil Plan	Page 4
7.0	Summary of Infiltration Rates	Page 5

## 1.0 Location of Practices:

The project proposes one location of infiltration for ground water recharge as well as channel flow protection purposes via Infiltration Pond #203.

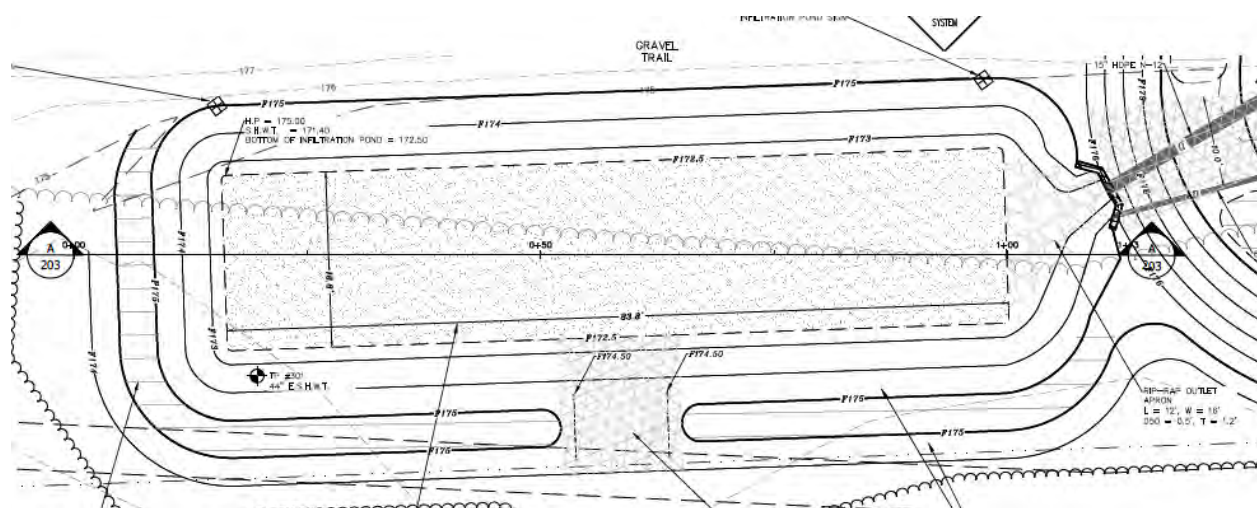
**Infiltration Pond #203 (Pond #203)** – This Infiltration Pond is on the easterly side of the parcel along the southeastern property line adjacent to the existing rec field to the northwest and Bioretention Pond w/ ISR #202 to the northeast. Treated flow is received from Bioretention W/ ISR #202 as well as overflow runoff during larger storms.

## 2.0 Existing Topography at the Location of the Practice

**Infiltration Pond #203 (Pond #103)** – The existing topography within the area is at a 2-3% slope. The area is currently vacant, unmaintained land with walking trails in the area.

## 3.0 Test Pit Locations

**Infiltration Pond #203 (Pond #203)** – The practice has a surface area of 1,575 SF at the lowest point. The practice is located over test pit #301. See test pit profile below. See test pit locations on Sheet P-203, Infiltration Pond #203 Plan. The test holes were completed in January & March 2024, (See Site Specific Soil Reports by John P Hayes III). The soils in the vicinity of this practice are Hinckley (12B) considered to be HSG A soil and Udorthents (400E) (Derived from Windsor and Deerfield soils) considered to be HSG B soil. The most restrictive published Ksat for both soils is 6 inches per hour. This practice was designed using 3 in. / hr.

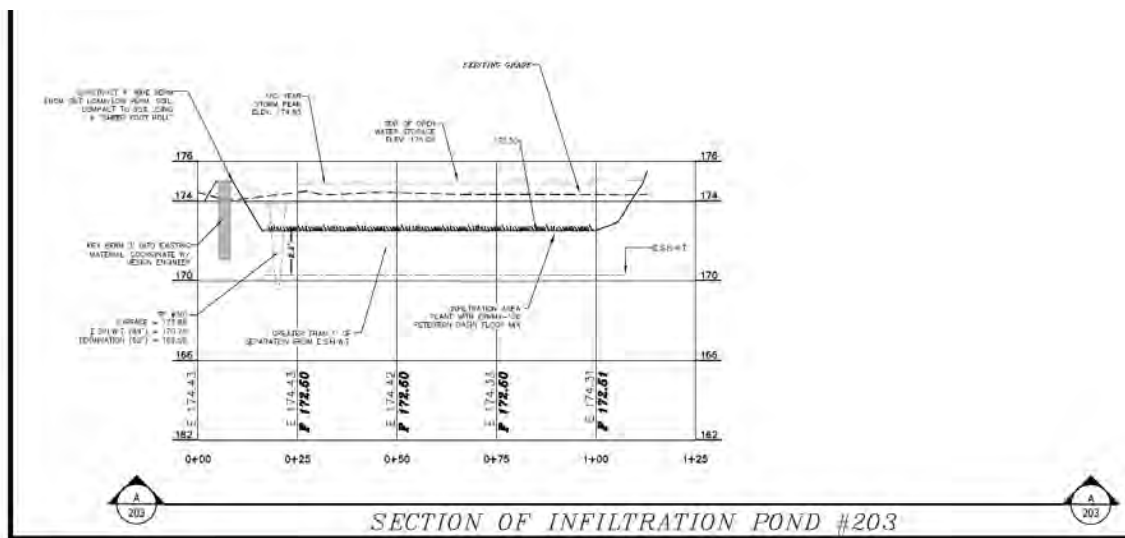


### 4.0 Seasonal High Water Table (SHWT) and Bedrock Elevations

TP #301:	Existing Surface Elevation of TP =	173.88'
	SHWT = 44 Inches	170.21'
	Bedrock > 52 Inches	<169.55'
	Ground Water > 52 Inches	<169.55'
	Deepest Elevation of TP = 52 Inches	169.55'

Infiltration Pond #203 (Pond #203): Inv. Pond Bottom 172.50'

See cross section below.



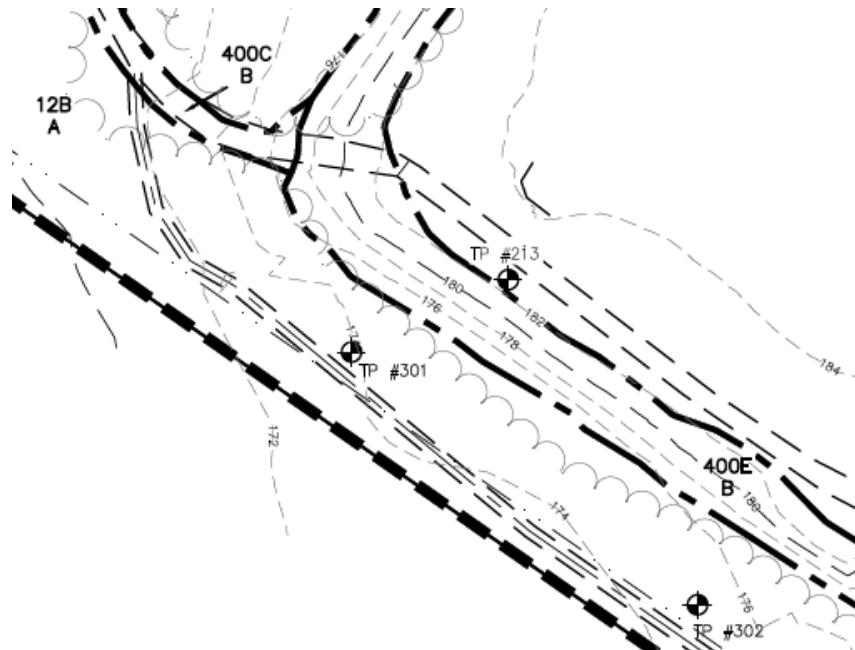
### 5.0 Profile descriptions

The following test pit data was collected, see profiles below.

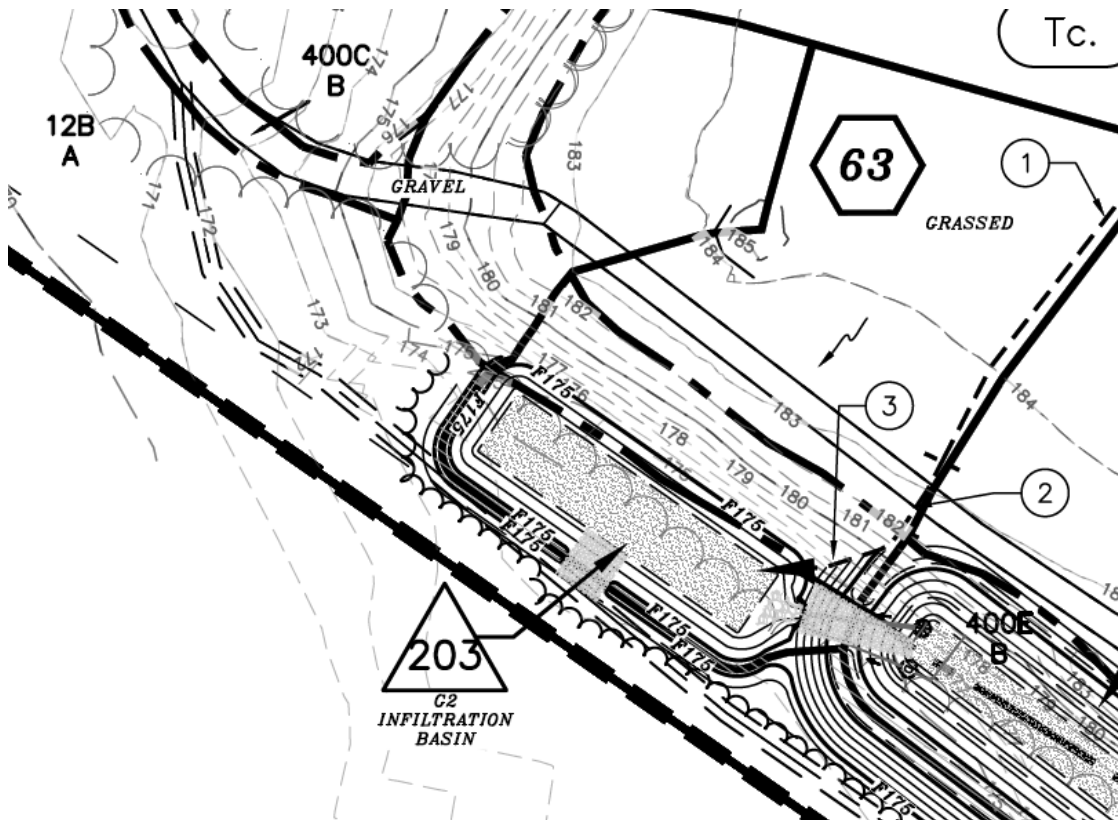
#### TEST PIT #301

- 0-8 10YR 3/2 DARK GRAYISH BROWN, LOAMY SAND, GRANULAR, FRIABLE
  - 8-18 10YR 5/6 YELLOWISH BROWN, GRAVELLY LOAMY SAND, GRANULAR, FRIABLE
  - 18-30 10YR 6/4 LIGHT YELLOWISH BROWN, GRAVELLY LOAMY SAND, GRANULAR, FRIABLE
  - 30-44 10YR 6/4 LIGHT YELLOWISH BROWN, SAND, SINGLE GRAIN, LOOSE
  - 44-52 2.5YR 5/3 LIGHT YELLOWISH BROWN, GRAVELLY SAND WITH REDOX. FEAT. PRESENT, SINGLE GRAIN, LOOSE
- E.S.H.W.T. @ 44"  
 RESTRICTIVE LAYER @ N/A  
 GROUND WATER @ N/A  
 TERMINATED @ 52"  
 REFUSAL @ N/A

## 6.0 Soil Plan in the Area of the Constructed Practice



Infiltration Pond #203 (Pond #203) is located over Hinckley and Udorthents (Derived from Windsor and Deerfield) soil. See Test Pits #301.



Infiltration Pond #203 (Pond #203)

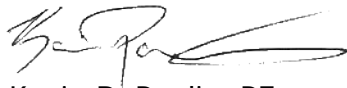
## 7.0 Summary of Infiltration Rate

Infiltration Pond #203 is located in Hinckley (12B) which is considered to be HSG A and Udorthents (400E) (Derived from Windsor and Deerfield) which is considered to be HSG B, soil area as mapped by Site Specific Soil Survey by John P. Hayes III, CSS, with a documented Ksat of 6 inches per hour. The design exfiltration rate for the infiltration practices is 3 inches per hour.

Amoozemeter testing was not conducted on site and the alternate method of using the USDA / NRCS published values was employed. Reference is made to K Sat Values for New Hampshire Soils (Including Hydrologic and DES Soil Lot Sizing Groups, sponsored by the Society of Soil Scientists of Northern New England, Publication #5 dated September 2009.

Respectfully submitted:

BERRY SURVEYING & ENGINEERING



Kevin R. Poulin, PE  
Project Engineer



Kenneth A. Berry, PE, LLS  
CPSWQ, CPESC, CESSWI  
Principal, VP – Technical  
Operations

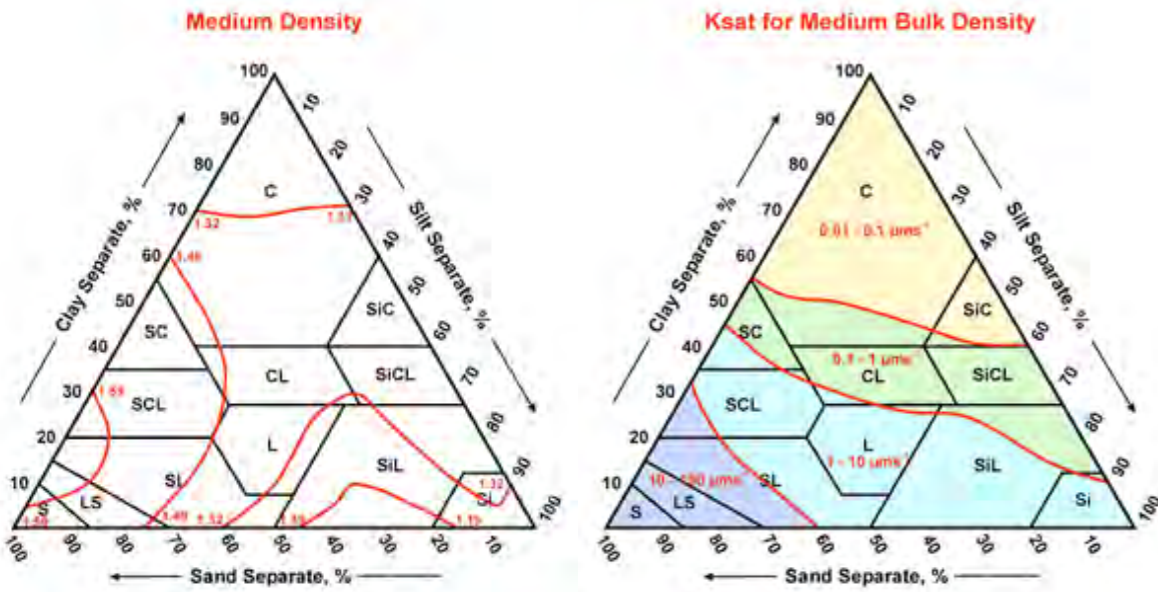


# **$K_{sat}$ VALUES**

## **FOR**

### **NEW HAMPSHIRE SOILS**

**(Including Hydrologic and DES Soil Lot Sizing Groups)**



From: Guide for Estimating Ksat from Soil Properties (Exhibit 618-9). (<http://soils.usda.gov/technical/handbook/contents/part618ex.html>)

Sponsored by the Society of Soil Scientists of Northern New England  
 SSSNNE Special Publication No. 5  
 September, 2009

# **K<sub>sat</sub> VALUES FOR NEW HAMPSHIRE SOILS**

## **ABOUT THE SOCIETY OF SOIL SCIENTISTS OF NORTHERN NEW ENGLAND**

The Society of Soil Scientists of Northern New England (SSSNNE) is a non-profit professional organization of soil scientists, both in the private and public sectors, which is dedicated to the advancement of soil science. The Society fosters the profession of soil classification, mapping and interpretation, and encourages the dissemination of information concerning soil science. With the intent of contributing to the general human welfare, the Society seeks to educate the public on the wise use of soils and the associated natural resources.

## **INTRODUCTION**

The publication “K<sub>sat</sub> Values for New Hampshire Soils” is designed to assist soil scientists, engineers, and other professionals by assembling tables of existing data for all soil series currently on the state soil legend with regard to K<sub>sat</sub> values and hydrologic groupings (Hyd.Grp.). The need for this information has become more important since the adoption by the New Hampshire Department of Environmental Services of the revised Alteration of Terrain rules for stormwater management. Additional information has been provided for each soil series with regard to landform, temperature regime (Temp.), soil textures, NHDES Soil Lot Size Groupings (Group), whether the soil is a Spodosol (Spodosol?) and other information which will be valuable to a variety of soil information users.

The data for each soil series has been sorted 3 ways for ease of searching:

Table A-Sorted by Numerical Legend

Table B-Sorted by Soil Series Name

Table C-Sorted by NHDES Soil Group for Establishing Lot Size

The report represents cumulative efforts by private soil scientists and NHDES staff with assistance from the USDA Natural Resource Conservation Service.

Comments or inquires on the information in this publication may be directed to the Board of Directors at the following address:

**Society of Soil Scientists  
of Northern New England  
PO Box 76  
Durham, NH 03824**

## SATURATED HYDRAULIC CONDUCTIVITY ( $K_{SAT}$ )

$K_{sat}$  refers to the ease with which pores in a saturated soil transmit water. The estimates presented here are expressed in terms of inches per hour (NRCS official data presents  $K_{sat}$  in both micrometers per second and inches per hour).  $K_{sat}$  values are based on soil characteristics observed in the field, particularly structure, consistence, porosity, and texture. (USDA NRCS, Web Soil Survey)

Saturated flow occurs when the soil water pressure is positive; that is, when the soil matric potential is zero (satiated wet condition). In most soils this situation takes place when about 95 percent of the total pore space is filled with water. The remaining 5 percent is filled with entrapped air. Saturated hydraulic conductivity cannot be used to describe water movement under unsaturated conditions. (Soil Survey Manual, 1993)

It is commonly known that soil features (and thus data) for a certain soil series name may be slightly different from one county soil survey to the next and the range in characteristics (via the Typical Pedon) may be slightly different. For example – a Marlow soil (series) in Carroll County may have a higher sand content in its B horizon as opposed to a Marlow soil (series) in Coos County; resulting in a slightly different  $K_{sat}$  range for the B horizon.

The  $K_{sat}$  data for this publication was obtained from the USDA-NRCS Soil Data Mart using the Typical Pedon from the county that best reflected the soil and/or had the most acres of that soil. This data is presented in B and C horizons only as it is assumed that the topsoil (A or  $A_p$  horizon) will be removed in typical construction practices.

### References:

Web Soil Survey. *Soil Survey Staff, Natural Resources Conservation Service, United States Department of Agriculture. Web Soil Survey. Available online at <http://websoilsurvey.nrcs.usda.gov/>.*

Soil Data Mart. <http://soildatamart.nrcs.usda.gov/>.

Soil Survey Manual. *Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18.*

## HYDROLOGIC SOIL GROUPS

Hydrologic group is a group of soils having the same runoff potential under similar storm and cover conditions.

Hydrologic groups are used in equations that estimate runoff from rainfall. These estimates are needed for solving hydrologic problems that arise in planning stormwater management, watershed protection, and flood-prevention projects and for planning or designing structures for the use, control, and disposal of water.

Classifications assigned to soils were based on the use of rainfall-runoff data from small watersheds and infiltrometer plots. From these data, relationships between soil properties and hydrologic groups were established. Assignment of soils to hydrologic groups is based on the relationship between soil properties and hydrologic groups. Wetness characteristics, permeability after prolonged wetting, and depth to very slowly permeable layers are properties that assist in estimating hydrologic groups. Minimum annual steady ponded infiltration rate for a bare ground surface determines the hydrologic soil groups.

Soil properties that influence runoff potential are those that influence the minimum rate of infiltration for a bare soil after prolonged wetting and when not frozen. These properties are depth to a seasonally high water table, intake rate and permeability after prolonged wetting, and depth to a very slowly permeable layer. (The influence of ground cover is treated independently, not in hydrologic soil groups.).

The soils in the United States are placed into four groups, A, B, C, and D, and three dual classes, *A/D*, *B/D*, and *C/D*. In the definitions of the classes, infiltration rate is the rate at which water enters the soil at the surface and is controlled by the surface conditions. Transmission rate is the rate at which water moves in the soil and is controlled by soil properties. Definitions of the classes are as follows:

**Group A-** Saturated hydraulic conductivity is very high or in the upper half of high and internal free water occurrence is very deep. Soils in this group have low runoff potential when thoroughly wet. Water is transmitted freely through the soil. Group A soils typically have less than 10 percent clay and more than 90 percent sand or gravel and have gravel or sand textures. Some soils having loamy sand, sandy loam, loam or silt loam textures may be placed in this group if they are well aggregated, of low bulk density, or contain greater than 35 percent rock fragments. The limits on the diagnostic physical characteristics of group A are as follows. The saturated hydraulic conductivity of all soil layers exceeds 40.0 micrometers per second (5.67 inches per hour). The depth to any water impermeable layer is greater than 50 centimeters [20 inches]. The depth to the water table is greater than 60 centimeters [24 inches]. Soils that are deeper than 100 centimeters [40 inches] to a water impermeable layer are in group A if the saturated hydraulic conductivity of all soil layers within 100 centimeters [40 inches] of the surface exceeds 10 micrometers per second (1.42 inches per hour).

**Group B-** Saturated hydraulic conductivity is in the lower half of high or in the upper half of moderately high and free water occurrence is deep or very deep. Soils in this group have moderately low runoff potential when thoroughly wet. Water transmission through the soil is unimpeded. Group B soils typically have between 10 percent and 20 percent clay and 50 percent to 90 percent sand and have loamy sand or sandy loam textures. Some soils having loam, silt loam, silt, or sandy clay loam textures may be placed in this group if they are well aggregated, of low bulk density, or contain greater than 35 percent rock fragments. The limits on the diagnostic physical characteristics of group B are as follows. The saturated hydraulic conductivity in the least transmissive layer between the surface and 50 centimeters [20 inches] ranges from 10.0 micrometers per second (1.42 inches per hour) to 40.0 micrometers per second (5.67 inches per hour). The depth to any water impermeable layer is greater than 50 centimeters [20 inches]. The depth to the water table is greater than 60 centimeters [24 inches]. Soils that are deeper than 100 centimeters [40 inches] to a water impermeable layer or water table are in group B if the saturated hydraulic conductivity of all soil layers within 100 centimeters [40 inches] of the surface exceeds 4.0 micrometers per second (0.57 inches per hour) but is less than 10.0 micrometers per second (1.42 inches per hour).

