DRAINAGE ANALYSIS & **EROSION AND SEDIMENT CONTROL PLAN**

Prepared for:

TSB Construction, LLC 44 Merryfield Lane EAST HAMPSTEAD, NH 03826

Land of

TSB Construction, LLC 44 Merryfield Lane EAST HAMPSTEAD, NH 03826

NH ROUTE 9, BARRINGTON, NH TAX MAP 234, LOT 25-1

Prepared by:

Berry Surveying & Engineering 335 Second Crown Point Road Barrington, NH 03825



Project Number: DB 2022-051

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USGS Quadrangle Location Map

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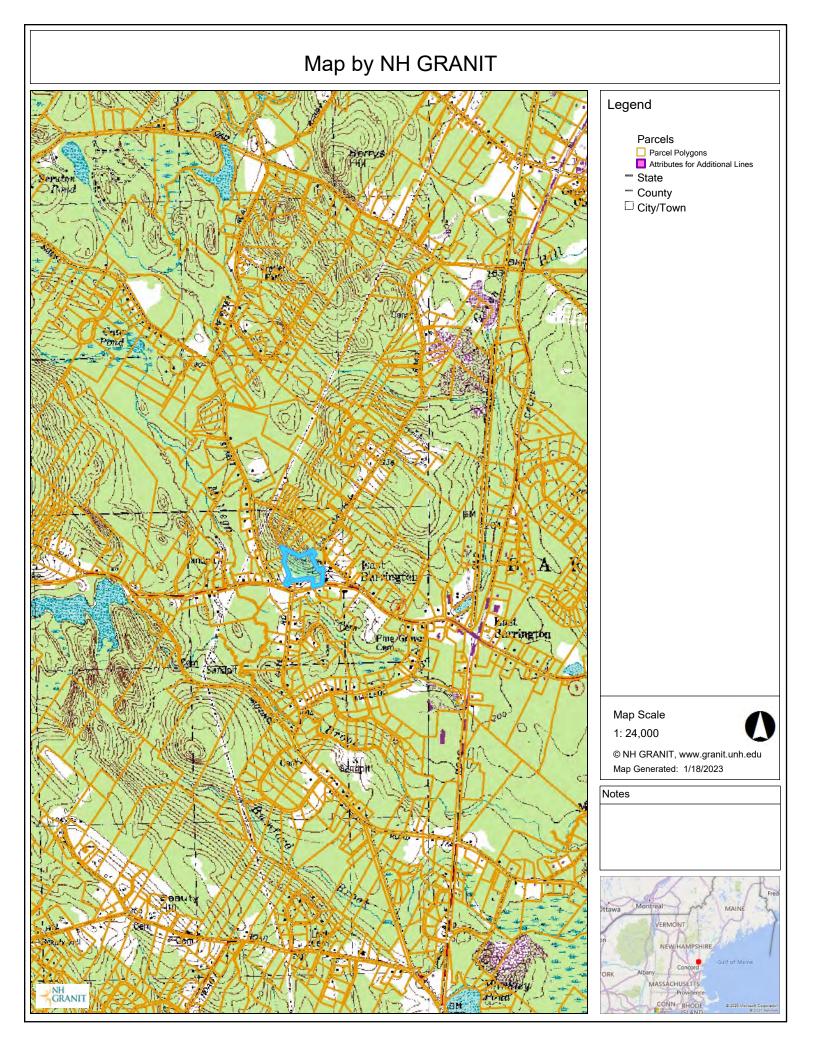
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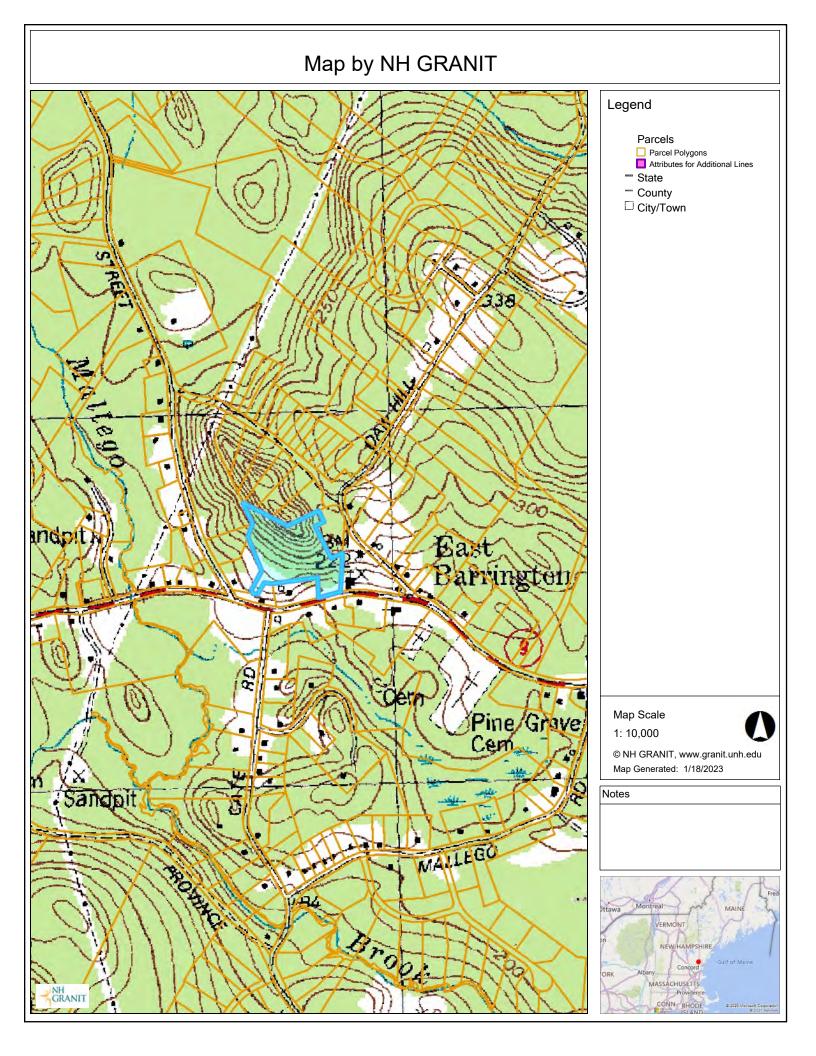
Appendix III - Calculations, Charts, & Graphs Extreme Precipitation Tables NCRS USDA Web-soil Map Site Specific Soils Survey Report & Plan Pond 202 Treatment Sheets Stormwater System Management: Inspection & Maintenance Manual & Plan Infiltration Feasibility Study & Report Filtrexx Construction Specifications

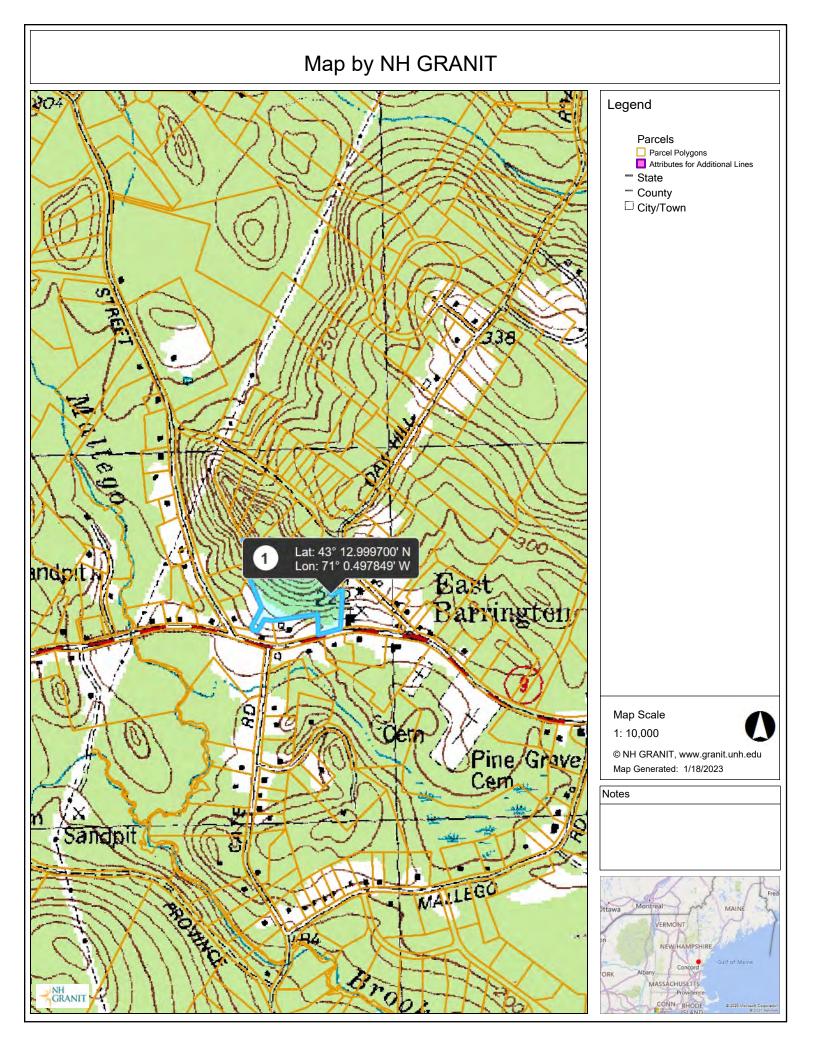
Enclosed:	W-1 Sheet	Existing Conditions Watershed Plan	Sheet 1
	W-2 Sheet	Post Construction Watershed Plan	Sheet 2
		Erosion and Sediment Control Plan	

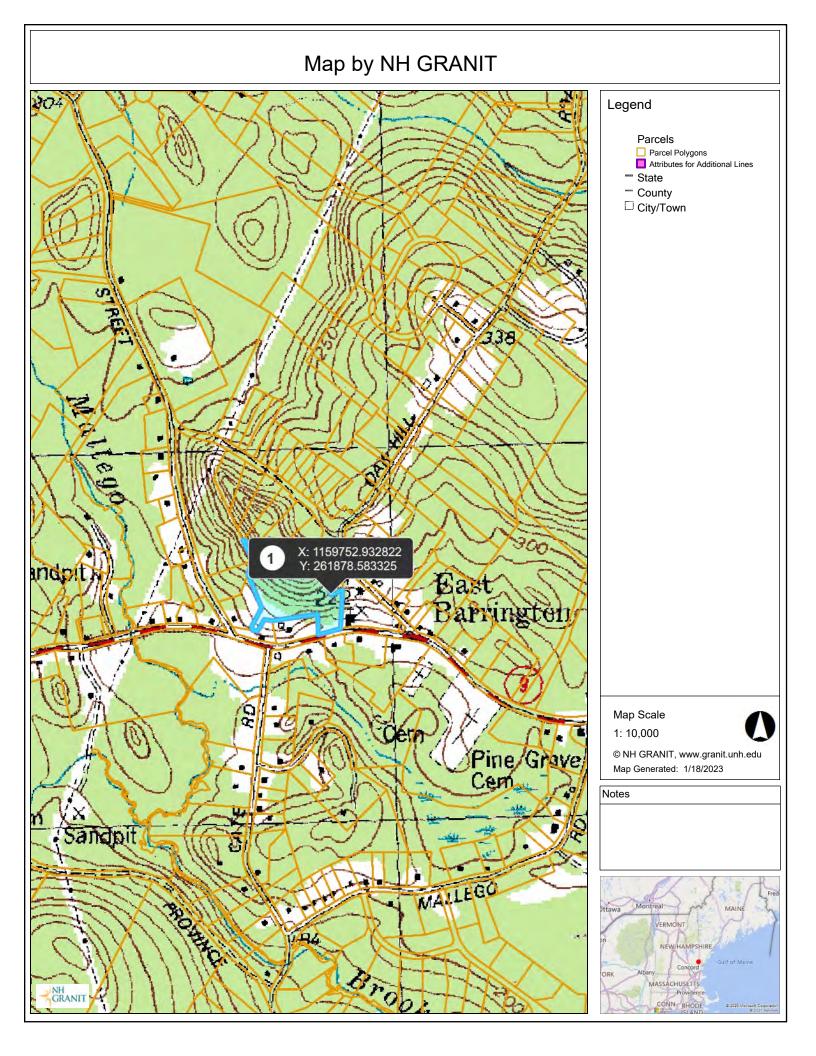












DESIGN METHOD OBJECTIVES

TSB Construction, LLC is proposing to develop two single family houses and a duplex residential building which will share a common driveway. The property is currently vacant land.

An Existing and Proposed Conditions analysis was conducted for the purpose of estimating the peak rate of stormwater run-off from the property and subsequently designing adequate mitigation of drainage. There are two drainage discharge points which were identified in the existing analysis and duplicated in the proposed analysis. Designing two watershed models we have compared the differences in these rates of peak run-off and surface water volume. Sheet W1, Existing Conditions Watershed Plan, outlines the characteristics of the site in its existing or pre-construction conditions. The second analysis displays the proposed (post-construction) conditions (See Sheet W2.) HydroCAD uses a series of node suffixes for numbering purposes (S = Subcatchment, P = Pond Device, R = Reach, to simplify annotation these suffixes are left off the watershed plans and node type is denoted by the symbol shape according to the displayed legend which coincides with HydroCAD graphics. The analysis was conducted using data for; 2 Yr - 24 Hr (3.08"), 10 Yr - 24 Hr (4.63"), 25 Yr - 24 Hr (5.85"), 50 Yr -24 Hr (6.99"), and 100 Yr - 24 Hr (8.36") storm events. Storm event analysis was accomplished using the USDA SCS TR-20 method within the HydroCAD Stormwater Modeling System environment and rainfall quantities are based on the Extreme Precipitation Table for this location from the Northeast Regional Climate Center / Cornell University (http://precip.eas.cornell.edu).

1.0 Existing Conditions Analysis:

Reference: Sheet W1 - Existing Conditions Watershed Plan (Enclosed) Existing Conditions Plan Site Specific Soils Map Plan

The existing property is currently vacant land. There are three subcatchments each contributing runoff to two final reaches. The combined area of the property is 9.44 acres and the area of the project analysis is 21.45 acres. As a result of the subdivisions on either side of Meetinghouse Road surface water runoff was diverted by swales, culverts, and rain gardens. While the analyses of Meetinghouse Road encompass a larger area only Subcatchments #1, #2, & #3 contribute runoff to Final Reaches #100 & #200. For this reason only these subcatchments were analyzed in this project. As such the resulting construction surrounding Meetinghouse Road results in a difference of 2.47 acres in contributing land area of the analyzed subcatchments between the preconstruction and post-construction analyses. There is a small wetland identified on the site. On-site soils were mapped by John P. Hayes III, CSS #87 and shown on the Site Specific Soils Map and included report.

The site is made up of a number of soils including soils from Hydrologic Soil Groups (HSG) A, B, & C. Off-site soils consist of Windsor (WdA/WdB), which is classified as Hydrologic Soil Group A, as well as Hollis-Charlton complex (HdC) which is classified as a combination of HSG A & D based on the USDA, NRCS Websoil.

Final Reach #100:

Subcatchment #1 consists of the majority of the parcel, extending from NH Route 9 north to a point along the northern boundary line. Runoff flows south to a cross culvert under NH Route 9 analyzed as **Pond #1** outletting to **Final Reach #100**.

Subcatchment #2 consists of mostly offsite land to the north and the west of the parcel. Runoff generally flows in a southerly direction to the edge of NH Route 9 where it flows in the roadside drainage measures to the cross culvert (Pond #1) and subsequently **Final Reach #100**.

Subcatchment #3 consists of offsite land northwest of the parcel. Runoff generally flows in a southerly direction along Oak Hill Road where it flows in the roadside drainage measures to the dividing line of **Subcatchment #2** and subsequently **Final Reach #100** through a pair of overland reaches (**Reaches #2 & #3**).

2.0 **Proposed Conditions Analysis:**

Reference: Sheet W2 - Proposed Conditions Watershed Plan (Enclosed) Proposed Site Plan Proposed Site Detail Plans Proposed Grading & Drainage Plans

The proposed conditions analysis consists of eight subcatchments. Final analysis points from the existing analysis are preserved.

Final Reach #100:

Subcatchments #2 & #3 remain unchanged by this development, however they are changed by the residential development on Meetinghouse Road and analyzed as such in this analysis.

Subcatchment #1 is greatly reduced in size due to the development of the site as a whole. The majority of Subcatchment one is now the offsite area along NH Route 9 **Final Reach #100**.

Subcatchment #21 is made up of the front portion of the parcel including area along NH Route 9 and the proposed entrance of Eagle Drive extending uphill close to

the eastern property line. Runoff is collected in an infiltration rain garden (**Pond #201**) and directed to **Final Reach #100** via a level spreader and overland reach (**Reach #201**).

Subcatchment #22 is made up of a portion of land at the end of Eagle Drive along the southern property line. Runoff is collected in an infiltration rain garden (**Pond #202**) and completely infiltrated, with no runoff leaving the constructed spillway at any analyzed storm event.

Subcatchment #23 is largely made up of undeveloped, wooded land aside from the grading associated with Eagle Drive and the second residential building. Runoff is directed to a small roadside detention pond and into a cross culvert under the driveway (Pond #23), which is directed to Pond #202 and subsequently Final Reach #100.

Subcatchment #24 is made up of undeveloped land as well as the middle portion of Eagle Drive and the associated grading. Runoff flows east and south and is collected in a small roadside detention pond near the corner of Eagle Drive and into a cross culvert under the roadway (**Pond #24**), which is directed to **Pond #201** (modeled within Pond #1 as an expansion of storage) and subsequently **Final Reach #100**.

Subcatchment #25 is a small portion of land uphill from Subcatchment #21 defined by the limits of a small detention pond designed to hold runoff from the small portion of Eagle Drive as well as runoff leaving **Pond #24** before directing runoff into **Pond #201** and subsequently **Final Reach #100**.

<u>Summary</u>

Runoff is partially infiltrated after treatment through the filter course of the rain garden (Pond #202). The peak rate of runoff from the proposal is reduced in the five storm events, at Final Reach #100, analyzed comparing pre-conditions to post-conditions.

3.0a Stormwater Treatment:

A portion of the surface water runoff from the development area is being treated by a rain garden w/ infiltration (Pond #202). Water Quality Volume calculations based on the Alteration of Terrain model along with Pond Storage Tables are included in Appendix 3 for the modeled practice.

3.0b Stormwater Infiltration:

Groundwater recharge volume requirements are satisfied by Rain Garden W/ Infiltration #202 (Pond #202) (Sheet P-202). See Infiltration Feasibility Study also prepared by Berry Surveying & Engineering and published on the same day.

3.1 FULL COMPARATIVE ANALYSIS

ANALYSIS COMPONENT: PEAK RATE DISCHARGE (Cubic Feet / Second)

		2 Yr	10 Yr	25 Yr	50 Yr	100 Yr
	Eviating	4.(0	10 ()	10.00	10.44	20.72
Final Reach #100	Existing	4.60	10.62	12.32	19.44	28.73
	Proposed	3.91	9.18	11.11	11.98	17.07
Final Reach #200	Existing	0.00	1.32	10.40	15.54	20.26
	Proposed	0.00	0.00	3.87	9.92	14.15

<u>ANALYSIS</u>

COMPONENT: VOLUME (Acre Feet)

		2 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Final Deach #100	Existing	0.866	2.221	3.144	4.125	5.455
Final Reach #100	Proposed	0.801	1.979	2.915	3.688	4.734
Final Deach #200	Existing	0.000	0.030	0.460	0.897	1.398
Final Reach #200	Proposed	0.000	0.000	0.149	0.512	0.925

4.0 EROSION & SEDIMENT CONTROL PLANS BEST MANAGEMENT PRACTICES (BMP's)

Reference: Proposed Site Plan and Grading Plan Erosion & Sediment Control Plan Erosion & Sediment Control Details

The proposed site development is protected from erosion and the abutting properties are protected from sediment by the use of Best Management Practices as outlined in the <u>New Hampshire Stormwater Manual, Volume 2, Post-Construction Best Management Practices Selection & Design</u> (December 2008, NHDES & US EPA). Any area disturbed by construction will be re-stabilized within 45 days (Env-Wq 1504.16) and abutting properties will not be adversely affected by this development. All swales and drainage structures will be constructed and stabilized prior to having run-off directed to them. Reference is also made to the <u>Stormwater System Management:</u> Inspection & Maintenance Manual which has been written specifically for this project and available to the owner. IAW EPA 2022 CGP 2.2.14, Site Stabilization will be initiated

immediately in any areas of exposed soil where construction activities have permanently ceased or will be temporarily inactive for 14 or more calendar days. The installation of stabilization will be completed as soon as practicable but no later than 14 calendar days.

As this proposed site redevelopment disturbs greater than one acre of land, a Notice of Intent is required to be submitted to the US EPA. This project will be covered by the Construction General Permit. As this project proposes less than 100,000 square feet of impact it does not require an issued permit by NHDES AoT, however construction is governed by a General Permit by Rule under NH RSA 485-A:17. Either a Stormwater Pollution Prevention Plan (SWPPP) or a Stormwater Management Plan (SWMP) as required by the Town of Barrington will be required.

Perimeter Control (Silt Fence / Silt Soxx)

The plan set demonstrates the location of perimeter sediment control. The Erosion and Sediment Control Details, Sheet E-101, has the specifications for installation and maintenance of the silt fence, Filtrexx mulch filled SiltSoxx (or approved equal), and Erosion Control Mix Berm. It is noted that the engineer prefers the mulch berms below as well as the Filtrexx product.

Bioretention System (Rain Garden)

<u>Description:</u> Rain Gardens, or bioretention areas are located close to the source of runoff. They are intended to integrate with the site landscaping and become an aesthetically attractive opportunity to provide highly effective stormwater treatment. The rain gardens associated with this proposed development contribute toward recharge of surface water run-off into the ground. It is important that sediment be removed from run-off prior to discharge into the bioretention area to preserve the mulch and soil mix ratio. During construction it is important that the ground surface not be exposed to traffic or construction specifications are included in the plan set and New Hampshire Stormwater Manual, Volume 2, 4-3 Treatment Practices, 4c Bioretention System.

<u>Construction Considerations</u>: After the stone and bio-media has been installed, Filtrexx Silt Soxx or approved equal, will be installed at the toe of slope intersection between the berm and bio-media and will remain until the slopes of the berm are stable.

See the Stormwater Management Inspection and Maintenance Manual for more details.

Detention Ponds

<u>Description:</u> A detention basin is an impoundment designed to temporarily store runoff and release it at a controlled rate, reducing the intensity of peak flows during storm

events. Conventional detention basins are typically designed to control peak runoff rates under a range of storm conditions, and can be used to control discharges as required under the AoT Regulations and other requirements. Construction specifications are included in the plan set and New Hampshire Stormwater Manual, Volume 2, 4-6 Conveyance Practices, 1, Detention Basin.

<u>Maintenance Considerations:</u> Detention Ponds should be inspected at least twice annually and following any rainfall event exceeding 0.25 inches in a twenty-four hour period. Maintenance rehabilitation will be conducted as warranted by each inspection. Trash and debris will be removed at each inspection.

Also on an annual basis the vegetation should be inspected to ensure healthy condition. Invasive species need to be removed along with dead or diseased vegetation.

Vegetated Stabilization

All areas that are disturbed during construction will be stabilized with vegetated material within 14 days of breaking ground. Construction will be managed in such a manner that erosion is prevented and that no abutter's property will be subjected to any siltation, unless otherwise permitted. All areas to be planted with grass for long-term cover will follow the specification and on Sheet E-102 using seeding mixture C, as follows:

Mixture	Pounds	Pounds per		
	per Acre	1,000 Sq. Ft.		
Tall Fescue	24	0.55		
Creeping Red Fescue	24	0.55		
Total	48	1.10		

Conservation Mix

Virginia Wild Rye	Native	FACW-
5		
Little Bluestem	Native	FACU
Big Bluestem	Native	FAC
Red Fescue	Native	FACU
Switch Grass	Native	FAC
Partridge Pea	Native	FACU
Showy Tick Trefoil	Native	FAC
Butterfly Milkweed	Native	NI
Beggar Ticks	Native	FACW
Purple Joe Pye Weed	Native	FAC
Black Eyed Susan	Native	FACU-
Total	25	0.57

Conservation Mix to be provided by New England Wetland Plants, Inc., Amherst, MA as outline in their New England Conservation / Wildlife Mix or approved equal. Mix to be applied at a rate of 25 lbs. per acre or one-lb. per 1750 square feet. Ratio of seed is proprietary and substitutions are not allowed.

Conservation Mix will be used to stabilize all 2:1 slopes.

Rain Garden Mix

The grass that is planted within a rain garden bio-filtration system within the biomedia must consist of a combination of warm season grass seed and cold season grass seed in order for the grass to start growing for stabilization and continue growing in the sandy well-drained environment. Planting specification will meet the requirements as outlined in 'Vegetation New Hampshire Sand and Gravel Pits' mix 1 (warm season grasses) (15 lbs/ac) and include annual and perennial rye grass seed (15 lbs/ac); the New England native warm season grass mix (23 lbs/ac) by New England Wetland Plants, Inc.; rain garden mix 180 (15 lbs/ac & 15 lbs/ac of rye) / rain garden grass mix 180-1 (20 lbs/ac & 10 lbs/ac of rye) by Ernst Conservation Seeds; or approved equal. Maintenance Considerations: Permanent seeded areas for long-term cover will be inspected on a periodic basis looking for signs of growth loss or erosion. Any areas found to be damaged will be repaired and replanted to reestablish the growth. The grass should be mowed at least twice per year and any dead material removed. Any woody growth that becomes established will need to be cut and removed.

Long-term maintenance of the land cover is critical and must be maintained at least 85% grass / vegetation coverage, must be inspected for concentrated flow, rills, and channels; and must be repaired as necessary to prevent erosion.

Environmental Dust Control

Dust will be controlled on the site by the use of multiple Best Management Practices. Mulching and temporary seeding will be the first line of protection to be utilized where problems occur. If dust problems are not solved by these applications, the use of water and calcium chloride can be applied. Calcium chloride will be applied at a rate that will keep the surface moist but not cause pollution.

Sediment Track-Out / Stabilized Construction Entrance / Exit

A temporary gravel construction entrance provides an area where mud can be dislodged from tires before the vehicle leaves the construction site to reduce the amount of mud and sediment transported onto paved municipal and state roads. The stone size for the pad should be 3-inch coarse aggregate, and the pad itself constructed to a minimum length of 75' for the full width of the access road with an entrance berm due to the sloping site. The aggregate should be placed at least six inches thick. A plan view and profile are shown on Construction Details Plan. The Construction Exit will

be maintained in a condition that will prevent tracking of sediment onto the public highway. When the control pad becomes ineffective, the stone should be removed along with the collected soil material, regraded on site, and stabilized. A plan view and profile are shown on the Construction Details Plan in E-101 Erosion & Sediment Control Details.

Off-site Vehicle Tracking: A stabilized construction entrance has been provided to help reduce vehicle tracking of sediments. The paved street adjacent to the site entrance will be swept every other week or as necessary to remove any excess mud, dirt or rock tracked from the site. After the binder pavement has been placed, any construction activity that causes the tracking of sediments will also result in street sweeping. Pavement should be swept prior to surface pavement and periodically to remove materials. Dump trucks hauling material from the construction site will be covered with a tarpaulin.

At a minimum, you must provide for maintenance that meets the following (EPA CGP Part 2.1.2.3.d) "Where sediment has been tracked-out from your site onto the surface of off-site streets, other paved areas, and sidewalks, you must remove the deposited sediment by the end of the same work day in which the track-out occurs or by the end of the next work day if track-out occurs on a non-work day. You must remove the track-out by sweeping, shoveling, or vacuuming these surfaces, or by using other similarly effective means of sediment removal. You are prohibited from hosing or sweeping tracked-out sediment into any stormwater conveyance (unless it is connected to a sediment basin, sediment trap, or similarly effective control), storm drain inlet, or surface water."

Stockpiled Sediment or Soil

Stockpiled materials including topsoil, excavated materials, borrow materials imported onto the site, construction aggregates, and sediment removed from temporary sediment traps will be located in designated areas at least 50 feet away form concentrated flows. All stockpiles will have erosion protection in the form of silt fence and diversion swales will be applied to protect the material and surrounding areas. Inactive stockpiles will be seeded for temporary stabilization. Erosion control measures will be inspected in accordance with the schedule for all other activities on site.

At a minimum, you must comply with following (EPA CGP Part 2.1.2.4d) "Do no hose down or sweep soil or sediment accumulated on pavement or other impervious surfaces into any stormwater conveyance (unless connected to a sediment basin, sediment trap, or similar effective control,) storm drain inlet, or surface water."

Dewatering Practices

Dewatering practices are not known to be required on this site. If during construction this becomes required, an addendum will be published specific for the

requirements. As a general rule, ground water that needs to be removed from an excavation will be pumped to a sediment basin or a storm drain inlet prior to discharge from the site.

At a minimum, you must comply with following (EPA CGP Part 2.1.3.4) "With backwash water, either haul it away for disposal or return it to the beginning of the treatment process; and replace and clean the filter media used in dewatering devices when the pressure differential equals or exceeds the manufacturer's specifications."

Regarding dewatering practices in the State of New Hampshire, specifically see Construction General Permit Section 9.1.2 NHR12000 State of New Hampshire and "Clarification of Section 9.1.2 ... and other New Hampshire specific information for the U.S. EPA 2012 NPDES Construction General Permit (CGP), May 3, 2012"

Please be advised that should dewatering become required, the EPA CGP 2022 requires daily inspections, monitoring, and reporting quarterly to the agency.

Drainage Swales / Stormwater Conveyance Channels

Drainage swales will be stabilized with vegetation for long term cover as outlined below, and on Sheet E-102 using seed mixture C. As a general rule, velocities in the swale should not exceed 3.0 feet per second for a vegetated swale although velocities as high as 4.5 FPS are allowed under certain soil conditions (If applicable).