**Group C-** Saturated hydraulic conductivity is in the lower half of moderately high or in the upper half of moderately low and internal free water occurrence is deeper than shallow. Soils in this group have moderately high runoff potential when thoroughly wet. Water transmission through the soil is somewhat restricted. Group C soils typically have between 20 percent and 40 percent clay and less than 50 percent sand and have loam, silt loam, sandy clay loam, clay loam, and silty clay loam textures. Some soils having clay, silty clay, or sandy clay textures may be placed in this group if they are well aggregated, of low bulk density, or contain greater than 35 percent rock fragments. The limits on the diagnostic physical characteristics of group C are as follows. The saturated hydraulic conductivity in the least transmissive layer between the surface and 50 centimeters [20 inches] is between 1.0 micrometers per second (0.14 inches per hour) and 10.0 micrometers per second (1.42 inches per hour). The depth to any water impermeable layer is greater than 50 centimeters [20 inches]. The depth to the water table is greater than 60 centimeters [24 inches]. Soils that are deeper than 100 centimeters [40 inches] to a restriction or water table are in group C if the saturated hydraulic conductivity of all soil layers within 100 centimeters [40 inches] of the surface exceeds 0.40 micrometers per second (0.06 inches per hour) but is less than 4.0 micrometers per second (0.57 inches per hour).

**Group D-** Saturated hydraulic conductivity is below the upper half of moderately low, and/or internal free water occurrence is shallow or very shallow and transitory through permanent. Soils in this group have high runoff potential when thoroughly wet. Water movement through the soil is restricted or very restricted. Group D soils typically have greater than 40 percent clay, less than 50 percent sand, and have clayey textures. In some areas, they also have high shrink-swell potential. All soils with a depth to a water impermeable layer less than 50 centimeters [20 inches] and all soils with a water table within 60 centimeters [24 inches] of the surface are in this group, although some may have a dual classification, as described in the next section, if they can be adequately drained. The limits on the physical diagnostic characteristics of group D are as follows. For soils with a water impermeable layer at a depth between 50 centimeters and 100 centimeters [20 and 40 inches], the saturated hydraulic conductivity in the least transmissive soil layer is less than or equal to 1.0 micrometers per second (0.14 inches per hour). For soils that are deeper than 100 centimeters [40 inches] to a restriction or water table, the saturated hydraulic

conductivity of all soil layers within 100 centimeters [40 inches] of the surface is less than or equal to 0.40 micrometers per second (0.06 inches per hour).

**Dual hydrologic soil groups**-Certain wet soils are placed in group D based solely on the presence of a water table within 60 centimeters [24 inches] of the surface even though the saturated hydraulic conductivity may be favorable for water transmission. If these soils can be adequately drained, then they are assigned to dual hydrologic soil groups (*A/D*, *B/D*, and *C/D*) based on their saturated hydraulic conductivity and the water table depth when drained. The first letter applies to the drained condition and the second to the undrained condition. For the purpose of hydrologic soil group, adequately drained means that the seasonal high water table is kept at least 60 centimeters [24 inches] below the surface in a soil where it would be higher in a natural state.

#### References:

National Engineering Handbook, Natural Resource Conservation Service, U.S. Department of Agriculture.

Soil Data Mart. <http://soildatamart.nrcs.usda.gov/>.

Soil Survey Manual. *Soil Survey Division Staff. 1993. Soil survey manual. Natural Resources Conservation Service. U.S. Department of Agriculture Handbook 18.*

**TABLE A**

**NUMERICAL LEGEND**

Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Occum	1	0.6	2.0	6.00	20.0	B	2	Flood Plain (Bottom Land)	mesic	loamy	no	loamy over loamy sand
Suncook	2	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	mesic	sandy	no	occasionally flooded
Lim	3	0.6	2.0	6.00	20.0	C	5	Flood Plain (Bottom Land)	mesic	loamy	no	
Pootatuck	4	0.6	6.0	6.00	20.0	B	3	Flood Plain (Bottom Land)	mesic	loamy	no	single grain in C
Rippowam	5	0.6	6.0	6.00	20.0	C	5	Flood Plain (Bottom Land)	mesic	loamy	no	
Saco	6	0.6	2.0	6.00	20.0	D	6	Flood Plain (Bottom Land)	mesic	silty	no	strata
Hadley	8	0.6	2.0	0.60	6.0	B	2	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand
Winooski	9	0.6	6.0	0.60	6.0	B		Flood Plain (Bottom Land)	mesic	silty over loamy	no	
Merrimac	10	2.0	20.0	6.00	20.0	A	1	Outwash and Stream Terraces	mesic	gravely sand	no	loamy cap
Gloucester	11	6.0	20.0	6.00	20.0	A	1	Sandy Till	mesic	sandy-skeletal	no	loamy cap
Hinckley	12	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	
Sheepscot	14	6.0	20.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravely coarse sand
Searsport	15	6.0	20.0	6.00	20.0	D	6	Outwash and Stream Terraces	frigid	sandy	no	organic over sand
Saugatuck	16	0.06	0.2	6.00	20.0	C	5	Outwash and Stream Terraces	mesic	sandy	yes	ortstein
Colton, gravelly	21	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravely surface
Colton	22	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	
Masardis	23	6.0	20.0	6.00	20.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	slate, loamy cap
Agawam	24	6.0	20.0	20.00	100.0	B	2	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Windsor	26	6.0	20.0	6.00	20.0	A	1	Outwash and Stream Terraces	mesic	sandy	no	
Groveton	27	0.6	2.0	0.60	6.0	B	2	Outwash and Stream Terraces	frigid	loamy	yes	loamy over sandy
Madawaska	28	0.6	2.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
Woodbridge	29	0.6	2.0	0.00	0.6	C	3	Firm, platy, loamy till	mesic	loamy	no	sandy loam in Cd
Unadilla	30	0.6	2.0	2.00	20.0	B	2	Terraces and glacial lake plains	mesic	silty	no	silty over gravelly
Hartland	31	0.6	2.0	0.20	2.0	B	2	Terraces and glacial lake plains	mesic	silty	no	very fine sandy loam
Boxford	32	0.1	0.2	0.00	0.2	C	3	Silt and Clay Deposits	mesic	fine	no	silty clay loam
Scitico	33	0.0	0.2	0.00	0.2	C	5	Silt and Clay Deposits	mesic	fine	no	
Wareham	34	6.0	20.0	6.00	20.0	C	5	Outwash and Stream Terraces	mesic	sandy	no	
Champlain	35	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	gravely sand	no	
Adams	36	6.0	20.0	20.00	99.0	A	1	Outwash and Stream Terraces	frigid	sandy	yes	
Melrose	37	2.0	6.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	silty clay loam in C
Eldridge	38	6.0	20.0	0.06	0.6	C	3	Sandy/loamy over silt/clay	mesic	sandy over loamy	no	
Millis	39					C	3	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Canton	42	2.0	6.0	6.00	20.0	B	2	Loose till, sandy textures	mesic	loamy over sandy	no	loamy over loamy sand
Montauk	44	0.6	6.0	0.06	0.6	C	3	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Henniker	46	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
Madawaska, aquatic	48	0.6	2.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
Whitman	49	0.0	0.2	0.00	0.2	D	6	Firm, platy, loamy till	mesic	loamy	no	mucky loam
Hermon	55	2.0	20.0	6.00	20.0	A	1	Sandy Till	frigid	sandy-skeletal	yes	loamy cap
Becket	56	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	yes	gravely sandy loam in Cd
Waumbeck	58	2.0	20.0	6.00	20.0	B	3	Loose till, sandy textures	frigid	sandy-skeletal	yes	very cobbly loamy sand
Charlton	62	0.6	6.0	0.60	6.0	B	2	Loose till, loamy textures	mesic	loamy	no	fine sandy loam
Paxton	66	0.6	2.0	0.00	0.2	C	3	Firm, platy, loamy till	mesic	loamy	no	
Sutton	68	0.6	6.0	0.60	6.0	B	3	Loose till, loamy textures	mesic	loamy	no	
Berkshire	72	0.6	6.0	0.60	6.0	B	2	Loose till, loamy textures	frigid	loamy	yes	fine sandy loam
Marlow	76	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Peru	78	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	
Thorndike	84	0.6	2.0	0.60	2.0	C/D	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	less than 20 in. deep
Hollis	86	0.6	6.0	0.60	6.0	C/D	4	Loose till, bedrock	mesic	loamy	no	less than 20 in. deep
Winnecook	88	0.6	2.0	0.60	2.0	C	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Chatfield	89	0.6	6.0	0.60	6.0	B	4	Loose till, bedrock	mesic	loamy	no	20 to 40 in. deep
Hogback	91	2.0	6.0	2.00	6.0	C	4	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Lyman	92	2.0	6.0	2.00	6.0	A/D	4	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Woodstock	93	2.0	6.0	2.00	6.0	C/D	4	Loose till, bedrock	frigid	loamy	no	less than 20 in. deep
Rawsonville	98	0.6	6.0	0.60	6.0	C	4	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Tunbridge	99	0.6	6.0	0.60	6.0	C	4	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep



Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Ondawa	101	0.6	6.0	6.00	20.0	B	2	Flood Plain (Bottom Land)	frigid	loamy	no	loamy over loamy sand
Sunday	102	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	frigid	sandy	no	occasionally flooded
Winooski	103	0.6	6.0	0.60	6.0	B	3	Flood Plain (Bottom Land)	mesic	silty	no	very fine sandy loam
Podunk	104	0.6	6.0	6.00	20.0	B	3	Flood Plain (Bottom Land)	frigid	loamy	no	loamy to coarse sand in C
Rumney	105	0.6	6.0	6.00	20.0	C	5	Flood Plain (Bottom Land)	frigid	loamy	no	
Hadley	108	0.6	2.0	0.60	6.0	B	2	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand, occ flooded
Limerick	109	0.6	2.0	0.60	2.0	C	5	Flood Plain (Bottom Land)	mesic	silty	no	
Scarboro	115	6.0	20.0	6.00	20.0	D	6	Outwash and Stream Terraces	mesic	sandy	no	organic over sand, non stony
Finch	116					C	3	Outwash and Stream Terraces	frigid	sandy	yes	cemented (ortstein)
Sudbury	118	2.0	6.0	2.00	20.0	B	3	Outwash and Stream Terraces	mesic	sandy	no	loam over gravelly sand
Telos	123	0.6	2.0	0.02	0.2	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Chesuncook	126	0.6	2.0	0.02	0.2	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Allagash	127	0.6	2.0	6.00	20.0	B	2	Outwash and Stream Terraces	frigid	loamy over sandy	yes	loamy over sandy
Elliottsville	128	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	frigid	loamy	yes	20 to 40 in. deep
Hitchcock	130	0.6	2.0	0.06	0.6	B	3	Terraces and glacial lake plains	mesic	silty	no	silt loam to silt in C
Burnham	131	0.2	6.0	0.02	0.2	D	6	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	organic over silt
Dartmouth	132	0.6	2.0	0.06	0.6	B	3	Terraces and glacial lake plains	mesic	silty	no	thin strata silty clay loam
Monson	133	0.6	2.0	0.60	2.0	D	4	Friable till, silty, schist & phyllite	frigid	loamy	yes	less than 20 in. deep
Maybid	134	0.0	0.2	0.00	0.2	D	6	Silt and Clay Deposits	mesic	fine	no	silt over clay
Shapleigh	136					C/D	4	Sandy Till	mesic	sandy	yes	less than 20 in. deep
Monadnock	142	0.6	2.0	2.00	6.0	B	2	Loose till, sandy textures	frigid	loamy over sandy, sandy-skeletal	yes	gravelly loamy sand in C
Acton	146	2.0	20.0	2.00	20.0	B	3	Loose till, sandy textures	mesic	sandy-skeletal	no	cobbly loamy sand
Vassalboro	150					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Success	154	2.0	6.0	6.00	20.0	A	1	Sandy Till	frigid	sandy-skeletal	yes	cemented
Canterbury	166	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Sunapee	168	0.6	2.0	0.60	6.0	B	3	Loose till, loamy textures	frigid	loamy	yes	
Waskish	195					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Ondawa	201	0.6	6.0	6.00	20.0	B	2	Flood Plain (Bottom Land)	frigid	loamy	no	occ flood, loamy over l. sand
Sunday	202	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	frigid	sandy	no	frequently flooded
Fryeburg	208	0.6	2.0	2.00	6.0	B	2	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Charles	209	0.6	100.0	0.60	100.0	C	5	Flood Plain (Bottom Land)	frigid	silty	no	
Warwick	210	2.0	6.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	loamy-skeletal	no	loamy over slate gravel
Naumburg	214	6.0	20.0	6.00	20.0	C	5	Outwash and Stream Terraces	frigid	sandy	yes	
Boscawen	220	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	no	loamy cap
Bemis	224	0.6	0.2	0.00	0.2	C	5	Firm, platy, loamy till	cryic	loamy	no	
Bice	226	0.6	6.0	0.60	6.0	B	2	Loose till, loamy textures	frigid	loamy	no	sandy loam
Lanesboro	228	0.6	2.0	0.06	0.2	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	channery silt loam in Cd
Poocham	230	0.6	2.0	0.20	2.0	B	3	Terraces and glacial lake plains	mesic	silty	no	silt loam in C
Buxton	232	0.1	0.6	0.00	0.2	C	3	Silt and Clay Deposits	frigid	fine	no	silty clay
Scantic	233	0.0	0.2	0.00	0.2	D	5	Silt and Clay Deposits	frigid	fine	no	
Biddeford	234	0.0	0.2	0.00	0.2	D	6	Silt and Clay Deposits	frigid	fine	no	organic over clay
Buckland	237	0.6	2.0	0.06	0.2	C	3	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Elmridge	238	2.0	6.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	mesic	loamy over clayey	no	
Brayton	240	0.6	2.0	0.06	0.6	C	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Lyme	246	0.6	6.0	0.60	6.0	C	5	Loose till, sandy textures	frigid	loamy	no	
Millsite	251	0.6	6.0	0.60	6.0	C	4	Loose till, bedrock	frigid	loamy	no	20 to 40 in. deep
Macomber	252	0.6	2.0	0.60	2.0	C	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Lombard	259	0.6	6.0	2.00	20.0	C/D	2	Weathered bedrock, phyllite	frigid	loamy	no	very channery
Sunapee var	269	0.6	2.0	0.60	6.0	B	3	Loose till, loamy textures	frigid	loamy	yes	frigid dystrodept
Chatfield Var.	289	0.6	6.0	0.60	6.0	B	3	Loose till, bedrock	mesic	loamy	no	mwd to swpd
Greenwood	295					A/D	6	Organic Materials - Freshwater	frigid	hemic	no	deep organic
Catden	296					A/D	6	Organic Materials - Freshwater	mesic	sapric	no	deep organic
Lovewell	307	0.6	2.0	0.60	2.0	B	3	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Quonset	310	2.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	shale
Deerfield	313	6.0	20.0	20.00	100.0	B	3	Outwash and Stream Terraces	mesic	sandy	no	single grain in C

Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Pipestone	314					B	5	Outwash and Stream Terraces	mesic	sandy	yes	
Mashpee	315	6.0	20.0	6.00	20.0	B	5	Outwash and Stream Terraces	mesic	sandy	yes	
Bernardston	330	0.6	2.0	0.06	0.2	C	3	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Roundabout	333	0.2	2.0	0.06	0.6	C	5	Terraces and glacial lake plains	frigid	silty	no	silt loam in the C
Pittstown	334	0.6	2.0	0.06	0.2	C	3	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Elmwood	338	2.0	6.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	
Stissing	340	0.6	2.0	0.06	0.2	C	5	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	
Cardigan	357	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	mesic	loamy	no	20 to 40 in. deep
Kearsarge	359	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	mesic	loamy	no	less than 20 in. deep
Dutchess	366	0.6	2.0	0.60	2.0	B	2	Friable till, silty, schist & phyllite	mesic	loamy	no	very channery
Dixfield	378	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Timakwa	393			6.00	100.0	D	6	Organic Materials - Freshwater	mesic	sandy or sandy-skeletal	no	organic over sand
Chocorua	395			6.00	20.0	D	6	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Ipswich	397					D	6	Tidal Flat	mesic	hemic/sapric	no	deep organic
Suncook	402	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	mesic	sandy	no	frequent flooding
Metallak	404	6.0	100.0	6.00	100.0	B	3	Flood Plain (Bottom Land)	frigid	loamy over sandy	no	sandy or sandy-skeletal
Medomak	406	0.6	2.0	0.60	2.0	D	6	Flood Plain (Bottom Land)	frigid	silty	no	organic over silt
Haven	410	0.6	2.0	20.00	100.0	B	2	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Duane	413	6.0	20.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	cemented (ortstein)
Moosilauke	414	6.0	20.0	6.00	20.0	C	5	Loose till, sandy textures	frigid	sandy	no	
Grange	433	0.6	2.0	0.60	2.0	C	5	Outwash and Stream Terraces	frigid	co. loamy over sandy (skeletal)	no	
Swanton	438	2.0	6.0	0.00	0.2	C	5	Sandy/loamy over silt/clay	frigid	co. loamy over clayey	no	
Shaker	439	2.0	6.0	0.00	0.2	C	5	Sandy/loamy over silt/clay	mesic	co. loamy over clayey	no	
Chichester	442	0.6	2.0	2.00	6.0	B	3	Loose till, sandy textures	frigid	loamy over sandy	no	loamy over loamy sand
Newfields	444	0.6	2.0	0.60	2.0	B	3	Loose till, sandy textures	mesic	loamy over sandy	no	sandy or sandy-skeletal
Scituate	448	0.6	2.0	0.06	0.2	C	3	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Metacomet	458	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
Pennichuck	460	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	mesic	loamy-skeletal	no	20 to 40 in. deep
Gilmanton	478	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	no	fine sandy loam in Cd
Ossipee	495			0.20	2.0	D	6	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Natchaug	496			0.20	2.0	D	6	Organic Materials - Freshwater	mesic	loamy	no	organic over loam
Pawcatuck	497			20.00	100.0	D	6	Tidal Flat	mesic	sandy or sandy-skeletal	no	organic over sand
Abenaki	501	0.6	2.0	6.00	99.0	B	2	Outwash and Stream Terraces	frigid	loamy over sandy-skeletal	no	loamy over gravelly
Cohas	505	0.6	2.0	0.60	100.0	C	5	Flood Plain (Bottom Land)	frigid	co. loamy over sandy (skeletal)	no	
Hoosic	510	2.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	slate, loamy cap
Ninigret	513	0.6	6.0	6.00	20.0	B	3	Outwash and Stream Terraces	mesic	loamy over sandy	no	sandy or sandy-skeletal
Leicester	514	0.6	6.0	0.60	20.0	C	5	Loose till, loamy textures	mesic	loamy	no	
Au Gres	516					B	5	Outwash and Stream Terraces	frigid	sandy	yes	single grain, loose
Machias	520	2.0	6.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	sandy or sandy-skeletal	yes	strata sand/gravel in C
Stetson	523	0.6	6.0	6.00	20.0	B	2	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	loamy over gravelly
Caesar	526	20.0	100.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	coarse sand	no	
Scio	531	0.6	2.0	0.60	2.0	B	3	Terraces and glacial lake plains	mesic	silty	no	gravelly sand in 2C
Belgrade	532	0.6	2.0	0.06	2.0	B	3	Terraces and glacial lake plains	mesic	silty	no	strata of fine sand
Raynham	533	0.2	2.0	0.06	0.2	C	5	Terraces and glacial lake plains	mesic	silty	no	
Binghamville	534	0.2	2.0	0.06	0.2	D	5	Terraces and glacial lake plains	mesic	silty	no	
Suffield	536	0.6	2.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	mesic	silty over clayey	no	deep to clay C
Squamscott	538	6.0	20.0	0.06	0.6	C	5	Sandy/loamy over silt/clay	mesic	sandy over loamy	yes	
Raypol	540	0.6	2.0	6.00	100.0	D	5	Outwash and Stream Terraces	mesic	co. loamy over sandy (skeletal)	no	
Walpole	546	2.0	6.0	6.00	20.0	C	5	Outwash and Stream Terraces	mesic	sandy	no	
Peacham	549	0.6	2.0	0.00	0.2	D	6	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	organic over loam
Skerry	558	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Plaisted	563	0.6	2.0	0.06	0.6	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Howland	566	0.6	2.0	0.06	0.2	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	silt loam, platy in Cd
Monarda	569	0.2	2.0	0.02	0.2	D	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Bangor	572	0.6	2.0	0.60	2.0	B	2	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam

Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Dixmont	578	0.6	2.0	0.60	2.0	C	3	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam, platy in C
Cabot	589	0.6	2.0	0.06	0.2	D	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Westbrook	597			0.00	2.0	D	6	Tidal Flat	mesic	loamy	no	organic over loam
Mundal	610	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	gravely sandy loam in Cd
Croghan	613	20.0	100.0	20.00	100.0	B	3	Outwash and Stream Terraces	frigid	sandy	yes	single grain in C
Kinsman	614	6.0	20.0	6.00	20.0	C	5	Outwash and Stream Terraces	frigid	sandy	yes	
Salmon	630	0.6	2.0	0.60	2.0	B	2	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Nicholville	632	0.6	2.0	0.60	2.0	C	3	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Pemi	633	0.6	2.0	0.06	0.6	C	5	Terraces and glacial lake plains	frigid	silty	no	
Pillsbury	646	0.6	2.0	0.06	0.2	C	5	Firm, platy, loamy till	frigid	silty	no	
Ridgebury	656	0.6	6.0	0.00	0.2	C	5	Firm, platy, loamy till	mesic	loamy	no	
Canaan	663	2.0	20.0	2.00	20.0	C	4	Weathered Bedrock Till	frigid	loamy-skeletal	yes	less than 20 in. deep
Redstone	665	2.0	6.0	6.00	20.0	A	1	Weathered Bedrock Till	frigid	fragmental	yes	loamy cap
Sisk	667	0.6	2.0	0.00	0.6	C	3	Firm, platy, loamy till	cryic	loamy	yes	sandy loam in Cd
Surplus	669	0.6	2.0	0.00	0.6	C	3	Firm, platy, loamy till	cryic	loamy	yes	mwd, sandy loam in Cd
Glebe	671	2.0	6.0	2.00	6.0	C	4	Loose till, bedrock	cryic	loamy	yes	20 to 40 in. deep
Saddleback	673	0.6	2.0	0.60	2.0	C/D	4	Loose till, bedrock	cryic	loamy	yes	less than 20 in. deep
Ricker	674	2.0	6.0	2.00	6.0	A	4	Organic over bedrock (up to 4" of mineral)	cryic	fibric to hemic	no	well drained, less than 20 in. deep
Houghtonville	795	0.6	6.0	0.60	6.0	B	2	Loose till, loamy textures	frigid	loamy	yes	cobbly fine sandy loam
Matunuck	797			20.00	100.0	D	6	Tidal Flat	mesic	sandy	no	organic over sand
Meadowsedge	894					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Bucksport	895					D	6	Organic Materials - Freshwater	frigid	sapric	no	deep organic
Colonel	927	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	loam in Cd
Pondicherry	992			6.00	20.0	D	6	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Wonsqueak	995			0.20	2.0	D	6	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Glover	NA	0.6	2.0	0.60	2	D	4	Friable till, silty, schist & phyllite	frigid	loamy	no	less than 20 in. deep



no longer recognized  
 organic materials

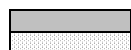
**TABLE B**  
**SOIL SERIES**

Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Abenaki	501	0.6	2.0	6.00	99.0	B	2	Outwash and Stream Terraces	frigid	loamy over sandy-skeletal	no	loamy over gravelly
Acton	146	2.0	20.0	2.00	20.0	B	3	Loose till, sandy textures	mesic	sandy-skeletal	no	cobbly loamy sand
Adams	36	6.0	20.0	20.00	99.0	A	1	Outwash and Stream Terraces	frigid	sandy	yes	
Agawam	24	6.0	20.0	20.00	100.0	B	2	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Allagash	127	0.6	2.0	6.00	20.0	B	2	Outwash and Stream Terraces	frigid	loamy over sandy	yes	loamy over sandy
Au Gres	516					B	5	Outwash and Stream Terraces	frigid	sandy	yes	single grain, loose
Bangor	572	0.6	2.0	0.60	2.0	B	2	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam
Becket	56	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	yes	gravelly sandy loam in Cd
Belgrade	532	0.6	2.0	0.06	2.0	B	3	Terraces and glacial lake plains	mesic	silty	no	strata of fine sand
Bemis	224	0.6	0.2	0.00	0.2	C	5	Firm, platy, loamy till	cryic	loamy	no	
Berkshire	72	0.6	6.0	0.60	6.0	B	2	Loose till, loamy textures	frigid	loamy	yes	fine sandy loam
Bernardston	330	0.6	2.0	0.06	0.2	C	3	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Bice	226	0.6	6.0	0.60	6.0	B	2	Loose till, loamy textures	frigid	loamy	no	sandy loam
Biddeford	234	0.0	0.2	0.00	0.2	D	6	Silt and Clay Deposits	frigid	fine	no	organic over clay
Binghamville	534	0.2	2.0	0.06	0.2	D	5	Terraces and glacial lake plains	mesic	silty	no	
Boscawen	220	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	no	loamy cap
Boxford	32	0.1	0.2	0.00	0.2	C	3	Silt and Clay Deposits	mesic	fine	no	silty clay loam
Brayton	240	0.6	2.0	0.06	0.6	C	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Buckland	237	0.6	2.0	0.06	0.2	C	3	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Bucksport	895					D	6	Organic Materials - Freshwater	frigid	sapric	no	deep organic
Burnham	131	0.2	6.0	0.02	0.2	D	6	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	organic over silt
Buxton	232	0.1	0.6	0.00	0.2	C	3	Silt and Clay Deposits	frigid	fine	no	silty clay
Cabot	589	0.6	2.0	0.06	0.2	D	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Caesar	526	20.0	100.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	coarse sand	no	
Canaan	663	2.0	20.0	2.00	20.0	C	4	Weathered Bedrock Till	frigid	loamy-skeletal	yes	less than 20 in. deep
Canterbury	166	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Canton	42	2.0	6.0	6.00	20.0	B	2	Loose till, sandy textures	mesic	loamy over sandy	no	loamy over loamy sand
Cardigan	357	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	mesic	loamy	no	20 to 40 in. deep
Catden	296					A/D	6	Organic Materials - Freshwater	mesic	sapric	no	deep organic
Champlain	35	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	gravelly sand	no	
Charles	209	0.6	100.0	0.60	100.0	C	5	Flood Plain (Bottom Land)	frigid	silty	no	
Charlton	62	0.6	6.0	0.60	6.0	B	2	Loose till, loamy textures	mesic	loamy	no	fine sandy loam
Chatfield	89	0.6	6.0	0.60	6.0	B	4	Loose till, bedrock	mesic	loamy	no	20 to 40 in. deep
Chatfield Var.	289	0.6	6.0	0.60	6.0	B	3	Loose till, bedrock	mesic	loamy	no	mwd to swpd
Chesuncook	126	0.6	2.0	0.02	0.2	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Chichester	442	0.6	2.0	2.00	6.0	B		Loose till, sandy textures	frigid	loamy over sandy	no	loamy over loamy sand
Chocorua	395			6.00	20.0	D	6	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Cohas	505	0.6	2.0	0.60	100.0	C	5	Flood Plain (Bottom Land)	frigid	co. loamy over sandy (skeletal)	no	
Colonel	927	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	loam in Cd
Colton	22	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	
Colton, gravelly	21	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravelly surface
Croghan	613	20.0	100.0	20.00	100.0	B	3	Outwash and Stream Terraces	frigid	sandy	yes	single grain in C
Dartmouth	132	0.6	2.0	0.06	0.6	B	3	Terraces and glacial lake plains	mesic	silty	no	thin strata silty clay loam
Deerfield	313	6.0	20.0	20.00	100.0	B	3	Outwash and Stream Terraces	mesic	sandy	no	single grain in C
Dixfield	378	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Dixmont	578	0.6	2.0	0.60	2.0	C	3	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam, platy in C
Duane	413	6.0	20.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	cemented (ortstein)
Dutchess	366	0.6	2.0	0.60	2.0	B	2	Friable till, silty, schist & phyllite	mesic	loamy	no	very channery
Eldridge	38	6.0	20.0	0.06	0.6	C	3	Sandy/loamy over silt/clay	mesic	sandy over loamy	no	
Elliottsville	128	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	frigid	loamy	yes	20 to 40 in. deep
Elmridge	238	2.0	6.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	mesic	loamy over clayey	no	
Elmwood	338	2.0	6.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	
Finch	116					C	3	Outwash and Stream Terraces	frigid	sandy	yes	cemented (ortstein)

Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Fryeburg	208	0.6	2.0	2.00	6.0	B	2	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Gilmanton	478	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	no	fine sandy loam in Cd
Glebe	671	2.0	6.0	2.00	6.0	C	4	Loose till, bedrock	cryic	loamy	yes	20 to 40 in. deep
Gloucester	11	6.0	20.0	6.00	20.0	A	1	Sandy Till	mesic	sandy-skeletal	no	loamy cap
Glover	NA	0.6	2.0	0.60	2	D	4	Friable till, silty, schist & phyllite	frigid	loamy	no	less than 20 in. deep
Grange	433	0.6	2.0	0.60	2.0	C	5	Outwash and Stream Terraces	frigid	co. loamy over sandy (skeletal)	no	
Greenwood	295					A/D	6	Organic Materials - Freshwater	frigid	hemic	no	deep organic
Groveton	27	0.6	2.0	0.60	6.0	B	2	Outwash and Stream Terraces	frigid	loamy	yes	loamy over sandy
Hadley	8	0.6	2.0	0.60	6.0	B	2	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand
Hadley	108	0.6	2.0	0.60	6.0	B	2	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand, occ flooded
Hartland	31	0.6	2.0	0.20	2.0	B	2	Terraces and glacial lake plains	mesic	silty	no	very fine sandy loam
Haven	410	0.6	2.0	20.00	100.0	B	2	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Henniker	46	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
Hermon	55	2.0	20.0	6.00	20.0	A	1	Sandy Till	frigid	sandy-skeletal	yes	loamy cap
Hinckley	12	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	
Hitchcock	130	0.6	2.0	0.06	0.6	B	3	Terraces and glacial lake plains	mesic	silty	no	silt loam to silt in C
Hogback	91	2.0	6.0	2.00	6.0	C	4	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Hollis	86	0.6	6.0	0.60	6.0	C/D	4	Loose till, bedrock	mesic	loamy	no	less than 20 in. deep
Hoosic	510	2.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	slate, loamy cap
Houghtonville	795	0.6	6.0	0.60	6.0	B	2	Loose till, loamy textures	frigid	loamy	yes	cobbly fine sandy loam
Howland	566	0.6	2.0	0.06	0.2	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	silt loam, platy in Cd
Ipswich	397					D	6	Tidal Flat	mesic	hemic/sapric	no	deep organic
Kearsarge	359	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	mesic	loamy	no	less than 20 in. deep
Kinsman	614	6.0	20.0	6.00	20.0	C	5	Outwash and Stream Terraces	frigid	sandy	yes	
Lanesboro	228	0.6	2.0	0.06	0.2	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	channery silt loam in Cd
Leicester	514	0.6	6.0	0.60	20.0	C	5	Loose till, loamy textures	mesic	loamy	no	
Lim	3	0.6	2.0	6.00	20.0	C	5	Flood Plain (Bottom Land)	mesic	loamy	no	
Limerick	109	0.6	2.0	0.60	2.0	C	5	Flood Plain (Bottom Land)	mesic	silty	no	
Lombard	259	0.6	6.0	2.00	20.0	C/D	2	Weathered bedrock, phyllite	frigid	loamy	no	very channery
Lovewell	307	0.6	2.0	0.60	2.0	B	3	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Lyman	92	2.0	6.0	2.00	6.0	A/D	4	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Lyme	246	0.6	6.0	0.60	6.0	C	5	Loose till, sandy textures	frigid	loamy	no	
Machias	520	2.0	6.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	sandy or sandy-skeletal	yes	strata sand/gravel in C
Macomber	252	0.6	2.0	0.60	2.0	C	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Madawaska	28	0.6	2.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
Madawaska, aquet	48	0.6	2.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
Marlow	76	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Masardis	23	6.0	20.0	6.00	20.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	slate, loamy cap
Mashpee	315	6.0	20.0	6.00	20.0	B	5	Outwash and Stream Terraces	mesic	sandy	yes	
Matunuck	797			20.00	100.0	D	6	Tidal Flat	mesic	sandy	no	organic over sand
Maybid	134	0.0	0.2	0.00	0.2	D	6	Silt and Clay Deposits	mesic	fine	no	silt over clay
Meadowsedge	894					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Medomak	406	0.6	2.0	0.60	2.0	D	6	Flood Plain (Bottom Land)	frigid	silty	no	organic over silt
Melrose	37	2.0	6.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	silty clay loam in C
Merrimac	10	2.0	20.0	6.00	20.0	A	1	Outwash and Stream Terraces	mesic	gravelly sand	no	loamy cap
Metacomet	458	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
Metallak	404	6.0	100.0	6.00	100.0	B	3	Flood Plain (Bottom Land)	frigid	loamy over sandy	no	sandy or sandy-skeletal
Millis	39					C	3	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Millsite	251	0.6	6.0	0.60	6.0	C	4	Loose till, bedrock	frigid	loamy	no	20 to 40 in. deep
Monadnock	142	0.6	2.0	2.00	6.0	B	2	Loose till, sandy textures	frigid	loamy over sandy, sandy-skeletal	yes	gravelly loamy sand in C
Monarda	569	0.2	2.0	0.02	0.2	D	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Monson	133	0.6	2.0	0.60	2.0	D	4	Friable till, silty, schist & phyllite	frigid	loamy	yes	less than 20 in. deep
Montauk	44	0.6	6.0	0.06	0.6	C	3	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Moosilauke	414	6.0	20.0	6.00	20.0	C	5	Loose till, sandy textures	frigid	sandy	no	

Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Mundal	610	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	gravely sandy loam in Cd
Natchaug	496			0.20	2.0	D	6	Organic Materials - Freshwater	mesic	loamy	no	organic over loam
Naumburg	214	6.0	20.0	6.00	20.0	C	5	Outwash and Stream Terraces	frigid	sandy	yes	
Newfields	444	0.6	2.0	0.60	2.0	B	3	Loose till, sandy textures	mesic	loamy over sandy	no	sandy or sandy-skeletal
Nicholville	632	0.6	2.0	0.60	2.0	C	3	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Ninigret	513	0.6	6.0	6.00	20.0	B	3	Outwash and Stream Terraces	mesic	loamy over sandy	no	sandy or sandy-skeletal
Occum	1	0.6	2.0	6.00	20.0	B	2	Flood Plain (Bottom Land)	mesic	loamy	no	loamy over loamy sand
Ondawa	101	0.6	6.0	6.00	20.0	B	2	Flood Plain (Bottom Land)	frigid	loamy	no	loamy over loamy sand
Ondawa	201	0.6	6.0	6.00	20.0	B	2	Flood Plain (Bottom Land)	frigid	loamy	no	occ flood, loamy over l. sand
Ossipee	495			0.20	2.0	D	6	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Pawcatuck	497			20.00	100.0	D	6	Tidal Flat	mesic	sandy or sandy-skeletal	no	organic over sand
Paxton	66	0.6	2.0	0.00	0.2	C	3	Firm, platy, loamy till	mesic	loamy	no	
Peacham	549	0.6	2.0	0.00	0.2	D	6	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	organic over loam
Pemi	633	0.6	2.0	0.06	0.6	C	5	Terraces and glacial lake plains	frigid	silty	no	
Pennichuck	460	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	mesic	loamy-skeletal	no	20 to 40 in. deep
Peru	78	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	
Pillsbury	646	0.6	2.0	0.06	0.2	C	5	Firm, platy, loamy till	frigid	silty	no	
Pipestone	314					B	5	Outwash and Stream Terraces	mesic	sandy	yes	
Pittstown	334	0.6	2.0	0.06	0.2	C	3	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Plaisted	563	0.6	2.0	0.06	0.6	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Podunk	104	0.6	6.0	6.00	20.0	B	3	Flood Plain (Bottom Land)	frigid	loamy	no	loamy to coarse sand in C
Pondicherry	992			6.00	20.0	D	6	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Poocham	230	0.6	2.0	0.20	2.0	B	3	Terraces and glacial lake plains	mesic	silty	no	silt loam in C
Pootatuck	4	0.6	6.0	6.00	20.0	B	3	Flood Plain (Bottom Land)	mesic	loamy	no	single grain in C
Quonset	310	2.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	shale
Rawsonville	98	0.6	6.0	0.60	6.0	C	4	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Raynham	533	0.2	2.0	0.06	0.2	C	5	Terraces and glacial lake plains	mesic	silty	no	
Raypol	540	0.6	2.0	6.00	100.0	D	5	Outwash and Stream Terraces	mesic	co. loamy over sandy (skeletal)	no	
Redstone	665	2.0	6.0	6.00	20.0	A	1	Weathered Bedrock Till	frigid	fragmental	yes	loamy cap
Ricker	674	2.0	6.0	2.00	6.0	A	4	Organic over bedrock (up to 4" of mineral)	cryic	fibric to hemic	no	well drained, less than 20 in. deep
Ridgebury	656	0.6	6.0	0.00	0.2	C	5	Firm, platy, loamy till	mesic	loamy	no	
Rippowam	5	0.6	6.0	6.00	20.0	C	5	Flood Plain (Bottom Land)	mesic	loamy	no	
Roundabout	333	0.2	2.0	0.06	0.6	C	5	Terraces and glacial lake plains	frigid	silty	no	silt loam in the C
Rumney	105	0.6	6.0	6.00	20.0	C	5	Flood Plain (Bottom Land)	frigid	loamy	no	
Saco	6	0.6	2.0	6.00	20.0	D	6	Flood Plain (Bottom Land)	mesic	silty	no	strata
Saddleback	673	0.6	2.0	0.60	2.0	C/D	4	Loose till, bedrock	cryic	loamy	yes	less than 20 in. deep
Salmon	630	0.6	2.0	0.60	2.0	B	2	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Saugatuck	16	0.06	0.2	6.00	20.0	C	5	Outwash and Stream Terraces	mesic	sandy	yes	ortstein
Scantic	233	0.0	0.2	0.00	0.2	D	5	Silt and Clay Deposits	frigid	fine	no	
Scarboro	115	6.0	20.0	6.00	20.0	D	6	Outwash and Stream Terraces	mesic	sandy	no	organic over sand, non stony
Scio	531	0.6	2.0	0.60	2.0	B	3	Terraces and glacial lake plains	mesic	silty	no	gravely sand in 2C
Scitico	33	0.0	0.2	0.00	0.2	C	5	Silt and Clay Deposits	mesic	fine	no	
Scituate	448	0.6	2.0	0.06	0.2	C	3	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Searsport	15	6.0	20.0	6.00	20.0	D	6	Outwash and Stream Terraces	frigid	sandy	no	organic over sand
Shaker	439	2.0	6.0	0.00	0.2	C	5	Sandy/loamy over silt/clay	mesic	co. loamy over clayey	no	
Shapleigh	136					C/D	4	Sandy Till	mesic	sandy	yes	less than 20 in. deep
Sheepscoot	14	6.0	20.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravely coarse sand
Sisk	667	0.6	2.0	0.00	0.6	C	3	Firm, platy, loamy till	cryic	loamy	yes	sandy loam in Cd
Skerry	558	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Squamscott	538	6.0	20.0	0.06	0.6	C	5	Sandy/loamy over silt/clay	mesic	sandy over loamy	yes	
Stetson	523	0.6	6.0	6.00	20.0	B	2	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	loamy over gravelly
Stissing	340	0.6	2.0	0.06	0.2	C	5	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	
Success	154	2.0	6.0	6.00	20.0	A	1	Sandy Till	frigid	sandy-skeletal	yes	cemented
Sudbury	118	2.0	6.0	2.00	20.0	B	3	Outwash and Stream Terraces	mesic	sandy	no	loam over gravelly sand

Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Suffield	536	0.6	2.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	mesic	silty over clayey	no	deep to clay C
Sunapee	168	0.6	2.0	0.60	6.0	B	3	Loose till, loamy textures	frigid	loamy	yes	
Sunapee var	269	0.6	2.0	0.60	6.0	B	3	Loose till, loamy textures	frigid	loamy	yes	frigid dystrodept
Suncook	2	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	mesic	sandy	no	occasionally flooded
Suncook	402	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	mesic	sandy	no	frequent flooding
Sunday	102	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	frigid	sandy	no	occasionally flooded
Sunday	202	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	frigid	sandy	no	frequently flooded
Surplus	669	0.6	2.0	0.00	0.6	C	3	Firm, platy, loamy till	cryic	loamy	yes	mwd, sandy loam in Cd
Sutton	68	0.6	6.0	0.60	6.0	B	3	Loose till, loamy textures	mesic	loamy	no	
Swanton	438	2.0	6.0	0.00	0.2	C	5	Sandy/loamy over silt/clay	frigid	co. loamy over clayey	no	
Telos	123	0.6	2.0	0.02	0.2	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Thorndike	84	0.6	2.0	0.60	2.0	C/D	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	less than 20 in. deep
Timakwa	393			6.00	100.0	D	6	Organic Materials - Freshwater	mesic	sandy or sandy-skeletal	no	organic over sand
Tunbridge	99	0.6	6.0	0.60	6.0	C	4	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Unadilla	30	0.6	2.0	2.00	20.0	B	2	Terraces and glacial lake plains	mesic	silty	no	silty over gravelly
Vassalboro	150					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Walpole	546	2.0	6.0	6.00	20.0	C	5	Outwash and Stream Terraces	mesic	sandy	no	
Wareham	34	6.0	20.0	6.00	20.0	C	5	Outwash and Stream Terraces	mesic	sandy	no	
Warwick	210	2.0	6.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	loamy-skeletal	no	loamy over slate gravel
Waskish	195					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Waumbeck	58	2.0	20.0	6.00	20.0	B	3	Loose till, sandy textures	frigid	sandy-skeletal	yes	very cobbly loamy sand
Westbrook	597			0.00	2.0	D	6	Tidal Flat	mesic	loamy	no	organic over loam
Whitman	49	0.0	0.2	0.00	0.2	D	6	Firm, platy, loamy till	mesic	loamy	no	mucky loam
Windsor	26	6.0	20.0	6.00	20.0	A	1	Outwash and Stream Terraces	mesic	sandy	no	
Winnecook	88	0.6	2.0	0.60	2.0	C	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Winooski	9	0.6	6.0	0.60	6.0	B		Flood Plain (Bottom Land)	mesic	silty over loamy	no	
Winooski	103	0.6	6.0	0.60	6.0	B	3	Flood Plain (Bottom Land)	mesic	silty	no	very fine sandy loam
Wonsqueak	995			0.20	2.0	D	6	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Woodbridge	29	0.6	2.0	0.00	0.6	C	3	Firm, platy, loamy till	mesic	loamy	no	sandy loam in Cd
Woodstock	93	2.0	6.0	2.00	6.0	C/D	4	Loose till, bedrock	frigid	loamy	no	less than 20 in. deep