Construction Sequence

- 1. Cut and remove trees in construction area only as required.
- 2. Construct and/or install temporary and permanent sediment erosion and detention control facilities as specified. Erosion and sediment control measures shall be installed prior to any soil land disturbance and must be reviewed and approved by the community services department.
- 3. Erosion, sediment and detention control facility shall be installed & stabilized prior to directing runoff to them, temporary diversions may be required. Post construction storm water management practices must be initiated and stabilized early in the process.
- 4. Clear, cut and dispose of debris in approved facility.
- 5. Construct temporary water diversions (swales, basins, etc.) as needed until site is stabilized.

- 6. All swales are to be installed prior to rough grading of the site. Temporary water diversion (swales, etc.) must be used as necessary until areas are stabilized.
- 7. Construct roadways for access to desired construction areas. All roads shall be stabilized immediately.
- 8. Install pipe and construction associated appurtenances as required or directed. Install rain gardens. All disturbed areas shall stabilized immediately after grading.
- 9. Begin permanent and temporary seeding and mulching. All cut and fill slopes and disturbed areas shall be seeded or mulched as required, or directed. Any area disturbed by construction will be re-stabilized within 45 days (Env-Wq 1504.16) and abutting properties will not be adversely affected by this development. All swales and drainage structures will be constructed and stabilized prior to having run-off directed to them. Iaw EPA 2022 CGP 2.2.14, site stabilization will be initiated immediately in any areas of exposed soil where construction activities have permanently ceased or will be temporarily inactive for 14 or more calendar days. The installation of stabilization will be completed as soon as practicable but no later than 14 calendar days. All roadways and parking areas shall be stabilized within 72 hours of achieving finished grades. All cut and fill slopes shall be stabilized within 72 hours of achieving finished grades.
- 10. Construct temporary berms, drains ditches, silt fences, sediment traps, etc. Mulch and seed as required.
- 11. Inspect and maintain all erosion and sediment control measures during construction. All SWPPP inspections must be conducted by a qualified professional such as a professional engineer (PE), a certified professional in erosion and sediment control (CPESC), a certified erosion sediment and storm water inspector (CESSWI), or a certified professional in storm water quality (CPSWQ). Inspection reports shall be submitted to the community services department. Inspections shall be conducted weekly and within 24 hours of a 0.25 inch rain event.
- 12. Complete permanent seeding and landscaping.
- 13. Remove temporary erosion control measures after seeding areas have established themselves and site improvements are complete.
- 14. Smooth and revegetate all disturbed areas. Stabilization should occur within 14 days or removing temporary measures.

15. Finish graveling all roadways.

Temporary Erosion Control Measures

- 1. The smallest practical area of land shall be exposed at any one time.
- 2. Erosion, sediment control measures shall be installed as shown on the plans and at locations as required, or directed by the engineer.
- 3. All disturbed areas shall be returned to original grades and elevations. Disturbed areas shall be loamed with a minimum of 4" of loam and seeded with not less than 1.10 pound of seed per 1,000 square feet (48 pounds per acre) of area.
- 4. Silt fences and other barriers shall be inspected periodically and after every rainstorm during the life of the project. All damaged areas shall be repaired, sediment deposits shall periodically be removed and properly disposed of.
- 5. After all disturbed areas have been stabilized, the temporary erosion control measures are to be removed and the area disturbed by the removal smoothed and re-vegetated.
- 6. Areas must be seeded and mulched within 5 days of final grading, permanently stabilized within 3 days of final grading, or temporarily stabilized within 14 days of initial disturbance of soil.

Inspection and Maintenance Schedule

Sediment control measures will be inspected during and after storm events to ensure that the practice still has integrity and is not allowing sediment to pass. Depending on SWPPP criteria, all controls will be inspected either once every 7 days and after storm events. Inspection reports must be submitted to Town of Barrington Planning Department. Sediment build-up in swales and level spreaders will be removed if it is deeper than six inches. See also <u>Stormwater System Operation</u>: Inspection & <u>Maintenance Manual</u> published separately also by Berry Surveying & Engineering. See also Storm Water Pollution Prevention Plan (SWPPP) and / or Stormwater Management Plan (SWMP) developed in accordance with EPA NPDES or Town of Barrington requirements.

Corrective Action measures will be made in accordance with Stormwater Management Plan (SWMP) / SWPPP requirements and records maintained on site by the Contractor.

5.0 CONCLUSION

Both peak rate of runoff and volume are reduced in the post-construction analysis, as compared to the pre-construction analysis at **Final Reaches #100 & #200**. This reduction occurs during the 2 Yr, 10 Yr, 25 Yr, 50 Yr, and 100 Yr-24 Hour storm events due to the infiltration of runoff in the rain garden.

A Site Specific, Terrain Alteration Permit (RSA 485: A-17) is not required for this site plan due to the area of disturbance being less than 100,000 SF and the site will operate as a General Permit by Rule under this rule. Because the site impact is greater than one acre, a Notice of Intent with the EPA is required.

Respectfully Submitted, BERRX SURVEYING & ENGINEERING

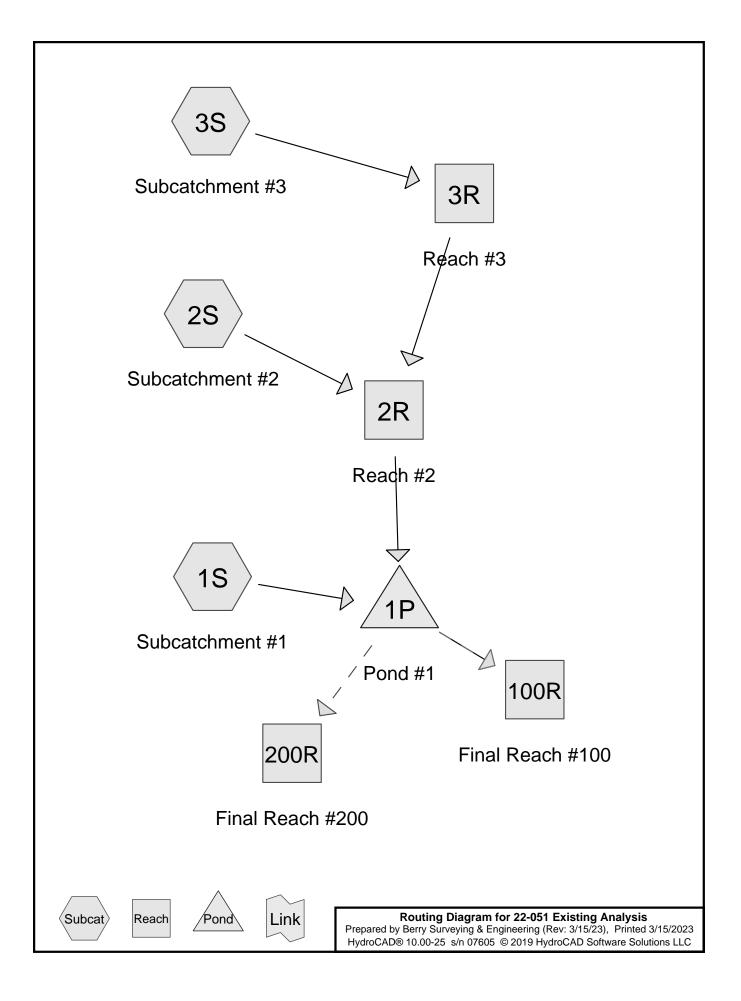
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Appendix I – Existing Conditions Analysis

25 Yr - 24 Hr. Full Summary 2 Yr - 24 Hr. Node Listing 10 Yr -24 Hr. Node Listing 25 Yr -24 Hr. Node Listing 50 Yr - 24 Hr. Node Listing 100 Yr -24 Hr. Node Listing



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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
2.178	39	>75% Grass cover, Good, HSG A (1S, 2S)
0.128	61	>75% Grass cover, Good, HSG B (2S)
0.585	74	>75% Grass cover, Good, HSG C (2S, 3S)
1.533	80	>75% Grass cover, Good, HSG D (2S, 3S)
0.020	96	Gravel surface, HSG C (2S)
0.065	96	Gravel surface, HSG D (2S, 3S)
0.333	98	Paved parking, HSG A (1S, 2S)
0.074	98	Paved parking, HSG B (2S)
0.080	98	Paved parking, HSG C (3S)
0.271	98	Paved parking, HSG D (2S, 3S)
0.203	98	Unconnected roofs, HSG A (1S, 2S)
0.009	98	Unconnected roofs, HSG B (2S)
0.045	98	Unconnected roofs, HSG C (2S)
0.081	98	Unconnected roofs, HSG D (2S)
2.323	30	Woods, Good, HSG A (1S, 2S)
1.616	55	Woods, Good, HSG B (1S, 2S)
11.262	70	Woods, Good, HSG C (1S, 2S, 3S)
0.640	77	Woods, Good, HSG D (2S, 3S)
21.446	64	TOTAL AREA

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
5.037	HSG A	1S, 2S
1.827	HSG B	1S, 2S
11.992	HSG C	1S, 2S, 3S
2.590	HSG D	2S, 3S
0.000	Other	
21.446		TOTAL AREA

22-051 Existing Analysis

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
2.178	0.128	0.585	1.533	0.000	4.424	>75% Grass cover, Good	1S, 2S, 3S
0.000	0.000	0.020	0.065	0.000	0.085	Gravel surface	2S, 3S
0.333	0.074	0.080	0.271	0.000	0.759	Paved parking	1S, 2S, 3S
0.203	0.009	0.045	0.081	0.000	0.338	Unconnected roofs	1S, 2S
2.323	1.616	11.262	0.640	0.000	15.841	Woods, Good	1S, 2S,
5.037	1.827	11.992	2.590	0.000	21.446	TOTAL AREA	3S

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Pipe Listing (all hodes)									
Line#	Node	In-Invert	Out-Invert	Length	Slope	n	Diam/Width	Height	Inside-Fill
	Number	(feet)	(feet)	(feet)	(ft/ft)		(inches)	(inches)	(inches)
 1	1P	194.12	193.74	40.5	0.0094	0.012	18.0	0.0	0.0

Pipe Listing (all nodes)

22-051 Existing Analysis	Type III 24-hr 25Yr24Hr. Rainfall=5.85"
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Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1S: Subcatchment#1	Runoff Area=298,718 sf 3.59% Impervious Runoff Depth>1.50" Flow Length=912' Tc=21.7 min CN=56 Runoff=7.00 cfs 0.857 af
Subcatchment 2S: Subcatchment #2 Flow Leng	Runoff Area=567,294 sf 4.96% Impervious Runoff Depth>2.31" th=1,706' Tc=38.6 min UI Adjusted CN=66 Runoff=17.40 cfs 2.505 af
Subcatchment3S: Subcatchment#3	Runoff Area=68,180 sf 13.06% Impervious Runoff Depth>3.24" Flow Length=932' Tc=19.7 min CN=76 Runoff=4.04 cfs 0.422 af
Reach 2R: Reach #2 n=0.022	Avg. Flow Depth=0.49' Max Vel=4.14 fps Inflow=20.18 cfs 2.926 af L=390.0' S=0.0168 '/' Capacity=20.98 cfs Outflow=20.15 cfs 2.922 af
Reach 3R: Reach #3 n=0.022	Avg. Flow Depth=0.18' Max Vel=3.69 fps Inflow=4.04 cfs 0.422 af L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=3.92 cfs 0.421 af
Reach 100R: Final Reach #100	Inflow=12.32 cfs 3.144 af Outflow=12.32 cfs 3.144 af
Reach 200R: Final Reach #200	Inflow=10.40 cfs 0.460 af Outflow=10.40 cfs 0.460 af
Pond 1P: Pond #1 Primary=11.91 cfs 3.141 af Secondary=10.	Peak Elev=196.83' Storage=14,456 cf Inflow=25.96 cfs 3.779 af 40 cfs 0.460 af Tertiary=0.41 cfs 0.004 af Outflow=23.74 cfs 3.777 af

0.173 af

Total Runoff Area = 21.446 acRunoff Volume = 3.784 afAverage Runoff Depth = 2.12"94.89% Pervious = 20.350 ac5.11% Impervious = 1.096 ac

Summary for Subcatchment 1S: Subcatchment #1

Runoff = 7.00 cfs @ 12.34 hrs, Volume= 0.857 af, Depth> 1.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25Yr.-24Hr. Rainfall=5.85"

A	rea (sf)	CN [Description				
	4,967	98 l	8 Unconnected roofs, HSG A				
	20,184	39 >	>75% Grass cover, Good, HSG A				
	5,749	98 F	Paved parking, HSG A				
	84,127	30 V	Voods, Go	od, HSG A			
	35,023	55 N	Voods, Go	od, HSG B			
1	48,668	70 V	Voods, Go	od, HSG C			
2	98,718	56 V	Veighted A	verage			
2	288,002	ç	96.41% Pei	vious Area			
	10,716	3	3.59% Impe	ervious Area	а		
	4,967	2	16.35% Un	connected			
Тс	Length	Slope	Velocity		Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
9.3	100	0.1700	0.18		Sheet Flow, Segment #1		
					Woods: Light underbrush n= 0.400 P2= 3.08"		
1.4	239	0.3353	2.90		Shallow Concentrated Flow, Segment #2		
					Woodland Kv= 5.0 fps		
1.1	119	0.1346	1.83		Shallow Concentrated Flow, Segment #3		
					Woodland Kv= 5.0 fps		
8.2	264	0.0114	0.53		Shallow Concentrated Flow, Segment #4		
					Woodland Kv= 5.0 fps		
1.7	190	0.0737	1.90		Shallow Concentrated Flow, Segment #5		
					Short Grass Pasture Kv= 7.0 fps		
21.7	912	Total					

22-051 Existing Analysis

Flow (cfs) 4

3-

2-

1-

0-

Ó

Runoff Depth>1.50"

Flow Length=912'

Tc=21.7 min

CN=56

Runoff

Subcatchment 1S: Subcatchment #1 Hydrograph 7.00 cfs Type III 24-hr 7 25Yr.-24Hr. Rainfall=5.85" 6-Runoff Area=298,718 sf Runoff Volume=0.857 af 5

11 12 13 14 15 16 17 18 19 20 21 22 23 2 Ś 4 5 6 7 8 9 10 24 1 Time (hours)

Summary for Subcatchment 2S: Subcatchment #2

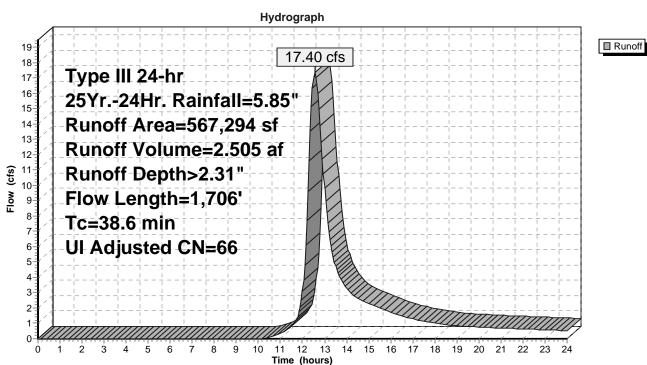
Runoff = 17.40 cfs @ 12.56 hrs, Volume= 2.505 af, Depth> 2.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25Yr.-24Hr. Rainfall=5.85"

A	rea (sf)	CN A	Adj Desc	ription				
	3,857	98	Unco	onnected ro	oofs, HSG A			
	389	98			ofs, HSG B			
	1,956	98		Unconnected roofs, HSG C				
	3,549	98		Unconnected roofs, HSG D				
	74,688	39	>75%	>75% Grass cover, Good, HSG A				
	5,582	61	>75%	6 Grass co	ver, Good, HSG B			
	12,591	74	>75%	6 Grass co	ver, Good, HSG C			
	59,930	80		>75% Grass cover, Good, HSG D				
	8,765	98	Pave	d parking,	HSG A			
	3,235	98	Pave	Paved parking, HSG B				
	6,387	98		Paved parking, HSG D				
	17,061	30		Woods, Good, HSG A				
	35,354	55		Woods, Good, HSG B				
	10,231	70		Woods, Good, HSG C				
	20,606	77		Woods, Good, HSG D				
	860	96		Gravel surface, HSG C				
	2,253	96		Gravel surface, HSG D				
	67,294	67		Weighted Average, UI Adjusted				
	39,156			95.04% Pervious Area				
	28,138			4.96% Impervious Area				
	9,751		34.6	34.65% Unconnected				
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•			
14.1	100	0.0600	0.12		Sheet Flow, Segment #1			
					Woods: Light underbrush n= 0.400 P2= 3.08"			
4.4	366	0.0765	1.38		Shallow Concentrated Flow, Segment #2			
					Woodland Kv= 5.0 fps			
1.0	71	0.0280	1.17		Shallow Concentrated Flow, Segment #3			
					Short Grass Pasture Kv= 7.0 fps			
13.8	676	0.0266	0.82		Shallow Concentrated Flow, Segment #4			
				Woodland Kv= 5.0 fps				
2.1	172	0.0754	1.37	1.37 Shallow Concentrated Flow, Segment #5				
				Woodland Kv= 5.0 fps				
3.2	321	0.1151	1.70		Shallow Concentrated Flow, Segment #6			
					Woodland Kv= 5.0 fps			
38.6	1 706	Total						

38.6 1,706 Total

22-051 Existing Analysis



Subcatchment 2S: Subcatchment #2

Summary for Subcatchment 3S: Subcatchment #3

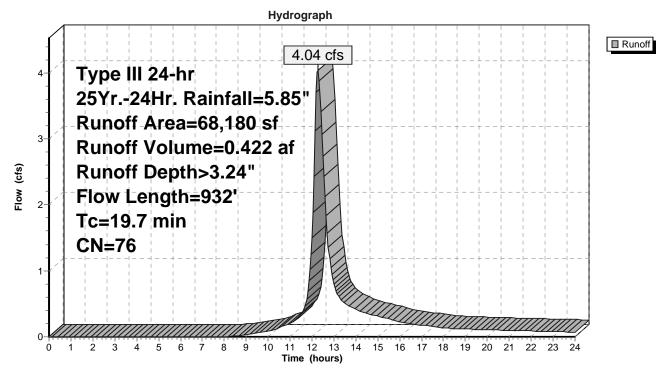
Runoff = 4.04 cfs @ 12.27 hrs, Volume= 0.422 af, Depth> 3.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25Yr.-24Hr. Rainfall=5.85"

A	rea (sf)	CN [Description				
	12,882	74 >	>75% Grass cover, Good, HSG C				
	6,834	80 >	>75% Grass cover, Good, HSG D				
	3,499	98 F	Paved parking, HSG C				
	5,407	98 F	Paved parking, HSG D				
	31,691		Woods, Good, HSG C				
	7,279		Woods, Good, HSG D				
	588	96 (Gravel surfa	ace, HSG [)		
	68,180		Veighted A				
	59,274	8	86.94% Pei	rvious Area			
	8,906	13.06% Impervious Area					
-				o ''			
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	Capacity (cfs)			
	0				Sheet Flow, Segment #1		
<u>(min)</u> 15.2	(feet) 100	(ft/ft) 0.0500	(ft/sec) 0.11		Sheet Flow, Segment #1 Woods: Light underbrush n= 0.400 P2= 3.08"		
(min)	(feet)	(ft/ft)	(ft/sec)		Sheet Flow, Segment #1 Woods: Light underbrush n= 0.400 P2= 3.08" Shallow Concentrated Flow, Segment #2		
(min) 15.2 3.7	(feet) 100 306	(ft/ft) 0.0500 0.0752	(ft/sec) 0.11 1.37	(cfs)	Sheet Flow, Segment #1 Woods: Light underbrush n= 0.400 P2= 3.08" Shallow Concentrated Flow, Segment #2 Woodland Kv= 5.0 fps		
<u>(min)</u> 15.2	(feet) 100	(ft/ft) 0.0500	(ft/sec) 0.11		Sheet Flow, Segment #1 Woods: Light underbrush n= 0.400 P2= 3.08" Shallow Concentrated Flow, Segment #2 Woodland Kv= 5.0 fps Parabolic Channel, Segment #3		
(min) 15.2 3.7	(feet) 100 306	(ft/ft) 0.0500 0.0752	(ft/sec) 0.11 1.37	(cfs)	Sheet Flow, Segment #1 Woods: Light underbrush n= 0.400 P2= 3.08" Shallow Concentrated Flow, Segment #2 Woodland Kv= 5.0 fps Parabolic Channel, Segment #3 W=5.50' D=1.00' Area=3.7 sf Perim=6.0'		
(min) 15.2 3.7	(feet) 100 306	(ft/ft) 0.0500 0.0752	(ft/sec) 0.11 1.37	(cfs)	Sheet Flow, Segment #1 Woods: Light underbrush n= 0.400 P2= 3.08" Shallow Concentrated Flow, Segment #2 Woodland Kv= 5.0 fps Parabolic Channel, Segment #3		

22-051 Existing Analysis

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Subcatchment 3S: Subcatchment #3

Summary for Reach 2R: Reach #2

[62] Hint: Exceeded Reach 3R OUTLET depth by 0.35' @ 12.65 hrs

 Inflow Area =
 14.588 ac, 5.83% Impervious, Inflow Depth > 2.41" for 25Yr.-24Hr. event

 Inflow =
 20.18 cfs @
 12.52 hrs, Volume=
 2.926 af

 Outflow =
 20.15 cfs @
 12.54 hrs, Volume=
 2.922 af, Atten= 0%, Lag= 1.1 min

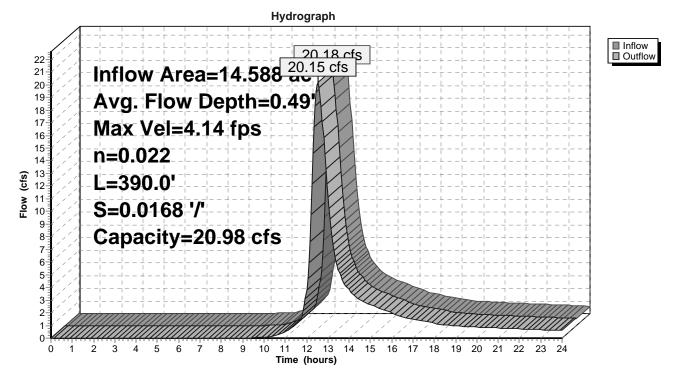
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Max. Velocity= 4.14 fps, Min. Travel Time= 1.6 min Avg. Velocity = 1.73 fps, Avg. Travel Time= 3.8 min

Peak Storage= 1,896 cf @ 12.54 hrs Average Depth at Peak Storage= 0.49' Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 20.98 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 390.0' Slope= 0.0168 '/' Inlet Invert= 200.66', Outlet Invert= 194.12'

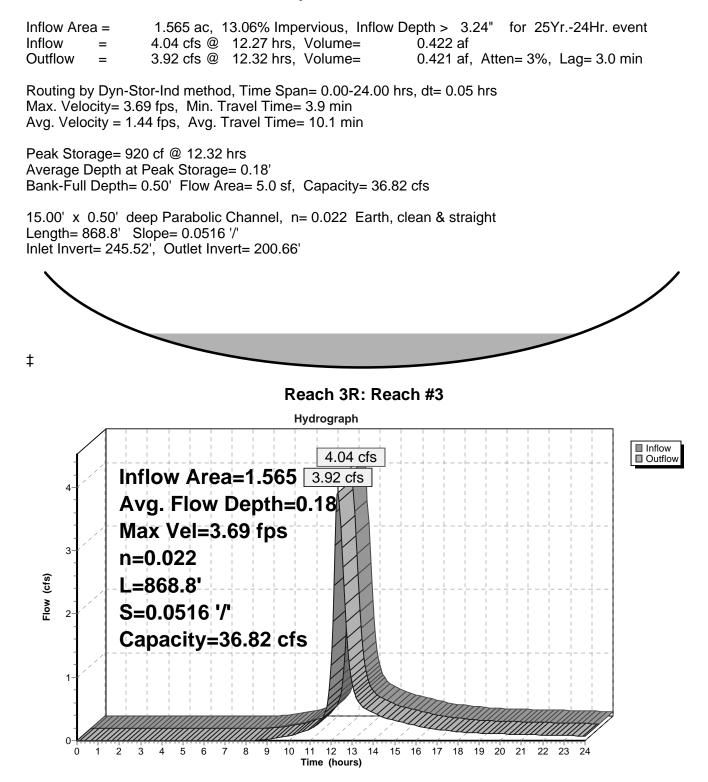
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Reach 2R: Reach #2



22-051 Existing AnalysisType III 24-hr 25Yr.-24Hr. Rainfall=5.85"Prepared by Berry Surveying & Engineering (Rev: 3/15/23)Printed 3/15/2023HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLCPage 14

Summary for Reach 3R: Reach #3

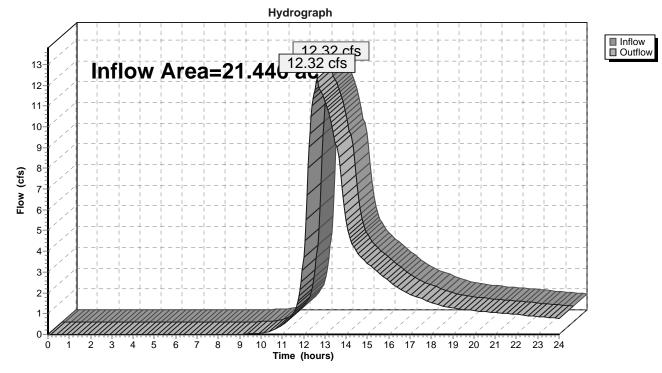


Summary for Reach 100R: Final Reach #100

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =	21.446 ac,	5.11% Impervious, Inflow I	Depth > 1.76"	for 25Yr24Hr. event
Inflow =	12.32 cfs @	12.65 hrs, Volume=	3.144 af	
Outflow =	12.32 cfs @	12.65 hrs, Volume=	3.144 af, Atte	en= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



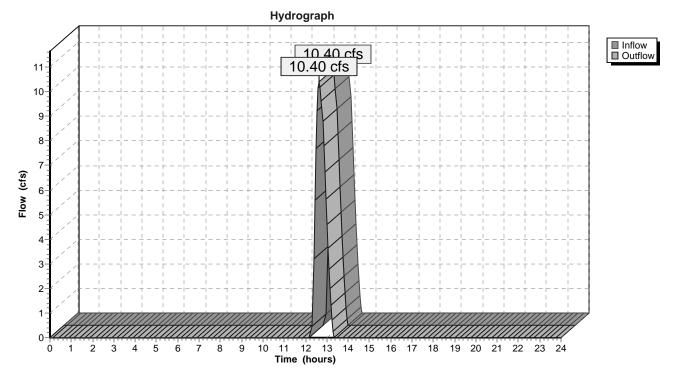
Reach 100R: Final Reach #100

Summary for Reach 200R: Final Reach #200

[40] Hint: Not Described (Outflow=Inflow)

Inflow	=	10.40 cfs @	12.65 hrs, Volume=	0.460 af
Outflow	=	10.40 cfs @	12.65 hrs, Volume=	0.460 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



Reach 200R: Final Reach #200

Summary for Pond 1P: Pond #1

[58] Hint: Peaked 0.03' above defined flood level [62] Hint: Exceeded Reach 2R OUTLET depth by 2.23' @ 12.70 hrs

Inflow Area =	21.446 ac,	5.11% Impervious, Inflow D	Depth > 2.11" for 25Yr24Hr. event
Inflow =	25.96 cfs @	12.49 hrs, Volume=	3.779 af
Outflow =	23.74 cfs @	12.65 hrs, Volume=	3.777 af, Atten= 9%, Lag= 9.7 min
Discarded =	1.02 cfs @	12.40 hrs, Volume=	0.173 af
Primary =	11.91 cfs @	12.65 hrs, Volume=	3.141 af
Secondary =		12.65 hrs, Volume=	0.460 af
Tertiary =	0.41 cfs @	12.65 hrs, Volume=	0.004 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 196.83' @ 12.65 hrs Surf.Area= 14,717 sf Storage= 14,456 cf Flood Elev= 196.80' Surf.Area= 14,717 sf Storage= 14,023 cf

Plug-Flow detention time= 6.5 min calculated for 3.770 af (100% of inflow) Center-of-Mass det. time= 6.2 min (880.8 - 874.6)

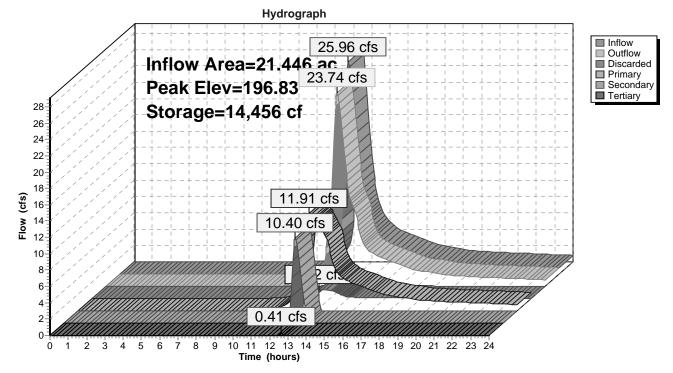
Volume	Invert	Avail.Sto	orage	Storage Description	n	
#1	194.12'	24,3	25 cf	Open Water Stora	i ge (Irregular) Listed	d below (Recalc)
Elevatio		urf.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>
194.1 196.0 196.5 197.5	00 50	14,717	14.0 469.6 797.7 797.7	0 4,384 5,224 14,717	0 4,384 9,608 24,325	12 17,551 50,641 51,439
Device	Routing	Invert	Outl	et Devices		
#1 #2	Discarded Primary	194.12' 194.12'	18.0 L= 4 Inlet	0 in/hr Infiltration 3 " Round 18" HDPE 0.5' CPP, square e / Outlet Invert= 194 0.012, Flow Area= 1	= edge headwall, Ke= .12' / 193.74' S= 0	
#3	Secondary	196.30'	Hea	' long x 40.0' bread d (feet) 0.20 0.40 (f. (English) 2.68 2.7	0.60 0.80 1.00 1.2	20 1.40 1.60
#4	Tertiary	196.80'	30.0 Hea	<pre>' long x 24.0' bread d (feet) 0.20 0.40 (f. (English) 2.68 2.7</pre>	dth Overflow Over 0.60 0.80 1.00 1.2	Rt 9 20 1.40 1.60

Discarded OutFlow Max=1.02 cfs @ 12.40 hrs HW=196.54' (Free Discharge) **1=Infiltration 3IN/HR** (Exfiltration Controls 1.02 cfs)