no longer recognized  
 organic materials



**TABLE C**

**NHDES SOIL GROUPINGS**

Soil Series	number	NHDES Soil Group	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Adams	36	1	6.0	20.0	20.00	99.0	A	Outwash and Stream Terraces	frigid	sandy	yes	
Boscawen	220	1	6.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	frigid	sandy-skeletal	no	loamy cap
Caesar	526	1	20.0	100.0	20.00	100.0	A	Outwash and Stream Terraces	mesic	coarse sand	no	
Champlain	35	1	6.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	frigid	gravelly sand	no	
Colton	22	1	6.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	
Colton, gravelly	21	1	6.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravelly surface
Gloucester	11	1	6.0	20.0	6.00	20.0	A	Sandy Till	mesic	sandy-skeletal	no	loamy cap
Hermon	55	1	2.0	20.0	6.00	20.0	A	Sandy Till	frigid	sandy-skeletal	yes	loamy cap
Hinckley	12	1	6.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	mesic	sandy-skeletal	no	
Hoosic	510	1	2.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	mesic	sandy-skeletal	no	slate, loamy cap
Masardis	23	1	6.0	20.0	6.00	20.0	A	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	slate, loamy cap
Merrimac	10	1	2.0	20.0	6.00	20.0	A	Outwash and Stream Terraces	mesic	gravelly sand	no	loamy cap
Quonset	310	1	2.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	mesic	sandy-skeletal	no	shale
Redstone	665	1	2.0	6.0	6.00	20.0	A	Weathered Bedrock Till	frigid	fragmental	yes	loamy cap
Success	154	1	2.0	6.0	6.00	20.0	A	Sandy Till	frigid	sandy-skeletal	yes	cemented
Suncook	2	1	6.0	20.0	6.00	20.0	A	Flood Plain (Bottomland)	mesic	sandy	no	occasionally flooded
Suncook	402	1	6.0	20.0	6.00	20.0	A	Flood Plain (Bottomland)	mesic	sandy	no	frequent flooding
Sunday	102	1	6.0	20.0	6.00	20.0	A	Flood Plain (Bottomland)	frigid	sandy	no	occasionally flooded
Sunday	202	1	6.0	20.0	6.00	20.0	A	Flood Plain (Bottomland)	frigid	sandy	no	frequently flooded
Warwick	210	1	2.0	6.0	20.00	100.0	A	Outwash and Stream Terraces	mesic	loamy-skeletal	no	loamy over slate gravel
Windsor	26	1	6.0	20.0	6.00	20.0	A	Outwash and Stream Terraces	mesic	sandy	no	
Abenaki	501	2	0.6	2.0	6.00	99.0	B	Outwash and Stream Terraces	frigid	loamy over sandy-skeletal	no	loamy over gravelly
Agawam	24	2	6.0	20.0	20.00	100.0	B	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Allagash	127	2	0.6	2.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	loamy over sandy	yes	loamy over sandy
Bangor	572	2	0.6	2.0	0.60	2.0	B	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam
Berkshire	72	2	0.6	6.0	0.60	6.0	B	Loose till, loamy textures	frigid	loamy	yes	fine sandy loam
Bice	226	2	0.6	6.0	0.60	6.0	B	Loose till, loamy textures	frigid	loamy	no	sandy loam
Canton	42	2	2.0	6.0	6.00	20.0	B	Loose till, sandy textures	mesic	loamy over sandy	no	loamy over loamy sand
Charlton	62	2	0.6	6.0	0.60	6.0	B	Loose till, loamy textures	mesic	loamy	no	fine sandy loam
Dutchess	366	2	0.6	2.0	0.60	2.0	B	Friable till, silty, schist & phyllite	mesic	loamy	no	very channery
Fryeburg	208	2	0.6	2.0	2.00	6.0	B	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Groveton	27	2	0.6	2.0	0.60	6.0	B	Outwash and Stream Terraces	frigid	loamy	yes	loamy over sandy
Hadley	8	2	0.6	2.0	0.60	6.0	B	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand
Hadley	108	2	0.6	2.0	0.60	6.0	B	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand, occ flooded
Hartland	31	2	0.6	2.0	0.20	2.0	B	Terraces and glacial lake plains	mesic	silty	no	very fine sandy loam
Haven	410	2	0.6	2.0	20.00	100.0	B	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Houghtonville	795	2	0.6	6.0	0.60	6.0	B	Loose till, loamy textures	frigid	loamy	yes	cobbly fine sandy loam
Lombard	259	2	0.6	6.0	2.00	20.0	C/D	Weathered bedrock, phyllite	frigid	loamy	no	very channery
Monadnock	142	2	0.6	2.0	2.00	6.0	B	Loose till, sandy textures	frigid	loamy over sandy, sandy-skeletal	yes	gravelly loamy sand in C
Occum	1	2	0.6	2.0	6.00	20.0	B	Flood Plain (Bottom Land)	mesic	loamy	no	loamy over loamy sand
Ondawa	101	2	0.6	6.0	6.00	20.0	B	Flood Plain (Bottom Land)	frigid	loamy	no	loamy over loamy sand
Ondawa	201	2	0.6	6.0	6.00	20.0	B	Flood Plain (Bottom Land)	frigid	loamy	no	occ flood, loamy over l. sand
Salmon	630	2	0.6	2.0	0.60	2.0	B	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Stetson	523	2	0.6	6.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	loamy over gravelly
Unadilla	30	2	0.6	2.0	2.00	20.0	B	Terraces and glacial lake plains	mesic	silty	no	silty over gravelly
Chichester	442	2	0.6	2.0	2.00	6.0	B	Loose till, sandy textures	frigid	loamy over sandy	no	loamy over loamy sand
Acton	146	3	2.0	20.0	2.00	20.0	B	Loose till, sandy textures	mesic	sandy-skeletal	no	cobbly loamy sand
Becket	56	3	0.6	2.0	0.06	0.6	C	Firm, platy, sandy till	frigid	loamy	yes	gravelly sandy loam in Cd
Belgrade	532	3	0.6	2.0	0.06	2.0	B	Terraces and glacial lake plains	mesic	silty	no	strata of fine sand
Bernardston	330	3	0.6	2.0	0.06	0.2	C	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Boxford	32	3	0.1	0.2	0.00	0.2	C	Silt and Clay Deposits	mesic	fine	no	silty clay loam

Soil Series	number	NHDES Soil Group	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Buckland	237	3	0.6	2.0	0.06	0.2	C	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Buxton	232	3	0.1	0.6	0.00	0.2	C	Silt and Clay Deposits	frigid	fine	no	silty clay
Canterbury	166	3	0.6	2.0	0.06	0.6	C	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Chatfield Var.	289	3	0.6	6.0	0.60	6.0	B	Loose till, bedrock	mesic	loamy	no	mwd to swpd
Chesuncook	126	3	0.6	2.0	0.02	0.2	C	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Colonel	927	3	0.6	2.0	0.06	0.6	C	Firm, platy, loamy till	frigid	loamy	yes	loam in Cd
Croghan	613	3	20.0	100.0	20.00	100.0	B	Outwash and Stream Terraces	frigid	sandy	yes	single grain in C
Dartmouth	132	3	0.6	2.0	0.06	0.6	B	Terraces and glacial lake plains	mesic	silty	no	thin strata silty clay loam
Deerfield	313	3	6.0	20.0	20.00	100.0	B	Outwash and Stream Terraces	mesic	sandy	no	single grain in C
Dixfield	378	3	0.6	2.0	0.06	0.6	C	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Dixmont	578	3	0.6	2.0	0.60	2.0	C	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam, platy in C
Duane	413	3	6.0	20.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	cemented (ortstein)
Eldridge	38	3	6.0	20.0	0.06	0.6	C	Sandy/loamy over silt/clay	mesic	sandy over loamy	no	
Elmridge	238	3	2.0	6.0	0.00	0.2	C	Sandy/loamy over silt/clay	mesic	loamy over clayey	no	
Elmwood	338	3	2.0	6.0	0.00	0.2	C	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	
Finch	116	3					C	Outwash and Stream Terraces	frigid	sandy	yes	cemented (ortstein)
Gilmanton	478	3	0.6	2.0	0.06	0.6	C	Firm, platy, loamy till	frigid	loamy	no	fine sandy loam in Cd
Henniker	46	3	0.6	2.0	0.06	0.6	C	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
Hitchcock	130	3	0.6	2.0	0.06	0.6	B	Terraces and glacial lake plains	mesic	silty	no	silt loam to silt in C
Howland	566	3	0.6	2.0	0.06	0.2	C	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	silt loam, platy in Cd
Lanesboro	228	3	0.6	2.0	0.06	0.2	C	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	channery silt loam in Cd
Lovewell	307	3	0.6	2.0	0.60	2.0	B	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Machias	520	3	2.0	6.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	sandy or sandy-skeletal	yes	strata sand/gravel in C
Madawaska	28	3	0.6	2.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
Madawaska, aquer	48	3	0.6	2.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
Marlow	76	3	0.6	2.0	0.06	0.6	C	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Melrose	37	3	2.0	6.0	0.00	0.2	C	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	silty clay loam in C
Metacomet	458	3	0.6	2.0	0.06	0.6	C	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
Metallak	404	3	6.0	100.0	6.00	100.0	B	Flood Plain (Bottom Land)	frigid	loamy over sandy	no	sandy or sandy-skeletal
Millis	39	3					C	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Montauk	44	3	0.6	6.0	0.06	0.6	C	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Mundal	610	3	0.6	2.0	0.06	0.6	C	Firm, platy, loamy till	frigid	loamy	yes	gravely sandy loam in Cd
Newfields	444	3	0.6	2.0	0.60	2.0	B	Loose till, sandy textures	mesic	loamy over sandy	no	sandy or sandy-skeletal
Nicholville	632	3	0.6	2.0	0.60	2.0	C	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Ninigret	513	3	0.6	6.0	6.00	20.0	B	Outwash and Stream Terraces	mesic	loamy over sandy	no	sandy or sandy-skeletal
Paxton	66	3	0.6	2.0	0.00	0.2	C	Firm, platy, loamy till	mesic	loamy	no	
Peru	78	3	0.6	2.0	0.06	0.6	C	Firm, platy, loamy till	frigid	loamy	yes	
Pittstown	334	3	0.6	2.0	0.06	0.2	C	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Plaisted	563	3	0.6	2.0	0.06	0.6	C	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Podunk	104	3	0.6	6.0	6.00	20.0	B	Flood Plain (Bottom Land)	frigid	loamy	no	loamy to coarse sand in C
Poocham	230	3	0.6	2.0	0.20	2.0	B	Terraces and glacial lake plains	mesic	silty	no	silt loam in C
Pootatuck	4	3	0.6	6.0	6.00	20.0	B	Flood Plain (Bottom Land)	mesic	loamy	no	single grain in C
Scio	531	3	0.6	2.0	0.60	2.0	B	Terraces and glacial lake plains	mesic	silty	no	gravely sand in C
Scituate	448	3	0.6	2.0	0.06	0.2	C	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Sheepscot	14	3	6.0	20.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravely coarse sand
Sisk	667	3	0.6	2.0	0.00	0.6	C	Firm, platy, loamy till	cryic	loamy	yes	sandy loam in Cd
Skerry	558	3	0.6	2.0	0.06	0.6	C	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Sudbury	118	3	2.0	6.0	2.00	20.0	B	Outwash and Stream Terraces	mesic	sandy	no	loam over gravely sand
Suffield	536	3	0.6	2.0	0.00	0.2	C	Sandy/loamy over silt/clay	mesic	silty over clayey	no	deep to clay C
Sunapee	168	3	0.6	2.0	0.60	6.0	B	Loose till, loamy textures	frigid	loamy	yes	
Sunapee var	269	3	0.6	2.0	0.60	6.0	B	Loose till, loamy textures	frigid	loamy	yes	frigid dystrodept
Surplus	669	3	0.6	2.0	0.00	0.6	C	Firm, platy, loamy till	cryic	loamy	yes	mwd, sandy loam in Cd
Sutton	68	3	0.6	6.0	0.60	6.0	B	Loose till, loamy textures	mesic	loamy	no	
Telos	123	3	0.6	2.0	0.02	0.2	C	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd

Soil Series	number	NHDES Soil Group	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Waumbeck	58	3	2.0	20.0	6.00	20.0	B	Loose till, sandy textures	frigid	sandy-skeletal	yes	very cobbly loamy sand
Winooski	103	3	0.6	6.0	0.60	6.0	B	Flood Plain (Bottom Land)	mesic	silty	no	very fine sandy loam
Woodbridge	29	3	0.6	2.0	0.00	0.6	C	Firm, platy, loamy till	mesic	loamy	no	sandy loam in Cd
Winooski	9	3	0.6	6.0	0.60	6.0	B	Flood Plain (Bottom Land)	mesic	silty over loamy	no	
Canaan	663	4	2.0	20.0	2.00	20.0	C	Weathered Bedrock Till	frigid	loamy-skeletal	yes	less than 20 in. deep
Cardigan	357	4	0.6	2.0	0.60	2.0	B	Friable till, silty, schist & phyllite	mesic	loamy	no	20 to 40 in. deep
Chatfield	89	4	0.6	6.0	0.60	6.0	B	Loose till, bedrock	mesic	loamy	no	20 to 40 in. deep
Elliottsville	128	4	0.6	2.0	0.60	2.0	B	Friable till, silty, schist & phyllite	frigid	loamy	yes	20 to 40 in. deep
Glebe	671	4	2.0	6.0	2.00	6.0	C	Loose till, bedrock	cryic	loamy	yes	20 to 40 in. deep
Glover	NA	4	0.6	2.0	0.60	2	D	Friable till, silty, schist & phyllite	frigid	loamy	no	less than 20 in. deep
Hogback	91	4	2.0	6.0	2.00	6.0	C	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Hollis	86	4	0.6	6.0	0.60	6.0	C/D	Loose till, bedrock	mesic	loamy	no	less than 20 in. deep
Kearsarge	359	4	0.6	2.0	0.60	2.0	B	Friable till, silty, schist & phyllite	mesic	loamy	no	less than 20 in. deep
Lyman	92	4	2.0	6.0	2.00	6.0	A/D	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Macomber	252	4	0.6	2.0	0.60	2.0	C	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Millsite	251	4	0.6	6.0	0.60	6.0	C	Loose till, bedrock	frigid	loamy	no	20 to 40 in. deep
Monson	133	4	0.6	2.0	0.60	2.0	D	Friable till, silty, schist & phyllite	frigid	loamy	yes	less than 20 in. deep
Pennichuck	460	4	0.6	2.0	0.60	2.0	B	Friable till, silty, schist & phyllite	mesic	loamy-skeletal	no	20 to 40 in. deep
Rawsonville	98	4	0.6	6.0	0.60	6.0	C	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Ricker	674	4	2.0	6.0	2.00	6.0	A	rganic over bedrock (up to 4" of miner)	cryic	fibric to hemic	no	well drained, less than 20 in. deep
Saddleback	673	4	0.6	2.0	0.60	2.0	C/D	Loose till, bedrock	cryic	loamy	yes	less than 20 in. deep
Shapleigh	136	4					C/D	Sandy Till	mesic	sandy	yes	less than 20 in. deep
Thorndike	84	4	0.6	2.0	0.60	2.0	C/D	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	less than 20 in. deep
Tunbridge	99	4	0.6	6.0	0.60	6.0	C	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Winnecook	88	4	0.6	2.0	0.60	2.0	C	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Woodstock	93	4	2.0	6.0	2.00	6.0	C/D	Loose till, bedrock	frigid	loamy	no	less than 20 in. deep
Au Gres	516	5					B	Outwash and Stream Terraces	frigid	sandy	yes	single grain, loose
Bemis	224	5	0.6	0.2	0.00	0.2	C	Firm, platy, loamy till	cryic	loamy	no	
Binghamville	534	5	0.2	2.0	0.06	0.2	D	Terraces and glacial lake plains	mesic	silty	no	
Brayton	240	5	0.6	2.0	0.06	0.6	C	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Cabot	589	5	0.6	2.0	0.06	0.2	D	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Charles	209	5	0.6	100.0	0.60	100.0	C	Flood Plain (Bottom Land)	frigid	silty	no	
Cohas	505	5	0.6	2.0	0.60	100.0	C	Flood Plain (Bottom Land)	frigid	co. loamy over sandy (skeletal)	no	
Grange	433	5	0.6	2.0	0.60	2.0	C	Outwash and Stream Terraces	frigid	co. loamy over sandy (skeletal)	no	
Kinsman	614	5	6.0	20.0	6.00	20.0	C	Outwash and Stream Terraces	frigid	sandy	yes	
Leicester	514	5	0.6	6.0	0.60	20.0	C	Loose till, loamy textures	mesic	loamy	no	
Lim	3	5	0.6	2.0	6.00	20.0	C	Flood Plain (Bottom Land)	mesic	loamy	no	
Limerick	109	5	0.6	2.0	0.60	2.0	C	Flood Plain (Bottom Land)	mesic	silty	no	
Lyme	246	5	0.6	6.0	0.60	6.0	C	Loose till, sandy textures	frigid	loamy	no	
Mashpee	315	5	6.0	20.0	6.00	20.0	B	Outwash and Stream Terraces	mesic	sandy	yes	
Monarda	569	5	0.2	2.0	0.02	0.2	D	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Moosilauke	414	5	6.0	20.0	6.00	20.0	C	Loose till, sandy textures	frigid	sandy	no	
Naumburg	214	5	6.0	20.0	6.00	20.0	C	Outwash and Stream Terraces	frigid	sandy	yes	
Pemi	633	5	0.6	2.0	0.06	0.6	C	Terraces and glacial lake plains	frigid	silty	no	
Pillsbury	646	5	0.6	2.0	0.06	0.2	C	Firm, platy, loamy till	frigid	silty	no	
Pipestone	314	5					B	Outwash and Stream Terraces	mesic	sandy	yes	
Raynham	533	5	0.2	2.0	0.06	0.2	C	Terraces and glacial lake plains	mesic	silty	no	
Raypol	540	5	0.6	2.0	6.00	100.0	D	Outwash and Stream Terraces	mesic	co. loamy over sandy (skeletal)	no	
Ridgebury	656	5	0.6	6.0	0.00	0.2	C	Firm, platy, loamy till	mesic	loamy	no	
Rippowam	5	5	0.6	6.0	6.00	20.0	C	Flood Plain (Bottom Land)	mesic	loamy	no	
Roundabout	333	5	0.2	2.0	0.06	0.6	C	Terraces and glacial lake plains	frigid	silty	no	silt loam in the C
Rumney	105	5	0.6	6.0	6.00	20.0	C	Flood Plain (Bottom Land)	frigid	loamy	no	

Sorted by DES Soil Group for Establishing Lot Size  
K<sub>sat</sub> B and C horizons  
SSSNNE pub no. 5

Soil Series	number	NHDES Soil Group	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Saugatuck	16	5	0.06	0.2	6.00	20.0	C	Outwash and Stream Terraces	mesic	sandy	yes	ortstein
Scantic	233	5	0.0	0.2	0.00	0.2	D	Silt and Clay Deposits	frigid	fine	no	
Scitico	33	5	0.0	0.2	0.00	0.2	C	Silt and Clay Deposits	mesic	fine	no	
Shaker	439	5	2.0	6.0	0.00	0.2	C	Sandy/loamy over silt/clay	mesic	co. loamy over clayey	no	
Squamscott	538	5	6.0	20.0	0.06	0.6	C	Sandy/loamy over silt/clay	mesic	sandy over loamy	yes	
Stissing	340	5	0.6	2.0	0.06	0.2	C	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	
Swanton	438	5	2.0	6.0	0.00	0.2	C	Sandy/loamy over silt/clay	frigid	co. loamy over clayey	no	
Walpole	546	5	2.0	6.0	6.00	20.0	C	Outwash and Stream Terraces	mesic	sandy	no	
Wareham	34	5	6.0	20.0	6.00	20.0	C	Outwash and Stream Terraces	mesic	sandy	no	
Biddeford	234	6	0.0	0.2	0.00	0.2	D	Silt and Clay Deposits	frigid	fine	no	organic over clay
Bucksport	895	6					D	Organic Materials - Freshwater	frigid	sapric	no	deep organic
Burnham	131	6	0.2	6.0	0.02	0.2	D	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	organic over silt
Catden	296	6					A/D	Organic Materials - Freshwater	mesic	sapric	no	deep organic
Chocorua	395	6			6.00	20.0	D	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Greenwood	295	6					A/D	Organic Materials - Freshwater	frigid	hemic	no	deep organic
Ipswich	397	6					D	Tidal Flat	mesic	hemic/sapric	no	deep organic
Matunuck	797	6			20.00	100.0	D	Tidal Flat	mesic	sandy	no	organic over sand
Maybid	134	6	0.0	0.2	0.00	0.2	D	Silt and Clay Deposits	mesic	fine	no	silt over clay
Meadowsedge	894	6					D	Organic Materials - Freshwater	frigid	peat	no	deep organic
Medomak	406	6	0.6	2.0	0.60	2.0	D	Flood Plain (Bottom Land)	frigid	silty	no	organic over silt
Natchaug	496	6			0.20	2.0	D	Organic Materials - Freshwater	mesic	loamy	no	organic over loam
Ossipee	495	6			0.20	2.0	D	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Pawcatuck	497	6			20.00	100.0	D	Tidal Flat	mesic	sandy or sandy-skeletal	no	organic over sand
Peacham	549	6	0.6	2.0	0.00	0.2	D	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	organic over loam
Pondicherry	992	6			6.00	20.0	D	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Saco	6	6	0.6	2.0	6.00	20.0	D	Flood Plain (Bottom Land)	mesic	silty	no	strata
Scarboro	115	6	6.0	20.0	6.00	20.0	D	Outwash and Stream Terraces	mesic	sandy	no	organic over sand, non stony
Searsport	15	6	6.0	20.0	6.00	20.0	D	Outwash and Stream Terraces	frigid	sandy	no	organic over sand
Timakwa	393	6			6.00	100.0	D	Organic Materials - Freshwater	mesic	sandy or sandy-skeletal	no	organic over sand
Vassalboro	150	6					D	Organic Materials - Freshwater	frigid	peat	no	deep organic
Waskish	195	6					D	Organic Materials - Freshwater	frigid	peat	no	deep organic
Westbrook	597	6			0.00	2.0	D	Tidal Flat	mesic	loamy	no	organic over loam
Whitman	49	6	0.0	0.2	0.00	0.2	D	Firm, platy, loamy till	mesic	loamy	no	mucky loam
Wonsqueak	995	6			0.20	2.0	D	Organic Materials - Freshwater	frigid	loamy	no	organic over loam

no longer recognized      organic materials      denotes break between Soil Group

# ORDER FORM

## **Ksat VALUES FOR NEW HAMPSHIRE SOILS (Including Hydrologic and Soil Lot Sizing Groups)**

SSSNNE Publication #5

DATE \_\_\_\_\_

Mail Copy of Publication #5 to:

Name \_\_\_\_\_

Address \_\_\_\_\_

City \_\_\_\_\_ State \_\_\_\_\_ Zip \_\_\_\_\_

Quantity of Publications \_\_\_\_\_ @ \$8.00 = Total Order: \$ \_\_\_\_\_

Amount Enclosed: \$ \_\_\_\_\_

Enclose check or money order for the full amount payable to:

**Society of Soil Scientists of Northern New England  
PO Box 76  
Durham, NH 03824**

Please allow 4 weeks for delivery.

**Each Watershed Report Card covers a single 12-digit Hydrologic Unit Code (HUC12), on average a 34 square mile area. Each Watershed Report Card has three components;**

1. REPORT CARD - A one page card that summarizes the overall use support for Aquatic Life Integrity, Primary Contact (i.e. Swimming), and Secondary Contact (i.e. Boating) Designated Uses on every Assessment Unit ID (AUID) within the HUC12.
2. HUC 12 MAP - A map of the watershed with abbreviated labels for each AUID within the HUC12.
3. ASSESSMENT DETAILS - Anywhere from one to forty pages with the detailed assessment information for each and every AUID in the Report Card and Map.

**How are the Surface Water Quality Assessment determinations made?**

All readily available data with reliable Quality Assurance/Quality Control is used in the biennial surface water quality assessments. For a full understanding of how the Surface Water Quality Standards (Env-Wq 1700) are translated into surface water quality assessments we urge the reader to review the 2020/2022 [Consolidated Assessment and Listing Methodology](#) (CALM).

**Where can I find more advanced mapping resources?**

GIS files are available by assessment cycle at the NHDES [FTP site](#).

**I'd like to see the more raw water quality data?**

The [web mapping tool](#) allows you to download the data used in the assessment of the primary contact and aquatic life designated uses by clicking on the “[Data Access Waterbody Data \(Aquatic Life and Swimming Uses\)](#)” link for any assessment unit.

**How are assessments coded in the report card?**

Assessment outcomes are displayed on a color scale as well as an alpha numeric scale that provides additional distinctions for the designated use and parameter level assessments as outlined in the table below.

	Severe	Poor	Likely Bad	No Data	Likely Good	Marginal	Good
	Not Supporting, Severe	Not Supporting, Marginal	Insufficient Information – Potentially Not Supporting	No Data	Insufficient Information – Potentially Full Supporting	Full Support, Marginal	Full Support, Good
CATEGORY	Description						
Category 2	Meets standards					2-M or 2-OBS	2-G
Category 3	Insufficient Information		3-PNS	3-ND	3-PAS		
Category 4	Does not Meet Standards;						
4A	TMDL* Completed	4A-P	4A-M or 4A-T				
4B	Other enforceable measure will correct the issue.	4B-P	4B-M or 4B-T				
4C	Non-pollutant (i.e. exotic weeds)	4C-P	4C-M				
Category 5	TMDL* Needed	5-P	5-M or 5-T				

\* [TMDL](#) stands for Total Maximum Daily Load studies

# Watershed 305(b) Assessment Summary Report:

Assessment Cycle: 2020/2022

HUC 12: 010600030903

HUC 12 Name: Bellamy River

(Locator map on next page only applies to this HUC12)

Good	Meets water quality standards/thresholds by a relatively large margin.
Marginal	Meets water quality standards/thresholds but only marginally.
Likely Good	Limited data available, however, the data that is available suggests that the parameter is Potentially Attaining Standards (PAS).
No Current Data	Insufficient information to make an assessment decision.
Likely Bad	Limited data available, however, the data that is available suggests that the parameter is Potentially Not Supporting (PNS) water quality standards.
Poor	Not meeting water quality standards/thresholds. The impairment is marginal.
Severe	Not meeting water quality standards/thresholds. The impairment is more severe and causes poor water quality.

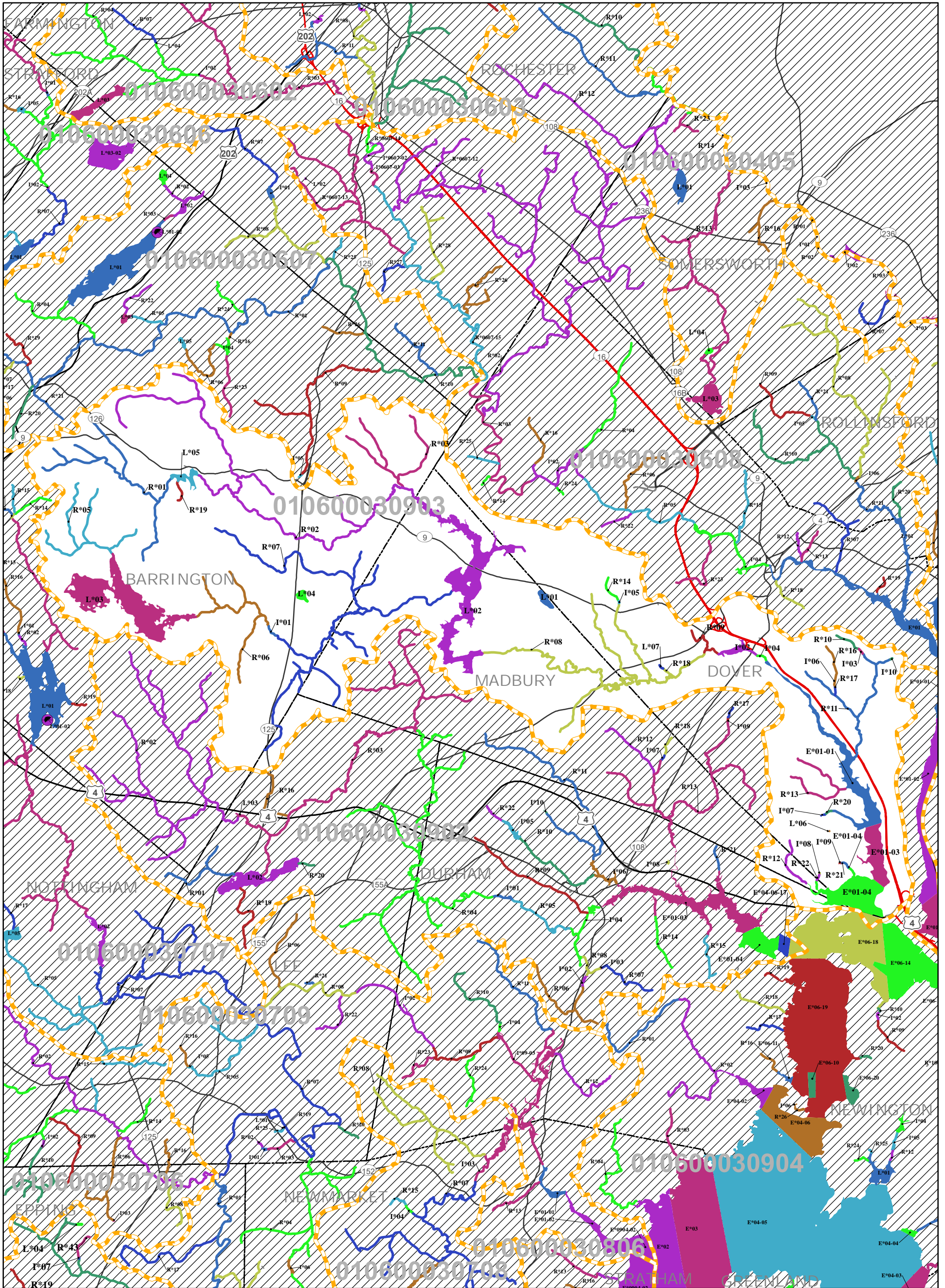


Assessment Unit ID	Map Label	Assessment Unit Name	Aquatic Life	Fish Consump.	Swimming	Boating
NHEST600030903-01-01	E*01-01	Bellamy River North		5-M	2-G	2-G
NHEST600030903-01-03	E*01-03	Bellamy River South Clement Point	5-P	5-M	2-G	2-G
NHEST600030903-01-04	E*01-04	Bellamy River South	5-P	5-M	2-G	2-G
NHIMP600030903-01	I*01	Bellamy River	3-ND	4A-M	3-ND	3-ND
NHIMP600030903-02	I*02	Bellamy River - Sawyers Mill Dam Pond	5-M	4A-M	5-M	3-ND
NHIMP600030903-03	I*03	Canney Brook - Wildlife Pond	3-ND	4A-M	3-ND	3-ND
NHIMP600030903-04	I*04	Bellamy River Iv Dam	3-ND	4A-M	3-ND	3-ND
NHIMP600030903-05	I*05	Knox Marsh Brook	3-ND	4A-M	3-ND	3-ND
NHIMP600030903-06	I*06	Unnamed Brook - Thornwood Commons Pond	3-ND	4A-M	3-ND	3-ND
NHIMP600030903-07	I*07	Unnamed Brook - Bellamy River Wildlife Pond	3-ND	4A-M	3-ND	3-ND
NHIMP600030903-08	I*08	Unnamed Brook - Farm Pond	3-ND	4A-M	3-ND	3-ND
NHIMP600030903-09	I*09	Unnamed Brook - Webster Brook Dam	3-ND	4A-M	3-ND	3-ND



NHIMP600030903-10	I*10	Unnamed Brook - Farm Pond	3-ND	4A-M	3-ND	3-ND
NHLAK600030903-01	L*01	Barbadoes Pond	3-ND	4A-M	3-ND	3-ND
NHLAK600030903-02	L*02	Bellamy Reservoir	5-M	4A-M	3-ND	3-ND
NHLAK600030903-03	L*03	Swains Lake	5-P	4A-M	3-PAS	3-ND
NHLAK600030903-04	L*04	Winkley Pond	5-P	4A-M	3-ND	3-ND
NHLAK600030903-05	L*05	Branch Mallego Brook Pond	3-ND	4A-M	3-ND	3-ND
NHLAK600030903-06	L*06	Farm Pond	3-ND	4A-M	3-ND	3-ND
NHLAK600030903-07	L*07	Unnamed Pond	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-01	R*01	Madla Brook	5-M	4A-M	3-ND	3-ND
NHRIV600030903-02	R*02	Mallego Brook	5-P	4A-M	3-ND	3-ND
NHRIV600030903-03	R*03	Calef Brook	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-05	R*05	Unnamed Brook - To Swains Lake	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-06	R*06	Bellamy River - Unnamed Brook	5-M	4A-M	3-ND	3-ND
NHRIV600030903-07	R*07	Bellamy River	5-P	4A-M	4A-P	4A-P
NHRIV600030903-08	R*08	Bellamy River - Kelly Brook - Knox Marsh Brook	5-P	4A-M	4A-P	3-PAS
NHRIV600030903-09	R*09	Bellamy River - Unnamed Brook	5-M	4A-M	4A-P	3-ND
NHRIV600030903-10	R*10	Canney Brook	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-11	R*11	Varney Brook - Canney Brook	3-ND	4A-M	4A-P	4A-M
NHRIV600030903-12	R*12	Unnamed Brook - To Bellamy River Royalls Cove	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-13	R*13	Garrison Brook	3-ND	4A-M	4A-P	3-ND
NHRIV600030903-14	R*14	Knox Marsh Brook	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-16	R*16	Unnamed Brook	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-17	R*17	Varney Brook	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-18	R*18	Unnamed Brook	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-19	R*19	Unnamed Brook	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-20	R*20	Unnamed Brook	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-21	R*21	Unnamed Brook	3-ND	4A-M	3-ND	3-ND
NHRIV600030903-22	R*22	Unnamed Brook	3-ND	4A-M	3-ND	3-ND

AUIDs For HUC12: 010600030903 - Bellamy River

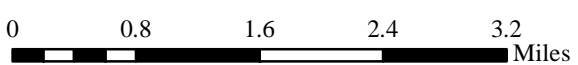


	HUC12 Boundaries	<b>Assessment Unit Coloring</b>	4 =
	Town Boundaries	<b>AUs Ending with:</b>	5 =
	Major Roads	0 =	6 =
	Interstate Highway	1 =	7 =
	US Highway	2 =	8 =
	State Highway	3 =	9 =



<b>Abbrev. Label</b>	<b>HUC 12</b>
L*03	010700060201
<b>AUID = NH LAK700060201-03</b>	

Assessment Unit IDs are derived from the HUC12 they reside within. The labels have been shortened on this map for presentation purposes. Example: the Label "L\*03" in HUC12 = 010700060201 represents AUID = "NHLAK700060201-03" In rare cases where an AUID extends beyond the boundary of a single HUC12, additional portions of the end of the HUC 12 number have also been replaced.



Scale: 1:78,820

Assessment Unit ID: NHRIV600030903-02

Size: 9.5430 MILES

2020/2022, 305(b)/303(d) - All

Assessment Unit Name: Mallego Brook

Assessment Unit Category: 5-P

Reviewed Parameters by Assessment

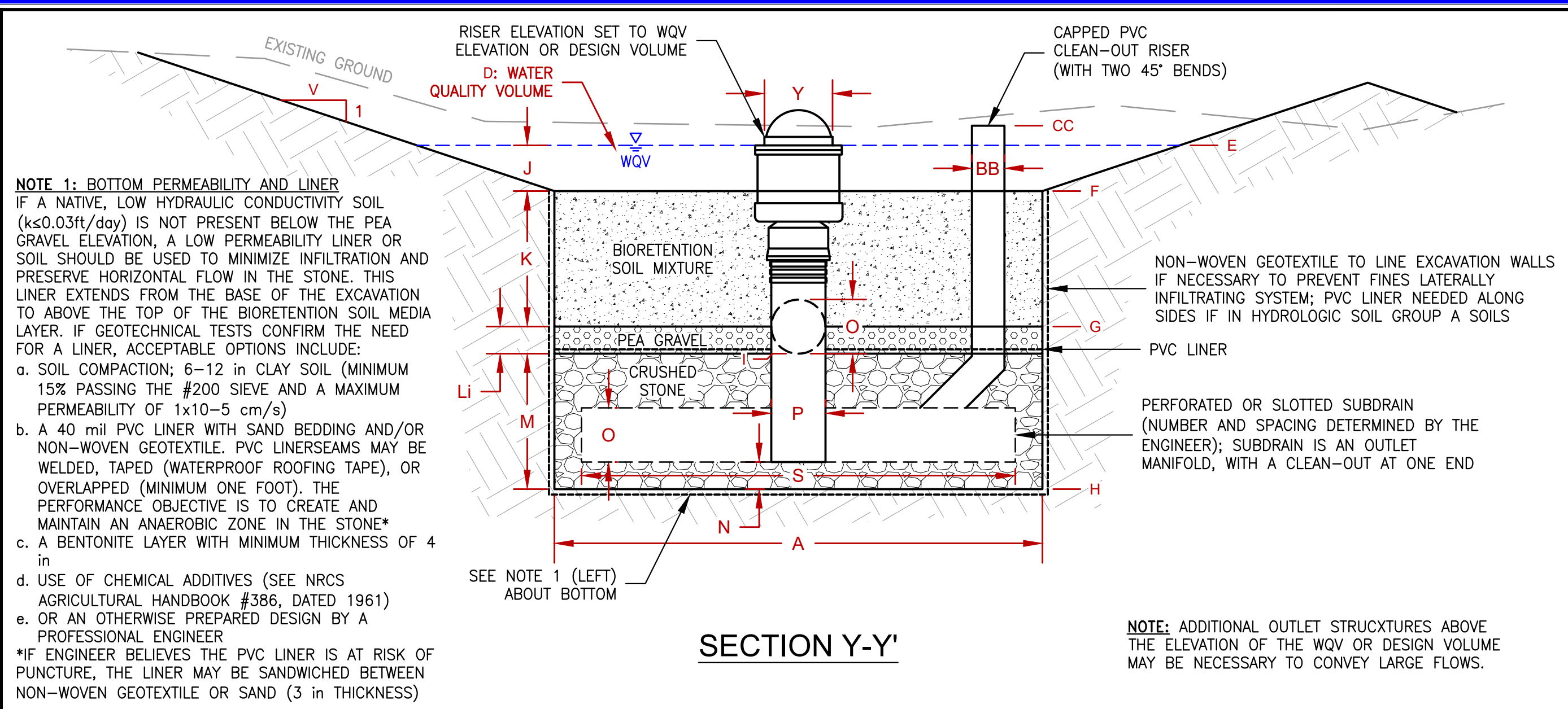
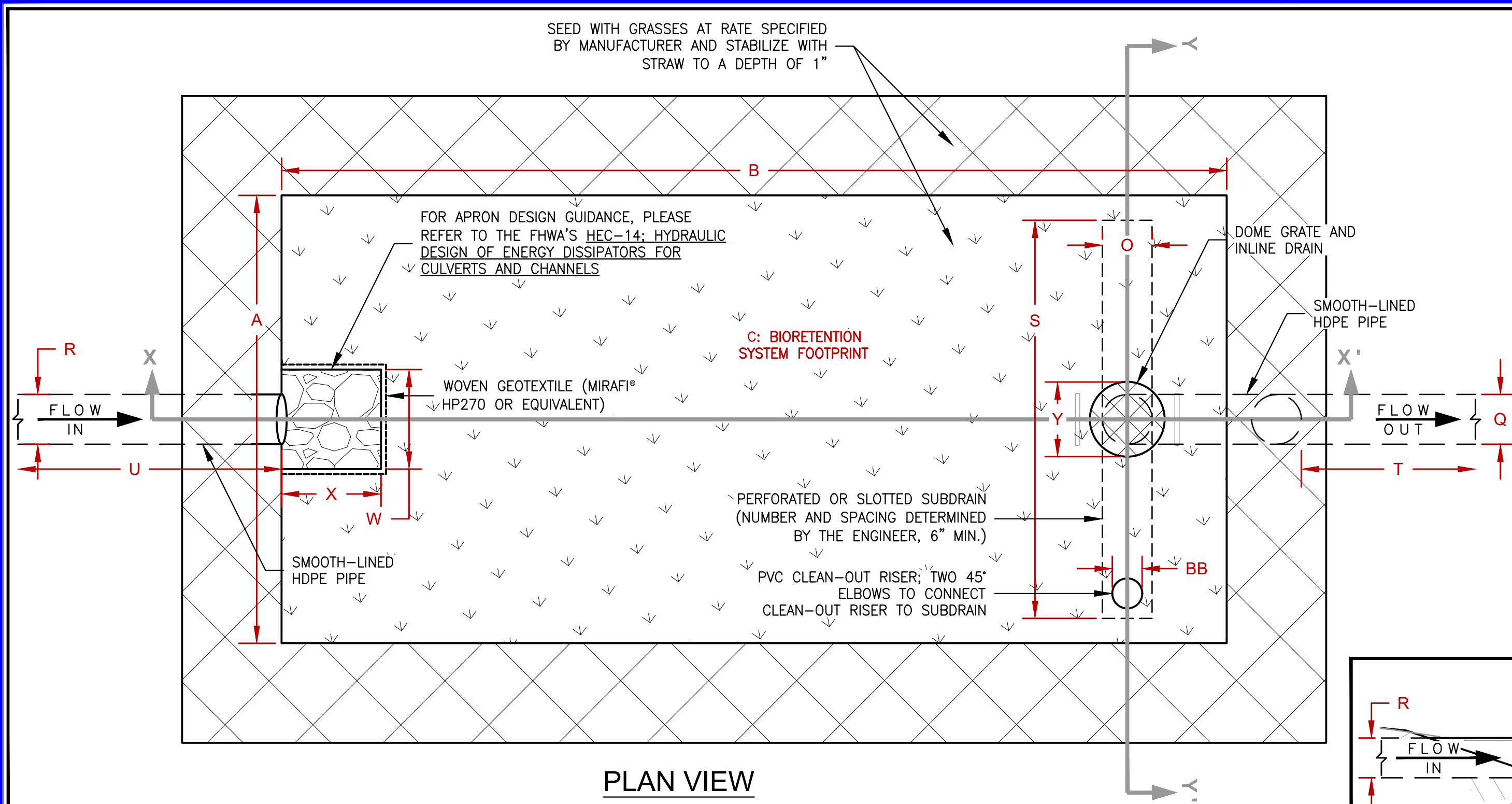
Town(s) Primary Town is Listed First: Barrington, Beach: N

Unit

Madbury

Designated Use Description	Desig. Use Category	Parameter Name	Parameter Threatened (Y/N)	Last Sample	Last Exceed	Parameter Category	TMDL Priority
Aquatic Life Integrity	5-P	AMMONIA (TOTAL)	N	2016	N/A	3-PAS	
		Benthic-Macroinvertebrate Bioassessments (Streams)	N			3-ND	
		CHLORIDE	N	2019	N/A	3-PAS	
		DISSOLVED OXYGEN SATURATION	N	2019	2017	5-M	LOW
		Fishes Bioassessments (Streams)	N			3-PAS	
		OXYGEN, DISSOLVED	N	2019	2018	5-M	LOW
		PH	N	2019	2019	5-P	LOW
		TURBIDITY	N	2019	N/A	3-PAS	
Fish Consumption	4A-M	MERCURY - FISH CONSUMPTION ADVISORY	N			4A-M	
Potential Drinking Water Supply	2-G						
Primary Contact Recreation	3-ND	Escherichia coli	N			3-ND	
Secondary Contact Recreation	3-ND	Escherichia coli	N			3-ND	
Wildlife	3-ND						

<p><b>Good</b> Meets water quality standards/thresholds by a relatively large margin.</p>	<p><b>Marginal</b> Meets water quality standards/thresholds but only marginally.</p>	<p><b>Likely Good</b> Limited data available. The data that is available suggests that the parameter is Potentially Attaining Standards (PAS)</p>	<p><b>No Current Data</b> Insufficient information to make an assessment decision.</p>	<p><b>Likely Bad</b> Limited data available The data that is available suggests that the parameter is Potentially Not Supporting (PNS) water quality standards.</p>	<p><b>Poor</b> Not meeting water quality standards/thresholds. The impairment is marginal.</p>	<p><b>Severe</b> Not meeting water quality standards/thresholds The impairment is more severe and causes poor water quality.</p>
---	--	---	--	---	--	--