Primary OutFlow Max=11.91 cfs @ 12.65 hrs HW=196.83' TW=0.00' (Dynamic Tailwater) **2=18" HDPE** (Inlet Controls 11.91 cfs @ 6.74 fps)

Secondary OutFlow Max=10.39 cfs @ 12.65 hrs HW=196.83' TW=0.00' (Dynamic Tailwater) -3=Overflow Over DW (Weir Controls 10.39 cfs @ 1.96 fps)

Tertiary OutFlow Max=0.40 cfs @ 12.65 hrs HW=196.83' TW=0.00' (Dynamic Tailwater) 4=Overflow Over Rt 9 (Weir Controls 0.40 cfs @ 0.46 fps)



Pond 1P: Pond #1

22-051 Existing Analysis	Type III 24-hr 2Yr24Hr. Rainfall=3.08"
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Subcatchment1S: Subcatchment#	1 Runoff Area=298,718 sf 3.59% Impervious Runoff Depth>0.24" Flow Length=912' Tc=21.7 min CN=56 Runoff=0.57 cfs 0.137 af
Subcatchment 2S: Subcatchment #2 Flow Le	2 Runoff Area=567,294 sf 4.96% Impervious Runoff Depth>0.58" ength=1,706' Tc=38.6 min UI Adjusted CN=66 Runoff=3.57 cfs 0.625 af
Subcatchment3S: Subcatchment#	3 Runoff Area=68,180 sf 13.06% Impervious Runoff Depth>1.06" Flow Length=932' Tc=19.7 min CN=76 Runoff=1.27 cfs 0.139 af
Reach 2R: Reach #2 n=0.02	Avg. Flow Depth=0.24' Max Vel=2.60 fps Inflow=4.42 cfs 0.763 af 2 L=390.0' S=0.0168 '/' Capacity=20.98 cfs Outflow=4.40 cfs 0.761 af
Reach 3R: Reach #3 n=0.02	Avg. Flow Depth=0.10' Max Vel=2.57 fps Inflow=1.27 cfs 0.139 af 2 L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=1.20 cfs 0.138 af
Reach 100R: Final Reach #100	Inflow=4.60 cfs 0.866 af Outflow=4.60 cfs 0.866 af
Reach 200R: Final Reach #200	Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Pond 1P: Pond #1 af Primary=4.60 cfs 0.866 af Secondary	Peak Elev=195.24' Storage=1,004 cf Inflow=4.96 cfs 0.899 af =0.00 cfs 0.000 af Tertiary=0.00 cfs 0.000 af Outflow=4.77 cfs 0.898 af

cfs 0.032

Total Runoff Area = 21.446 acRunoff Volume = 0.901 afAverage Runoff Depth = 0.50"94.89% Pervious = 20.350 ac5.11% Impervious = 1.096 ac

22-051 Existing Analysis	Type III 24-hr 10Yr24Hr. Rainfall=4.63"
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Runoff Area=298,718 sf 3.59% Impervious Runoff Depth>0.85" Flow Length=912' Tc=21.7 min CN=56 Runoff=3.48 cfs 0.486 af
2 Runoff Area=567,294 sf 4.96% Impervious Runoff Depth>1.47" gth=1,706' Tc=38.6 min UI Adjusted CN=66 Runoff=10.67 cfs 1.590 af
Runoff Area=68,180 sf 13.06% Impervious Runoff Depth>2.22" Flow Length=932' Tc=19.7 min CN=76 Runoff=2.75 cfs 0.290 af
Avg. Flow Depth=0.39' Max Vel=3.58 fps Inflow=12.57 cfs 1.879 af L=390.0' S=0.0168 '/' Capacity=20.98 cfs Outflow=12.55 cfs 1.876 af
Avg. Flow Depth=0.15' Max Vel=3.28 fps Inflow=2.75 cfs 0.290 af 2 L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=2.65 cfs 0.289 af
Inflow=10.62 cfs 2.221 af Outflow=10.62 cfs 2.221 af
Inflow=1.32 cfs 0.030 af Outflow=1.32 cfs 0.030 af
Peak Elev=196.43' Storage=8,683 cf Inflow=15.57 cfs 2.363 af I.32 cfs 0.030 af Tertiary=0.00 cfs 0.000 af Outflow=12.87 cfs 2.362 af

0.111 af

Total Runoff Area = 21.446 acRunoff Volume = 2.367 afAverage Runoff Depth = 1.32"94.89% Pervious = 20.350 ac5.11% Impervious = 1.096 ac

22-051 Existing Analysis	Type III 24-hr 25Yr24Hr. Rainfall=5.85"
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Subcatchment1S: Subcatchment#1	Runoff Area=298,718 sf 3.59% Impervious Runoff Depth>1.50" Flow Length=912' Tc=21.7 min CN=56 Runoff=7.00 cfs 0.857 af
Subcatchment 2S: Subcatchment #2 Flow Lengt	Runoff Area=567,294 sf 4.96% Impervious Runoff Depth>2.31" th=1,706' Tc=38.6 min UI Adjusted CN=66 Runoff=17.40 cfs 2.505 af
Subcatchment3S: Subcatchment#3	Runoff Area=68,180 sf 13.06% Impervious Runoff Depth>3.24" Flow Length=932' Tc=19.7 min CN=76 Runoff=4.04 cfs 0.422 af
Reach 2R: Reach #2 n=0.022	Avg. Flow Depth=0.49' Max Vel=4.14 fps Inflow=20.18 cfs 2.926 af L=390.0' S=0.0168 '/' Capacity=20.98 cfs Outflow=20.15 cfs 2.922 af
Reach 3R: Reach #3 n=0.022	Avg. Flow Depth=0.18' Max Vel=3.69 fps Inflow=4.04 cfs 0.422 af L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=3.92 cfs 0.421 af
Reach 100R: Final Reach #100	Inflow=12.32 cfs 3.144 af Outflow=12.32 cfs 3.144 af
Reach 200R: Final Reach #200	Inflow=10.40 cfs 0.460 af Outflow=10.40 cfs 0.460 af
Pond 1P: Pond #1 Primary=11.91 cfs 3.141 af Secondary=10.4	Peak Elev=196.83' Storage=14,456 cf Inflow=25.96 cfs 3.779 af 40 cfs 0.460 af Tertiary=0.41 cfs 0.004 af Outflow=23.74 cfs 3.777 af

0.173 af

Total Runoff Area = 21.446 acRunoff Volume = 3.784 afAverage Runoff Depth = 2.12"94.89% Pervious = 20.350 ac5.11% Impervious = 1.096 ac

22-051 Existing Analysis	Type III 24-hr 50Yr24Hr. Rainfall=6.99"
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Subcatchment1S: Subcatchment#1	Runoff Area=298,718 sf 3.59% Impervious Runoff Depth>2.20" Flow Length=912' Tc=21.7 min CN=56 Runoff=10.81 cfs 1.257 af
Subcatchment 2S: Subcatchment #2 Flow Lengt	Runoff Area=567,294 sf 4.96% Impervious Runoff Depth>3.17" h=1,706' Tc=38.6 min UI Adjusted CN=66 Runoff=24.21 cfs 3.439 af
Subcatchment3S: Subcatchment#3	Runoff Area=68,180 sf 13.06% Impervious Runoff Depth>4.23" Flow Length=932' Tc=19.7 min CN=76 Runoff=5.28 cfs 0.552 af
Reach 2R: Reach #2 n=0.022	Avg. Flow Depth=0.58' Max Vel=4.53 fps Inflow=27.85 cfs 3.990 af L=390.0' S=0.0168 '/' Capacity=20.98 cfs Outflow=27.80 cfs 3.985 af
Reach 3R: Reach #3 n=0.022	Avg. Flow Depth=0.20' Max Vel=4.02 fps Inflow=5.28 cfs 0.552 af L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=5.14 cfs 0.551 af
Reach 100R: Final Reach #100	Inflow=19.44 cfs 4.125 af Outflow=19.44 cfs 4.125 af
Reach 200R: Final Reach #200	Inflow=15.54 cfs 0.897 af Outflow=15.54 cfs 0.897 af
Pond 1P: Pond #1 Primary=12.41 cfs 3.927 af Secondary=15.5	Peak Elev=197.00' Storage=16,923 cf Inflow=36.65 cfs 5.242 af 54 cfs 0.897 af Tertiary=7.03 cfs 0.198 af Outflow=36.00 cfs 5.240 af

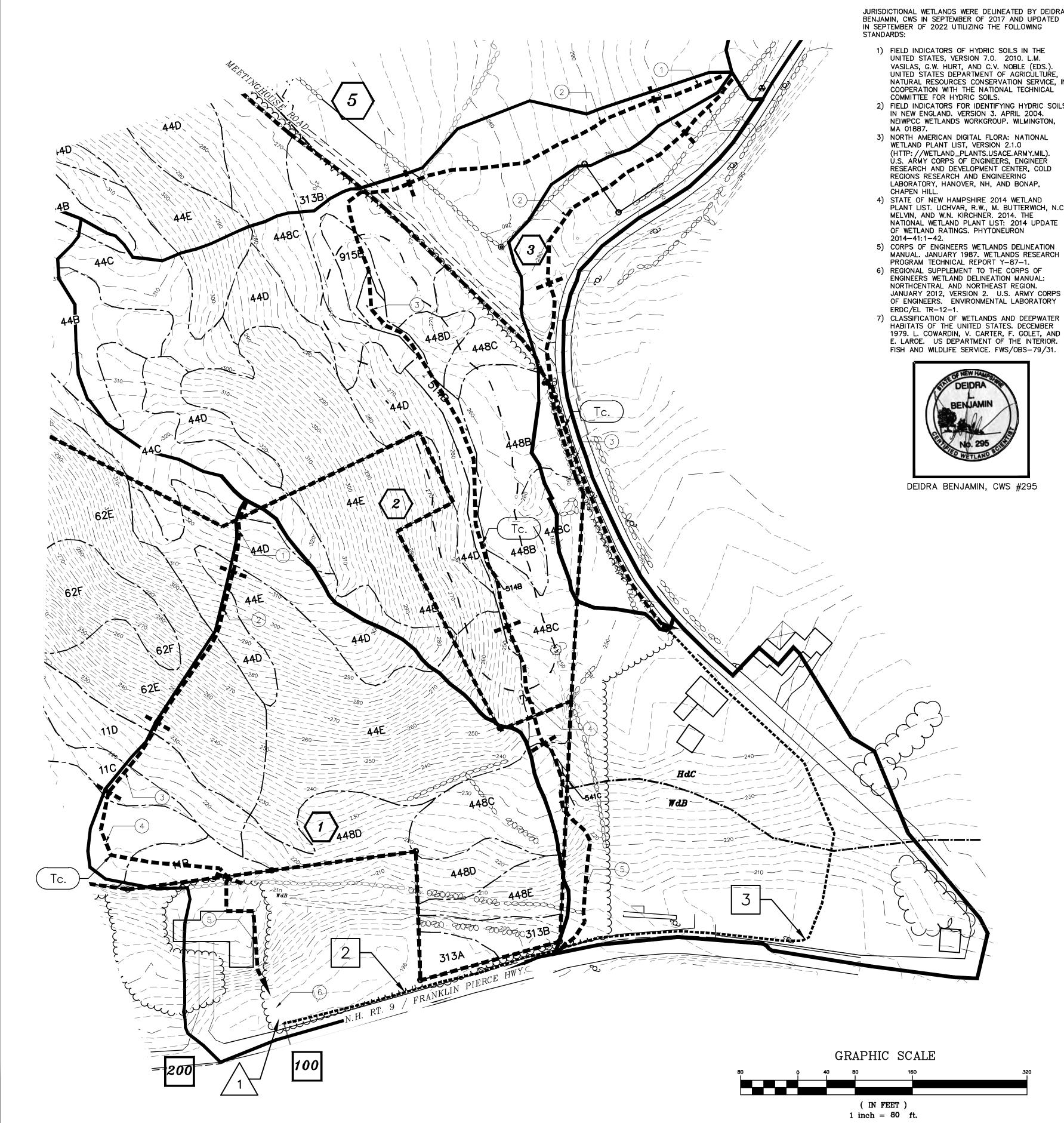
0.218 af

Total Runoff Area = 21.446 acRunoff Volume = 5.248 afAverage Runoff Depth = 2.94"94.89% Pervious = 20.350 ac5.11% Impervious = 1.096 ac

22-051 Existing Analysis	Type III 24-hr	100Yr24Hr. Rainfall=8.36"
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Subcatchment1S: Subcatchment#1	Runoff Area=298,718 sf 3.59% Impervious Runoff Depth>3.13" Flow Length=912' Tc=21.7 min CN=56 Runoff=15.91 cfs 1.789 af
Subcatchment 2S: Subcatchment #2 Flow Length	Runoff Area=567,294 sf 4.96% Impervious Runoff Depth>4.27" n=1,706' Tc=38.6 min UI Adjusted CN=66 Runoff=32.85 cfs 4.633 af
Subcatchment3S: Subcatchment#3	Runoff Area=68,180 sf 13.06% Impervious Runoff Depth>5.47" Flow Length=932' Tc=19.7 min CN=76 Runoff=6.79 cfs 0.713 af
Reach 2R: Reach #2 n=0.022 L	Avg. Flow Depth=0.68' Max Vel=4.84 fps Inflow=37.51 cfs 5.345 af
Reach 3R: Reach #3 n=0.022	Avg. Flow Depth=0.23' Max Vel=4.35 fps Inflow=6.79 cfs 0.713 af L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=6.64 cfs 0.711 af
Reach 100R: Final Reach #100	Inflow=28.73 cfs 5.455 af Outflow=28.73 cfs 5.455 af
Reach 200R: Final Reach #200	Inflow=20.26 cfs 1.398 af Outflow=20.26 cfs 1.398 af
Pond 1P: Pond #1 272 af Primary=12.82 cfs 4.832 af Secondary=20.26	Peak Elev=197.14' Storage=19,005 cf Inflow=50.33 cfs 7.128 af 5 cfs 1.398 af Tertiary=15.92 cfs 0.623 af Outflow=50.01 cfs 7.126 af

Total Runoff Area = 21.446 acRunoff Volume = 7.135 afAverage Runoff Depth = 3.99"94.89% Pervious = 20.350 ac5.11% Impervious = 1.096 ac



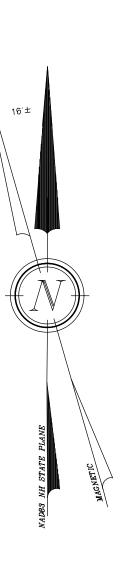
JURISDICTIONAL WETLANDS WERE DELINEATED BY DEIDRA

- VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.). UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL
- 2) FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND. VERSION 3. APRIL 2004. NEIWPCC WETLANDS WORKGROUP. WILMINGTON,
- 3) NORTH AMERICAN DIGITAL FLORA: NATIONAL (HTTP: //WETLAND_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD LABORATORY, HANOVER, NH, AND BONAP,
- 4) STATE OF NEW HAMPSHIRE 2014 WETLAND PLANT LIST. LICHVAR, R.W., M. BUTTERWICH, N.C. MELVIN, AND W.N. KIRCHNER. 2014. THE NATIONAL WETLAND PLANT LIST: 2014 UPDATE
- 5) CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL, JANUARY 1987. WETLANDS RESEARCH
- 6) REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTHCENTRAL AND NORTHEAST REGION. JANUARY 2012, VERSION 2. U.S. ARMY CORPS OF ENGINEERS. ENVIRONMENTAL LABORATORY
- 7) CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES. DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. US DEPARTMENT OF THE INTERIOR. FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.

DEIDRA BENJAMIN, CWS #295



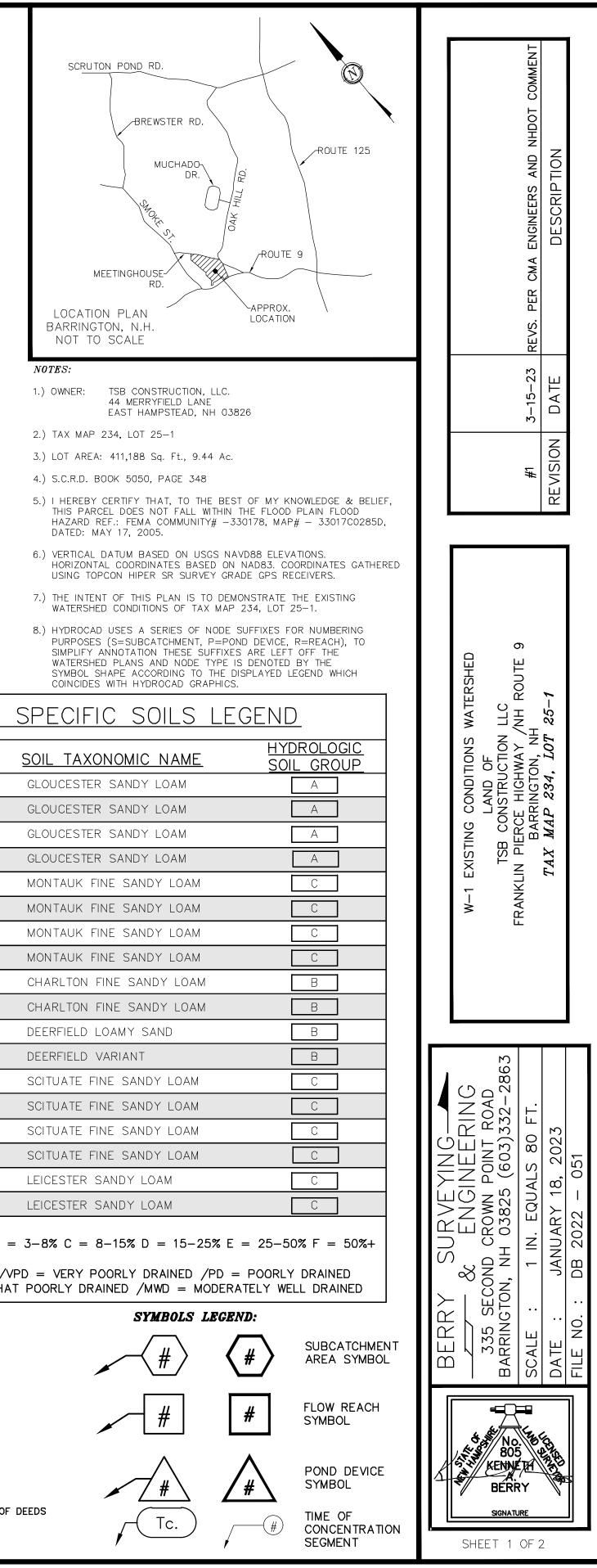
JOHN P. HAYES III, CSS #87



	<u>SITE</u>
SYMBOL	
11A	
11B	
11C	
11D	
44B	
44C	
44D	
44E	
62E	
62F	
313B	
915B	
448B	
448C	
448D	
448E	
514P	
514C	
SLOPE: A =	= 0-3% B :
DENOM /SWPD	1INATOR: /\ = SOMEWHA

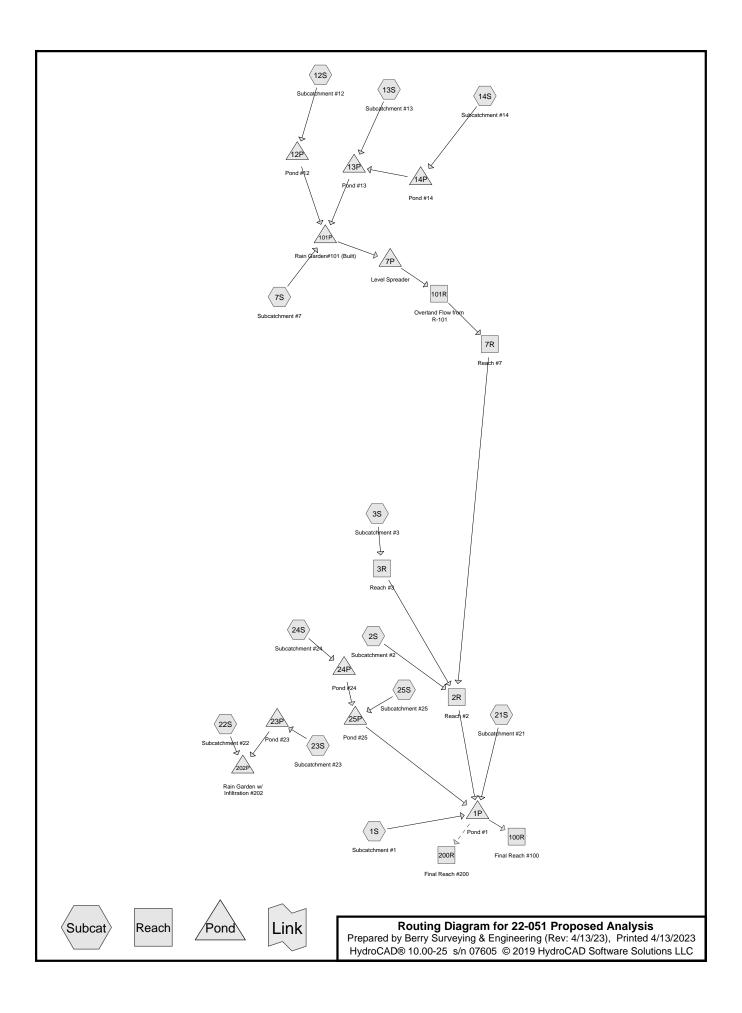
LEGEND:

EXISTING CONTOUR MINOR — — — — 299 — — — — — — EXISTING CONTOUR MAJOR STONE WALL WETLAND LINE _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ _ PROPERTY LINE -----SOIL LINE 448A SOIL SERIES NRCS SOIL LINE NRCS SOIL LABEL HfB LIMIT OF WATERSHED TIME OF CONCENTRATION PATH FLOW REACH **4**-----MATCH LINE S.C.R.D. TYP. FND TBR STRAFFORD COUNTY REGISTRY OF DEEDS TYPICAL FOUND TO BE REMOVED



Appendix II - Proposed Conditions Analysis

25 Yr - 24 Hr. Full Summary 2 Yr - 24 Hr. Node Listing 10 Yr -24 Hr. Node Listing 25 Yr -24 Hr. Node Listing 50 Yr - 24 Hr. Node Listing 100 Yr -24 Hr. Node Listing



Project Notes

While the analyses of Meetinghouse Road encompass a larger area only Subcatchments #1, #2, & #3 contribute runoff to Final Reaches #100 & #200. For this reason only these subcatchments were analyzed in this project. As such the resulting construction surrounding Meetinghouse Road results in a difference of 2.47 acres in contributing land area of the analyzed subcatchments between the pre-construction and post-construction analyses.

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
2.693	39	>75% Grass cover, Good, HSG A (1S, 2S, 21S, 22S, 23S)
0.931	61	>75% Grass cover, Good, HSG B (1S, 2S, 7S, 21S, 22S, 23S)
3.126	74	>75% Grass cover, Good, HSG C (1S, 2S, 3S, 7S, 12S, 13S, 14S, 21S, 23S, 24S,
		25S)
1.623	80	>75% Grass cover, Good, HSG D (2S, 3S, 12S, 13S)
0.552	98	Paved parking, HSG A (1S, 2S, 21S, 22S, 24S)
0.144	98	Paved parking, HSG B (1S, 2S, 21S, 22S, 24S)
0.430	98	Paved parking, HSG C (2S, 3S, 7S, 12S, 13S, 14S, 21S, 22S, 23S, 24S, 25S)
0.319	98	Paved parking, HSG D (2S, 3S, 12S, 13S, 14S)
0.115	98	Roofs, HSG A (1S, 23S)
0.027	98	Roofs, HSG B (22S, 23S)
0.042	98	Roofs, HSG C (23S, 24S, 25S)
0.098	98	Unconnected roofs, HSG A (2S, 22S)
0.009	98	Unconnected roofs, HSG B (2S)
0.064	98	Unconnected roofs, HSG C (2S, 21S)
0.081	98	Unconnected roofs, HSG D (2S)
1.575	30	Woods, Good, HSG A (1S, 2S, 21S)
0.443	55	Woods, Good, HSG B (2S, 21S, 22S, 23S)
6.140	70	Woods, Good, HSG C (1S, 2S, 3S, 12S, 13S, 21S, 22S, 23S, 24S, 25S)
0.566	77	Woods, Good, HSG D (2S, 3S)
18.980	66	TOTAL AREA

Soil Listing (all nodes)

Area	a Soil	Subcatchment
(acres) Group	Numbers
5.034	4 HSG A	1S, 2S, 21S, 22S, 23S, 24S
1.554	4 HSG B	1S, 2S, 7S, 21S, 22S, 23S, 24S
9.802	2 HSG C	1S, 2S, 3S, 7S, 12S, 13S, 14S, 21S, 22S, 23S, 24S, 25S
2.590) HSG D	2S, 3S, 12S, 13S, 14S
0.000	O Other	
18.98	0	TOTAL AREA

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					,		
HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchmer Numbers
2.693	0.931	3.126	1.623	0.000	8.374	>75% Grass cover, Good	3S, 7S, 12S,
							13S, 14S,
							21S,
							22S,
							23S,
							24S, 25S
0.552	0.144	0.430	0.319	0.000	1.446	Paved parking	1S, 2S,
							3S, 7S,
							12S,
							13S,
							14S,
							21S,
							22S, 23S,
							233, 24S, 25S
0.115	0.027	0.042	0.000	0.000	0.184	Roofs	1S, 22S,
0.110	0.021	0.012	0.000	0.000	0.101		23S,
							24S, 25S
0.098	0.009	0.064	0.081	0.000	0.252	Unconnected roofs	2S, 21S,
							22S
1.575	0.443	6.140	0.566	0.000	8.724	Woods, Good	1S, 2S,
							3S, 12S,
							13S,
							21S,
							22S,
							23S,
							24S, 25S
5.034	1.554	9.802	2.590	0.000	18.980	TOTAL AREA	

Ground Covers (all nodes)

22-051	Proposed	Analysis
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Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	1P	194.12	193.74	40.5	0.0094	0.012	18.0	0.0	0.0
2	12P	269.20	269.00	40.0	0.0050	0.012	15.0	0.0	0.0
3	13P	271.00	270.00	45.0	0.0222	0.012	18.0	0.0	0.0
4	14P	271.50	271.10	21.9	0.0183	0.012	12.0	0.0	0.0
5	23P	224.00	219.00	65.0	0.0769	0.012	15.0	0.0	0.0
6	24P	208.50	208.00	38.0	0.0132	0.012	15.0	0.0	0.0
7	25P	200.50	197.50	38.0	0.0789	0.012	18.0	0.0	0.0
8	101P	263.40	263.28	24.0	0.0050	0.012	24.0	0.0	0.0

Notes Listing (all nodes)

 Line#	Number	Noles
1	Project	While the analyses of Meetinghouse Road encompass a larger area only Subcatchments #1, #2, & #3 contribute runoff to Final Reaches #100 & #200. For this reason only these subcatchments were analyzed in this project. As such the resulting construction surrounding Meetinghouse Road results in a difference of 2.47 acres in contributing land area of the analyzed subcatchments between the pre-construction and post-construction analyses.

22-051 Proposed Analysis	Type III 24-hr 25Yr24Hr. Rainfall=5.85"
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Subcatchment1S: Subcatchment#1	Runoff Area=110,110 sf 12.30% Impervious Runoff Depth>0.87" Flow Length=145' Tc=7.9 min CN=47 Runoff=1.57 cfs 0.183 af
Subcatchment 2S: Subcatchment #2	Runoff Area=394,636 sf 7.55% Impervious Runoff Depth>2.31" Flow Length=1,118' Tc=37.4 min CN=66 Runoff=12.28 cfs 1.743 af
Subcatchment3S: Subcatchment#3	Runoff Area=28,734 sf 17.82% Impervious Runoff Depth>3.54" Tc=6.0 min CN=79 Runoff=2.68 cfs 0.195 af
Subcatchment7S: Subcatchment#7	Runoff Area=19,980 sf 10.51% Impervious Runoff Depth>3.24" Flow Length=721' Tc=12.7 min CN=76 Runoff=1.39 cfs 0.124 af
Subcatchment 12S: Subcatchment #1	2 Runoff Area=47,185 sf 2.86% Impervious Runoff Depth>2.86" Flow Length=485' Tc=18.5 min CN=72 Runoff=2.52 cfs 0.258 af
Subcatchment 13S: Subcatchment #1	3 Runoff Area=30,563 sf 5.93% Impervious Runoff Depth>2.86" Flow Length=400' Tc=17.4 min CN=72 Runoff=1.67 cfs 0.167 af
Subcatchment14S: Subcatchment#1	4 Runoff Area=6,937 sf 53.05% Impervious Runoff Depth>4.37" Tc=6.0 min CN=87 Runoff=0.78 cfs 0.058 af
Subcatchment21S: Subcatchment#2	1 Runoff Area=18,611 sf 24.79% Impervious Runoff Depth>3.05" Flow Length=312' Tc=14.4 min CN=74 Runoff=1.17 cfs 0.109 af
Subcatchment22S: Subcatchment#2	2 Runoff Area=41,415 sf 15.74% Impervious Runoff Depth>1.82" Flow Length=442' Tc=13.2 min CN=60 Runoff=1.50 cfs 0.144 af
Subcatchment23S: Subcatchment#2	3 Runoff Area=57,287 sf 2.85% Impervious Runoff Depth>2.59" Flow Length=348' Tc=10.1 min CN=69 Runoff=3.41 cfs 0.284 af
Subcatchment24S: Subcatchment#2	4 Runoff Area=63,055 sf 13.75% Impervious Runoff Depth>3.15" Flow Length=340' Tc=10.2 min CN=75 Runoff=4.60 cfs 0.380 af
Subcatchment 25S: Subcatchment #2	5 Runoff Area=8,261 sf 38.37% Impervious Runoff Depth>3.95" Tc=6.0 min CN=83 Runoff=0.85 cfs 0.062 af
Reach 2R: Reach #2 n=0.022	Avg. Flow Depth=0.40' Max Vel=4.05 fps Inflow=14.33 cfs 2.489 af L=31.0' S=0.0213 '/' Capacity=23.64 cfs Outflow=14.32 cfs 2.488 af
Reach 3R: Reach #3 n=0.022	Avg. Flow Depth=0.14' Max Vel=3.16 fps Inflow=2.68 cfs 0.195 af L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=2.34 cfs 0.194 af
Reach 7R: Reach #7 n=0.022	Avg. Flow Depth=0.10' Max Vel=3.07 fps Inflow=1.38 cfs 0.553 af L=772.0' S=0.0762 '/' Capacity=44.74 cfs Outflow=1.38 cfs 0.552 af
Reach 100R: Final Reach #100	Inflow=11.11 cfs 2.915 af Outflow=11.11 cfs 2.915 af

		ysisType III 24-hr25Yr24Hr. Rainfall=5.85"ying & Engineering (Rev: 4/13/23)Printed 4/13/2023605 © 2019 HydroCAD Software Solutions LLCPage 9
	Reach 101R: Overland Flo	w from R-101 Avg. Flow Depth=0.16' Max Vel=1.58 fps Inflow=1.38 cfs 0.553 af n=0.045 L=32.0' S=0.0469 '/' Capacity=17.15 cfs Outflow=1.38 cfs 0.553 af
	Reach 200R: Final Reach	#200 Inflow=3.87 cfs 0.149 af Outflow=3.87 cfs 0.149 af
0.157 af	Pond 1P: Pond #1 Primary=11.11 cfs 2.915 af S	Peak Elev=196.57' Storage=10,734 cf Inflow=17.77 cfs 3.222 af econdary=3.87 cfs 0.149 af Tertiary=0.00 cfs 0.000 af Outflow=16.00 cfs 3.221 af
	Pond 7P: Level Spreader	Peak Elev=264.08' Storage=519 cf Inflow=1.38 cfs 0.564 af Outflow=1.38 cfs 0.553 af
	Pond 12P: Pond #12	Peak Elev=270.20' Storage=0 cf Inflow=2.52 cfs 0.258 af 15.0" Round Culvert n=0.012 L=40.0' S=0.0050 '/' Outflow=2.52 cfs 0.258 af
	Pond 13P: Pond #13	Peak Elev=271.66' Storage=14 cf Inflow=2.07 cfs 0.225 af 18.0" Round Culvert n=0.012 L=45.0' S=0.0222 '/' Outflow=2.07 cfs 0.225 af
	Pond 14P: Pond #14	Peak Elev=271.95' Storage=6 cf Inflow=0.78 cfs 0.058 af 12.0" Round Culvert n=0.012 L=21.9' S=0.0183 '/' Outflow=0.78 cfs 0.058 af
	Pond 23P: Pond #23	Peak Elev=224.93' Storage=543 cf Inflow=3.41 cfs 0.284 af 15.0" Round Culvert n=0.012 L=65.0' S=0.0769 '/' Outflow=3.24 cfs 0.283 af
	Pond 24P: Pond #24	Peak Elev=209.72' Storage=27 cf Inflow=4.60 cfs 0.380 af Primary=4.59 cfs 0.380 af Secondary=0.00 cfs 0.000 af Outflow=4.59 cfs 0.380 af
	Pond 25P: Pond #25	Peak Elev=201.65' Storage=96 cf Inflow=5.28 cfs 0.442 af Primary=5.29 cfs 0.442 af Secondary=0.00 cfs 0.000 af Outflow=5.29 cfs 0.442 af
	Pond 101P: Rain Garden#	101 (Built) Peak Elev=267.16' Storage=9,858 cf Inflow=5.85 cfs 0.607 af Primary=1.38 cfs 0.564 af Secondary=0.00 cfs 0.000 af Outflow=1.38 cfs 0.564 af
	Pond 202P: Rain Garden v	v/ Infiltration #202 Peak Elev=215.77' Storage=9,669 cf Inflow=4.73 cfs 0.427 af Outflow=0.28 cfs 0.298 af
	Total Runoff	Area = 18.980 ac Runoff Volume = 3.707 af Average Runoff Depth = 2.34" 90.08% Pervious = 17.098 ac 9.92% Impervious = 1.882 ac

Summary for Subcatchment 1S: Subcatchment #1

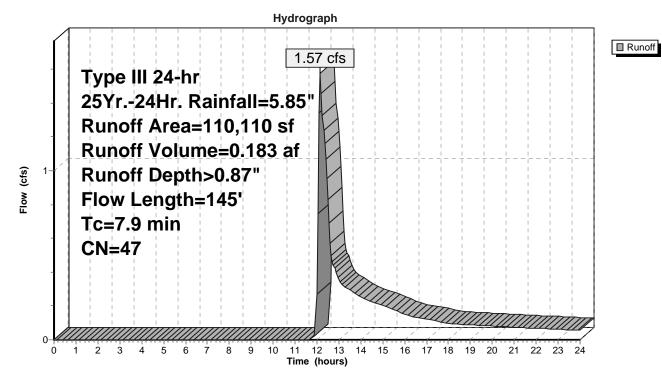
Runoff = 1.57 cfs @ 12.16 hrs, Volume= 0.183 af, Depth> 0.87"

A	rea (sf)	CN I	CN Description					
	4,968	98 I	Roofs, HSG	βA				
	26,401	39 :	>75% Gras	s cover, Go	bod, HSG A			
	8,559	98 I	Paved park	ing, HSG A				
	50,493			od, HSG A				
	7,119	61 >	>75% Gras	s cover, Go	bod, HSG B			
	16			ing, HSG B				
	7,711				ood, HSG C			
	4,843	70 \	<u> Noods, Go</u>	<u>od, HSG C</u>				
1	10,110	47 \	Veighted A	verage				
	96,567	8	37.70% Pe	rvious Area				
	13,543		2.30% Imp	pervious Ar	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.3	77	0.1823	0.17		Sheet Flow, Segment #1			
					Woods: Light underbrush n= 0.400 P2= 3.08"			
0.6	68	0.1332	1.82		Shallow Concentrated Flow, Segment #2			
					Woodland Kv= 5.0 fps			
7.9	145	Total						

22-051 Proposed Analysis

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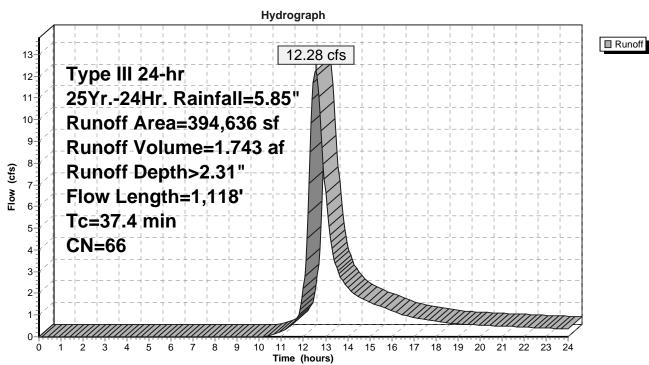


Summary for Subcatchment 2S: Subcatchment #2

Runoff = 12.28 cfs @ 12.55 hrs, Volume= 1.743 af, Depth> 2.31"

A	rea (sf)	CN I	Description		
	3,856	98	Jnconnecte	ed roofs, H	SG A
	389	98	Jnconnecte	ed roofs, H	SG B
	2,494			ed roofs, H	
	3,549	98	Jnconnecte	ed roofs, H	SG D
	74,868	39 :	>75% Gras	s cover, Go	bod, HSG A
	21,849	61 :	>75% Gras	s cover, Go	bod, HSG B
	48,235	74 :	>75% Gras	s cover, Go	bod, HSG C
	60,403	80 :	>75% Gras	s cover, Go	bod, HSG D
	8,483	98	Paved park	ing, HSG A	N Contraction of the second seco
	4,101			ing, HSG E	
	530	98	Paved park	ing, HSG C	
	6,387			ing, HSG E	
	17,053			od, HSG A	
	5,351			od, HSG B	
	19,594			od, HSG C	
	17,494	77 \	Noods, Go	od, HSG D	
3	94,636		Neighted A		
	64,847			rvious Area	
	29,789			ervious Are	а
	10,288		34.54% Un	connected	
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description
21.9	100	0.0200	0.08	(010)	Sheet Flow, Segment #1
21.5	100	0.0200	0.00		Woods: Light underbrush n= 0.400 P2= 3.08"
10.2	525	0.0295	0.86		Shallow Concentrated Flow, Segment #2
10.2	020	0.0200	0.00		Woodland Kv= 5.0 fps
2.1	172	0.0754	1.37		Shallow Concentrated Flow, Segment #3
					Woodland $Kv = 5.0$ fps
3.2	321	0.1151	1.70		Shallow Concentrated Flow, Segment #4
					Woodland Kv= 5.0 fps
37.4	1,118	Total			÷

22-051 Proposed Analysis



Subcatchment 2S: Subcatchment #2

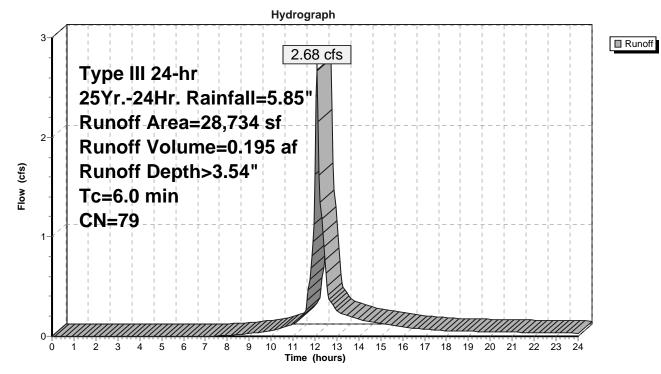
Summary for Subcatchment 3S: Subcatchment #3

Runoff = 2.68 cfs @ 12.09 hrs, Volume= 0.195 af, Depth> 3.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25Yr.-24Hr. Rainfall=5.85"

Area (sf)	CN	Description					
583	74	>75% Grass cover, Good, HSG C					
5,749	80	>75% Grass cover, Good, HSG D					
822	98	Paved parking, HSG C					
4,299	98	Paved parking, HSG D					
10,131	70	Woods, Good, HSG C					
7,150	77	Woods, Good, HSG D					
28,734	79	Weighted Average					
23,613		82.18% Pervious Area					
5,121		17.82% Impervious Area					
Tc Length							
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)					
6.0		Direct Entry, Direct					

Subcatchment 3S: Subcatchment #3



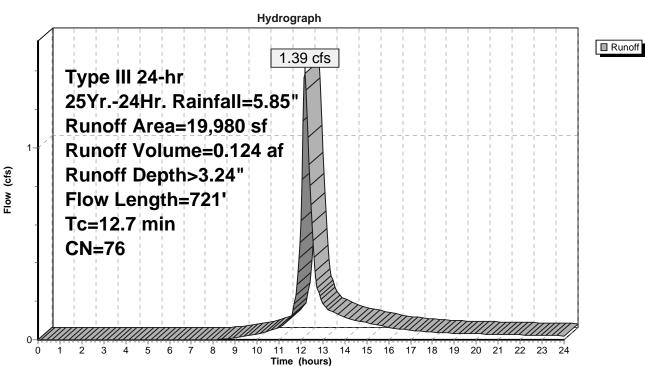
Summary for Subcatchment 7S: Subcatchment #7

Runoff = 1.39 cfs @ 12.18 hrs, Volume= 0.124 af, Depth> 3.24"

A	rea (sf)	CN D	escription		
	963	61 >	75% Gras	s cover, Go	bod, HSG B
	16,917	74 >	75% Gras	s cover, Go	bod, HSG C
	2,100	98 P	aved park	ing, HSG C	
	19,980	76 V	/eighted A	verage	
	17,880			vious Area	
	2,100	1	0.51% Imp	pervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.5	100	0.1600	0.17		Sheet Flow, Segment #1
					Woods: Light underbrush n= 0.400 P2= 3.08"
0.9	116	0.1726	2.08		Shallow Concentrated Flow, Segment #2
					Woodland Kv= 5.0 fps
0.5	76	0.1570	2.77		Shallow Concentrated Flow, Segment #3
					Short Grass Pasture Kv= 7.0 fps
0.2	64	0.0625	5.08		Shallow Concentrated Flow, Segment #4
					Paved Kv= 20.3 fps
0.9	100	0.0700	1.85		Shallow Concentrated Flow, Segment #5
					Short Grass Pasture Kv= 7.0 fps
0.7	265	0.0135	6.24	37.41	
					Bot.W=4.00' D=1.00' Z= 2.0 '/' Top.W=8.00'
					n= 0.022 Earth, clean & straight
12.7	721	Total			

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Subcatchment 7S: Subcatchment #7

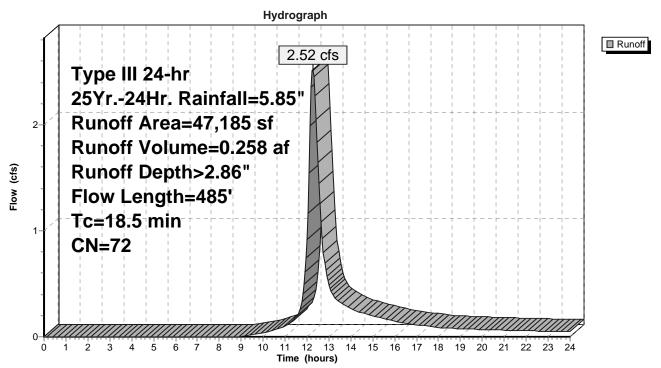
Summary for Subcatchment 12S: Subcatchment #12

Runoff = 2.52 cfs @ 12.26 hrs, Volume= 0.258 af, Depth> 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25Yr.-24Hr. Rainfall=5.85"

A	rea (sf)	CN E	CN Description						
	7,608	74 >	74 >75% Grass cover, Good, HSG C						
	4,476	80 >	75% Gras	s cover, Go	bod, HSG D				
	103	98 F	Paved park	ing, HSG C					
	1,246	98 F	Paved park	ing, HSG D)				
	33,752	70 V	Voods, Go	od, HSG C					
	47,185	72 V	Veighted A	verage					
	45,836	9	7.14% Per	vious Area					
	1,349	2	86% Impe	ervious Are	a				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
14.1	100	0.0600	0.12		Sheet Flow, Segment #1				
					Woods: Light underbrush n= 0.400 P2= 3.08"				
3.0	253	0.0790	1.41		Shallow Concentrated Flow, Segment #2				
					Woodland Kv= 5.0 fps				
1.4	132	0.0531	1.61		Shallow Concentrated Flow, Segment #3				
					Short Grass Pasture Kv= 7.0 fps				
18.5	485	Total							

Subcatchment 12S: Subcatchment #12



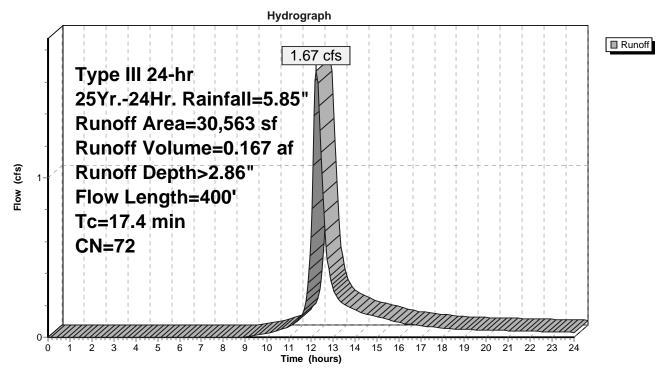
Summary for Subcatchment 13S: Subcatchment #13

Runoff = 1.67 cfs @ 12.25 hrs, Volume= 0.167 af, Depth> 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25Yr.-24Hr. Rainfall=5.85"

A	rea (sf)	CN [CN Description						
	4,701	74 >	75% Gras	s cover, Go	ood, HSG C				
	83				ood, HSG D				
	940	98 F	Paved park	ing, HSG C					
	872			ing, HSG D					
	23,967	70 V	Voods, Go	od, HSG C					
	30,563	72 V	Veighted A	verage					
	28,751	ç	94.07% Per	vious Area					
	1,812	5	5.93% Impe	ervious Area	a				
Tc	Length	Slope		Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
14.1	100	0.0600	0.12		Sheet Flow, Segment #1				
					Woods: Light underbrush n= 0.400 P2= 3.08"				
3.3	300	0.0901	1.50		Shallow Concentrated Flow, Segment #2				
					Woodland Kv= 5.0 fps				
17.4	400	Total							

Subcatchment 13S: Subcatchment #13



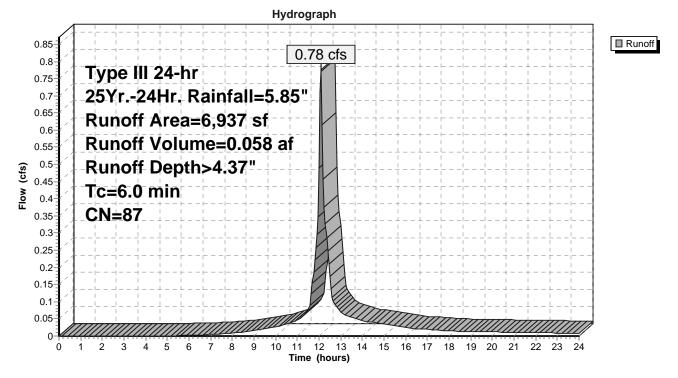
Summary for Subcatchment 14S: Subcatchment #14

Runoff = 0.78 cfs @ 12.09 hrs, Volume= 0.058 af, Depth> 4.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25Yr.-24Hr. Rainfall=5.85"

A	rea (sf)	CN	Description					
	3,257	74	>75% Gras	s cover, Go	bod, HSG C			
	2,589		Paved parking, HSG C					
	1,091	98	Paved parking, HSG D					
	6,937	87	Weighted A	verage				
	3,257		46.95% Per	vious Area	1			
	3,680		53.05% Imp	pervious Ar	ea			
То	Longth	Slope	Velocity	Capacity	Description			
Tc (min)	Length	Slope			Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry, Direct			

Subcatchment 14S: Subcatchment #14



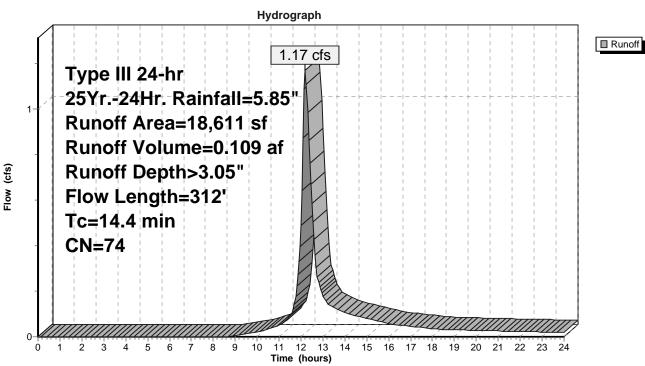
Summary for Subcatchment 21S: Subcatchment #21

Runoff = 1.17 cfs @ 12.20 hrs, Volume= 0.109 af, Depth> 3.05"

А	vrea (sf)	CN E	CN Description					
	442	39 >	75% Gras	s cover, Go	bod, HSG A			
	1,531			ing, HSG A				
	1,079	30 V	Voods, Go	od, HSG A				
	3,013	61 >	75% Gras	s cover, Go	bod, HSG B			
	1,338	98 F	Paved park	ing, HSG E	3			
	544			od, HSG B				
	273			ed roofs, H				
	8,262				bod, HSG C			
	1,471			ing, HSG C				
	658	70 V	70 Woods, Good, HSG C					
	18,611	74 V	Veighted A	verage				
	13,998	7	'5.21% Pei	vious Area	l			
	4,613			pervious Ar	ea			
	273	5	.92% Unco	onnected				
_		. .						
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
11.5	100	0.1000	0.14		Sheet Flow, Segment #1			
					Woods: Light underbrush n= 0.400 P2= 3.08"			
0.3	30	0.1007	1.59		Shallow Concentrated Flow, Segment #2			
					Woodland Kv= 5.0 fps			
0.5	76	0.2889	2.69		Shallow Concentrated Flow, Segment #3			
. .	/ a -				Woodland Kv= 5.0 fps			
2.1	106	0.0282	0.84		Shallow Concentrated Flow, Segment #4			
					Woodland Kv= 5.0 fps			
14.4	312	Total						

22-051 Proposed Analysis

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Subcatchment 21S: Subcatchment #21

Summary for Subcatchment 22S: Subcatchment #22

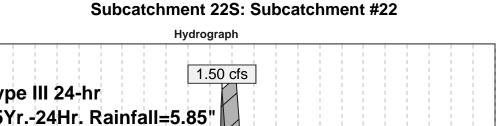
Runoff = 1.50 cfs @ 12.20 hrs, Volume= 0.144 af, Depth> 1.82"

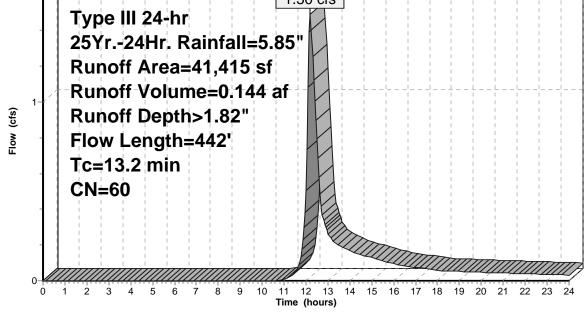
Area (sf) CN	Description							
432	2 98	98 Unconnected roofs, HSG A							
15,020			>75% Grass cover, Good, HSG A						
3,076	<u> </u>	Paved park	ing, HSG A						
395	5 98	Roofs, HSC	Β́Β						
2,730				bod, HSG B					
212		Paved park							
6,235		Woods, Go							
2,405		Paved park							
10,910) 70	Woods, Go	od, HSG C						
41,415		Weighted A							
34,895		84.26% Pe							
6,520		15.74% lm		ea					
432	2	6.63% Unconnected							
To Lower			0	Description					
Tc Lengt			Capacity	Description					
(min) (fee	,	/ /	(cfs)						
7.6 6	0.127	72 0.15		Sheet Flow, Segment #1					
3.6 3	3 0.196	60 0.15		Woods: Light underbrush n= 0.400 P2= 3.08" Sheet Flow, Segment #2					
3.0 3	5 0.190	0.15		Woods: Light underbrush n= 0.400 P2= 3.08"					
1.3 23	1 0.335	58 2.90		Shallow Concentrated Flow, Segment #3					
1.5 25	0.55	2.90		Woodland Kv= 5.0 fps					
0.7 11	1 0.157	79 2.78		Shallow Concentrated Flow, Segment #4					
0.7 11	1 0.101	2.10		Short Grass Pasture Kv= 7.0 fps					
13.2 44	2 Total								

22-051 Proposed Analysis

Runoff

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Summary for Subcatchment 23S: Subcatchment #23

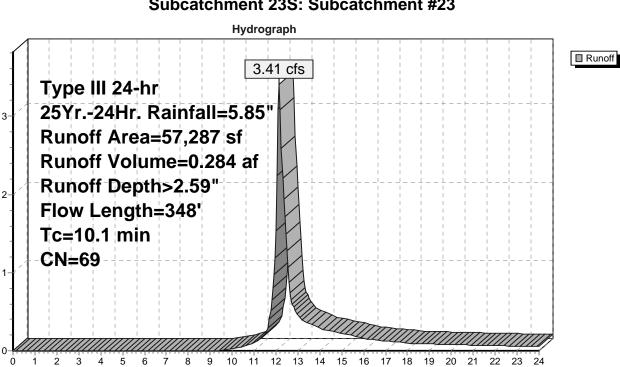
Runoff = 3.41 cfs @ 12.15 hrs, Volume= 0.284 af, Depth> 2.59"

A	rea (sf)	CN	Description					
	61	98	Roofs, HSG	βA				
	575	39	>75% Gras	s cover, Go	ood, HSG A			
	767	98	Roofs, HSG	βB				
	4,895	61	>75% Gras	s cover, Go	ood, HSG B			
	7,160	55	Woods, Go	od, HSG B				
	529	98	Roofs, HSG	G C				
	10,113				ood, HSG C			
	276		Paved park					
	32,911	70	70 Woods, Good, HSG C					
	57,287	69	Weighted A	verage				
	55,654		97.15% Per	vious Area				
	1,633		2.85% lmpe	ervious Area	а			
Tc	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)		(cfs)				
8.7	100	0.2000	0.19		Sheet Flow, Segment #1			
					Woods: Light underbrush n= 0.400 P2= 3.08"			
1.1	184	0.3147	2.80		Shallow Concentrated Flow, Segment #2			
					Woodland Kv= 5.0 fps			
0.3	64	0.2814	3.71		Shallow Concentrated Flow, Segment #3			
					Short Grass Pasture Kv= 7.0 fps			
10.1	348	Total						

22-051 Proposed Analysis

Flow (cfs)

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Time (hours)

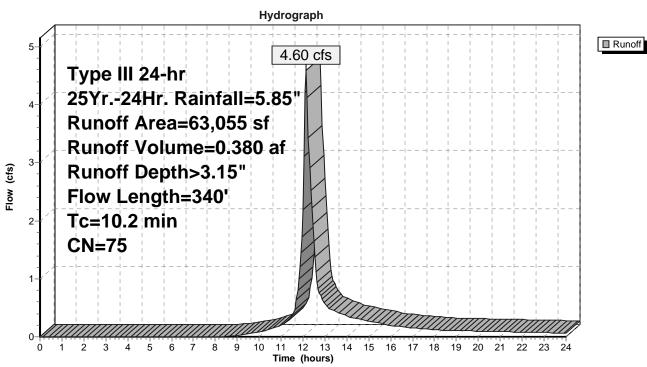
Subcatchment 23S: Subcatchment #23

Summary for Subcatchment 24S: Subcatchment #24

Runoff = 4.60 cfs @ 12.15 hrs, Volume= 0.380 af, Depth> 3.15"

A	rea (sf)	CN [Description				
	2,405	98 F	B Paved parking, HSG A				
	601	98 F	Paved park	ing, HSG B	5		
	1,058	98 F	Roofs, HSC	G C			
	24,321			,	ood, HSG C		
	4,603			ing, HSG C			
	30,067	70 \	Woods, Good, HSG C				
	63,055	75 \	Veighted A	verage			
	54,388	8	36.25% Pe	vious Area			
	8,667	1	3.75% Imp	pervious Ar	ea		
т.	1	0	Mala di	0	Description		
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
8.5	100	0.2100	0.20		Sheet Flow, Segment #1		
					Woods: Light underbrush n= 0.400 P2= 3.08"		
1.1	175	0.2852	2.67		Shallow Concentrated Flow, Segment #2		
					Woodland Kv= 5.0 fps		
0.6	65	0.1228	1.75		Shallow Concentrated Flow, Segment #3		
					Woodland Kv= 5.0 fps		
10.2	340	Total					

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Subcatchment 24S: Subcatchment #24

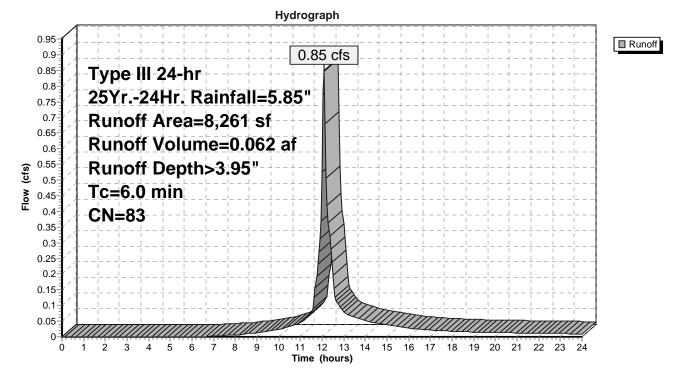
Summary for Subcatchment 25S: Subcatchment #25

Runoff = 0.85 cfs @ 12.09 hrs, Volume= 0.062 af, Depth> 3.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25Yr.-24Hr. Rainfall=5.85"

A	rea (sf)) CN Description							
	257	98	Roofs, HSG C						
	4,481	74	>75% Gras	s cover, Go	ood, HSG C				
	2,913	98	Paved parking, HSG C						
	610	70	Woods, Good, HSG C						
	8,261	83	Weighted A	verage					
	5,091	61.63% Pervious Area							
	3,170		38.37% Imp	pervious Ar	ea				
Tc	Length	Slope		Capacity	Description				
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
6.0					Direct Entry, Direct Entry				

Subcatchment 25S: Subcatchment #25



Summary for Reach 2R: Reach #2

 [62] Hint: Exceeded Reach 3R OUTLET depth by 0.32' @ 12.60 hrs

 [62] Hint: Exceeded Reach 7R OUTLET depth by 0.30' @ 12.50 hrs

 Inflow Area =
 12.122 ac, 8.30% Impervious, Inflow Depth > 2.46" for 25Yr.-24Hr. event

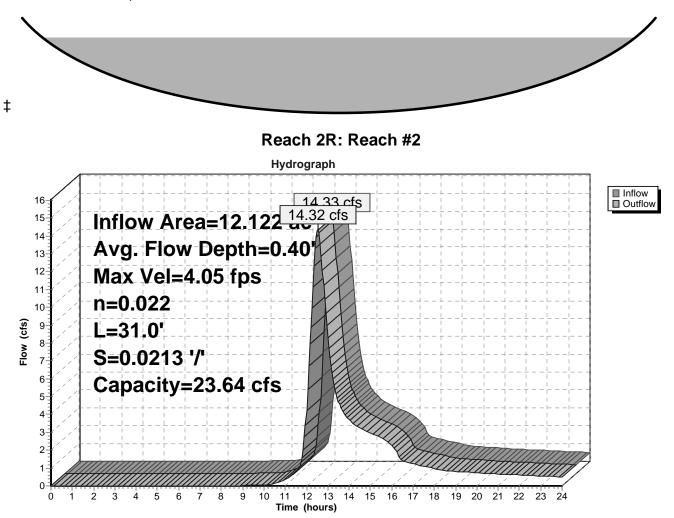
 Inflow =
 14.33 cfs @ 12.52 hrs, Volume=
 2.489 af

 Outflow =
 14.32 cfs @ 12.53 hrs, Volume=
 2.488 af, Atten= 0%, Lag= 0.2 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Max. Velocity= 4.05 fps, Min. Travel Time= 0.1 min Avg. Velocity = 1.73 fps, Avg. Travel Time= 0.3 min