**NOTE 1: BOTTOM PERMEABILITY AND LINER**  
 IF A NATIVE, LOW HYDRAULIC CONDUCTIVITY SOIL ( $k \leq 0.03 \text{ ft/day}$ ) IS NOT PRESENT BELOW THE PEA GRAVEL ELEVATION, A LOW PERMEABILITY LINER OR SOIL SHOULD BE USED TO MINIMIZE INFILTRATION AND PRESERVE HORIZONTAL FLOW IN THE STONE. THIS LINER EXTENDS FROM THE BASE OF THE EXCAVATION TO ABOVE THE TOP OF THE BIORETENTION SOIL MEDIA LAYER. IF GEOTECHNICAL TESTS CONFIRM THE NEED FOR A LINER, ACCEPTABLE OPTIONS INCLUDE:  
 a. SOIL COMPACTION; 6-12 in CLAY SOIL (MINIMUM 15% PASSING THE #200 SIEVE AND A MAXIMUM PERMEABILITY OF  $1 \times 10^{-5} \text{ cm/s}$ )  
 b. A 40 mil PVC LINER WITH SAND BEDDING AND/OR NON-WOVEN GEOTEXTILE. PVC LINERSEAMS MAY BE WELDED, TAPED (WATERPROOF ROOFING TAPE), OR OVERLAPPED (MINIMUM ONE FOOT). THE PERFORMANCE OBJECTIVE IS TO CREATE AND MAINTAIN AN ANAEROBIC ZONE IN THE STONE\*  
 c. A BENTONITE LAYER WITH MINIMUM THICKNESS OF 4 in  
 d. USE OF CHEMICAL ADDITIVES (SEE NRCS AGRICULTURAL HANDBOOK #386, DATED 1961)  
 e. OR AN OTHERWISE PREPARED DESIGN BY A PROFESSIONAL ENGINEER  
 \*IF ENGINEER BELIEVES THE PVC LINER IS AT RISK OF PUNCTURE, THE LINER MAY BE SANDWICHED BETWEEN NON-WOVEN GEOTEXTILE OR SAND (3 in THICKNESS)

**NOTES:**

- FOR FULL BIORETENTION STORMWATER SYSTEM SPECIFICATIONS, PLEASE REFER TO THE UNH STORMWATER CENTER'S BIORETENTION SPECIFICATIONS PUBLICATION, DATED FEBRUARY 2017, FOUND AT: [https://www.unh.edu/unhsc/sites/default/files/media/unhsc\\_bsm\\_spec\\_2-28-17\\_0.pdf](https://www.unh.edu/unhsc/sites/default/files/media/unhsc_bsm_spec_2-28-17_0.pdf)
- SYSTEM FOOTPRINT NEED NOT BE RECTANGULAR. ANY SHAPE IS POSSIBLE. THESE DETAILS USE THE RECTANGULAR SHAPE AS AN EXAMPLE.
- THESE DETAILS ARE NOT TO SCALE; FOR DIMENSIONS AND SPECIFICATIONS, REFERENCE EACH LETTER TO THE TABLE OF METRICS.
- BIORETENTION SOIL MIX SHALL NOT BE PLACED UNTIL AFTER ENGINEERING APPROVAL AND INSPECTION OF SUBGRADE.
- BIORETENTION SYSTEM IS RECOMMENDED TO HAVE PRETREATMENT (FOREBAY, SWALE, OR OTHER APPROVED STRUCTURE). PRETREATMENT IS REQUIRED FOR PROJECTS REQUIRING ALTERATION OF TERRAIN (AOT) PERMITTING.
- PLANT THE SYSTEM AS SPECIFIED; AT A MINIMUM, SEED THE SYSTEM FLOOR AND SIDE SLOPES WITH RYE GRASS MIXTURE CONTAINING PERENNIAL AND WINTER RYES, AT A RATE SPECIFIED BY THE MANUFACTURER. STABILIZE THE SLOPES WITH STRAW TO A DEPTH OF 1".
- GENERAL CONSTRUCTION GUIDELINES:
  - VERIFY THAT NO FOREIGN OR DELETERIOUS MATERIAL OR LIQUID SUCH AS PAINT, PAINT WASHOUT, CONCRETE SLURRY, ASPHALT/CONCRETE LAYERS OR CHUNKS, CEMENT, PLASTER, OILS, GASOLINE, DIESEL FUEL, PAINT THINNER, TURPENTINE, TAR, ROOFING COMPOUND, SOLID WASTE, OR ACID HAS BEEN DEPOSITED IN PLANTING SOIL (BIORETENTION MEDIA OR LOAM ON SIDE SLOPES).
  - PROCEED WITH PLACEMENT OF ANY SUBSURFACE MATERIALS ONLY AFTER UNSATISFACTORY CONDITIONS HAVE BEEN CORRECTED.
  - COMPACT EACH BLENDED LIFT OF BIORETENTION SOIL MEDIA TO 75% OF MAXIMUM STANDARD PROCTOR DENSITY ACCORDING TO ASTM D698.
  - GRADE SOIL MEDIA TO A SMOOTH, UNIFORM SURFACE PLANE WITH LOOSE, UNIFORMLY FINE TEXTURE. ROLL AND RAKE, REMOVE RIDGES, AND FILL DEPRESSIONS TO MEET FINISH GRADES.
  - LIGHTLY COMPACT FINISHED FLOOR ELEVATION AND FINISHED SLOPES USING THE BUCKET OF AN EXCAVATOR, NON-MOTORIZED ROLLER, HAND TAMP, OR OTHER MEANS, THEN ROUGHEN SURFACE WITH A RAKE TO LOOSEN SOILS BEFORE SEEDING.
  - DO NOT COMPACT THE SUBGRADE AT THE BOTTOM OF EXCAVATION UNLESS PERMEABILITY EXCEEDS  $1 \times 10^{-5} \text{ cm/s}$
- BIORETENTION SOIL MEDIA (BSM) MIXTURE SPECIFICATIONS:
  - STICKS AND ROOTS SHOULD BE MINIMIZED IN THE BSM MIXTURE, AND PREFERABLY LIMITED TO NOTHING LARGER THAN 4.76 mm (0.187 in).
  - DEBRIS AND OTHER FOREIGN MATERIALS SHOULD BE MINIMIZED.
  - ORGANIC MATTER SHOULD MAKE UP A MINIMUM OF 3% BY VOLUME AND A MAXIMUM 8% BY VOLUME OF THE BSM.
  - BSM MIXTURE SHOULD HAVE A SOIL REACTION pH OF 6 TO 7.
  - CATION EXCHANGE CAPACITY (CEC) OF BSM SHOULD BE A MINIMUM OF 10 meq PER 100 mL AT A pH OF 7.0.
- IF BSM IS PURCHASED FROM A MANUFACTURER, BSM MIXTURE SHALL NOT CONTAIN THE FOLLOWING:
  - UNACCEPTABLE MATERIALS: CONCRETE SLURRY, CONCRETE LAYERS OR CHUNKS,

- CEMENT, PLASTER, BUILDING DEBRIS, ASPHALT, BRICKS, OILS, GASOLINE, DIESEL FUEL, PAINT THINNER, TURPENTINE, TAR, ROOFING COMPOUND, ACID, SOLID WASTE, OR OTHER EXTRANEIOUS MATERIALS THAT ARE HARMFUL TO PLANTS.
- UNUSABLE MATERIALS: STONES, ROOTS, PLANTS, SOD, CLAY LUMPS, OR POCKETS OF COARSE SAND THAT EXCEED A COMBINED MAXIMUM OF 5% BY DRY WEIGHT OF THE MANUFACTURED SOIL.
- LARGE MATERIALS: STONES, CLODS, ROOTS, CLAY LUMPS EXCEEDING 0.187 in (4.76 mm) IN ANY DIMENSION.
- ORGANIC SOIL AMENDMENTS:
  - NO COMPOST SHOULD BE USED IN THE PLANTING MIX (USED ON THE SIDE SLOPES AND SURROUNDING AREA) UNLESS SPECIFIED BY THE ENGINEER.
  - SPHAGNUM PEAT: PARTIALLY DECOMPOSED SPHAGNUM PEAT MOSS, FINELY DIVIDED OR OF GRANULAR TEXTURE WITH 100% PASSING THROUGH A 1/2-in (13 mm) SIEVE, WITH A pH OF 3.4 TO 4.8.
  - WOOD DERIVATIVES: SHREDDED WOOD, WOOD CHIPS, GROUND BARK, OR WOOD WASTE; OF UNIFORM TEXTURE AND FREE OF STONES, STICKS, SOIL, OR TOXIC MATERIAL.
- THE CRUSHED STONE LAYER SHOULD CONSIST OF AASHTO #5 STONE (3/4-in).
- THE VOLUME OF WATER CONTAINED ABOVE THE BSM ELEVATION AND BELOW THE HIGH FLOW SPILLWAY IS STATISTICALLY DESIGNED TO HOLD A SPECIFIC RUNOFF VOLUME.
- THE DESIGN VOLUME ABOVE THE BSM IS PREFERABLY THE WQV. THIS VOLUME MAY NOT BE ACHIEVABLE FOR RETROFIT INSTALLATIONS

BIORETENTION SYSTEM DESIGN METRICS				
ID	DESIGN PARAMETER	MIN	DESIGN	UNITS
A	SYSTEM FLOOR WIDTH			FT
B	SYSTEM FLOOR LENGTH			FT
C	BIORETENTION FOOTPRINT AREA			SF
D	WATER QUALITY VOLUME			CF
E	WQV AND RISER CAP ELEVATION			FT
F	SYSTEM FLOOR ELEVATION			FT
G	BOTTOM BSM ELEVATION			FT
H	BOTTOM STONE ELEVATION			FT
I	TOP STONE/OUTLET INVERT ELEVATION			FT
J	WQV PONDING DEPTH			IN
K	BSM MEDIA DEPTH	18		IN
Li	INLET END PEA GRAVEL DEPTH			IN
Lo	OUTLET END PEA GRAVEL DEPTH	3		IN
Mi	INLET END CRUSHED STONE DEPTH			IN
Mo	OUTLET END CRUSHED STONE DEPTH	14		IN
N	SUBDRAIN DEPTH ABOVE BOTTOM	4		IN
O	PERFORATED SUBDRAIN DIAMETER	6		IN

BIORETENTION SYSTEM DESIGN METRICS				
ID	DESIGN PARAMETER	MIN	DESIGN	UNITS
P	RISER PIPE DIAMETER	6		IN
Q	OUTLET PIPE DIAMETER	6		IN
R	INFLOW PIPE DIAMETER			IN
S	PERFORATED SUBDRAIN LENGTH			FT
T	OUTLET PIPE LENGTH			FT
U	INFLOW PIPE LENGTH			FT
V	SLOPE GRADE (RUN PER 1ft RISE)			FT
W	ROCK APRON WIDTH			FT
X	ROCK APRON LENGTH			FT
Y	RISER DOME GRATE DIAMETER			IN
Z	PVC LINER SLOPE			%
AA	OUTLET PIPE SLOPE			%
BB	CLEAN-OUT RISER DIAMETER			IN
CC	CLEAN-OUT RISER ELEVATION			FT
DD	PVC LINER GAP	0.1*B		FT
EE	OUTLET PIPE ORIFICE DIAMETER	1		IN

ACCEPTABLE PARTICLE SIZE DISTRIBUTION OF FINAL BIORETENTION SOIL MIX				
MEDIA TYPE	SIEVE #	SIZE (in)	SIZE (mm)	% PASSING
COARSE SAND	4	0.187	4.76	100
MEDIUM SAND	10	0.079	2.00	95
FINE SAND	40	0.017	0.42	40-15
SILTS	200	0.003	0.075	10-20
CLAYS	<200	PAN	PAN	0-5

**BIORETENTION SOIL MEDIA COMPONENTS:\***  
 - AMOUNTS MIXED BY TOTAL VOLUME  
 • 60-85% - SAND (0.5 TO 2.0 mm) (SEE SPECS ABOVE)  
 • 15-25% - LOAM OR TOPSOIL  
 • 3-8% - ORGANIC MATTER  
 • 0-5% - WATER TREATMENT RESIDUALS OR IRON FILINGS\*\*  
 \*ALTERNATELY, USE MEDIA SPECIFIED IN THE ALTERATION OF TERRAIN RULES, Env-Wq 1508.07(k)  
 \*\*THIS IS AN AMENDMENT USED FOR ENHANCED PHOSPHORUS ADSORPTION

**INTERNAL STORAGE RESERVOIR (ISR) NOTES:**  
 • THE HYBRID BIORETENTION SYSTEM HARBORS AN ANAEROBIC INTERNAL STORAGE RESERVOIR FOR NITROGEN REMOVAL.  
 • THE ISR IS SEPARATED BY AN IMPERMEABLE PVC LINER BETWEEN THE PEA GRAVEL AND CRUSHED STONE LAYERS.  
 • THE PVC LINER SLOPES FROM THE OUTLET TOWARDS THE INLET TO MAXIMIZE STORAGE RETENTION AND PROVIDE EXTRA TREATMENT/FILTER TIME VIA PLUG FLOW THROUGH CRUSHED STONE.  
 • DESIGN GUIDELINES FOR THE SUBSURFACE GRAVEL WETLAND SPECIFICATIONS (UNHSC, 2016) IDENTIFIED THAT THE WATER VOLUME IN THE ISR BE AT LEAST 0.26\*WQV [WATER QUALITY VOLUME], OR 26% OF THE WQV.  
 • PVC LINER THICKNESS OF 40 TO 60 mil, PREFERABLY SEAMLESS. IF SEAMS ARE UNAVOIDABLE, THE SEAMS SHOULD BE SEALED.

The Contractor shall verify and be responsible for all dimensions. DO NOT scale the drawing - any errors or omissions shall be reported to UNHSC without delay. The Copyrights to all designs and drawings are the property of UNHSC. Reproduction or use for any purpose other than that authorized by UNHSC is forbidden.



University of New Hampshire  
 35 Colovos Road  
 Durham, NH 03824  
 Phone (603) 862-2818  
 Fax (603) 862-3957  
<http://www.unh.edu/unhsc>

02	10 Sept 2019	DES Revisions
01	12 Mar 2019	Initial design
No.	Date	Revision
Designed:	JCB	Checked:
		TPB/JJH
Approved:		TPB/JJH

GRAPHIC SCALE  
 N/A - DRAWING NOT TO SIZE  
  
 Original Drawing Size = 34 x 22 in.

Project: **STANDARD DETAIL BIORETENTION ISR**  
 STORMWATER SYSTEM

Date: 21 FEB 2020  
 Sheet No. 01 of 01

## **SWPPP Cut Sheet:**

### **Filtrex<sup>®</sup> Inlet Protection** *Sediment & Perimeter Control Technology*

#### **PURPOSE & DESCRIPTION**

Filtrex<sup>®</sup> Inlet protection is a three-dimensional tubular sediment control and storm water runoff filtration device typically used for storm drain **inlet protection** of sediment and soluble pollutants (such as phosphorus and petroleum hydrocarbons) on and around construction activities.

#### **APPLICATION**

**Drain inlets** are located in areas that receive runoff from surrounding lands, often exposed and disturbed soils, and are located at a low point, or in a sump. Inlet protection used around drain inlets (or *Drain Inlet protection*) should completely enclose the circumference of the drain and where possible should not be placed on a grade or slope. Inlet protection used around drain inlets should never be the only form of site sediment control and should be accompanied by erosion control/slope stabilization practices, such as Slope protection or rolled erosion control blankets (RECB). Inlet protection should never be placed where they divert runoff flow from the drain inlet, or on top of the inlet, which can cause flooding. Under high runoff and sediment loading conditions placement of 1-2 in (25-50 mm) diameter rock (AASHTO #2) may be placed around the outer circumference of the Inlet protection up to ½ the height of the Inlet protection. This will slow runoff velocity as it contacts the Inlet protection and will reduce sediment build-up and clogging of the Inlet protection.

**Curb inlets** are generally located on paved surfaces and are designed to rapidly drain storm runoff from roadways to prevent flooding that poses a hazard to vehicular traffic. Inlet protection devices should be placed in a manner which intercepts runoff prior to entering the inlet, but does not block or divert runoff from the inlet. To prevent diversion of runoff, Inlet protection used around curbs (or *Curb*

*inlet protection*) should be used in low points, or sumps, and minor slopes or grades. Inlet protection should never be placed in or on the curb inlet drain, or placed in a manner than obstructs vehicular traffic. Inlet protection height should be at least 1 in (25 mm) lower than top of curb inlet to allow for overflow into the drain and not over the curb. Maximum sediment removal efficiency occurs when minor ponding exists behind Inlet protection but should never lead to flooding.

**Curb sediment containment systems** are used to reduce the sediment and pollutant load flowing to a curb inlet. They are generally placed on paved surfaces perpendicular to runoff flow and should be lower than the height of the curb. Curb sediment containment systems should never cause flooding or placed where they are a hazard to vehicular traffic. Inlet protection used for curb sediment containment (or *Curb Sediment Containment Inlet protection*) can be placed on a grade but should never be placed directly upslope from curb inlet where it may inadvertently divert runoff from entering curb inlet.

#### **INSTALLATION**

1. Inlet protection used for inlet protection to reduce sediment and soluble pollutants entering storm drains shall meet Filtrex<sup>®</sup> FilterSoxx<sup>™</sup> Material Specifications and use Certified Filtrex<sup>®</sup> FilterMedia<sup>™</sup>.
2. Contractor is required to be a Filtrex<sup>®</sup> Certified<sup>™</sup> Installer as determined by Filtrex<sup>®</sup> International, LLC (440-926-2607 or visit web site at [Filtrex.com](http://Filtrex.com)). Certification shall be considered current if appropriate identification is shown during time of bid or at time of application (current list of installers can be found at [www.filtrex.com](http://www.filtrex.com)). Look for the Filtrex<sup>®</sup> Certified<sup>™</sup> Installer Seal.



3. Filtrexx® Inlet protection shall be placed at locations indicated on plans as directed by the Engineer. Inlet protection should be installed in a pattern that allows complete protection of the inlet area.
4. Installation of curb Inlet protection will ensure a minimal overlap of at least 1 ft (300mm) on either side of the opening being protected. The Inlet protection will be anchored to the soil behind the curb using staples, stakes or other devices capable of holding the Inlet protection in place.
5. Standard Inlet protection for curb inlet protection and curb sediment containment will use 8 in (200mm) diameter Inlet protection, and drain inlets on soil will use 12 in (300mm) or 18 in (450mm) diameter Inlet protection. In severe flow situations, larger Inlet protection may be specified by the Engineer. During curb installation, Inlet protection shall be compacted to be slightly shorter than curb height.
6. If Inlet protection becomes clogged with debris and sediment, they shall be maintained so as to assure proper drainage and water flow into the storm drain. In severe storm events, overflow of the Inlet protection may be acceptable in order to keep the area from flooding.
7. Curb and drain Inlet protection shall be positioned so as to provide a permeable physical barrier to the drain itself, allowing sediment to collect on the outside of the Inlet protection.
8. For drains and inlets that have only curb cuts, without street grates, a spacer is required in order to keep the Inlet protection away from the drain opening. This spacer should be a hog wire screen bent to overlap the grate opening and keep the sock from falling into the opening. Use at least one spacer for every 4 ft (1.2m) of curb drain opening. The wire grid also prevents other floatable waste from passing over the Inlet protection.
9. Stakes shall be installed through the middle of the drain Inlet protection on 5 ft (1.5m) centers, using 2 in (50mm) x 2 in (50mm) x 3 ft (1m) wood stakes.
10. Staking depth for sand and silt loam soils shall be 12 in (300mm), and 8 in (200mm) for clay soils.

### INSPECTION AND MAINTENANCE

Routine inspection should be conducted within 24 hrs of a runoff event or as designated by the regulating authority. Inlet protection should be regularly inspected to make sure they maintain their

shape and are producing adequate hydraulic flow-through. If ponding becomes excessive, additional Inlet protection may be required or sediment removal may be necessary. Inlet protection shall be inspected until contributing drainage area has been permanently stabilized and construction activity has ceased

1. The Contractor shall maintain the Inlet protection in a functional condition at all times and it shall be routinely inspected.
2. If the Inlet protection has been damaged, it shall be repaired, or replaced if beyond repair.
3. The Contractor shall remove sediment at the base of the upslope side of the Inlet protection when accumulation has reached 1/2 of the effective height of the Inlet protection, or as directed by the Engineer. Alternatively, for drain Inlet protection a new Soxx™ may be placed on top of the original increasing the sediment storage capacity without soil disturbance.
4. Inlet protection shall be maintained until disturbed area above or around the device has been permanently stabilized and construction activity has ceased.
5. Regular maintenance includes lifting the Inlet protection and cleaning around and under them as sediment collects.
6. The FilterMedia™ will be removed from paved areas or dispersed on site soil or behind curb once disturbed area has been permanently stabilized, construction activity has ceased, or as determined by the Engineer.

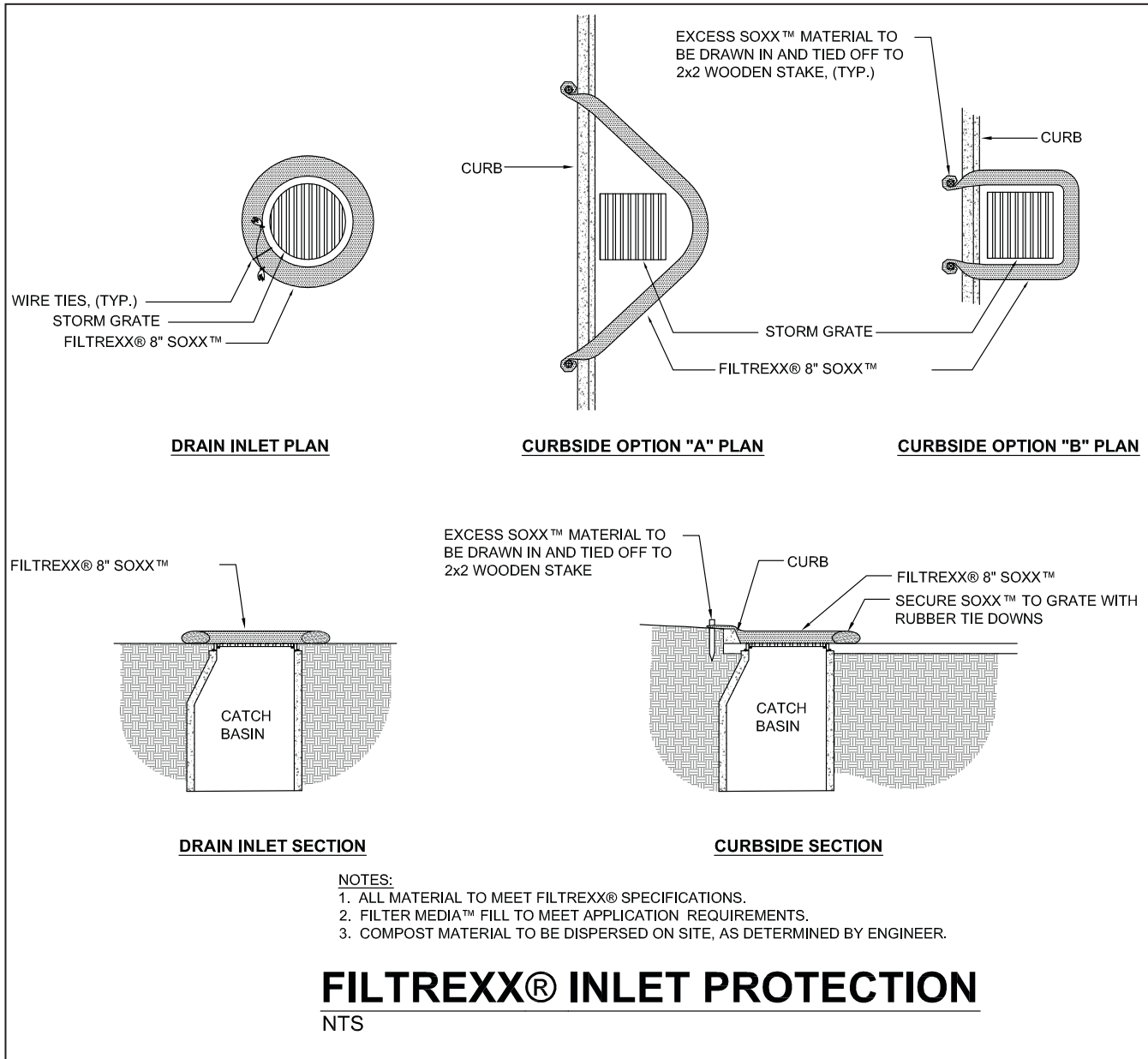
**Table 2.4** Spacing for Curb Sediment Containment Systems.