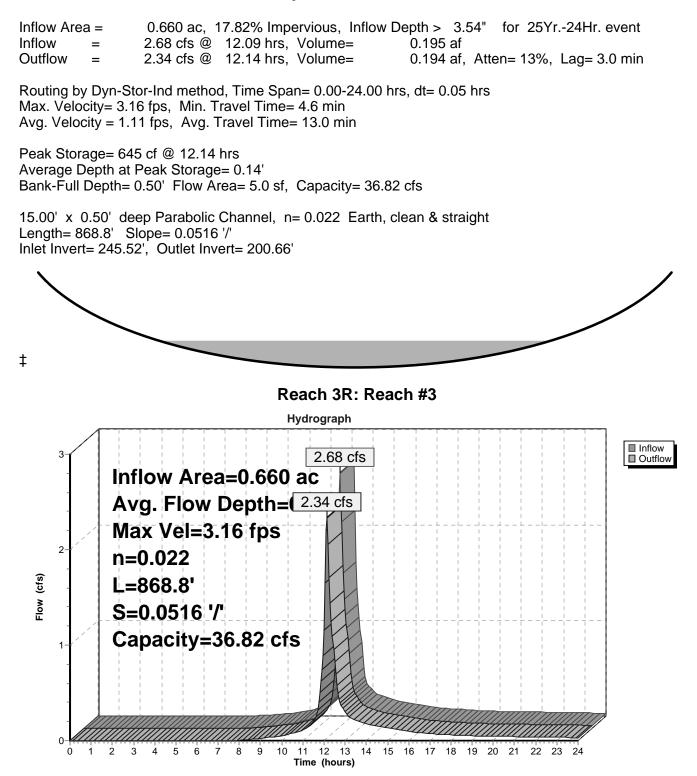
Peak Storage= 110 cf @ 12.53 hrs Average Depth at Peak Storage= 0.40' Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 23.64 cfs

15.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 31.0' Slope= 0.0213 '/' Inlet Invert= 200.66', Outlet Invert= 200.00'



22-051 Proposed AnalysisType III 24-hr 25Yr.-24Hr. Rainfall=5.85"Prepared by Berry Surveying & Engineering (Rev: 4/13/23)Printed 4/13/2023HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLCPage 30

Summary for Reach 3R: Reach #3



Summary for Reach 7R: Reach #7

[61] Hint: Exceeded Reach 101R outlet invert by 0.10' @ 12.90 hrs

 Inflow Area =
 2.403 ac,
 8.54% Impervious, Inflow Depth > 2.76" for 25Yr.-24Hr. event

 Inflow =
 1.38 cfs @
 12.81 hrs, Volume=
 0.553 af

 Outflow =
 1.38 cfs @
 12.89 hrs, Volume=
 0.552 af, Atten= 0%, Lag= 5.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Max. Velocity= 3.07 fps, Min. Travel Time= 4.2 min Avg. Velocity = 2.08 fps, Avg. Travel Time= 6.2 min

Peak Storage= 347 cf @ 12.89 hrs Average Depth at Peak Storage= 0.10' Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 44.74 cfs

2 3

5 6

4

7 8

9

Time (hours)

15.00' x 0.50' deep Parabolic Channel, n= 0.022 Earth, clean & straight Length= 772.0' Slope= 0.0762 '/' Inlet Invert= 259.50', Outlet Invert= 200.66'

‡ Reach 7R: Reach #7 Hydrograph Inflow 1 38 cfs Outflow Inflow Area=2.403 a Avg. Flow Depth=0.10 Max Vel=3.07 fps n=0.022 (cfs) L=772.0' Flow S=0.0762 '/' Capacity=44.74 cfs 0-

10 11 12 13 14 15 16 17 18 19 20 21

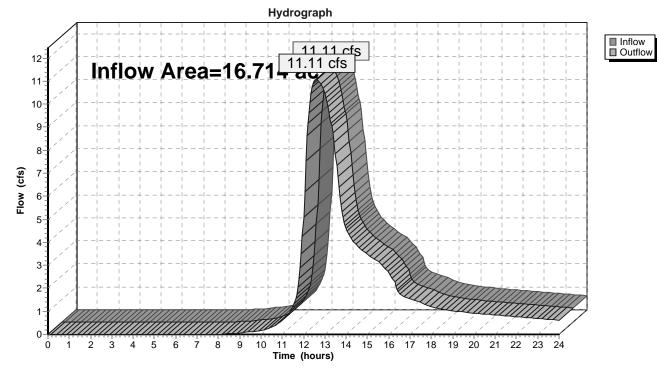
22 23

Summary for Reach 100R: Final Reach #100

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	16.714 ac, 10.14% Impervious, Inflow Dep	oth > 2.09" for 25Yr24Hr. event
Inflow	=	11.11 cfs @ 12.62 hrs, Volume= 2	2.915 af
Outflow	=	11.11 cfs @ 12.62 hrs, Volume= 2	2.915 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



Reach 100R: Final Reach #100

Summary for Reach 101R: Overland Flow from R-101

2.403 ac, 8.54% Impervious, Inflow Depth > 2.76" for 25Yr.-24Hr. event

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area =

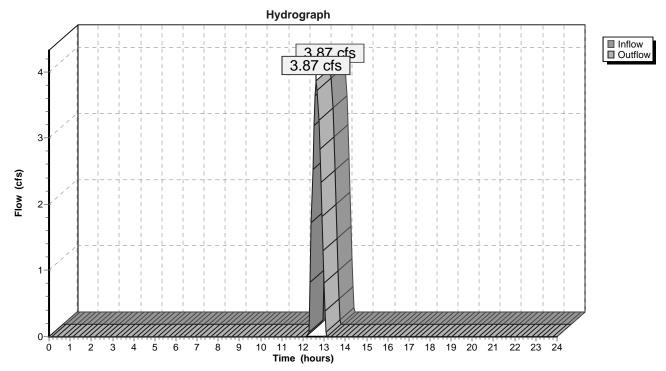
1.38 cfs @ 12.84 hrs, Volume= 0.553 af Inflow = 1.38 cfs @ 12.81 hrs, Volume= Outflow 0.553 af, Atten= 0%, Lag= 0.0 min = Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Max. Velocity= 1.58 fps, Min. Travel Time= 0.3 min Avg. Velocity = 1.07 fps, Avg. Travel Time= 0.5 min Peak Storage= 28 cf @ 12.81 hrs Average Depth at Peak Storage= 0.16' Bank-Full Depth= 0.50' Flow Area= 5.0 sf, Capacity= 17.15 cfs 15.00' x 0.50' deep Parabolic Channel, n= 0.045 Length= 32.0' Slope= 0.0469 '/' Inlet Invert= 261.00', Outlet Invert= 259.50' ‡ Reach 101R: Overland Flow from R-101 Hydrograph Inflow 1 38 cfs Outflow Inflow Area=2.403 a Avg. Flow Depth=0.16 Max Vel=1.58 fps n=0.045 (cfs) L=32.0' Flow S=0.0469 '/' Capacity=17.15 cfs 0-9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 2 3 4 5 6 7 8 Time (hours)

Summary for Reach 200R: Final Reach #200

[40] Hint: Not Described (Outflow=Inflow)

Inflow	=	3.87 cfs @	12.62 hrs, Volume=	0.149 af
Outflow	=	3.87 cfs @	12.62 hrs, Volume=	0.149 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



Reach 200R: Final Reach #200

Summary for Pond 1P: Pond #1

Inflow Area =	16.714 ac, 10.14% Impervious, Inflow Depth > 2.31" for 25Yr24Hr. event
Inflow =	17.77 cfs @ 12.44 hrs, Volume= 3.222 af
Outflow =	16.00 cfs @ 12.62 hrs, Volume= 3.221 af, Atten= 10%, Lag= 10.6 min
Discarded =	1.02 cfs @ 12.45 hrs, Volume= 0.157 af
Primary =	11.11 cfs @ 12.62 hrs, Volume= 2.915 af
Secondary =	3.87 cfs @ 12.62 hrs, Volume= 0.149 af
Tertiary =	0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 196.57' @ 12.62 hrs Surf.Area= 14,717 sf Storage= 10,734 cf Flood Elev= 197.00' Surf.Area= 14,717 sf Storage= 17,094 cf

Plug-Flow detention time= 6.0 min calculated for 3.214 af (100% of inflow) Center-of-Mass det. time= 5.8 min (876.3 - 870.5)

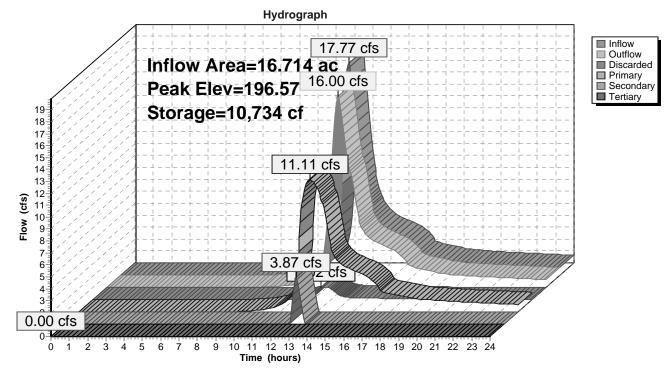
Volume	Invert	Avail.S	Storage	Storage Descriptio	n		
#1	194.12'	24	,325 cf	Existing Depress	ion (Irregular)List	ted below (Recalc)	
#2	196.25'		631 cf			ted below (Recalc) - In	npervious
		24	,956 cf	Total Available Sto	orage	· · ·	
Elevatio		.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee	, ,	sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
194.1		12	14.0	0	0	12	
196.0		6,700	469.6	4,384	4,384	17,551	
196.5		4,717	797.7	5,224	9,608	50,641	
197.5	0 14	4,717	797.7	14,717	24,325	51,439	
Elevatio		.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(fee		sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
`	1	<i>i i</i>		· · ·	· · ·		
196.2		15	15.0	0	0	15	
196.5		139	48.8	17	17	187	
197.0		314	87.9	110	127	613	
198.0	0	721	136.7	504	631	1,493	
Device	Routing	Inve	ert Outle	et Devices			
#1	Discarded	194.1	2' 3.00	0 in/hr Infiltration 3	BIN/HR over Surf	ace area	
#2	Primary	194.1	2' 18.0	" Round 18" HDPE			
	,		L= 4	0.5' CPP, square e	edge headwall. K	e= 0.500	
						0.0094 '/' Cc= 0.900	
			n= 0	.012, Flow Area= 1	.77 sf		
#3	Secondary	196.3	0' 10.0 '	long x 40.0' bread	dth Overflow Over	er DW	
				d (feet) 0.20 0.40			
			Coef	. (English) 2.68 2.1	70 2.70 2.64 2.6	3 2.64 2.64 2.63	
#4	Tertiary	196.8		' long x 24.0' bread			
	-			d (feet) 0.20 0.40			
				. (English) 2.68 2.1			

Discarded OutFlow Max=1.02 cfs @ 12.45 hrs HW=196.51' (Free Discharge) **1=Infiltration 3IN/HR** (Exfiltration Controls 1.02 cfs)

Primary OutFlow Max=11.10 cfs @ 12.62 hrs HW=196.57' TW=0.00' (Dynamic Tailwater) **2=18" HDPE** (Inlet Controls 11.10 cfs @ 6.28 fps)

Secondary OutFlow Max=3.84 cfs @ 12.62 hrs HW=196.57' TW=0.00' (Dynamic Tailwater) -3=Overflow Over DW (Weir Controls 3.84 cfs @ 1.40 fps)

Tertiary OutFlow Max=0.00 cfs @ 0.00 hrs HW=194.12' TW=0.00' (Dynamic Tailwater) 4=Overflow Over Rt 9 (Controls 0.00 cfs)



Pond 1P: Pond #1

Summary for Pond 7P: Level Spreader

Inflow Area =	2.403 ac,	8.54% Impervious, Inflow	Depth > 2.82"	for 25Yr24Hr. event
Inflow =	1.38 cfs @	12.83 hrs, Volume=	0.564 af	
Outflow =	1.38 cfs @	12.84 hrs, Volume=	0.553 af, Atte	n= 0%, Lag= 0.4 min
Primary =	1.38 cfs @	12.84 hrs, Volume=	0.553 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 264.08' @ 12.84 hrs Surf.Area= 496 sf Storage= 519 cf

Plug-Flow detention time= 14.6 min calculated for 0.553 af (98% of inflow) Center-of-Mass det. time= 4.5 min (902.5 - 898.0)

Volume	Inv	vert Avail	.Storage	Storage Description	on		
#1	262.	50'	531 cf	Open Water Stor	age (Irregular) Lis	ted below (Recalc)	
Elevatio (fee 262.5	et)	Surf.Area (sq-ft) 176	Perim. (feet) 53.0	Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0	Wet.Area (sq-ft) 176	
263.0 264.0 264.7	00	262 496 496	59.3 88.2 88.2	109 373 50	109 482 531	239 586 595	
Device	Routing	Inv	vert Outle	et Devices			
#1	Primary	264.	Head 2.50 Coef	3.00 3.50	0.60 0.80 1.00	1.20 1.40 1.60 1.80 2.0 66 2.70 2.77 2.89 2.88	

Primary OutFlow Max=1.38 cfs @ 12.84 hrs HW=264.08' TW=261.16' (Dynamic Tailwater)

Hydrograph Inflow Area=2.403 1.38 cfs InflowPrimary Peak Elev=264.08' Storage=519 cf 1 Flow (cfs) 0-10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 Time (hours) 1 2 ò Ś 4 5 7 8 ġ 6

Pond 7P: Level Spreader

Summary for Pond 12P: Pond #12

[44] Hint: Outlet device #1 is below defined storage

Inflow Area =	= 1.083 ac,	2.86% Impervious, Inflow I	Depth > 2.86"	for 25Yr24Hr. event
Inflow =	2.52 cfs @	12.26 hrs, Volume=	0.258 af	
Outflow =	2.52 cfs @	12.26 hrs, Volume=	0.258 af, Atte	en= 0%, Lag= 0.0 min
Primary =	2.52 cfs @	12.26 hrs, Volume=	0.258 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 270.20' @ 12.26 hrs Surf.Area= 114 sf Storage= 0 cf

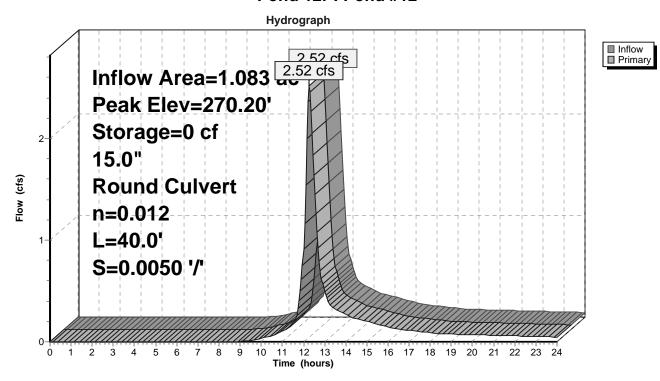
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 0.0 min (844.1 - 844.1)

Volume	Inv	ert Avai	I.Storage	Storage Description	on		
#1	270.2	20'	312 cf	Open Water Stor	a ge (Irregular) List	ed below (Recalc)	
Elevatior (feet	•	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
270.20)	114	48.0	0	0	114	
271.00)	114	48.0	91	91	152	
272.00)	348	122.2	220	312	1,161	
	<u>Routing</u> Primary	 269.	.20' 15.0 L= 4 Inlet	et Devices " Round 15" HDP 0.0' CPP, end-sed / Outlet Invert= 26 .012, Flow Area=	ction conforming to 9.20' / 269.00' S=	fill, Ke= 0.500 0.0050 '/' Cc= 0.900	

Primary OutFlow Max=2.80 cfs @ 12.26 hrs HW=270.20' TW=266.51' (Dynamic Tailwater) **1=15" HDPE N-12** (Barrel Controls 2.80 cfs @ 3.64 fps)

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Summary for Pond 13P: Pond #13

Inflow Area =	0.861 ac, 14.65% Impervious	, Inflow Depth > 3.14" for 25Yr24Hr. event
Inflow =	2.07 cfs @ 12.21 hrs, Volum	e= 0.225 af
Outflow =	2.07 cfs @ 12.21 hrs, Volum	e= 0.225 af, Atten= 0%, Lag= 0.1 min
Primary =	2.07 cfs @ 12.21 hrs, Volum	e= 0.225 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 271.66' @ 12.21 hrs Surf.Area= 35 sf Storage= 14 cf

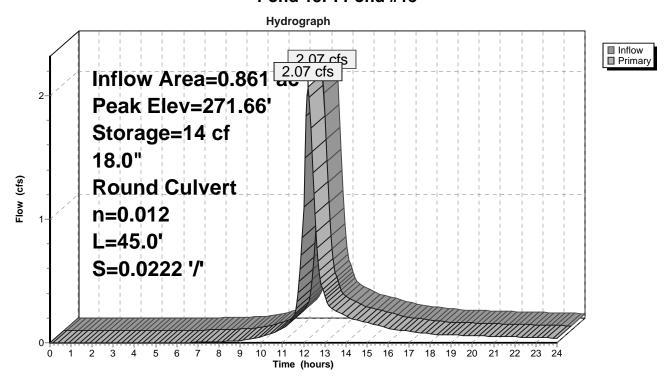
Plug-Flow detention time= 0.2 min calculated for 0.225 af (100% of inflow) Center-of-Mass det. time= 0.1 min (831.0 - 830.9)

Volume	Inv	ert Avail	.Storage	Storage Description	on		
#1	271.	00'	491 cf	Open Water Stor	age (Irregular)Lis	sted below (Recalc)	
Elevatio (fee 271.0 272.0 273.0 274.0	et) 00 00 00	Surf.Area (sq-ft) 10 54 204 499	Perim. (feet) 11.6 29.4 75.4 169.9	Inc.Store (cubic-feet) 0 29 121 341	Cum.Store (cubic-feet) 0 29 150 491	Wet.Area (sq-ft) 10 72 459 2,308	
Device	Routing	Inv	ert Outle	et Devices			
#1	Primary	271.	L= 4 Inlet				

Primary OutFlow Max=2.06 cfs @ 12.21 hrs HW=271.66' TW=266.37' (Dynamic Tailwater) **1=18" HDPE N-12** (Inlet Controls 2.06 cfs @ 2.76 fps)

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Summary for Pond 14P: Pond #14

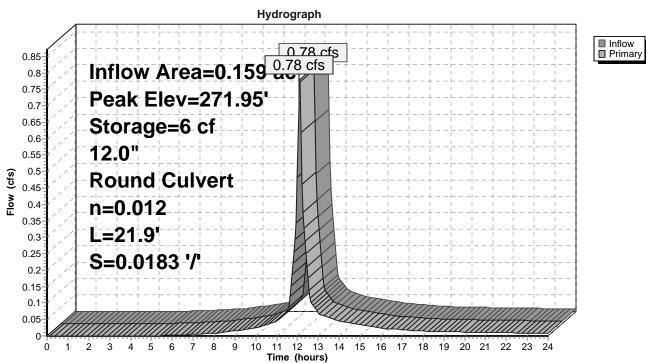
Inflow Area =	0.159 ac, 53.05% Impervious, Inflow Depth > 4.37" for 25Yr24Hr. event	
Inflow =	0.78 cfs @ 12.09 hrs, Volume= 0.058 af	
Outflow =	0.78 cfs @ 12.09 hrs, Volume= 0.058 af, Atten= 0%, Lag= 0.1 min	
Primary =	0.78 cfs @ 12.09 hrs, Volume= 0.058 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 271.95' @ 12.10 hrs Surf.Area= 13 sf Storage= 6 cf

Plug-Flow detention time= 0.4 min calculated for 0.058 af (100% of inflow) Center-of-Mass det. time= 0.3 min (795.4 - 795.1)

Volume	Invert	Avail.Storage	Storage Description
#1	271.50'	85 cf	4.00'D x 6.75'H Catch Basin
Device	Routing	Invert Out	et Devices
#1	Primary	L= 2 Inlet	P" Round 12" HDPE N-12 21.9' CPP, end-section conforming to fill, Ke= 0.500 t / Outlet Invert= 271.50' / 271.10' S= 0.0183 '/' Cc= 0.900 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.70 cfs @ 12.09 hrs HW=271.95' TW=271.61' (Dynamic Tailwater) **1=12" HDPE N-12** (Outlet Controls 0.70 cfs @ 3.05 fps)



Pond 14P: Pond #14

Summary for Pond 23P: Pond #23

Inflow Area =	1.315 ac,	2.85% Impervious, Inflow D	Depth > 2.59" for 25Yr24Hr. event
Inflow =	3.41 cfs @	12.15 hrs, Volume=	0.284 af
Outflow =	3.24 cfs @	12.19 hrs, Volume=	0.283 af, Atten= 5%, Lag= 2.2 min
Primary =	3.24 cfs @	12.19 hrs, Volume=	0.283 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 224.93' @ 12.19 hrs Surf.Area= 689 sf Storage= 543 cf Flood Elev= 227.00' Surf.Area= 1,295 sf Storage= 2,535 cf

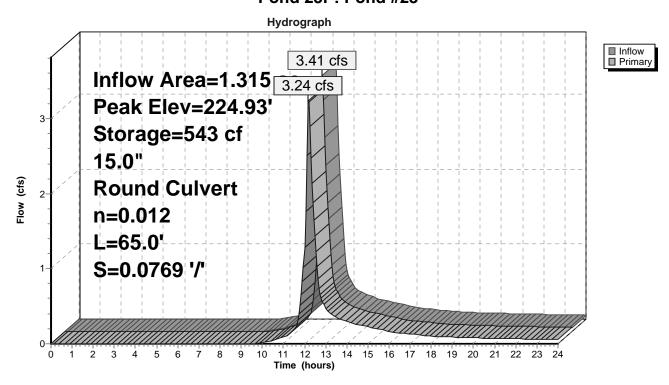
Plug-Flow detention time= 6.6 min calculated for 0.282 af (99% of inflow) Center-of-Mass det. time= 4.2 min (848.7 - 844.5)

Volume	Inv	ert Avai	I.Storage	Storage Description			
#1	224.	00'	2,535 cf	Open Water Stor	r age (Irregular) Lis	ted below (Recalc)	
Elevatio (fee	t)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
224.0		480	105.8	0	0	480	
225.0	0	705	118.4	589	589	731	
226.0	0	954	131.0	826	1,415	1,011	
227.0	0	1,295	172.2	1,120	2,535	2,016	
Device	Routing Invert Outlet Devices						
#1	Primary	224	.00' 15.0	" Round 15" HDP	PE N-12		
	,		L= 6	5.0' CPP, square	edge headwall, K	e= 0.500	
						= 0.0769 '/' Cc= 0.900	
			n= 0	.012, Flow Area=	1.23 sf		
Driment OutFlow May 2.20 of @ 12.40 bro LIW 224.02 TW 244.27 (Durantia Taihustar)							

Primary OutFlow Max=3.20 cfs @ 12.19 hrs HW=224.93' TW=214.37' (Dynamic Tailwater) **1=15" HDPE N-12** (Inlet Controls 3.20 cfs @ 3.28 fps)

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Pond 23P: Pond #23



Summary for Pond 24P: Pond #24

Inflow Area =	1.448 ac, 13.75% Impervious, Inflow De	epth > 3.15" for 25Yr24Hr. event
Inflow =	4.60 cfs @ 12.15 hrs, Volume=	0.380 af
Outflow =	4.59 cfs @ 12.15 hrs, Volume=	0.380 af, Atten= 0%, Lag= 0.2 min
Primary =	4.59 cfs @ 12.15 hrs, Volume=	0.380 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 209.72' @ 12.15 hrs Surf.Area= 56 sf Storage= 27 cf Flood Elev= 212.00' Surf.Area= 370 sf Storage= 487 cf

Plug-Flow detention time= 0.1 min calculated for 0.379 af (100% of inflow) Center-of-Mass det. time= 0.1 min (830.4 - 830.4)

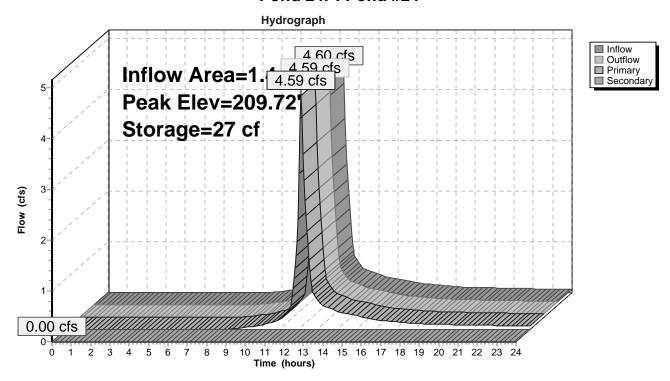
Volume	Inve	ert Avail.S	Storage	rage Storage Description			
#1	208.5	50'	672 cf	Open Water Stor	age (Irregular)Lis	sted below (Recale	c)
Elevatic (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
208.5	50	2	6.0	0	0	2	
209.0	0	13	16.3	3	3	21	
210.0	0	81	43.8	42	45	156	
211.0	0	225	81.3	147	192	535	
212.0	0	370	90.3	295	487	684	
212.5	50	370	90.3	185	672	730	
Device	Routing	Inve	rt Outle	et Devices			
#1	Primary	208.5	0' 15.0	" Round 15" HDP	E N-12		
#2	Seconda	ıry 212.2	Inlet n= 0 5' 5.0' Head	L= 38.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 208.50' / 208.00' S= 0.0132 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf 5.0' long x 24.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63			

Primary OutFlow Max=4.59 cfs @ 12.15 hrs HW=209.72' TW=201.65' (Dynamic Tailwater) **1=15" HDPE N-12** (Inlet Controls 4.59 cfs @ 3.76 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=208.50' TW=200.50' (Dynamic Tailwater)

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Pond 24P: Pond #24



Summary for Pond 25P: Pond #25

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area =	1.637 ac, 16.60% Impervious, Inflow De	epth > 3.24" for 25Yr24Hr. event
Inflow =	5.28 cfs @ 12.14 hrs, Volume=	0.442 af
Outflow =	5.29 cfs @ 12.15 hrs, Volume=	0.442 af, Atten= 0%, Lag= 0.5 min
Primary =	5.29 cfs @ 12.15 hrs, Volume=	0.442 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 201.65' @ 12.15 hrs Surf.Area= 186 sf Storage= 96 cf Flood Elev= 203.00' Surf.Area= 550 sf Storage= 579 cf

Plug-Flow detention time= 0.2 min calculated for 0.442 af (100% of inflow) Center-of-Mass det. time= 0.2 min (827.3 - 827.1)

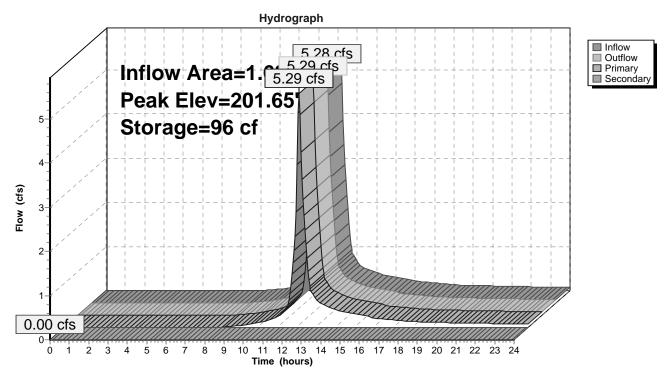
Volume	Inve	ert Avai	.Storage	Storage Description	on		
#1	200.5	0'	854 cf	Open Water Stor	age (Irregular) Lis	ted below (Recale	c)
Elevatic (fee 200.5 201.0 202.0 203.0	50 50 00 00	Surf.Area (sq-ft) 4 72 271 550	Perim. (feet) 25.0 78.4 112.5 142.4	Inc.Store (cubic-feet) 0 15 161 402	Cum.Store (cubic-feet) 0 15 176 579	Wet.Area (sq-ft) 4 444 971 1,591	
203.5		550	142.4	275	854	1,662	
Device #1 #2	Routing Primary Seconda	200.	50' 18.0 L= 3 Inlet n= 0 .00' 5.0' Head	142.4 275 854 1,662 Outlet Devices 18.0" Round 18" HDPE N-12 L= 38.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 200.50' / 197.50' S= 0.0789 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf 5.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64			

Primary OutFlow Max=5.28 cfs @ 12.15 hrs HW=201.65' TW=195.91' (Dynamic Tailwater) **1=18" HDPE N-12** (Inlet Controls 5.28 cfs @ 3.64 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.50' TW=194.12' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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Summary for Pond 101P: Rain Garden#101 (Built)

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=66)

Inflow Area =	2.403 ac,	8.54% Impervious, Inflow De	epth > 3.03"	for 25Yr24Hr. event
Inflow =	5.85 cfs @	12.22 hrs, Volume=	0.607 af	
Outflow =	1.38 cfs @	12.83 hrs, Volume=	0.564 af, Atte	n= 76%, Lag= 36.4 min
Primary =	1.38 cfs @	12.83 hrs, Volume=	0.564 af	
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0.000 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 267.16' @ 12.83 hrs Surf.Area= 5,872 sf Storage= 9,858 cf Flood Elev= 271.00' Surf.Area= 6,756 sf Storage= 49,781 cf

Plug-Flow detention time= 97.5 min calculated for 0.563 af (93% of inflow) Center-of-Mass det. time= 61.7 min (898.0 - 836.3)

Volume	Invert	Avail.Storage	Storage Description
#1	263.50'	1,843 cf	Stone (Irregular)Listed below (Recalc) -Impervious
			4,642 cf Overall - 34 cf Embedded = 4,608 cf x 40.0% Voids
#2	264.50'	1,393 cf	
			6,963 cf Overall x 20.0% Voids
#3	266.00'		Open Water Storage (Irregular) Listed below (Recalc) -Impervious
#4	266.00'		Forebay (Irregular) Listed below (Recalc)
#5	263.50'	15 cf	6.0" Round 6" U.D. Inside #1
			L= 75.0'
#6	263.50'	8 cf	6.0" Round 6" U.D. Inside #1
			L= 40.0'
#7	263.50'	12 cf	6.0" Round 6" U.D. Inside #1
			L= 60.0'

49,781 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>
263.50	4,642	276.7	0	0	4,642
264.50	4,642	276.7	4,642	4,642	4,919
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
264.50	4,642	276.7	0	0	4,642
266.00	4,642	276.7	6,963	6,963	5,057
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
266.00	4,642	276.7	0	0	4,642
267.00	5,208	289.3	4,922	4,922	5,276
267.50	5,543	300.2	2,687	7,610	5,808
268.00	8,513	630.7	3,488	11,097	30,292
269.00	10,465	882.3	9,472	20,569	60,595
270.00	12,286	915.0	11,363	31,933	65,357
271.00	14,130	928.0	13,197	45,130	67,475

Type III 24-hr 25Yr.-24Hr. Rainfall=5.85"

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Elevatio (fee		Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
266.0	00	434	223.0	0	0	434
267.0	00	895	236.0	651	651	961
267.5	50	2,114	483.4	731	1,381	15,126
Device	Routing	Inver	t Outle	t Devices		
#1	Primary	263.40		Round 24" HDPE		
				.0' CPP, end-sectio	5	
				Outlet Invert= 263.4		0050 '/' Cc= 0.900
	.	000 50		012, Flow Area= 3.1		
#2	Device 1		•••	/ert. 6" Vertical C=		
#3	Device 1	270.00		Horiz. 24" Top Plat		
			Limite	ed to weir flow at low	heads	
#4	Device 2	263.50)' 0.7" \	/ert75" Orifice - A	naerobic C= 0.60	00
#5	Device 2	264.50)' 10.00	0 in/hr Exfiltration	over Surface area	1
#6	Seconda	ary 270.50)' 15.0'	long x 8.0' breadth	E-Spillway	
						0 1.40 1.60 1.80 2.00
			2.50	3.00 3.50 4.00 4.5	0 5.00 5.50	
			Coef.	(English) 2.43 2.54	2.70 2.69 2.68	2.68 2.66 2.64 2.64
				2.65 2.65 2.66 2.6		

Primary OutFlow Max=1.38 cfs @ 12.83 hrs HW=267.16' TW=264.08' (Dynamic Tailwater)

1=24" HDPE N-12 (Passes 1.38 cfs of 25.13 cfs potential flow)

-2=6" Vertical (Passes 1.38 cfs of 1.66 cfs potential flow) -4=.75" Orifice - Anaerobic (Orifice Controls 0.02 cfs @ 8.46 fps) -5=Exfiltration (Exfiltration Controls 1.36 cfs)

-3=24" Top Plate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=263.50' TW=262.50' (Dynamic Tailwater) G=E-Spillway (Controls 0.00 cfs)

Hydrograph Inflow 5.85 cfs Outflow
 Primary
 Secondary Inflow Area=2.403 ac Peak Elev=267.16' 6 Storage=9,858 cf 5-Flow (cfs) 3 1.38 cfs 1.38 cfs 2 0.00 cfs 0 1 2 3 11 12 13 14 15 16 17 18 19 20 21 22 23 24 4 5 6 ģ 7 8 10 Time (hours)

Pond 101P: Rain Garden#101 (Built)

Summary for Pond 202P: Rain Garden w/ Infiltration #202

Inflow Area =	2.266 ac,	8.26% Impervious, Inflow De	epth > 2.26" for 25Yr24Hr. event
Inflow =	4.73 cfs @	12.19 hrs, Volume=	0.427 af
Outflow =	0.28 cfs @	11.80 hrs, Volume=	0.298 af, Atten= 94%, Lag= 0.0 min
Discarded =	0.28 cfs @	11.80 hrs, Volume=	0.298 af

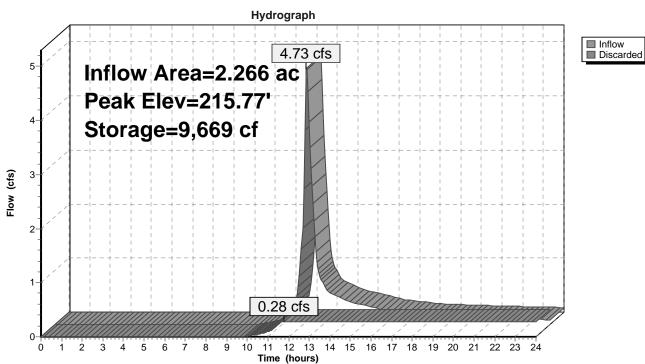
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 215.77' @ 15.81 hrs Surf.Area= 4,000 sf Storage= 9,669 cf Flood Elev= 218.25' Surf.Area= 4,000 sf Storage= 28,301 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 196.4 min (1,052.0 - 855.7)

Volume	Invert Ava	ail.Storage	Storage Description	on		
#1	212.00'	1,600 cf		Listed below (Rec	alc)	
			4,000 cf Overall			
#2	213.00'	1,200 cf			Recalc) - Imperviou	IS
#3	214.50'	25 501 of	6,000 cf Overall		tod bolow (Booolo	
	214.30	25,501 cf			ted below (Recalc	<u>) -impervio</u> us
		28,301 cf	Total Available St	orage		
Elevation	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)		(cubic-feet)	(cubic-feet)	(sq-ft)	
212.00	4,000	258.6	0	0	4,000	
213.00	4,000	258.6	4,000	4,000	4,259	
Elevation	Surf.Area		Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
213.00	4,000	258.6	0	0	4,000	
214.50	4,000	258.6	6,000	6,000	4,388	
Elevation	Surf.Area		Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
214.50	4,000	258.6	0	0	4,000	
214.75	5,115	327.0	1,137	1,137	7,188	
215.00	5,365	332.7	1,310	2,446	7,499	
216.00	6,398	353.1	5,874	8,320	8,664	
217.00	7,486	371.9	6,935	15,255	9,807	
218.00	8,630	390.8	8,051	23,306	11,016	
218.25	8,925		2,194	25,501	11,326	
Device Routing Invert Outlet Devices						
-						

#1 Discarded 212.00' **3.000 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.28 cfs @ 11.80 hrs HW=212.08' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.28 cfs)



Pond 202P: Rain Garden w/ Infiltration #202

22-051 Proposed Analysis	Type III 24-hr 2Yr24Hr. Rainfall=3.08"
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Subcatchment1S: Subcatchment#1	Runoff Area=110,110 sf 12.30% Impervious Runoff Depth>0.06" Flow Length=145' Tc=7.9 min CN=47 Runoff=0.02 cfs 0.012 af
Subcatchment 2S: Subcatchment #2	Runoff Area=394,636 sf 7.55% Impervious Runoff Depth>0.58" Flow Length=1,118' Tc=37.4 min CN=66 Runoff=2.53 cfs 0.435 af
Subcatchment3S: Subcatchment#3	Runoff Area=28,734 sf 17.82% Impervious Runoff Depth>1.25" Tc=6.0 min CN=79 Runoff=0.93 cfs 0.068 af
Subcatchment7S: Subcatchment#7	Runoff Area=19,980 sf 10.51% Impervious Runoff Depth>1.07" Flow Length=721' Tc=12.7 min CN=76 Runoff=0.44 cfs 0.041 af
Subcatchment 12S: Subcatchment #12	Runoff Area=47,185 sf 2.86% Impervious Runoff Depth>0.85" Flow Length=485' Tc=18.5 min CN=72 Runoff=0.68 cfs 0.077 af
Subcatchment 13S: Subcatchment #13	Runoff Area=30,563 sf 5.93% Impervious Runoff Depth>0.85" Flow Length=400' Tc=17.4 min CN=72 Runoff=0.45 cfs 0.050 af
Subcatchment14S: Subcatchment#14	Runoff Area=6,937 sf 53.05% Impervious Runoff Depth>1.81" Tc=6.0 min CN=87 Runoff=0.33 cfs 0.024 af
Subcatchment 21S: Subcatchment #21	Runoff Area=18,611 sf 24.79% Impervious Runoff Depth>0.96" Flow Length=312' Tc=14.4 min CN=74 Runoff=0.34 cfs 0.034 af
Subcatchment 22S: Subcatchment #22	Runoff Area=41,415 sf 15.74% Impervious Runoff Depth>0.36" Flow Length=442' Tc=13.2 min CN=60 Runoff=0.18 cfs 0.029 af
Subcatchment 23S: Subcatchment #23	Runoff Area=57,287 sf 2.85% Impervious Runoff Depth>0.71" Flow Length=348' Tc=10.1 min CN=69 Runoff=0.81 cfs 0.078 af
Subcatchment 24S: Subcatchment #24	Runoff Area=63,055 sf 13.75% Impervious Runoff Depth>1.01" Flow Length=340' Tc=10.2 min CN=75 Runoff=1.40 cfs 0.122 af
Subcatchment 25S: Subcatchment #25	Runoff Area=8,261 sf 38.37% Impervious Runoff Depth>1.51" Tc=6.0 min CN=83 Runoff=0.33 cfs 0.024 af
Reach 2R: Reach #2 n=0.022	Avg. Flow Depth=0.21' Max Vel=2.65 fps Inflow=3.60 cfs 0.640 af L=31.0' S=0.0213 '/' Capacity=23.64 cfs Outflow=3.60 cfs 0.639 af
Reach 3R: Reach #3 n=0.022 L	Avg. Flow Depth=0.08' Max Vel=2.22 fps Inflow=0.93 cfs 0.068 af .=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=0.75 cfs 0.068 af
Reach 7R: Reach #7 n=0.022 L	Avg. Flow Depth=0.08' Max Vel=2.66 fps Inflow=1.04 cfs 0.137 af
Reach 100R: Final Reach #100	Inflow=3.91 cfs 0.801 af Outflow=3.91 cfs 0.801 af

	S Type III 24-hr 2 Yr24Hr. Rainfall=3.08" g & Engineering (Rev: 4/13/23) Printed 4/13/2023 © 2019 HydroCAD Software Solutions LLC Page 2
Reach 101R: Overland Flow fi	rom R-101 Avg. Flow Depth=0.14' Max Vel=1.45 fps Inflow=0.91 cfs 0.137 af n=0.045 L=32.0' S=0.0469 '/' Capacity=17.15 cfs Outflow=1.04 cfs 0.137 af
Reach 200R: Final Reach #200	D Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Pond 1P: Pond #1 cfs 0.030 af Primary=3.91 cfs 0.801 af Sec	Peak Elev=195.13' Storage=755 cf Inflow=4.21 cfs 0.831 af ondary=0.00 cfs 0.000 af Tertiary=0.00 cfs 0.000 af Outflow=4.06 cfs 0.831 af
Pond 7P: Level Spreader	Peak Elev=264.06' Storage=510 cf Inflow=0.95 cfs 0.148 af Outflow=0.91 cfs 0.137 af
Pond 12P: Pond #12	Peak Elev=270.20' Storage=0 cf Inflow=0.68 cfs 0.077 af 15.0" Round Culvert n=0.012 L=40.0' S=0.0050 '/' Outflow=0.68 cfs 0.077 af
Pond 13P: Pond #13	Peak Elev=271.35' Storage=5 cf Inflow=0.62 cfs 0.074 af 18.0" Round Culvert n=0.012 L=45.0' S=0.0222 '/' Outflow=0.62 cfs 0.074 af
Pond 14P: Pond #14	Peak Elev=271.78' Storage=4 cf Inflow=0.33 cfs 0.024 af 12.0" Round Culvert n=0.012 L=21.9' S=0.0183 '/' Outflow=0.33 cfs 0.024 af
Pond 23P: Pond #23	Peak Elev=224.41' Storage=212 cf Inflow=0.81 cfs 0.078 af 15.0" Round Culvert n=0.012 L=65.0' S=0.0769 '/' Outflow=0.75 cfs 0.077 af
Pond 24P: Pond #24 Prim	Peak Elev=209.07' Storage=4 cf Inflow=1.40 cfs 0.122 af hary=1.40 cfs 0.122 af Secondary=0.00 cfs 0.000 af Outflow=1.40 cfs 0.122 af
Pond 25P: Pond #25 Prim	Peak Elev=201.09' Storage=22 cf Inflow=1.67 cfs 0.146 af hary=1.67 cfs 0.146 af Secondary=0.00 cfs 0.000 af Outflow=1.67 cfs 0.146 af
Pond 101P: Rain Garden#101 Prim	(Built) Peak Elev=264.94' Storage=2,284 cf Inflow=1.69 cfs 0.191 af hary=0.95 cfs 0.148 af Secondary=0.00 cfs 0.000 af Outflow=0.95 cfs 0.148 af
Pond 202P: Rain Garden w/ Ir	filtration #202 Peak Elev=212.53' Storage=843 cf Inflow=0.90 cfs 0.106 af Outflow=0.28 cfs 0.106 af
Total Runoff Are	ea = 18.980 ac Runoff Volume = 0.993 af Average Runoff Depth = 0.63" 90.08% Pervious = 17.098 ac 9.92% Impervious = 1.882 ac

22-051 Proposed Analysis	Type III 24-hr 10Yr24Hr. Rainfa	ll=4.63"
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Subcatchment1S: Subcatchment#1	Runoff Area=110,110 sf 12.30% Impervious Runoff Depth>0.41" Flow Length=145' Tc=7.9 min CN=47 Runoff=0.46 cfs 0.087 af
Subcatchment 2S: Subcatchment #2	Runoff Area=394,636 sf 7.55% Impervious Runoff Depth>1.47" Flow Length=1,118' Tc=37.4 min CN=66 Runoff=7.54 cfs 1.107 af
Subcatchment 3S: Subcatchment #3	Runoff Area=28,734 sf 17.82% Impervious Runoff Depth>2.48" Tc=6.0 min CN=79 Runoff=1.89 cfs 0.137 af
Subcatchment7S: Subcatchment#7	Runoff Area=19,980 sf 10.51% Impervious Runoff Depth>2.23" Flow Length=721' Tc=12.7 min CN=76 Runoff=0.95 cfs 0.085 af
Subcatchment 12S: Subcatchment #12	Runoff Area=47,185 sf 2.86% Impervious Runoff Depth>1.91" Flow Length=485' Tc=18.5 min CN=72 Runoff=1.66 cfs 0.172 af
Subcatchment 13S: Subcatchment #13	Runoff Area=30,563 sf 5.93% Impervious Runoff Depth>1.91" Flow Length=400' Tc=17.4 min CN=72 Runoff=1.10 cfs 0.112 af
Subcatchment14S: Subcatchment#14	Runoff Area=6,937 sf 53.05% Impervious Runoff Depth>3.22" Tc=6.0 min CN=87 Runoff=0.58 cfs 0.043 af
Subcatchment 21S: Subcatchment #21	Runoff Area=18,611 sf 24.79% Impervious Runoff Depth>2.07" Flow Length=312' Tc=14.4 min CN=74 Runoff=0.79 cfs 0.074 af
Subcatchment 22S: Subcatchment #22	Runoff Area=41,415 sf 15.74% Impervious Runoff Depth>1.09" Flow Length=442' Tc=13.2 min CN=60 Runoff=0.82 cfs 0.086 af
Subcatchment 23S: Subcatchment #23	Runoff Area=57,287 sf 2.85% Impervious Runoff Depth>1.69" Flow Length=348' Tc=10.1 min CN=69 Runoff=2.17 cfs 0.185 af
Subcatchment 24S: Subcatchment #24	Runoff Area=63,055 sf 13.75% Impervious Runoff Depth>2.15" Flow Length=340' Tc=10.2 min CN=75 Runoff=3.12 cfs 0.259 af
Subcatchment 25S: Subcatchment #25	Runoff Area=8,261 sf 38.37% Impervious Runoff Depth>2.84" Tc=6.0 min CN=83 Runoff=0.62 cfs 0.045 af
Reach 2R: Reach #2 n=0.022	Avg. Flow Depth=0.32' Max Vel=3.55 fps Inflow=9.29 cfs 1.599 af L=31.0' S=0.0213 '/' Capacity=23.64 cfs Outflow=9.29 cfs 1.599 af
Reach 3R: Reach #3 n=0.022 L	Avg. Flow Depth=0.12' Max Vel=2.82 fps Inflow=1.89 cfs 0.137 af
Reach 7R: Reach #7 n=0.022 L	Avg. Flow Depth=0.10' Max Vel=2.98 fps Inflow=1.36 cfs 0.358 af _=772.0' S=0.0762 '/' Capacity=44.74 cfs Outflow=1.24 cfs 0.356 af
Reach 100R: Final Reach #100	Inflow=9.18 cfs 1.979 af

Outflow=9.18 cfs 1.979 af

	lysis eying & Engineering (Rev: 4/13/ ⁶⁰⁵ © 2019 HydroCAD Software So	23)	"10Yr24Hr. Rainfall=4.63 Printed 4/13/2023 Page 4	6
Reach 101R: Overland Flo	ow from R-101 Avg. Flow Depth=0 n=0.045 L=32.0' S=0.0469 '/).15' Max Vel=1.5 " Capacity=17.15	7 fps Inflow=1.56 cfs 0.358 af cfs Outflow=1.36 cfs 0.358 af	:
Reach 200R: Final Reach	#200		Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af	
Pond 1P: Pond #1 Primary=9.18 cfs 1.979 af	Peak Elev=196 Secondary=0.00 cfs 0.000 af Tertia		3 cf Inflow=11.41 cfs 2.063 af af Outflow=9.75 cfs 2.063 af	
Pond 7P: Level Spreader	Peak Elev=2	264.08' Storage=5	22 cf Inflow=1.24 cfs 0.369 af Outflow=1.56 cfs 0.358 af	
Pond 12P: Pond #12			=0 cf Inflow=1.66 cfs 0.172 af) '/' Outflow=1.66 cfs 0.172 af	
Pond 13P: Pond #13			10 cf Inflow=1.39 cfs 0.154 af 2 '/' Outflow=1.39 cfs 0.154 af	
Pond 14P: Pond #14			=5 cf Inflow=0.58 cfs 0.043 af 3 '/' Outflow=0.58 cfs 0.043 af	
Pond 23P: Pond #23			94 cf Inflow=2.17 cfs 0.185 af 9 '/' Outflow=2.06 cfs 0.184 af	
Pond 24P: Pond #24	Peak Elev= Primary=3.12 cfs 0.259 af Second		13 cf Inflow=3.12 cfs 0.259 af af Outflow=3.12 cfs 0.259 af	
Pond 25P: Pond #25	Peak Elev= Primary=3.63 cfs 0.304 af Second		58 cf Inflow=3.62 cfs 0.304 af af Outflow=3.63 cfs 0.304 af	
Pond 101P: Rain Garden	#101 (Built) Peak Elev=26 Primary=1.24 cfs 0.369 af Second		31 cf Inflow=3.90 cfs 0.412 af af Outflow=1.24 cfs 0.369 af	
Pond 202P: Rain Garden	w/ Infiltration #202 Peak Elev=21	4.92' Storage=4,8	07 cf Inflow=2.88 cfs 0.270 af Outflow=0.28 cfs 0.270 af	
Total Runof	f Area = 18.980 ac Runoff Volu 90.08% Perviou		verage Runoff Depth = 1.5 9.92% Impervious = 1.882 a	

22-051 Proposed Analysis	Type III 24-hr 25Yr24Hr. Rainfall=5.85"
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Subcatchment1S: Subcatchment#1	Runoff Area=110,110 sf 12.30% Impervious Runoff Depth>0.87" Flow Length=145' Tc=7.9 min CN=47 Runoff=1.57 cfs 0.183 af
Subcatchment 2S: Subcatchment #2	Runoff Area=394,636 sf 7.55% Impervious Runoff Depth>2.31" Flow Length=1,118' Tc=37.4 min CN=66 Runoff=12.28 cfs 1.743 af
Subcatchment3S: Subcatchment#3	Runoff Area=28,734 sf 17.82% Impervious Runoff Depth>3.54" Tc=6.0 min CN=79 Runoff=2.68 cfs 0.195 af
Subcatchment7S: Subcatchment#7	Runoff Area=19,980 sf 10.51% Impervious Runoff Depth>3.24" Flow Length=721' Tc=12.7 min CN=76 Runoff=1.39 cfs 0.124 af
Subcatchment 12S: Subcatchment #1	2 Runoff Area=47,185 sf 2.86% Impervious Runoff Depth>2.86" Flow Length=485' Tc=18.5 min CN=72 Runoff=2.52 cfs 0.258 af
Subcatchment13S: Subcatchment#1	3 Runoff Area=30,563 sf 5.93% Impervious Runoff Depth>2.86" Flow Length=400' Tc=17.4 min CN=72 Runoff=1.67 cfs 0.167 af
Subcatchment14S: Subcatchment#1	4 Runoff Area=6,937 sf 53.05% Impervious Runoff Depth>4.37" Tc=6.0 min CN=87 Runoff=0.78 cfs 0.058 af
Subcatchment21S: Subcatchment#2	1 Runoff Area=18,611 sf 24.79% Impervious Runoff Depth>3.05" Flow Length=312' Tc=14.4 min CN=74 Runoff=1.17 cfs 0.109 af
Subcatchment22S: Subcatchment#2	2 Runoff Area=41,415 sf 15.74% Impervious Runoff Depth>1.82" Flow Length=442' Tc=13.2 min CN=60 Runoff=1.50 cfs 0.144 af
Subcatchment 23S: Subcatchment #2	3 Runoff Area=57,287 sf 2.85% Impervious Runoff Depth>2.59" Flow Length=348' Tc=10.1 min CN=69 Runoff=3.41 cfs 0.284 af
Subcatchment24S: Subcatchment#2	4 Runoff Area=63,055 sf 13.75% Impervious Runoff Depth>3.15" Flow Length=340' Tc=10.2 min CN=75 Runoff=4.60 cfs 0.380 af
Subcatchment 25S: Subcatchment #2	5 Runoff Area=8,261 sf 38.37% Impervious Runoff Depth>3.95" Tc=6.0 min CN=83 Runoff=0.85 cfs 0.062 af
Reach 2R: Reach #2 n=0.022	Avg. Flow Depth=0.40' Max Vel=4.05 fps Inflow=14.33 cfs 2.489 af L=31.0' S=0.0213 '/' Capacity=23.64 cfs Outflow=14.32 cfs 2.488 af
Reach 3R: Reach #3 n=0.022	Avg. Flow Depth=0.14' Max Vel=3.16 fps Inflow=2.68 cfs 0.195 af L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=2.34 cfs 0.194 af
Reach 7R: Reach #7 n=0.022	Avg. Flow Depth=0.10' Max Vel=3.07 fps Inflow=1.38 cfs 0.553 af L=772.0' S=0.0762 '/' Capacity=44.74 cfs Outflow=1.38 cfs 0.552 af
Reach 100R: Final Reach #100	Inflow=11.11 cfs 2.915 af Outflow=11.11 cfs 2.915 af

		Type III 24-hr25Yr24Hr. Rainfall=5.85"ying & Engineering (Rev: 4/13/23)Printed 4/13/2023605 © 2019 HydroCAD Software Solutions LLCPage 6
	Reach 101R: Overland Flo	w from R-101 Avg. Flow Depth=0.16' Max Vel=1.58 fps Inflow=1.38 cfs 0.553 af n=0.045 L=32.0' S=0.0469 '/' Capacity=17.15 cfs Outflow=1.38 cfs 0.553 af
	Reach 200R: Final Reach	#200 Inflow=3.87 cfs 0.149 af Outflow=3.87 cfs 0.149 af
0.157 af	Pond 1P: Pond #1 Primary=11.11 cfs 2.915 af S	Peak Elev=196.57' Storage=10,734 cf Inflow=17.77 cfs 3.222 af econdary=3.87 cfs 0.149 af Tertiary=0.00 cfs 0.000 af Outflow=16.00 cfs 3.221 af
	Pond 7P: Level Spreader	Peak Elev=264.08' Storage=519 cf Inflow=1.38 cfs 0.564 af Outflow=1.38 cfs 0.553 af
	Pond 12P: Pond #12	Peak Elev=270.20' Storage=0 cf Inflow=2.52 cfs 0.258 af 15.0" Round Culvert n=0.012 L=40.0' S=0.0050 '/' Outflow=2.52 cfs 0.258 af
	Pond 13P: Pond #13	Peak Elev=271.66' Storage=14 cf Inflow=2.07 cfs 0.225 af 18.0" Round Culvert n=0.012 L=45.0' S=0.0222 '/' Outflow=2.07 cfs 0.225 af
	Pond 14P: Pond #14	Peak Elev=271.95' Storage=6 cf Inflow=0.78 cfs 0.058 af 12.0" Round Culvert n=0.012 L=21.9' S=0.0183 '/' Outflow=0.78 cfs 0.058 af
	Pond 23P: Pond #23	Peak Elev=224.93' Storage=543 cf Inflow=3.41 cfs 0.284 af 15.0" Round Culvert n=0.012 L=65.0' S=0.0769 '/' Outflow=3.24 cfs 0.283 af
	Pond 24P: Pond #24	Peak Elev=209.72' Storage=27 cf Inflow=4.60 cfs 0.380 af Primary=4.59 cfs 0.380 af Secondary=0.00 cfs 0.000 af Outflow=4.59 cfs 0.380 af
	Pond 25P: Pond #25	Peak Elev=201.65' Storage=96 cf Inflow=5.28 cfs 0.442 af Primary=5.29 cfs 0.442 af Secondary=0.00 cfs 0.000 af Outflow=5.29 cfs 0.442 af
	Pond 101P: Rain Garden#	101 (Built) Peak Elev=267.16' Storage=9,858 cf Inflow=5.85 cfs 0.607 af Primary=1.38 cfs 0.564 af Secondary=0.00 cfs 0.000 af Outflow=1.38 cfs 0.564 af
	Pond 202P: Rain Garden	v/ Infiltration #202 Peak Elev=215.77' Storage=9,669 cf Inflow=4.73 cfs 0.427 af Outflow=0.28 cfs 0.298 af
	Total Runoff	Area = 18.980 ac Runoff Volume = 3.707 af Average Runoff Depth = 2.34" 90.08% Pervious = 17.098 ac 9.92% Impervious = 1.882 ac

22-051 Proposed Analysis	Type III 24-hr 50Yr24Hr. Rainfall=6.99"
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Subcatchment1S: Subcatchment#1	Runoff Area=110,110 sf 12.30% Impervious Runoff Depth>1.40" Flow Length=145' Tc=7.9 min CN=47 Runoff=3.06 cfs 0.294 af
Subcatchment 2S: Subcatchment #2	Runoff Area=394,636 sf 7.55% Impervious Runoff Depth>3.17" Flow Length=1,118' Tc=37.4 min CN=66 Runoff=17.10 cfs 2.393 af
Subcatchment 3S: Subcatchment #3	Runoff Area=28,734 sf 17.82% Impervious Runoff Depth>4.57" Tc=6.0 min CN=79 Runoff=3.44 cfs 0.251 af
Subcatchment7S: Subcatchment#7	Runoff Area=19,980 sf 10.51% Impervious Runoff Depth>4.24" Flow Length=721' Tc=12.7 min CN=76 Runoff=1.82 cfs 0.162 af
Subcatchment 12S: Subcatchment #1	2 Runoff Area=47,185 sf 2.86% Impervious Runoff Depth>3.81" Flow Length=485' Tc=18.5 min CN=72 Runoff=3.37 cfs 0.344 af
Subcatchment 13S: Subcatchment #1	3 Runoff Area=30,563 sf 5.93% Impervious Runoff Depth>3.81" Flow Length=400' Tc=17.4 min CN=72 Runoff=2.23 cfs 0.223 af
Subcatchment14S: Subcatchment#1	4 Runoff Area=6,937 sf 53.05% Impervious Runoff Depth>5.47" Tc=6.0 min CN=87 Runoff=0.96 cfs 0.073 af
Subcatchment21S: Subcatchment#2	1 Runoff Area=18,611 sf 24.79% Impervious Runoff Depth>4.02" Flow Length=312' Tc=14.4 min CN=74 Runoff=1.55 cfs 0.143 af
Subcatchment22S: Subcatchment#2	2 Runoff Area=41,415 sf 15.74% Impervious Runoff Depth>2.59" Flow Length=442' Tc=13.2 min CN=60 Runoff=2.20 cfs 0.205 af
Subcatchment23S: Subcatchment#2	3 Runoff Area=57,287 sf 2.85% Impervious Runoff Depth>3.50" Flow Length=348' Tc=10.1 min CN=69 Runoff=4.65 cfs 0.384 af
Subcatchment24S: Subcatchment#2	4 Runoff Area=63,055 sf 13.75% Impervious Runoff Depth>4.13" Flow Length=340' Tc=10.2 min CN=75 Runoff=6.03 cfs 0.499 af
Subcatchment25S: Subcatchment#2	5 Runoff Area=8,261 sf 38.37% Impervious Runoff Depth>5.01" Tc=6.0 min CN=83 Runoff=1.07 cfs 0.079 af
Reach 2R: Reach #2 n=0.022	Avg. Flow Depth=0.46' Max Vel=4.46 fps Inflow=19.58 cfs 3.388 af L=31.0' S=0.0213 '/' Capacity=23.64 cfs Outflow=19.58 cfs 3.388 af
Reach 3R: Reach #3 n=0.022	Avg. Flow Depth=0.16' Max Vel=3.42 fps Inflow=3.44 cfs 0.251 af L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=3.04 cfs 0.250 af
Reach 7R: Reach #7 n=0.022	Avg. Flow Depth=0.11' Max Vel=3.21 fps Inflow=1.59 cfs 0.746 af L=772.0' S=0.0762 '/' Capacity=44.74 cfs Outflow=1.59 cfs 0.745 af
Reach 100R: Final Reach #100	Inflow=11.98 cfs 3.688 af Outflow=11.98 cfs 3.688 af