Grade (%)	Spacing (ft)	Spacing (mm)
0.5	100	30
1.0	50	15
2.0	25	8
3.0	16	5
4.0	13	4
5.0	10	3

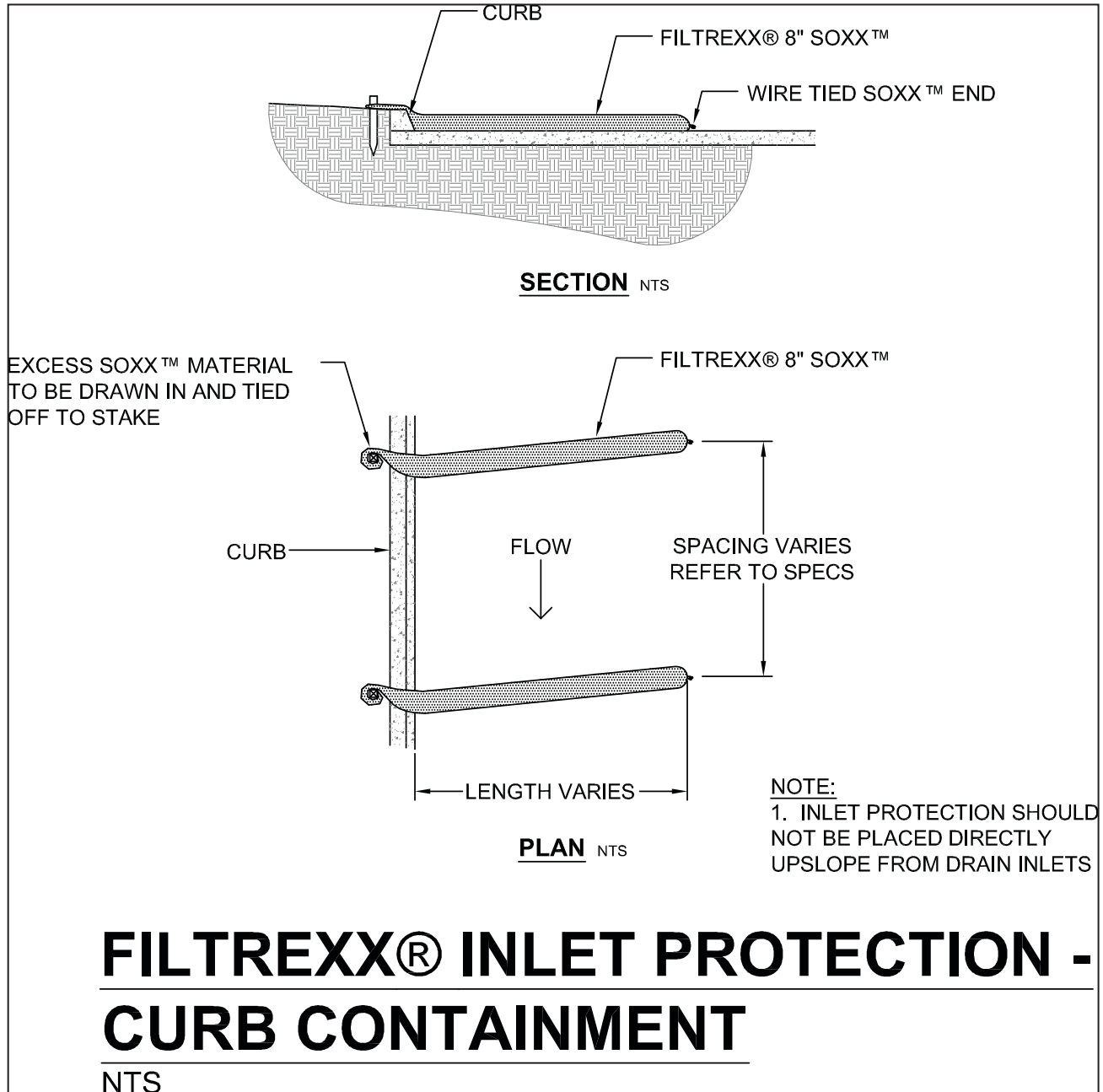
Source: Fifield, 2001.



Figure 2.1. Engineering Design Drawing for Curb and Drain Inlet Protection



**Figure 2.2.** Engineering Design Drawing for Curb Sediment Containment Inlet Protection





# SWPPP Cut Sheet:

## Filtrex<sup>®</sup> Sediment Control

### Sediment & Perimeter Control Technology

#### PURPOSE & DESCRIPTION

Filtrex<sup>®</sup> Sediment control is a three-dimensional tubular sediment control and storm water runoff filtration device typically used for **perimeter control** of sediment and other soluble pollutants (such as phosphorus and petroleum hydrocarbons), on and around construction activities.

#### APPLICATION

Filtrex<sup>®</sup> Sediment control is to be installed down slope of any disturbed area requiring erosion and sediment control and filtration of soluble pollutants from runoff. Sediment control is effective when installed perpendicular to sheet or low concentrated flow. Acceptable applications include:

- Site perimeters
- Above and below disturbed areas subject to sheet runoff, interrill and rill erosion
- Above and below exposed and erodable slopes
- Around area drains or inlets located in a 'sump'
- On compacted soils where trenching of silt fence is difficult or impossible
- Around sensitive trees where trenching of silt fence is not beneficial for tree survival or may unnecessarily disturb established vegetation.
- On frozen ground where trenching of silt fence is impossible.
- On paved surfaces where trenching of silt fence is impossible.

#### INSTALLATION

1. Sediment control used for perimeter control of sediment and soluble pollutants in storm runoff shall meet Filtrex<sup>®</sup> Soxx<sup>™</sup> Material Specifications and use Certified Filtrex<sup>®</sup> FilterMedia<sup>™</sup>.
2. Contractor is required to be Filtrex<sup>®</sup> Certified<sup>™</sup>, or use pre-filled Filtrex<sup>®</sup> Sediment control

products manufactured by a Filtrex<sup>®</sup> Certified Manufacturer<sup>™</sup> as determined by Filtrex<sup>®</sup> International, LLC (440-926-2607 or visit [www.filtrex.com](http://www.filtrex.com)). Certification shall be considered current if appropriate identification is shown during time of bid or at time of application. Look for the Filtrex<sup>®</sup> Certified<sup>™</sup> Seal.

3. Sediment control will be placed at locations indicated on plans as directed by the Engineer.
4. Sediment control should be installed parallel to the base of the slope or other disturbed area. In extreme conditions (i.e., 2:1 slopes), a second Sediment control shall be constructed at the top of the slope.
5. Effective Soxx<sup>™</sup> height in the field should be as follows: 8" Diameter Sediment control = 6.5" high, 12" Diameter Sediment control = 9.5" high, 18" Diameter SiltSoxx<sup>™</sup> = 14.5" high, 24" Diameter Sediment control = 19" high.
6. Stakes shall be installed through the middle of the Sediment control on 10 ft (3m) centers, using 2 in (50mm) by 2 in (50mm) by 3 ft (1m) hard wood stakes. In the event staking is not possible, i.e., when Sediment control is used on pavement, heavy concrete blocks shall be used behind the Sediment control to help stabilize during rainfall/runoff events.
7. Staking depth for sand and silt loam soils shall be 12 in (300mm), and 8 in (200mm) for clay soils.
8. Loose compost may be backfilled along the upslope side of the Sediment control, filling the seam between the soil surface and the device, improving filtration and sediment retention.
9. If the Sediment control is to be left as a permanent filter or part of the natural landscape, it may be seeded at time of installation for



establishment of permanent vegetation. The Engineer will specify seed requirements.

10. Filtrexx® Sediment control is not to be used in perennial, ephemeral, or intermittent streams.

See design drawing schematic for correct Filtrexx® Sediment control installation (Figure 1.1).

### INSPECTION AND MAINTENANCE

Routine inspection should be conducted within 24 hrs of a runoff event or as designated by the regulating authority. Sediment control should be regularly inspected to make sure they maintain their shape and are producing adequate hydraulic flow-through. If ponding becomes excessive, additional Sediment control may be required to reduce effective slope length or sediment removal may be necessary. Sediment control shall be inspected until area above has been permanently stabilized and construction activity has ceased

1. The Contractor shall maintain the Sediment control in a functional condition at all times and it shall be routinely inspected.
2. If the Sediment control has been damaged, it shall be repaired, or replaced if beyond repair.

3. The Contractor shall remove sediment at the base of the upslope side of the Sediment control when accumulation has reached 1/2 of the effective height of the Sediment control, or as directed by the Engineer. Alternatively, a new Sediment control can be placed on top of and slightly behind the original one creating more sediment storage capacity without soil disturbance.
4. Sediment control shall be maintained until disturbed area above the device has been permanently stabilized and construction activity has ceased.
5. The FilterMedia™ will be dispersed on site once disturbed area has been permanently stabilized, construction activity has ceased, or as determined by the Engineer.
6. For long-term sediment and pollution control applications, Sediment control can be seeded at the time of installation to create a vegetative filtering system for prolonged and increased filtration of sediment and soluble pollutants (contained vegetative filter strip). The appropriate seed mix shall be determined by the Engineer.

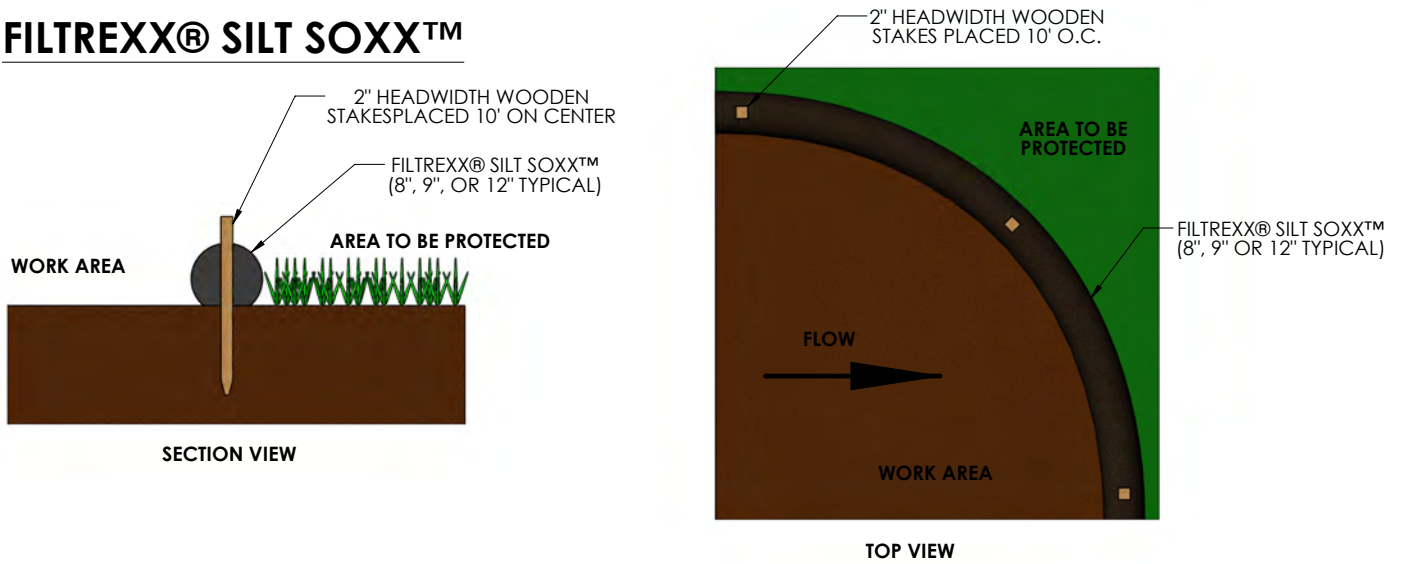
Slope Percent	Maximum Slope Length Above Sediment Control in Feet (meters)*				
	8 in (200 mm) Sediment control	12 in (300 mm) Sediment control	18 in (450 mm) Sediment control	24 in (600mm) Sediment control	32 in (800mm) Sediment control
	6.5 in (160 mm)**	9.5 in (240 mm) **	14.5 in (360 mm) **	19 in (480 mm) **	26 in (650 mm) **
2 (or less)	600 (180)	750 (225)	1000 (300)	1300 (400)	1650 (500)
5	400 (120)	500 (150)	550 (165)	650 (200)	750 (225)
10	200 (60)	250 (75)	300 (90)	400 (120)	500 (150)
15	140 (40)	170 (50)	200 (60)	325 (100)	450 (140)
20	100 (30)	125 (38)	140 (42)	260 (80)	400 (120)
25	80 (24)	100 (30)	110 (33)	200 (60)	275 (85)
30	60 (18)	75 (23)	90 (27)	130 (40)	200 (60)
35	60 (18)	75 (23)	80 (24)	115 (35)	150 (45)
40	60 (18)	75 (23)	80 (24)	100 (30)	125 (38)
45	40 (12)	50 (15)	60 (18)	80 (24)	100 (30)
50	40 (12)	50 (15)	55 (17)	65 (20)	75 (23)

\* Based on a failure point of 36 in (0.9 m) super silt fence (wire reinforced) at 1000 ft (303 m) of slope, watershed width equivalent to receiving length of sediment control device, 1 in/ 24 hr (25 mm/24 hr) rain event.

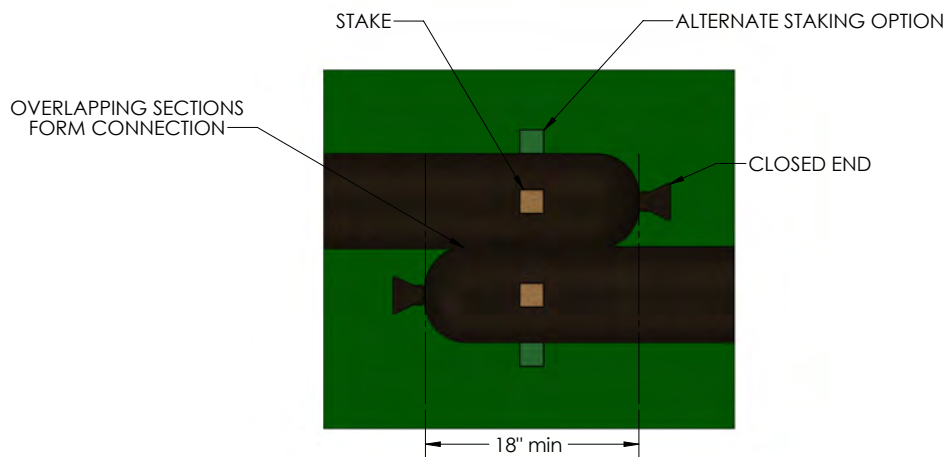
\*\* Effective height of Sediment control after installation and with constant head from runoff as determined by Ohio State University.



## FILTREXX® SILT SOXX™



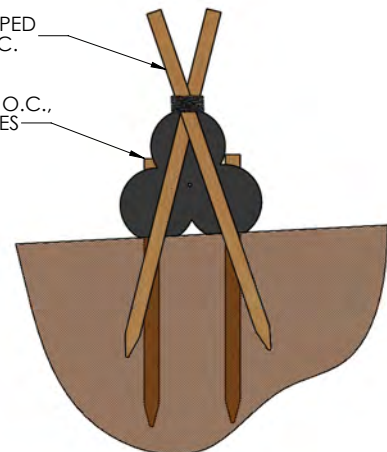
## COMPOST SOCK CONNECTION/ATTACHMENT DETAIL



## FILTREXX® PYRAMID STAKING DETAIL

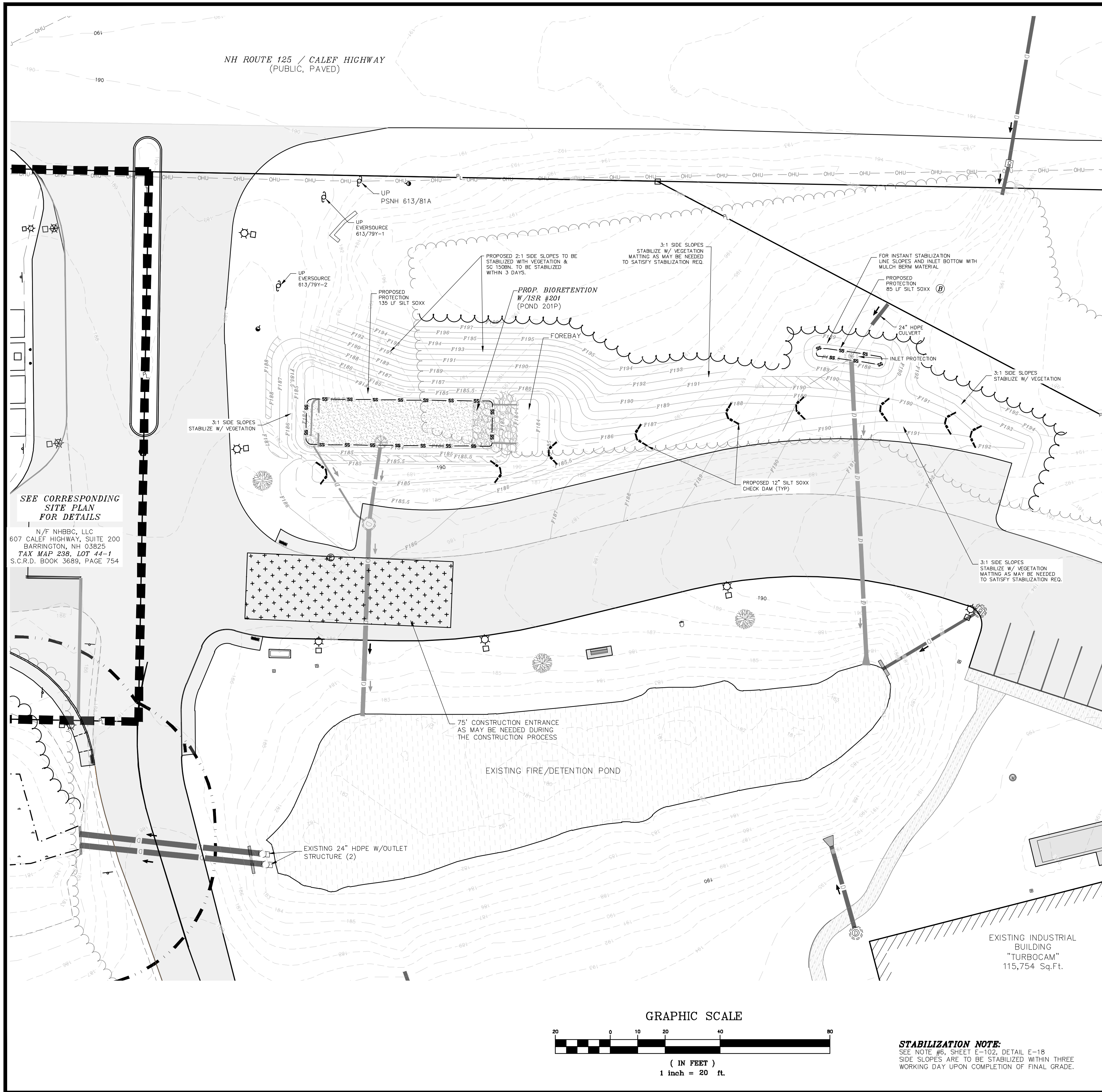
(2) 2"x2"x48+" HARDWOOD STAKES, WRAPPED TOGETHER WITH 16 GAUGE WIRE, 10' O.C.

2"x2"x36" HARDWOOD STAKE, 10' O.C., STARTING 5' FROM ANGLED STAKES



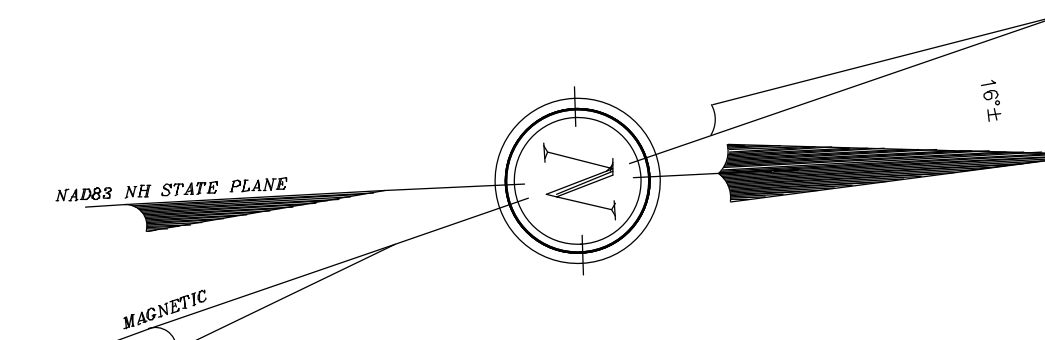
- NOTES:**
1. ALL MATERIAL TO MEET FILTREXX® SPECIFICATIONS.
  2. SILT SOXX™ FILL TO MEET APPLICATION REQUIREMENTS.
  3. COMPOST MATERIAL TO BE DISPERSED ON SITE, AS DETERMINED BY ENGINEER.