		Ysis Type III 24-hr 50Yr24Hr. Rainfall=6.99" ing & Engineering (Rev: 4/13/23) Printed 4/13/2023 05 © 2019 HydroCAD Software Solutions LLC Page 8
	Reach 101R: Overland Flow	v from R-101 Avg. Flow Depth=0.17' Max Vel=1.65 fps Inflow=1.59 cfs 0.747 af n=0.045 L=32.0' S=0.0469 '/' Capacity=17.15 cfs Outflow=1.59 cfs 0.746 af
	Reach 200R: Final Reach #	200 Inflow=9.92 cfs 0.512 af Outflow=9.92 cfs 0.512 af
0.203 af	Pond 1P: Pond #1 Primary=11.86 cfs 3.687 af Se	Peak Elev=196.81' Storage=14,292 cf Inflow=24.32 cfs 4.403 af econdary=9.92 cfs 0.512 af Tertiary=0.12 cfs 0.001 af Outflow=22.92 cfs 4.402 af
	Pond 7P: Level Spreader	Peak Elev=264.08' Storage=523 cf Inflow=1.59 cfs 0.758 af Outflow=1.59 cfs 0.747 af
	Pond 12P: Pond #12	Peak Elev=270.33' Storage=15 cf Inflow=3.37 cfs 0.344 af 15.0" Round Culvert n=0.012 L=40.0' S=0.0050 '/' Outflow=3.38 cfs 0.344 af
	Pond 13P: Pond #13	Peak Elev=271.77' Storage=18 cf Inflow=2.73 cfs 0.295 af 18.0" Round Culvert n=0.012 L=45.0' S=0.0222 '/' Outflow=2.73 cfs 0.295 af
	Pond 14P: Pond #14	Peak Elev=272.03' Storage=7 cf Inflow=0.96 cfs 0.073 af 12.0" Round Culvert n=0.012 L=21.9' S=0.0183 '/' Outflow=0.96 cfs 0.073 af
	Pond 23P: Pond #23	Peak Elev=225.16' Storage=706 cf Inflow=4.65 cfs 0.384 af 15.0" Round Culvert n=0.012 L=65.0' S=0.0769 '/' Outflow=4.36 cfs 0.382 af
	Pond 24P: Pond #24	Peak Elev=210.16' Storage=60 cf Inflow=6.03 cfs 0.499 af Primary=6.01 cfs 0.499 af Secondary=0.00 cfs 0.000 af Outflow=6.01 cfs 0.499 af
	Pond 25P: Pond #25	Peak Elev=201.88' Storage=147 cf Inflow=6.86 cfs 0.578 af Primary=6.83 cfs 0.578 af Secondary=0.00 cfs 0.000 af Outflow=6.83 cfs 0.578 af
	Pond 101P: Rain Garden# ⁴ F	01 (Built) Peak Elev=267.71' Storage=13,560 cf Inflow=7.70 cfs 0.801 af Primary=1.59 cfs 0.758 af Secondary=0.00 cfs 0.000 af Outflow=1.59 cfs 0.758 af
	Pond 202P: Rain Garden w	/ Infiltration #202 Peak Elev=216.58' Storage=15,042 cf Inflow=6.56 cfs 0.587 af Outflow=0.28 cfs 0.310 af
	Total Runoff	Area = 18.980 ac Runoff Volume = 5.049 af Average Runoff Depth = 3.19" 90.08% Pervious = 17.098 ac 9.92% Impervious = 1.882 ac

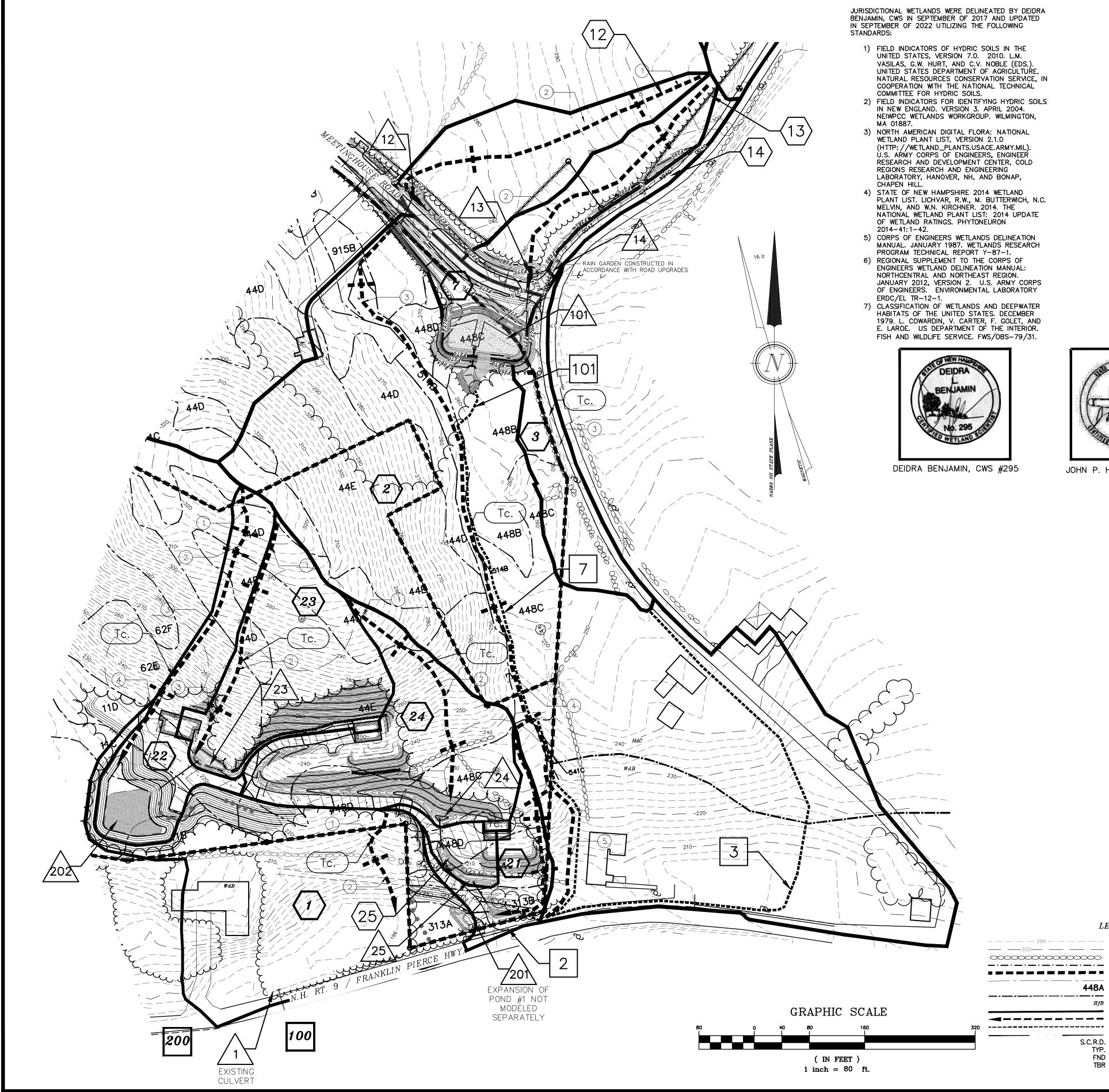
22-051 Proposed Analysis	Type III 24-hr	100Yr24Hr. Rainfall=8.36"
Prepared by Berry Surveying & Engineering (Rev: 4/13	/23)	Printed 4/13/2023
HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software S	olutions LLC	Page 9

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1S: Subcatchment#1	Runoff Area=110,110 sf 12.30% Impervious Runoff Depth>2.14" Flow Length=145' Tc=7.9 min CN=47 Runoff=5.15 cfs 0.451 af
Subcatchment 2S: Subcatchment #2	Runoff Area=394,636 sf 7.55% Impervious Runoff Depth>4.27" Flow Length=1,118' Tc=37.4 min CN=66 Runoff=23.22 cfs 3.224 af
Subcatchment3S: Subcatchment#3	Runoff Area=28,734 sf 17.82% Impervious Runoff Depth>5.84" Tc=6.0 min CN=79 Runoff=4.36 cfs 0.321 af
Subcatchment7S: Subcatchment#7	Runoff Area=19,980 sf 10.51% Impervious Runoff Depth>5.48" Flow Length=721' Tc=12.7 min CN=76 Runoff=2.35 cfs 0.209 af
Subcatchment12S: Subcatchment#1	2 Runoff Area=47,185 sf 2.86% Impervious Runoff Depth>4.99" Flow Length=485' Tc=18.5 min CN=72 Runoff=4.42 cfs 0.451 af
Subcatchment13S: Subcatchment#1	3 Runoff Area=30,563 sf 5.93% Impervious Runoff Depth>5.00" Flow Length=400' Tc=17.4 min CN=72 Runoff=2.93 cfs 0.292 af
Subcatchment14S: Subcatchment#1	4 Runoff Area=6,937 sf 53.05% Impervious Runoff Depth>6.80" Tc=6.0 min CN=87 Runoff=1.18 cfs 0.090 af
Subcatchment 21S: Subcatchment #2	1 Runoff Area=18,611 sf 24.79% Impervious Runoff Depth>5.24" Flow Length=312' Tc=14.4 min CN=74 Runoff=2.01 cfs 0.186 af
Subcatchment 22S: Subcatchment #2	2 Runoff Area=41,415 sf 15.74% Impervious Runoff Depth>3.60" Flow Length=442' Tc=13.2 min CN=60 Runoff=3.12 cfs 0.285 af
Subcatchment 23S: Subcatchment #2	3 Runoff Area=57,287 sf 2.85% Impervious Runoff Depth>4.65" Flow Length=348' Tc=10.1 min CN=69 Runoff=6.19 cfs 0.510 af
Subcatchment 24S: Subcatchment #2	4 Runoff Area=63,055 sf 13.75% Impervious Runoff Depth>5.36" Flow Length=340' Tc=10.2 min CN=75 Runoff=7.79 cfs 0.646 af
Subcatchment 25S: Subcatchment #2	5 Runoff Area=8,261 sf 38.37% Impervious Runoff Depth>6.32" Tc=6.0 min CN=83 Runoff=1.33 cfs 0.100 af
Reach 2R: Reach #2 n=0.022	Avg. Flow Depth=0.52' Max Vel=4.86 fps Inflow=25.99 cfs 4.530 af L=31.0' S=0.0213 '/' Capacity=23.64 cfs Outflow=25.99 cfs 4.530 af
Reach 3R: Reach #3 n=0.022	Avg. Flow Depth=0.18' Max Vel=3.68 fps Inflow=4.36 cfs 0.321 af L=868.8' S=0.0516 '/' Capacity=36.82 cfs Outflow=3.90 cfs 0.320 af
Reach 7R: Reach #7 n=0.022	Avg. Flow Depth=0.11' Max Vel=3.21 fps Inflow=1.59 cfs 0.988 af L=772.0' S=0.0762 '/' Capacity=44.74 cfs Outflow=1.59 cfs 0.986 af
Reach 100R: Final Reach #100	Inflow=17.07 cfs 4.734 af

Outflow=17.07 cfs 4.734 af

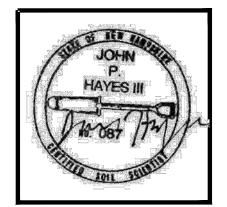
22-051 Proposed A Prepared by Berry Sur HydroCAD® 10.00-25 s/n	nalysis Type III 24-hr 100Yr24Hr. Rainfall=8.36" veying & Engineering (Rev: 4/13/23) Printed 4/13/2023 07605 © 2019 HydroCAD Software Solutions LLC Page 10
Reach 101R: Overland	Flow from R-101 Avg. Flow Depth=0.17' Max Vel=1.65 fps Inflow=1.59 cfs 0.988 af n=0.045 L=32.0' S=0.0469 '/' Capacity=17.15 cfs Outflow=1.59 cfs 0.988 af
Reach 200R: Final Read	ch #200 Inflow=14.15 cfs 0.925 af Outflow=14.15 cfs 0.925 af
Pond 1P: Pond #1 0.252 af Primary=12.28 cfs 4.585 af	Peak Elev=196.95' Storage=16,382 cf Inflow=32.58 cfs 5.913 af Secondary=14.15 cfs 0.925 af Tertiary=4.79 cfs 0.149 af Outflow=32.25 cfs 5.911 af
Pond 7P: Level Spread	er Peak Elev=264.08' Storage=523 cf Inflow=1.59 cfs 0.999 af Outflow=1.59 cfs 0.988 af
Pond 12P: Pond #12	Peak Elev=270.57' Storage=43 cf Inflow=4.42 cfs 0.451 af 15.0" Round Culvert n=0.012 L=40.0' S=0.0050 '/' Outflow=4.40 cfs 0.451 af
Pond 13P: Pond #13	Peak Elev=271.89' Storage=24 cf Inflow=3.54 cfs 0.382 af 18.0" Round Culvert n=0.012 L=45.0' S=0.0222 '/' Outflow=3.54 cfs 0.382 af
Pond 14P: Pond #14	Peak Elev=272.11' Storage=8 cf Inflow=1.18 cfs 0.090 af 12.0" Round Culvert n=0.012 L=21.9' S=0.0183 '/' Outflow=1.18 cfs 0.090 af
Pond 23P: Pond #23	Peak Elev=225.52' Storage=990 cf Inflow=6.19 cfs 0.510 af 15.0" Round Culvert n=0.012 L=65.0' S=0.0769 '/' Outflow=5.60 cfs 0.508 af
Pond 24P: Pond #24	Peak Elev=210.79' Storage=150 cf Inflow=7.79 cfs 0.646 af Primary=7.63 cfs 0.646 af Secondary=0.00 cfs 0.000 af Outflow=7.63 cfs 0.646 af
Pond 25P: Pond #25	Peak Elev=202.26' Storage=255 cf Inflow=8.66 cfs 0.746 af Primary=8.55 cfs 0.746 af Secondary=0.00 cfs 0.000 af Outflow=8.55 cfs 0.746 af
Pond 101P: Rain Garde	en#101 (Built) Peak Elev=268.35' Storage=18,816 cf Inflow=10.06 cfs 1.042 af Primary=1.59 cfs 0.999 af Secondary=0.00 cfs 0.000 af Outflow=1.59 cfs 0.999 af
Pond 202P: Rain Garde	en w/ Infiltration #202 Peak Elev=217.55' Storage=22,352 cf Inflow=8.71 cfs 0.793 af Outflow=0.28 cfs 0.324 af
Total Run	off Area = 18.980 ac Runoff Volume = 6.765 af Average Runoff Depth = 4.28" 90.08% Pervious = 17.098 ac 9.92% Impervious = 1.882 ac



JURISDICTIONAL WETLANDS WERE DELINEATED BY DEIDRA BENJAMIN, CWS IN SEPTEMBER OF 2017 AND UPDATED IN SEPTEMBER OF 2022 UTILIZING THE FOLLOWING

- 1) FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, VERSION 7.0. 2010. L.M. VASILAS, G.W. HURT, AND C.V. NOBLE (EDS.) UNITED STATES DEPARTMENT OF AGRICULTURE, NATURAL RESOURCES CONSERVATION SERVICE, IN COOPERATION WITH THE NATIONAL TECHNICAL
- 2) FIELD INDICATORS FOR IDENTIFYING HYDRIC SOILS IN NEW ENGLAND. VERSION 3. APRIL 2004. NEIWPCC WETLANDS WORKGROUP. WILMINGTON,
- 3) NORTH AMERICAN DIGITAL FLORA: NATIONAL (HTTP://WETLAND_PLANTS.USACE.ARMY.MIL). U.S. ARMY CORPS OF ENGINEERS, ENGINEER RESEARCH AND DEVELOPMENT CENTER, COLD LABORATORY, HANOVER, NH, AND BONAP,
- MELVIN, AND W.N. KIRCHNER. 2014. THE NATIONAL WETLAND PLANT LIST: 2014 UPDATE
- 5) CORPS OF ENGINEERS WETLANDS DELINEATION MANUAL. JANUARY 1987. WETLANDS RESEARCH
- 6) REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTHCENTRAL AND NORTHEAST REGION. JANUARY 2012, VERSION 2. U.S. ARMY CORPS
- 7) CLASSIFICATION OF WETLANDS AND DEEPWATER HABITATS OF THE UNITED STATES. DECEMBER 1979. L. COWARDIN, V. CARTER, F. GOLET, AND E. LAROE. US DEPARTMENT OF THE INTERIOR. FISH AND WILDLIFE SERVICE. FWS/OBS-79/31.

DEIDRA BENJAMIN, CWS #295



JOHN P. HAYES III, CSS #87

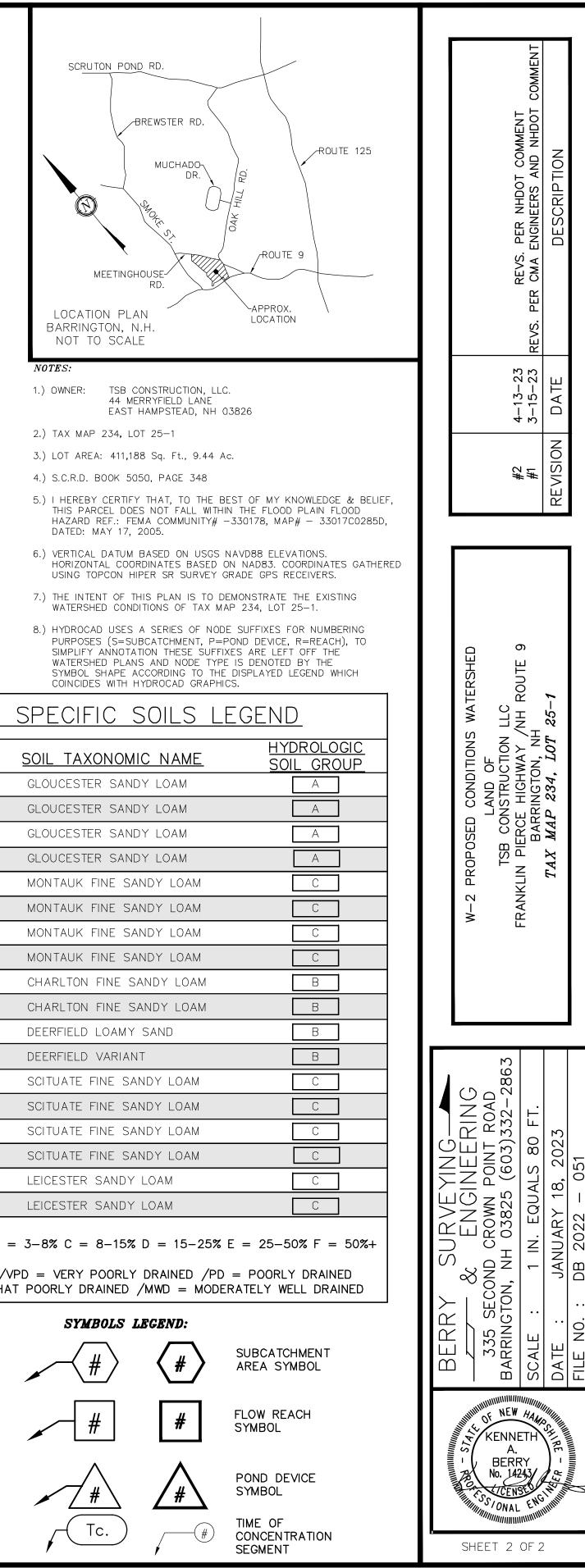
	<u>SITE</u>
<u>SYMBOL</u>	
11A	
11B	
11C	
11D	
44B	
44C	
44D	
44E	
62E	
62F	
313B	
915B	
448B	
448C	
448D	
448E	
514P	
514C	
SLOPE: A =	0-3% B =
DENOMIN /SWPD =	ATOR: /\ SOMEWHA

LEGEND:

448A

НfВ

EXISTING CONTOUR MINOR -------------------------------EXISTING CONTOUR MAJOR STONE WALL WETLAND LINE PROPERTY LINE SOIL LINE SOIL SERIES NRCS SOIL LINE NRCS SOIL LABEL LIMIT OF WATERSHED TIME OF CONCENTRATION PATH FLOW REACH MATCH LINE S.C.R.D. STRAFFORD COUNTY REGISTRY OF DEEDS TYP. TYPICAL FND FOUND TBR TO BE REMOVED



Appendix III - Calculations, Charts, & Graphs

Extreme Precipitation Tables Rip-Rap Calculations NHDES AOT Spreadsheet USDA / NRCS Websoil Site Specific Soil Survey Report & Plan Infiltration Feasibility Study KSat NH Manual Stormwater System Management: Inspection and Maintenance Manual Filtrexx Silt Soxx Specifications Cut Sheets

Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes
State	New Hampshire
Location	
Longitude	71.006 degrees West
Latitude	43.216 degrees North
Elevation	0 feet
Date/Time	Mon, 10 Oct 2022 13:45:07 -0400

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.49	0.65	0.81	1.02	1yr	0.70	0.98	1.19	1.53	1.97	2.56	2.81	1yr	2.26	2.70	3.11	3.84	4.40	1yr
2yr	0.32	0.49	0.61	0.80	1.01	1.28	2yr	0.87	1.16	1.49	1.89	2.41	3.08	3.42	2yr	2.72	3.29	3.79	4.52	5.15	2yr
5yr	0.37	0.57	0.72	0.96	1.23	1.57	5yr	1.06	1.44	1.84	2.36	3.02	3.88	4.38	5yr	3.44	4.21	4.83	5.69	6.44	5yr
10yr	0.40	0.63	0.80	1.09	1.42	1.84	10yr	1.22	1.69	2.17	2.80	3.60	4.63	5.27	10yr	4.10	5.07	5.80	6.78	7.63	10yr
25yr	0.46	0.74	0.94	1.30	1.73	2.27	25yr	1.49	2.08	2.69	3.49	4.53	5.85	6.75	25yr	5.18	6.49	7.40	8.56	9.56	25yr
50yr	0.52	0.83	1.07	1.49	2.01	2.67	50yr	1.73	2.45	3.17	4.15	5.40	6.99	8.14	50yr	6.19	7.83	8.89	10.21	11.35	50yr
100yr	0.58	0.94	1.21	1.72	2.34	3.13	100yr	2.02	2.88	3.74	4.92	6.42	8.36	9.83	100yr	7.39	9.45	10.69	12.19	13.47	100yr
200yr	0.65	1.06	1.37	1.97	2.72	3.68	200yr	2.35	3.39	4.43	5.85	7.66	9.99	11.86	200yr	8.84	11.40	12.87	14.55	16.00	200yr
500yr	0.76	1.26	1.64	2.38	3.33	4.56	500yr	2.88	4.21	5.50	7.32	9.66	12.65	15.21	500yr	11.20	14.63	16.44	18.41	20.10	500yr

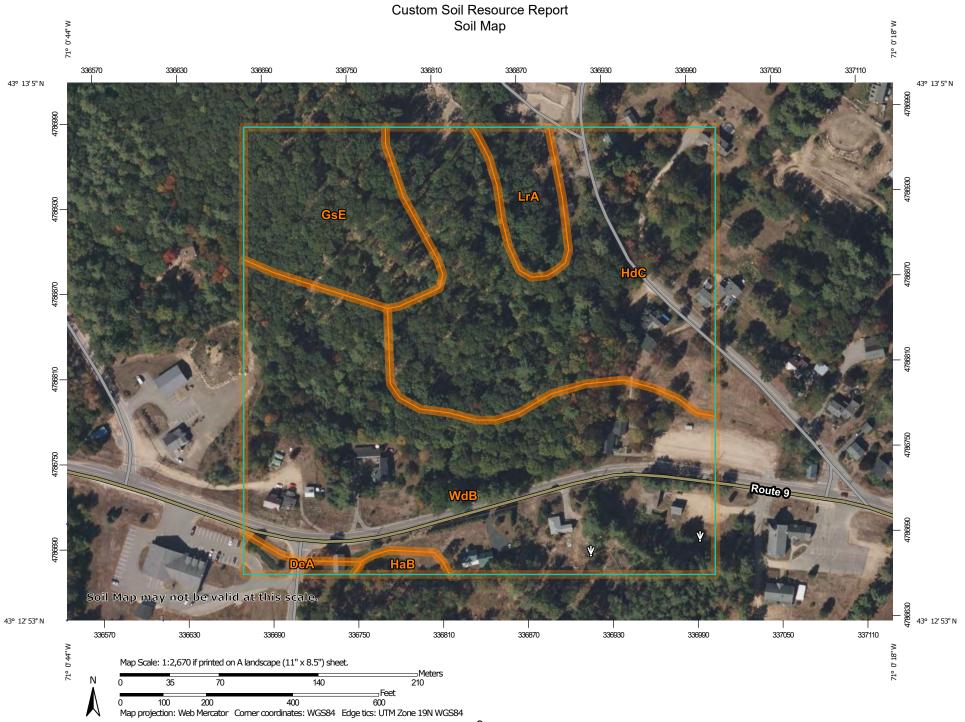
Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.24	0.36	0.44	0.60	0.73	0.90	1yr	0.63	0.88	0.91	1.25	1.52	1.94	2.48	1yr	1.72	2.39	2.92	3.28	3.94	1yr
2yr	0.31	0.48	0.60	0.81	0.99	1.18	2yr	0.86	1.15	1.35	1.81	2.34	2.98	3.33	2yr	2.64	3.20	3.68	4.40	5.02	2yr
5yr	0.35	0.54	0.67	0.91	1.16	1.40	5yr	1.00	1.37	1.61	2.14	2.77	3.59	4.03	5yr	3.18	3.88	4.49	5.34	6.01	5yr
10yr	0.38	0.59	0.73	1.02	1.32	1.60	10yr	1.14	1.56	1.81	2.43	3.12	4.11	4.67	10yr	3.64	4.49	5.20	6.17	6.87	10yr
25yr	0.44	0.67	0.83	1.19	1.57	1.91	25yr	1.35	1.87	2.12	2.84	3.63	4.90	5.63	25yr	4.34	5.42	6.34	7.46	8.24	25yr
50yr	0.49	0.74	0.92	1.33	1.79	2.19	50yr	1.54	2.15	2.38	3.20	4.06	5.60	6.49	50yr	4.95	6.24	7.36	8.62	9.51	50yr
100yr	0.55	0.83	1.03	1.49	2.05	2.52	100yr	1.77	2.46	2.67	3.60	4.53	6.37	7.47	100yr	5.64	7.18	8.55	9.96	10.87	100yr
200yr	0.61	0.92	1.16	1.68	2.35	2.89	200yr	2.03	2.83	3.00	4.04	5.06	7.25	8.81	200yr	6.42	8.47	9.95	11.50	12.45	200yr
500yr	0.72	1.06	1.37	1.99	2.83	3.50	500yr	2.44	3.42	3.51	4.72	5.87	8.55	10.67	500yr	7.57	10.26	12.17	13.93	14.83	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.43	0.53	0.71	0.87	1.07	1yr	0.75	1.05	1.23	1.72	2.18	2.76	3.02	1yr	2.44	2.91	3.34	4.12	4.74	1yr
2yr	0.33	0.50	0.62	0.84	1.03	1.24	2yr	0.89	1.21	1.46	1.94	2.50	3.19	3.54	2yr	2.82	3.40	3.92	4.65	5.30	2yr
5yr	0.39	0.60	0.75	1.02	1.30	1.57	5yr	1.12	1.53	1.83	2.47	3.16	4.18	4.71	5yr	3.70	4.53	5.18	6.05	6.84	5yr
10yr	0.45	0.70	0.87	1.21	1.56	1.90	10yr	1.35	1.86	2.21	3.00	3.80	5.16	5.86	10yr	4.56	5.64	6.42	7.39	8.32	10yr
25yr	0.55	0.84	1.05	1.50	1.97	2.44	25yr	1.70	2.38	2.84	3.90	4.87	6.83	7.84	25yr	6.05	7.54	8.51	9.73	10.72	25yr
50yr	0.64	0.97	1.21	1.74	2.35	2.94	50yr	2.02	2.87	3.44	4.74	5.89	8.46	9.79	50yr	7.48	9.42	10.55	11.94	13.12	50yr
100yr	0.75	1.13	1.41	2.04	2.80	3.54	100yr	2.41	3.46	4.17	5.79	7.15	10.47	12.23	100yr	9.27	11.76	13.07	14.66	16.00	100yr
200yr	0.86	1.30	1.65	2.39	3.33	4.27	200yr	2.87	4.18	5.06	7.06	8.65	13.01	15.03	200yr	11.52	14.46	16.19	17.99	19.56	200yr
500yr	1.06	1.57	2.02	2.94	4.18	5.47	500yr	3.61	5.34	6.51	9.21	11.17	17.38	20.13	500yr	15.38	19.36	21.49	23.64	25.53	500yr





	MAP L	EGEND)	MAP INFORMATION
Area of Int	terest (AOI)	00	Spoil Area	The soil surveys that comprise your AOI were mapped at
	Area of Interest (AOI)	۵	Stony Spot	1:20,000.
Soils	Call Man Link Dahmana	Ø	Very Stony Spot	Warning: Soil Map may not be valid at this scale.
	Soil Map Unit Polygons	Ŷ	Wet Spot	
~	Soil Map Unit Lines	Δ	Other	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil
	Soil Map Unit Points		Special Line Features	line placement. The maps do not show the small areas of
Special	Point Features Blowout	Water Fea	atures	contrasting soils that could have been shown at a more detailed scale.
× ×	Borrow Pit	\sim	Streams and Canals	
×	Clay Spot	Transport		Please rely on the bar scale on each map sheet for map
Ô	Closed Depression	••••	Rails	measurements.
×	Gravel Pit	~	Interstate Highways	Source of Map: Natural Resources Conservation Service
°. נש	Gravelly Spot	~	US Routes	Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
	Landfill	~	Major Roads	
Ň.	Lava Flow	~	Local Roads	Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts
n. alis	Marsh or swamp	Backgrou	nd Aerial Photography	distance and area. A projection that preserves area, such as the
*	Mine or Quarry		, tonar i notography	Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
0	Miscellaneous Water			
0	Perennial Water			This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
0	Rock Outcrop			
~	Saline Spot			Soil Survey Area: Strafford County, New Hampshire Survey Area Data: Version 23, Sep 9, 2022
+	Sandy Spot			
0 0 0 0	5 .			Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.
÷	Severely Eroded Spot			
0	Sinkhole			Date(s) aerial images were photographed: Jun 19, 2020—Sep 20, 2020
	Slide or Slip			
ø	Sodic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
DeA	Deerfield loamy fine sand, 0 to 3 percent slopes	0.3	1.1%
GsE	Gloucester very stony fine sandy loam, 25 to 60 percent slopes	3.4	12.9%
НаВ	Hinckley loamy sand, 3 to 8 percent slopes	0.2	0.9%
HdC	Hollis-Charlton very rocky fine sandy loams, 8 to 15 percent slopes	9.4	36.0%
LrA	Leicester-Ridgebury fine sandy loams, 0 to 3 percent slopes, very stony	1.1	4.3%
WdB	Windsor loamy sand, 3 to 8 percent slopes	11.7	44.8%
Totals for Area of Interest		26.1	100.0%

Map Unit Legend

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit

descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.