**NOTES:**

- 1.) OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- 1A.) APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- 2.) TAX MAP 238, LOT 44
- 3.) LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
- 4.) S.C.R.D. BOOK 2948, PAGE 332
- 5.) EROSION AND SEDIMENT CONTROL INSPECTIONS TO BE CONDUCTED ONCE PER EVERY SEVEN DAYS AND AT AN INCREASED FREQUENCY INCLUDING WITHIN 24-HOURS OF A 0.25 INCH RAIN EVENT. INSPECTIONS TO BE CONDUCTED BY A "QUALIFIED PERSON" AS DEFINED BY EPA CGP 4.1.1 AND INSPECTION REPORTS SUBMITTED TO THE TOWN OF BARRINGTON, LAND USE DEPARTMENT WITHIN 24 HOURS IN ACCORDANCE WITH CGP 4.1.7 AND MAINTAINED BY THE OWNER FOR A PERIOD OF THREE YEARS AFTER THE PROJECT IS COMPLETED.
- 6.) PER EPA CGP Z.1.2.2 (INSTALL PERIMETER CONTROL), "YOU MUST INSTALL SEDIMENT CONTROLS ALONG THOSE PERIMETER AREAS OF YOUR SITE THAT WILL RECEIVE STORMWATER FROM EARTH DISTURBING ACTIVITIES." AS A RESULT OF SWPPP INSPECTIONS, THE CONTRACTOR MAY HAVE TO EXPAND PERIMETER CONTROLS TO MEET THIS REQUIREMENT. THE E&SC PLAN IS INITIAL GUIDANCE AS TO THE ANTICIPATED REQUIREMENTS AND IT IS THE CONTRACTORS RESPONSIBILITY TO ENSURE THAT STORMWATER VIOLATIONS DO NOT OCCUR.
- 7.) CONTRACTOR IS REQUIRED TO HAVE A CONSTRUCTION ENTRANCE 3" ANGULAR STONE IS REQUIRED.
- 8.) CONTRACTOR IS RESPONSIBLE FOR SWEEPING THE DRIVEWAY TO ENSURE THAT NO SEDIMENT IS BEING TRACKED ONTO CALEF HIGHWAY OR THE REMAINDER OF THE EXISTING TURBOCAM SITE.
- 9.) CONTRACTOR IS RESPONSIBLE FOR CLEANING AND MAINTAINING THE INLET PROTECTION ONCE INSTALLED.
- 10.) FUGITIVE DUST IS TO BE CONTROLLED THROUGHOUT THE CONSTRUCTION PROCESS IN ACCORDANCE WITH ENV-A 1000.
- 11.) CONTRACTOR IS TO MEET THE REQUIREMENTS SPECIFIED IN RSA 430:51-57 AND AGR 3800, RELATING TO INVASIVE SPECIES.
- 12.) CONTRACTOR IS RESPONSIBLE FOR PROTECTING THE WATER QUALITY FROM ANY RUNOFF DURING THE CONSTRUCTION PROCESS, IN ACCORDANCE WITH ENV-WQ 1507, IN ORDER TO PREVENT VIOLATIONS OF THE STORMWATER QUALITY STANDARDS.
- 13.) CONTRACTOR CAN USE SILT FENCE, SILT SOXX OR MULCH BERM FOR PERIMETER CONTROL. SPECIFIC PRACTICES MAY BE SPECIFIED, SEE PLAN. SILT FENCE OR SILT SOXX REQUIRED WHEN UPGRADIENT DISTURBED SOIL IS GREATER THAN 5%. MULCH BERM CAN BE USED WHEN THE UPGRADIENT DISTURBED SOIL IS 5% OR LESS.



**SOILS & DEWATERING:**

313	DEERFIELD	LOAMY SAND	K= 0.15
12	HINCKLEY	LOAMY SAND	K= 0.17
26	WINDSOR	LOAMY SAND	K= 0.17

SEE SITE SPECIFIC SOILS MAP (SSSM)  
SEE WEBSOIL USDA-NRCS  
ERODIBILITY FACTOR - K, CPESC MANUAL, ENVIROCERT INTERNATIONAL INC. & ROCKINGHAM COUNTY SOIL SURVEY, ROCKWEB SOIL ATTRIBUTES.

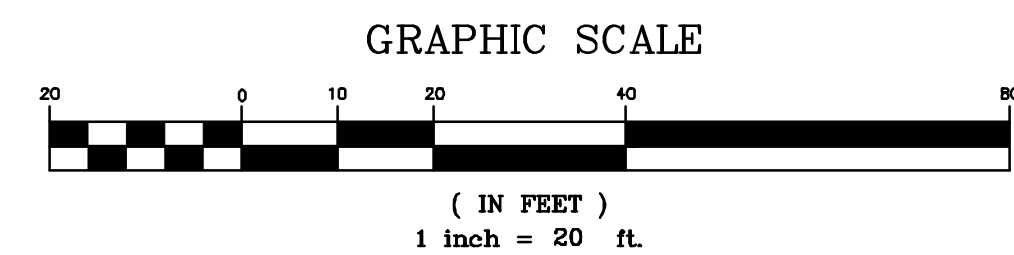
CONTRACTOR TO BE AWARE OF THE SOIL PROFILES AND ENSURE THAT PROPER EROSION PREVENTION AND SEDIMENT CONTROL MEASURES ARE TAKEN AT ALL TIMES. ANY DEWATERING REQUIREMENTS IN NEW HAMPSHIRE REQUIRE SPECIAL PROVISIONS IN ACCORDANCE WITH THE "CLARIFICATION OF SECTION 9.1.2 (STATE OF NEW HAMPSHIRE CONDITIONS) AND OTHER NH SPECIFIC INFORMATION FOR THE U.S. EPA 2022 NPDES CONSTRUCTION GENERAL PERMIT (CGP)" DATED FEBRUARY 17, 2022 INCLUDED IN THE SWPPP.

COVER MANAGEMENT DURING CONSTRUCTION FOR EXPOSED SOIL WILL INCLUDE HAY / STRAW APPLIED AT A RATE OF 2.0 TONS PER ACRE, TEMPORARY SEEDING OF ANNUAL RYE GRASS, AND PERMANENT SEEDING AT THE EARLIEST OPPORTUNITY. SEE ADDITIONAL REQUIREMENT FOR STABILIZATION ON THE EROSION AND SEDIMENT CONTROL DETAIL SHEETS, E-101 AND E-102.

THE CONSTRUCTION SCHEDULE WILL BE MANAGED SO THAT ALL STORMWATER STRUCTURES WILL BE BUILT AND STABILIZED PRIOR TO RECEIVING SURFACE WATER RUNOFF. CONTRACTOR TO BE RESPONSIBLE FOR ALL DIVERSIONS DURING CONSTRUCTION AND FOR INTERIM SEDIMENT AND EROSION CONTROL MEASURES.

**LEGEND:**

- GRANITE/CONCRETE BOUND ~FND~
- PROPOSED CATCH BASIN W/ STRUCTURE
- PROPOSED STORMWATER BMP OUTLET STRUCTURE
- TEMPORARY BENCHMARK (T.B.M.)
- DETAIL SHEET / DETAIL
- POORLY DRAINED JURISDICTIONAL WETLAND
- 50' WETLAND BUFFER
- CONTOUR MINOR, EXISTING
- CONTOUR MAJOR, EXISTING
- CONTOUR MINOR, PROPOSED
- CONTOUR MAJOR, PROPOSED
- CENTER LINE
- EXISTING DRAINAGE LINE
- PROPOSED DRAINAGE LINE
- RIP RAP
- STORMWATER BEST MANAGEMENT PRACTICE (BMP)
- BERM
- SILT FENCE / EROSION MIX BERM
- FILTREX 8" - 12" SILT SOXX AS SPECIFIED
- ORANGE CONSTRUCTION PERIMETER FENCE
- PERIMETER CONTROL
- STORMWATER BMP PROTECTION
- PERIMETER CONTROL
- RESIDENTIAL/ROADWAY CONSTRUCTION



**STABILIZATION NOTE:**  
SEE NOTE #6, SHEET E-102, DETAIL E-18  
SIDE SLOPES ARE TO BE STABILIZED WITHIN THREE WORKING DAY UPON COMPLETION OF FINAL GRADE.

SEE CORRESPONDING SITE PLAN FOR DETAILS

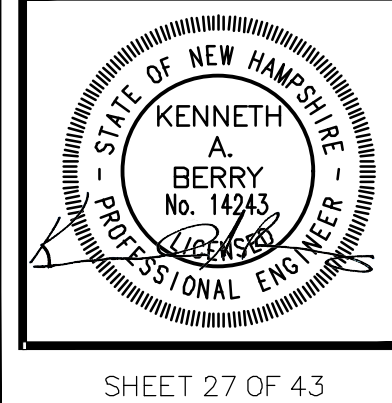
N/F NHBBC, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825  
TAX MAP 238, LOT 44-1  
S.C.R.D. BOOK 3689, PAGE 754

EROSION AND SEDIMENT CONTROL PLAN

REVISION	DATE	DESCRIPTION

FOR TURBOCAM, INC.  
LAND OF VIRTUOUS REALTY, LLC  
NH ROUTE 125/CALEF HIGHWAY  
BARRINGTON, N.H.  
TAX MAP 238, LOT 44

BERRY SURVEYING & ENGINEERING  
335 SECOND CROWN POINT ROAD  
BARRINGTON, NH 03825 (603)332-2863  
SCALE : 1 IN. EQUALS 20 FT.  
DATE : APRIL 17, 2024  
FILE NO. : DB 2023 - 017

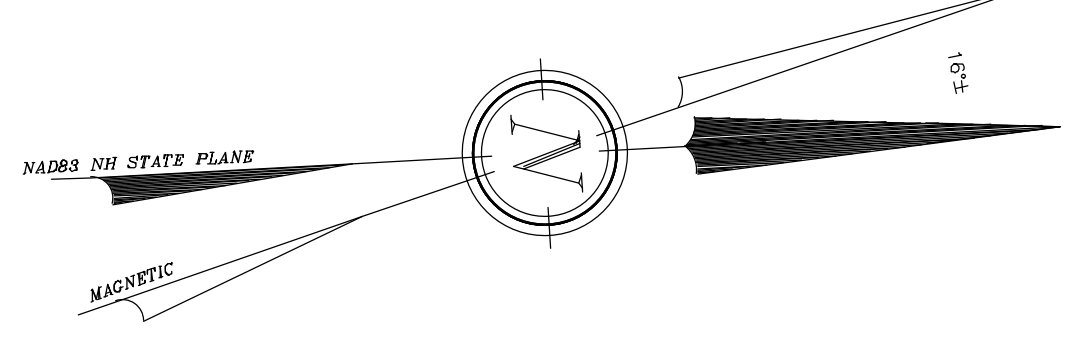


FOR TOWN APPROVAL PURPOSES :  
  
THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.



**NOTES:**

- 1.) OWNER: VIRTUOUS REALTY, LLC  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- 1A.) APPLICANT: TURBOCAM, INC.  
607 CALEF HIGHWAY, SUITE 200  
BARRINGTON, NH 03825
- 2.) TAX MAP 238, LOT 44
- 3.) LOT AREA: 1,217,250 Sq.Ft., 27.94 Ac.
- 4.) S.C.R.D. BOOK 2948, PAGE 332
- 5.) EROSION AND SEDIMENT CONTROL INSPECTIONS TO BE CONDUCTED ONCE PER EVERY SEVEN DAYS AND AT AN INCREASED FREQUENCY INCLUDING WITHIN 24-HOURS OF A 0.25 INCH RAIN EVENT. INSPECTIONS TO BE CONDUCTED BY A "QUALIFIED PERSON" AS DEFINED BY EPA CGP 4.1.1 AND INSPECTION REPORTS SUBMITTED TO THE TOWN OF BARRINGTON, LAND USE DEPARTMENT WITHIN 24 HOURS IN ACCORDANCE WITH CGP 4.1.7 AND MAINTAINED BY THE OWNER FOR A PERIOD OF THREE YEARS AFTER THE PROJECT IS COMPLETED.
- 6.) PER EPA CGP 2.1.2.2 (INSTALL PERIMETER CONTROL), "YOU MUST INSTALL SEDIMENT CONTROLS ALONG THOSE PERIMETER AREAS OF YOUR SITE THAT WILL RECEIVE STORMWATER FROM EARTH DISTURBING ACTIVITIES." AS A RESULT OF SWPPP INSPECTIONS, THE CONTRACTOR MAY HAVE TO EXPAND PERIMETER CONTROLS TO MEET THIS REQUIREMENT. THE E&S PLAN IS INITIAL GUIDANCE AS TO THE ANTICIPATED REQUIREMENTS AND IT IS THE CONTRACTORS RESPONSIBILITY TO ENSURE THAT STORMWATER VIOLATIONS DO NOT OCCUR.
- 7.) CONTRACTOR IS REQUIRED TO HAVE A CONSTRUCTION ENTRANCE 3" ANGULAR STONE IS REQUIRED.
- 8.) CONTRACTOR IS RESPONSIBLE FOR SWEEPING THE DRIVEWAY TO ENSURE THAT NO SEDIMENT IS BEING TRACKED ONTO CALEF HIGHWAY OR THE REMAINDER OF THE EXISTING TURBOCAM SITE.
- 9.) CONTRACTOR IS RESPONSIBLE FOR CLEANING AND MAINTAINING THE INLET PROTECTION ONCE INSTALLED.
- 10.) FUGITIVE DUST IS TO BE CONTROLLED THROUGHOUT THE CONSTRUCTION PROCESS IN ACCORDANCE WITH ENV-A 1000.
- 11.) CONTRACTOR IS TO MEET THE REQUIREMENTS SPECIFIED IN RSA 430:51-57 AND AGR 3800, RELATING TO INVASIVE SPECIES.
- 12.) CONTRACTOR IS RESPONSIBLE FOR PROTECTING THE WATER QUALITY FROM ANY RUNOFF DURING THE CONSTRUCTION PROCESS, IN ACCORDANCE WITH ENV-WQ 1507, IN ORDER TO PREVENT VIOLATIONS OF THE STORMWATER QUALITY STANDARDS.
- 13.) CONTRACTOR CAN USE SILT FENCE, SILT SOXX OR MULCH BERM FOR PERIMETER CONTROL. SPECIFIC PRACTICES MAY BE SPECIFIED, SEE PLAN. SILT FENCE OR SILT SOXX REQUIRED WHEN UPGRADING DISTURBED SOIL IS GREATER THAN 5%. MULCH BERM CAN BE USED WHEN THE UPGRADING DISTURBED SOIL IS 5% OR LESS.



**SOILS & DEWATERING:**

313	DEERFIELD	LOAMY SAND	K= 0.15
12	HINCKLEY	LOAMY SAND	K= 0.17
26	WINDSOR	LOAMY SAND	K= 0.17

SEE SITE SPECIFIC SOILS MAP (SSSM)  
SEE WEBSOIL USDA-NRCS  
ERODIBILITY FACTOR - K, CPESC MANUAL, ENVIROCERT INTERNATIONAL INC. & ROCKINGHAM COUNTY SOIL SURVEY, ROCKWEB SOIL ATTRIBUTES.

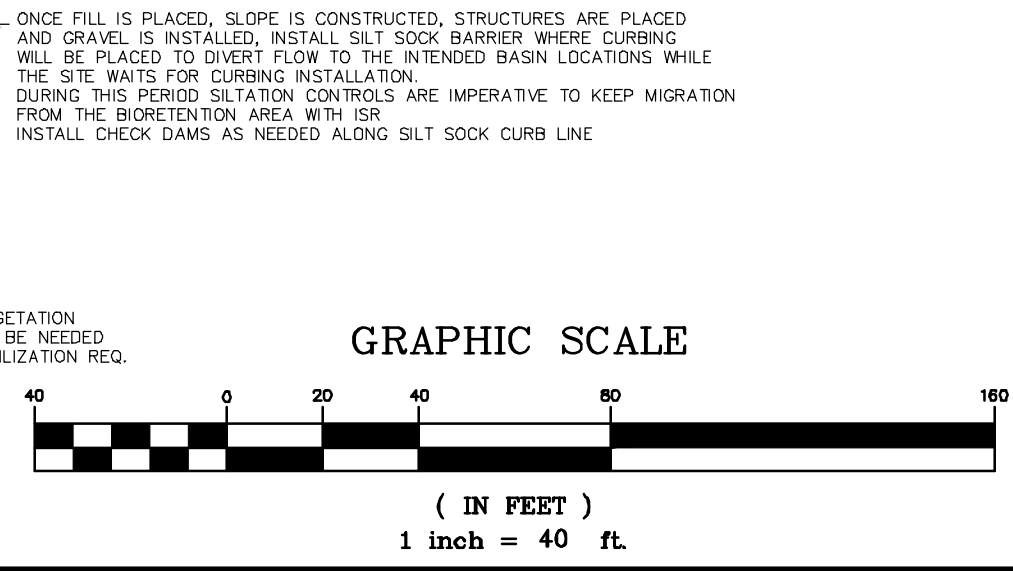
CONTRACTOR TO BE AWARE OF THE SOIL PROFILES AND ENSURE THAT PROPER EROSION PREVENTION AND SEDIMENT CONTROL MEASURES ARE TAKEN AT ALL TIMES. ANY DEWATERING REQUIREMENTS IN NEW HAMPSHIRE REQUIRE SPECIAL PROVISIONS IN ACCORDANCE WITH THE "CLARIFICATION OF SECTION 9.1.2 (STATE OF NEW HAMPSHIRE CONDITIONS) AND OTHER NH SPECIFIC INFORMATION FOR THE U.S. EPA 2022 NPDES CONSTRUCTION GENERAL PERMIT (CGP)" DATED FEBRUARY 17, 2022 INCLUDED IN THE SWPPP.

COVER MANAGEMENT DURING CONSTRUCTION FOR EXPOSED SOIL WILL INCLUDE HAY / STRAW APPLIED AT A RATE OF 2.0 TONS PER ACRE, TEMPORARY SEEDING OF ANNUAL RYE GRASS, AND PERMANENT SEEDING AT THE EARLIEST OPPORTUNITY. SEE ADDITIONAL REQUIREMENT FOR STABILIZATION ON THE EROSION AND SEDIMENT CONTROL DETAIL SHEETS, E-101 AND E-102.

THE CONSTRUCTION SCHEDULE WILL BE MANAGED SO THAT ALL STORMWATER STRUCTURES WILL BE BUILT AND STABILIZED PRIOR TO RECEIVING SURFACE WATER RUNOFF. CONTRACTOR TO BE RESPONSIBLE FOR ALL DIVERSIONS DURING CONSTRUCTION AND FOR INTERIM SEDIMENT AND EROSION CONTROL MEASURES.

**LEGEND:**

- ☐ GRANITE/CONCRETE BOUND ~FND~
- ☉ PROPOSED CATCH BASIN W/ STRUCTURE
- ☉ PROPOSED STORMWATER BMP OUTLET STRUCTURE
- ☉ TEMPORARY BENCHMARK (T.B.M.)
- ☉ DETAIL SHEET / DETAIL
- ☉ POORLY DRAINED JURISDICTIONAL WETLAND
- ☉ 50' WETLAND BUFFER
- ☉ CONTOUR MINOR, EXISTING
- ☉ CONTOUR MAJOR, EXISTING
- ☉ CONTOUR MINOR, PROPOSED
- ☉ CONTOUR MAJOR, PROPOSED
- ☉ CENTER LINE
- ☉ EXISTING DRAINAGE LINE
- ☉ PROPOSED DRAINAGE LINE
- ☉ RIP RAP
- ☉ STORMWATER BEST MANAGEMENT PRACTICE (BMP)
- ☉ BERM
- ☉ SILT FENCE / EROSION MIX BERM
- ☉ FILTERREX 8" - 12" SILT SOXX AS SPECIFIED
- ☉ ORANGE CONSTRUCTION PERIMETER FENCE
- ☉ PERIMETER CONTROL
- ☉ STORMWATER BMP PROTECTION
- ☉ PERIMETER CONTROL
- ☉ RESIDENTIAL/ROADWAY CONSTRUCTION

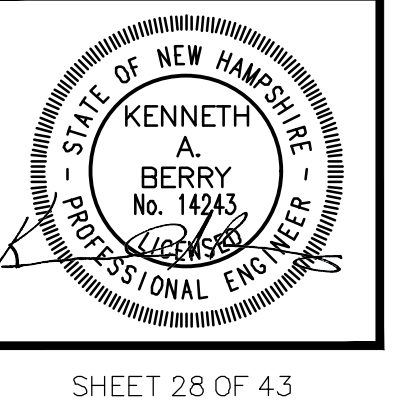


**STABILIZATION NOTE:**  
SEE NOTE #3, SHEET E-102, DETAIL E-18  
SIDE SLOPES ARE TO BE STABILIZED WITHIN THREE WORKING DAY UPON COMPLETION OF FINAL GRADE.

REVISION	DATE	DESCRIPTION

EROSION AND SEDIMENT CONTROL PLAN  
 FOR  
 TURBOCAM, INC.  
 LAND OF  
 VIRTUOUS REALTY, LLC  
 NH ROUTE 125/CALEF HIGHWAY  
 BARRINGTON, N.H.  
 TAX MAP 238, LOT 44

BERRY SURVEYING & ENGINEERING  
 335 SECOND CROWN POINT ROAD  
 BARRINGTON, NH 03825 (603)332-2863  
 SCALE : 1 IN. EQUALS 40 FT.  
 DATE : APRIL 17, 2024  
 FILE NO. : DB 2023 - 017



FOR TOWN APPROVAL PURPOSES :  
 THE SITE REVIEW REGULATIONS OF THE TOWN OF BARRINGTON ARE A PART OF THIS PLAN, AND APPROVAL OF THIS PLAN IS CONTINGENT UPON COMPLETION OF ALL REQUIREMENTS OF SAID SITE REVIEW REGULATIONS, EXCEPTING ONLY MODIFICATIONS MADE IN WRITING BY THE BOARD AND ATTACHED HERETO.