GROUNDWATER RECHARGE VOLULME (GRV) CALCULATION (Env-Wq 1507.04)

0.24	ас	Area of HSG A soil that was replaced by impervious cover	0.40"
0.10	ac	Area of HSG B soil that was replaced by impervious cover	0.25"
0.39	ac	Area of HSG C soil that was replaced by impervious cover	0.10"
(0.02)	ac	Area of HSG D soil or impervious cover that was replaced by impervious cover	0.0"
0.22	inches	Rd = Weighted groundwater recharge depth	
0.158	ac-in	GRV = AI * Rd	
574	cf	GRV conversion (ac-in x 43,560 sf/ac x 1ft/12")	

Provide calculations below showing that the project meets the groundwater recharge requirements (Env-Wq 1507.04):

Due to the development of Meetinghouse Road resulting in a change in subcatchments off site from the proposed development area of this project there is a minor decrease in impervious HSG D soils. A portion of the HSG D soils was captured in a different subcatchment of the Meetinghouse Road project and sent to a different final reach.



FILTRATION PRACTICE DESIGN CRITERIA (Env-Wq 1508.07)

Type/Node Name:

Infiltration Rain Garden #202

Enter the type of filtration practice (e.g., bioretention system) and the node name in the drainage analysis, if applicable.

		Check if you reviewed the restrictions on unlined systems outlined in Env-Wq 1508.07	7(a).
2.27	ac	A = Area draining to the practice	. ,
0.19	-	A _l = Impervious area draining to the practice	
0.08	decimal	I = Percent impervious area draining to the practice, in decimal form	
	unitless	$Rv = Runoff coefficient = 0.05 + (0.9 \times I)$	
0.28	ac-in	WQV= 1" x Rv x A	
1,023	cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
256	cf	25% x WQV (check calc for sediment forebay volume)	
767	cf	75% x WQV (check calc for surface sand filter volume)	
Sedimen	t Forebay	Method of Pretreatment? (not required for clean or roof runoff)	
2,535	cf	V _{SED} = Sediment forebay volume, if used for pretreatment	<u>></u> 25%WQV
Calculate ti	me to drain	if system IS NOT underdrained:	
4,000	sf	A _{SA} = Surface area of the practice	
3.00	iph	Ksat _{DESIGN} = Design infiltration rate ¹	
	-	If Ksat (prior to factor of safety) is < 0.50 iph, has an underdrain been provided?	
N/A	Yes/No	(Use the calculations below)	
1.0	hours	$T_{DRAIN} = Drain time = V / (A_{SA} * I_{DESIGN})$	<u><</u> 72-hrs
Calculate ti	me to drain	if system IS underdrained:	
	ft	E_{WQV} = Elevation of WQV (attach stage-storage table)	
	cfs	Q_{WQV} = Discharge at the E_{WQV} (attach stage-discharge table)	
-	hours	$T_{DRAIN} = Drain time = 2WQV/Q_{WQV}$	<u><</u> 72-hrs
213.00	feet	E _{FC} = Elevation of the bottom of the filter course material ²	
	feet	E_{UD} = Invert elevation of the underdrain (UD), if applicable	
209.58	feet	E_{SHWT} = Elevation of SHWT (if none found, enter the lowest elevation of the test p	it)
207.33	feet	E_{ROCK} = Elevation of bedrock (if none found, enter the lowest elevation of the test	pit)
213.00	feet	$D_{FC to UD}$ = Depth to UD from the bottom of the filter course	<u>></u> 1'
5.67	feet	D _{FC to ROCK} = Depth to bedrock from the bottom of the filter course	<u>></u> 1'
3.42	feet	D _{FC to SHWT} = Depth to SHWT from the bottom of the filter course	<u>></u> 1'
216.58	ft	Peak elevation of the 50-year storm event (infiltration can be used in analysis)	
218.25	ft	Elevation of the top of the practice	
YES		50 peak elevation <u><</u> Elevation of the top of the practice	← yes
	sand filter	or underground sand filter is proposed:	
YES	ас	Drainage Area check.	< 10 ac
	cf	V = Volume of storage ³ (attach a stage-storage table)	<u>></u> 75%WQV
	inches	D _{FC} = Filter course thickness	18", or 24" if
	_		within GPA
Sheet	lesson and the second sec	Note what sheet in the plan set contains the filter course specification.	
	Yes/No	Access grate provided?	← yes

If a biorete	ention area	is proposed:	
YES	ас	Drainage Area no larger than 5 ac?	← yes
23,225	cf	V = Volume of storage ³ (attach a stage-storage table)	<u>></u> WQV
18.0	inches	D _{FC} = Filter course thickness	18", or 24" if within GPA
Sheet	t P-202	Note what sheet in the plan set contains the filter course specification	
3.0) :1	Pond side slopes	<u>> 3</u> :1
Sheet	t P-202	Note what sheet in the plan set contains the planting plans and surface cover	
If porous p	oavement is	proposed:	
		Type of pavement proposed (Concrete? Asphalt? Pavers? Etc.)	
	acres	A _{SA} = Surface area of the pervious pavement	
	:1	Ratio of the contributing area to the pervious surface area	≤ 5:1
	inches	D _{FC} = Filter course thickness	12", or 18" if within GPA
Sheet	t	Note what sheet in the plan set contains the filter course spec.	mod. 304.1 (see spec)

1. Rate of the limiting layer (either the filter course or the underlying soil). Ksat_{design} includes factor of safey. See Env-Wq 1504.14 for guidance on determining the infiltration rate.

2. See lines 34, 40 and 48 for required depths of filter media.

3. Volume without depending on infiltration. The volume includes the storage above the filter (but below the invert of the outlet stucture, if any), the filter media voids, and the pretreatment area. The storage above the filter media shall not include the volume above the outlet structure, if any.

Designer's Notes:

NHDES Alteration of Terrain

Last Revised: January 2019

Summary for Pond 202P: Rain Garden w/ Infiltration #202

Inflow Area =	2.266 ac,	8.26% Impervious, Inflow Dep	pth > 3.11" for 50Yr24Hr. event
Inflow =	6.56 cfs @	12.19 hrs, Volume=	0.587 af
Outflow =	0.28 cfs @	11.60 hrs, Volume=	0.310 af, Atten= 96%, Lag= 0.0 min
Discarded =	0.28 cfs @	11.60 hrs, Volume=	0.310 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 216.58' @ 16.73 hrs Surf.Area= 4,000 sf Storage= 15,042 cf Flood Elev= 218.25' Surf.Area= 4,000 sf Storage= 28,301 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 190.0 min (1,036.2 - 846.3)

Volume	Invert Av	ail.Storage	Storage Descripti	on			
#1	212.00'	1,600 cf		Listed below (Rec	alc)		
			4,000 cf Overall				
#2	213.00'	1,200 cf			Recalc) -Imperviou	IS	
#0				6,000 cf Overall x 20.0% Voids Open Water Storage (Irregular)Listed below (Recalc) -Impervious			
#3	214.50'	25,501 cf			ted below (Recalc	<u>) - Impervio</u> us	
		28,301 cf	Total Available St	orage			
Elevation	Surf.Area	a Perim.	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
212.00	4,000) 258.6	0	0	4,000		
213.00	4,000	258.6	4,000	4,000	4,259		
Elevation	Surf.Area	a Perim.	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
213.00	4,000	258.6	0	0	4,000		
214.50	4,000	258.6	6,000	6,000	4,388		
-	• • • •	. .					
Elevation	Surf.Area	-	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
214.50	4,000) 258.6	0	0	4,000		
214.75	5,115	5 327.0	1,137	1,137	7,188		
215.00	5,365	5 332.7	1,310	2,446	7,499		
216.00	6,398	353.1	5,874	8,320	8,664		
217.00	7,486	371.9	6,935	15,255	9,807		
218.00	8,630	390.8	8,051	23,306	11,016		
218.25	8,92	5 395.5	2,194	25,501	11,326		
Device R	outing	Invert Outl	et Devices				
#1 Discarded 212.00' 3.000 in/br Exfiltration over Surface area							

#1 Discarded 212.00' **3.000 in/hr Exfiltration over Surface area**

Discarded OutFlow Max=0.28 cfs @ 11.60 hrs HW=212.08' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.28 cfs) Prepared by Berry Surveying & Engineering (Rev: 3/15/23) HydroCAD® 10.00-25 s/n 07605 © 2019 HydroCAD Software Solutions LLC

Stage-Area-Storage for Pond 202P: Rain Garden w/ Infiltration #202

Elevation	Surface	Storage	Elevation	Surface	Storage	
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)	
212.00	4,000	0	217.30	4,000	20,351	
212.10	4,000	160	217.40	4,000	21,139	
212.20	4,000	320	217.50	4,000	21,938	
212.30	4,000	480	217.60	4,000	22,748	
212.40	4,000	640	217.70	4,000	23,570	
212.50	4,000	800	217.80	4,000	<u> </u>	
212.60	4,000	960	217.90	4,000	25 249	Spillway Elev:
212.70	4,000	1,120	218.00	4,000	26,106	217.85=24,825
212.80	4,000	1,280	218.10	4,000	26,975	
212.90	4,000	1,440	218.20	4,000	27,856	
213.00	4,000	1,600		,	,	
213.10	4,000	1,680				
213.20	4,000	1,760				
213.30	4,000	1,840	04.00			-
213.40	4,000	1,920	24,825	o CF Storage	e at E Spillwa	У
213.50	4,000	2,000	-1.60	0 CF Storad	e within ston	e
213.60	4,000	2,080		-	BMP Storage	
213.70	4,000	2,160	23,		Divit Storay	
213.80	4,000	2,240				
213.90	4,000	2,320				
214.00	4,000	2,400				
214.10	4,000	2,480				
214.20	4,000	2,560				
214.30	4,000	2,640				
214.40	4,000	2,720				
214.50	4,000	2,800				
214.60	4,000	3,221				
214.00	4,000	3,687				
214.80	4,000	4,194				
214.90	4,000	4,715				
214.90	4,000	5,246				
215.10	4,000	5,788				
215.20	4,000	6,339				
215.30	4,000	6,901				
215.40	4,000	7,472				
215.50 215.60	4,000	8,054 8,646				
215.60	4,000	8,646				
215.70	4,000	9,249				
215.80	4,000	9,862				
215.90	4,000	10,486				
216.00	4,000	11,120				
216.10	4,000	11,765				
216.20	4,000	12,421 13,087				
216.30	4,000					
216.40	4,000	13,764				
216.50	4,000	14,452				
216.60	4,000	15,150				
216.70	4,000	15,860				
216.80	4,000	16,581				
216.90	4,000	17,312				
217.00	4,000	18,055				
217.10	4,000	18,809				
217.20	4,000	19,575				
		, i				

RIP RAP CALCULATIONS

22-051 TSB Construction, LLC

Dover, NH

Berry Surveying & Engineering

335 Second Crown Point Road

Barrington, NH

1/18/2023: Rev 4/13/23

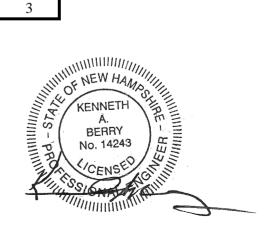
Rip Rap equations were obtained from the *Stormwater Management and Erosion Control Handbook for Urban and Developing Areas in New Hampshire.* Rip Rap was sized for the 25 year storm event. (Some d50 sizes and T values have been modified)

TAILWATER < HALF THE Do

La = (1.8 x Q) / Do 3/2 + (7 x Do) Q = Peak Flow & Do is Pipe Diameter W = La + 3*Do or defined channel width								
$d50 = (0.02 \text{ x } \text{Q4/3}) / (\text{Tw x Do}) \qquad \text{Tw} = \text{Tailwater Depth}$								
T = Largest Stone Size x 1.5								
Culvert or	Tailwater	Discharge	Diameter	Length of	Width of	d50-Stone		
Catch Basin	(Feet)	(C.F.S.)	of Pipe	Rip Rap	Rip Rap	Rip Rap	Actual	
	Tw	Q	Do	La (feet)	W (feet)	d50(ft.)	Size	Thickness
15" HDPE (Pond #23)	0.25	3.24	1.25	12.9	16.7	0.31	0.50	1.20
15" HDPE (Pond #24)	0.25	4.59	1.25	14.7	18.4	0.49	0.50	1.20
18" HDPE (Pond #25)	0.30	5.29	1.50	15.7	20.2	0.41	0.50	1.20

Please note that the designer chose to use the 25 Year Event for the dimensional calculations.

Table 7-24 Recommended Rip Rap Gradation Ranges					
d50 Size =	0.5	Feet	6	Inches	
% of Weight Smaller	% of Weight Smaller Size of Stone (Inches)				
Than the Given d50 Size		From		То	
100%		9		12	
85%		8		11	
50%		6		9	
15%		2		3	





BERRY SURVEYING & ENGINEERING

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Inspection and Maintenance Manual

Stormwater System Management

Tax Map 234, Lot 25-1 Franklin Pierce Highway Barrington, NH 03825

Prepared for:

TSB Construction, LLC 44 Merryfield Lane East Hampstead, NH 03826

Prepared By

Berry Surveying & Engineering 335 Second Crown Point Road Barrington, NH 03825 603-332-2863

2063 HILLINGENEW HAMSON HENNETH A. BERRY No. 14243 HENNETH A. BERRY No. 14243 HENNETH A. BERRY No. 14243 HENNETH HENNETH A. BERRY NO. 14243 HENNETH HENNEH

File Number DB2022-051

January 18, 2023 Revised: March 15, 2023

Inspection and Maintenance Manual

Stormwater System Management

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Owner Certification	Page 16
Operation & Maintenance Plans	Attached – 1 Page
Control of Invasive Plants, NH Department of Agriculture	Attached – 4 Pages
NHDES Green SnoPro Utilization Chart	Attached – 1 Page

Introduction

The Best Management Practices (BMP) described in this manual are specified in more detail within the plan set giving design details and specifications. The <u>New Hampshire</u> <u>Stormwater Manual</u>, Volume 2, Post-Construction Best Management Practices Selection & Design (December 2008, NHDES & US EPA) is included by reference to this manual. Additional details, construction specifications, and example drawings are provided within this reference. (<u>http://des.nh.gov/organization/divisions/water/stormwater/</u>)

The BMP's are covered below in the general order in which the storm water flows. Each BMP has a description and maintenance consideration listed. A Check List table is proved after the narrative to summarize the maintenance responsibilities and schedule. A Log Form is also provided for the owners use.

For details regarding the design of the Storm Water System see also <u>Drainage Analysis</u> <u>& Sediment and Erosion</u>, **January 18**, **2023**, as revised. See also plan set completed for **TSB Construction**, **LLC**, originally dated **January 18**, **2023**, as revised.

TSB Construction, LLC, development company with David Coish as the operator, is responsible for the Operation, Inspection, and Maintenance of the Stormwater Management System. A significant step in this responsibility is the Inspection and Maintenance of each component of the system. Ongoing, semi-annual, and annual inspection and maintenance requirement are documented below and must be followed diligently. Failure of any component of the system can result in surface water run-off ponding and/or freezing in the roadway and parking lots, leaving the developed site untreated, and/or causing violations to issued permits. The owner / developer must maintain, and have available, plans of the Stormwater System in order properly inspect and maintain the system. (Reduced copies attached.) The responsible party will conduct the inspections, complete the required maintenance, and will maintain the Inspection & Maintenance Check Lists and Logs, and will provide copies with the Annual Report to the Town of Barrington, Planning Department by December 15th of each year. If the development is converted from rental units or is sold to a subsequent party, copies of the Inspection & Maintenance Manual complete with attachments and plans must be transferred to the incoming management / ownership team. The

The owners of Tax Map 234, Lot 25-1, TSB Construction, LLC, are proposing a residential multi-family site development consisting of 4 residential units made up of one duplex and two single family homes and 618 linear feet of shared driveway.

The following practices and drainage features will all require periodic inspections and maintenance based on this manual and drainage layout:

Drainage Pipes

Conveyance Swales

Detention Pond #201 with forebay

Rain Garden w/ Infiltration #202 with forebay and spillway

Conveyance Swales (Grassed & Rip-Rap)

<u>Description</u>: "Swales are stabilized channels designed to convey runoff at non-erosive velocities." (NHDES SWM) They will be trapezoidal or parabolic in section view. A conveyance swale is intended to move surface water runoff from one point to another where as a treatment swale will slow the velocity to a point where sediment will settle out of the stormwater flow. A treatment swale will be constructed to a width of between four and eight feet and have a minimum length of 100 feet. The flow characteristics will also meet design criteria. See SWM Volume 2, 4.3 Treatment Practices, 5. Treatment Swales, page 123.

<u>Project Intent</u>: The swales are individually designed in the drainage analysis and specified on the design plans. The designed swales must have greater than 85% vegetated growth prior to receiving runoff. The bottom of a treatment swale must be above the seasonal high water table.

<u>Maintenance Considerations:</u> Grassed swales will be inspected twice annually, removing accumulated sediment and gross solids. Grass will be mowed periodically but to a depth of not less than 4 inches. Any damage to the vegetation will be repaired and woody vegetation and invasive vegetation will be removed. Rip-rap lined swales should be inspected to ensure the rip-rap is not being displaced by flow or vegetation. Excess sedimentation should be removed.

Culvert Pipes, Flared End Sections / Headwalls

<u>Description</u>: Culvert pipes are placed to route surface water runoff from catch basins to drain manholes, and drain manholes to a discharge point conveying the runoff in such a manner that erosion does not take place. Culvert pipes are often terminated with flared end sections or headwalls.

<u>Maintenance Considerations</u>: The entrance and exit of the culvert pipe should be cleaned of any trash and sediment build-up. The culvert should be clear to let runoff pass through the culvert unobstructed. Flared end sections and headwalls should be inspected for erosion and destabilization, with repairs made as required.

Sediment Forebay

<u>Description:</u> A sediment forebay is designed to reduce the velocity of incoming surface water runoff allowing sediment to fall out of suspension initially pre-treating the runoff before it is sent to a treatment structure. This earthen basin will have vegetated side-slopes and a check dam to further reduce and pretreat the runoff. At the point of incoming runoff, the basin will be protected by rip rap outlet protection construction and the outgoing edge will be protected with rip rap. The check dam will be constructed from one side of the basin to the other and cause runoff to either go through or over. The volume of the forebay is generally 10% the volume of the Water Quality Volume (WQV) for gravel wetlands, and 25% for rain gardens. A dewatering drain is designed into the two-foot berm of the forebay. Construction specifications are included in the plan set and New Hampshire Stormwater Manual, Volume 2, 4-4 Pretreatment Practices 1, Sediment Forebays.

<u>Maintenance Considerations:</u> The basin and slopes will be periodically mowed, at least twice per year ensuring that woody material does not get an opportunity to grow. Sediment accumulated in the basin will be removed and properly disposed of when it reaches half the height of the check dam. Erosion or other damage to the basin will be repaired and revegetated. (See Outlet Protection) Inspect and clean the dewatering drain to ensure runoff is not trapped for more than 72 hours in the forebay.

Detention Ponds

Description:

A detention basin is an impoundment designed to temporarily store runoff and release it at a controlled rate, reducing the intensity of peak flows during storm events. Conventional detention basins are typically designed to control peak runoff rates under a range of storm conditions, and can be used to control discharges as required under the AoT Regulations and other requirements. Construction specifications are included in the plan set and New Hampshire Stormwater Manual, Volume 2, 4-6 Conveyance Practices, 1, Detention Basin.

Maintenance Considerations:

Detention Ponds should be inspected at least twice annually and following any rainfall event exceeding 0.25 inches in a twenty-four hour period. Maintenance rehabilitation will be conducted as warranted by each inspection. Trash and debris will be removed at each inspection.

Also on an annual basis the vegetation should be inspected to ensure healthy condition. Invasive species need to be removed along with dead or diseased vegetation.

Bio-Filtration System (Rain Gardens)

Description:

Rain Gardens, or bio-filtration areas are located close to the source of runoff. They are intended to integrate with the site landscaping an become an aesthetically attractive opportunity to provide highly effective stormwater treatment. The rain gardens associated with this proposed development contribute toward recharge of surface water run-off into the ground. It is important that sediment be removed from run-off prior to discharge into the bio-filtration area to preserve the mulch and soil mix ratio. During construction it is important that the ground surface not be exposed to traffic or construction specifications are included in the plan set and New Hampshire Stormwater Manual, Volume 2, 4-3 Treatment Practices, 4c Bioretention System. (Bio-media and bio-filtration mean bioretention filter media.)

Maintenance Considerations:

Rain Gardens should be inspected at least twice annually and following any rainfall event exceeding 0.25 inches in a twenty-four hour period. Maintenance rehabilitation will be conducted as warranted by each inspection. Trash and debris will be removed at each inspection.

On an annual basis the infiltration capabilities need to be confirmed by evaluation the drawdown time. If the bio-filtration system does not drain within 72-hours following a rainfall event, a qualified professional will assess the condition of the rain garden to determine measures required to restore the infiltration function. This is normally the direct result of sediment accumulation which will be removed to restore the filter media ratio.

Also on an annual basis the vegetation should be inspected to ensure healthy condition. Invasive species need to be removed along with dead or diseased vegetation.

Rip Rap Outlet Protection, Level Spreaders, & Emergency Spillways

<u>Description</u>: Outlet Protection consists of a riprap apron or preformed scour hole that is designed to provide velocity reduction of the surface water run-off that is leaving a culvert. The design is dependent on the culvert size, soil conditions, velocity, and quantity of the run-off. There are to be no bend or curves at the intersection of the

conduit and apron. Level spreaders are intended to provide a level lip where surface water runoff is allowed to continue downhill closer to sheet flow. The level lip is to be constructed as level as possible for the entire length. Emergency Spillways are rip rap reinforced outlets near the top of the berm that allow runoff to leave a practice during periods of very high flow. Ref.: NHDES SWM Volume 2, Section 4-6 Conveyance Practices, 6. Outlet Protection and 1. Detention Ponds, Note 3, Page 158.

<u>Maintenance Considerations:</u> The riprap outlet protection will be inspected annually for damage, which must be corrected immediately. Any sediment buildup will be removed and disposed of correctly. Sediment and subsequent vegetation will build up in the Level Spreader. This material will be cleaned out along with any gross solids and disposed of properly. (See invasive species below) Any rip rap that has been displaced from the original construction will be repaired, especially recreating the level lip.

Stabilization for Long Term Cover

Vegetated Stabilization – Original Planting

All areas that are disturbed during construction will be stabilized with vegetated material within 30 days of breaking ground. Construction will be managed in such a manner that erosion is prevented and that no abutter's property will be subjected to any siltation, unless otherwise permitted. All areas to be planted with grass for long-term cover will follow the specification and on Sheet E-102 using seeding mixture C, as follows:

Mixturo	e Tall Fescue Creeping Red Fescue Total	Pounds per Acre 24 24 48	Pounds per 1,000 Sq. Ft. 0.55 0.55 1.10
Conse	ervation Mix		
	Virginia Wild Rye Little Bluestem Big Bluestem Red Fescue Switch Grass Partridge Pea Showy Tick Trefoil Butterfly Milkweed Beggar Ticks Purple Joe Pye Weed	Native Native Native Native Native Native Native Native Native Native	FACW- FACU FAC FACU FAC FACU FAC NI FACW FAC

Black Eyed Susan	Native	FACU-
Total	25	0.57

Conservation Mix will used to stabilize all 2:1 slopes. As the site is to be stabilized with erosion control mix as a mulch, the vegetation should be established with a high percentage of white clover for growth to be established.

Rain Garden Mix

The grass that is planted within a rain garden bio-filtration system within the bio-media must consist of a combination of warm season grass seed and cold season grass seed in order for the grass to start growing for stabilization and continue growing in the sandy well-drained environment. Planting specification will meet the requirements as outlined in 'Vegetation New Hampshire Sand and Gravel Pits' mix 1 (warm season grasses) (15 lbs/ac) and include annual and perennial rye grass seed (15 lbs/ac); the New England native warm season grass mix (23 lbs/ac) by New England Wetland Plants, Inc.; rain garden mix 180 (15 lbs/ac & 15 lbs/ac of rye) / rain garden grass mix 180-1 (20 lbs/ac & 10 lbs/ac of rye) by Ernst Conservation Seeds; or approved equal.

<u>Maintenance Considerations:</u> Permanent seeded areas for long-term cover will be inspected on a periodic basis looking for signs of growth loss or erosion. Any areas found to be damaged will be repaired and replanted to reestablish the growth. The grass should be mowed at least twice per year and any dead material removed. Any woody growth that becomes established will need to be cut and removed.

Long-term maintenance of the land cover is critical and must be maintained at least 85% grass / vegetation coverage, must be inspected for concentrated flow, rills, and channels; and must be repaired as necessary to prevent erosion.

CONTROL OF INVASIVE PLANTS

During maintenance activities, check for the presence of invasive plants and remove in a safe manner as described on the following pages. They should be controlled as described on the following pages.

Invasive plants are introduced, alien, or non-native plants, which have been moved by people from their native habitat to a new area. Some exotic plants are imported for human use such as landscaping, erosion control, or food crops. They also can arrive as "hitchhikers" among shipments of other plants, seeds, packing materials, or fresh produce. Some exotic plants become invasive and cause harm by:

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- becoming weedy and overgrown;
- killing established shade trees;
- obstructing pipes and drainage systems;
- forming dense beds in water;
- lowering water levels in lakes, streams, and wetlands;
- destroying natural communities;
- promoting erosion on stream banks and hillsides; and
- resisting control except by hazardous chemical.

Annual Report

Description: The owner is responsible to keep an **I & M Activity Log** that documents inspection, maintenance and repairs to the storm water management system, and a **Deicing Log** to track the amount and type of deicing material applied to the site. The original owner is responsible to ensure that any subsequent owner (s) have copies of the <u>Inspection & Maintenance Manual</u>, Stormwater System Management, copies of past logs and check lists. This includes any owner association that might become involved with the property. The Annual Report will be prepared and submitted to Town of Barrington, Planning Department with copies of both logs and check lists no later than December 15th of each year. Upon an ownership change, the Annual Report will include the Transfer of Ownership Responsibility Forms duplicated from the form found below.

The plans that accompany this manual includes four sheets, "Drainage Operation, Inspection, & Maintenance Plan". The owners and municipality will also maintain a complete set of the approved original design plans.

Respectfully, BERRY SURVEYING & ENGINEERING

Kevin R. Poulin, EIT Project Engineer / Manager

Kenneth A. Berry, PE, LLS CPSWQ, CPESC, CESSWI Principal, VP – Technical Operations

STORMWATER SYSTEM: INSPECTION AND MAINTENANCE MANUAL

Inspection & Maintenance Manual Checklist

Franklin Pierce Highway, Barrington, NH 03825

TSB Construction, LLC 44 Merryfield Lane, Hampstead, NH 03826

R	Date	BMP / System	Minimum Inspection Frequency	Minimum Inspection Requirements	Maintenance / Cleanout Threshold
		Pavement Sweeping	Three Times Per Year	N/A	N/A
		Litter/Trash Removal	Routinely	Inspect swales, inlet sumps, and yard areas.	Parcel will be free of litter/trash.
		Deicing Agents	N/A	N/A	Use salt as the primary agent for roadway safety during winter.
		Invasive Species	Two times per year.	Inspect for Invasive Species	Remove and dispose invasive species.
		Closed Draina	ige System:		
		Drainage Pipes & Inlet Sumps	2 times per year	Check for sediment accumulation & clogging.	Less than 2" sediment depth
	Date	BMP / System	Minimum Inspection Frequency	Minimum Inspection Requirements	Maintenance / Cleanout Threshold

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Detention Ponds, Infiltration Ponds	2 times per year	Check for sediment and debris accumulation buildup.	Remove sediment & debris when required. Remove Invasive Species
Riprap Outlet Protection	Annually	Check for sediment buildup and structure damage.	Remove excess sediment and repair damage.
Winter Maintenance	Ongoing	Remove snow as directed.	Ongoing
Post Winter Maintenance	Annually	Remove excess sand, gross solids, and repair vegetation and plantings	Parcel will be free of excess sand, litter/trash. Vegetation per approved plans.
Annual Report	1 time per year	Submit Annual Report to Barrington Planning Dept. and kept on file by the owner.	Report to be submitted on or before December 15th each year. Copies submitted to NHDES by that date.

The following practices and drainage features will all require periodic inspections and maintenance based on this manual and drainage layout:

Drainage Pipes

Conveyance Swales

Detention Pond #201 with forebay

Rain Garden w/ Infiltration #202 with forebay and spillway

Т

STORMWATER SYSTEM OPERATION AND MAINTENANCE PLAN

Inspection & Maintenance Manual Log Form

Franklin Pierce Highway, Barrington, NH 03825

TSB Construction, LLC 44 Merryfield Lane, Hampstead, NH 03826

BMP / System	Date Inspected	Inspector	Cleaning/Repair (List Items & Comments)	Repair Date	Performed By:

STORMWATER SYSTEM OPERATION AND MAINTENANCE PLAN

Deicing Log Form

Franklin Pierce Highway, Barrington, NH 03825

TSB Construction, LLC 44 Merryfield Lane, Hampstead, NH 03826

Date	Amount Applied	Performed By:	Date	Amount Applied	Performed By:

STORMWATER SYSTEM OPERATION & MAINTENANCE PLAN CERTIFICATION

Owner	Responsibility
TSB Construction, LLC Owners / Developers David Coish, Operator e: (603) 765-9176 (Mobile) pishdavid@yahoo.com	The owner is responsible for the conduct of all construction activities, and ultimate compliance with all the provisions of the Stormwater System Operation & Maintenance Plan and the implementation of the Inspection and Maintenance Manual.

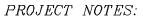
OWNER CERTIFICATION

I certify under penalty of law that this document and all attachments were prepared under my direction and supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Signed: _____ Date:

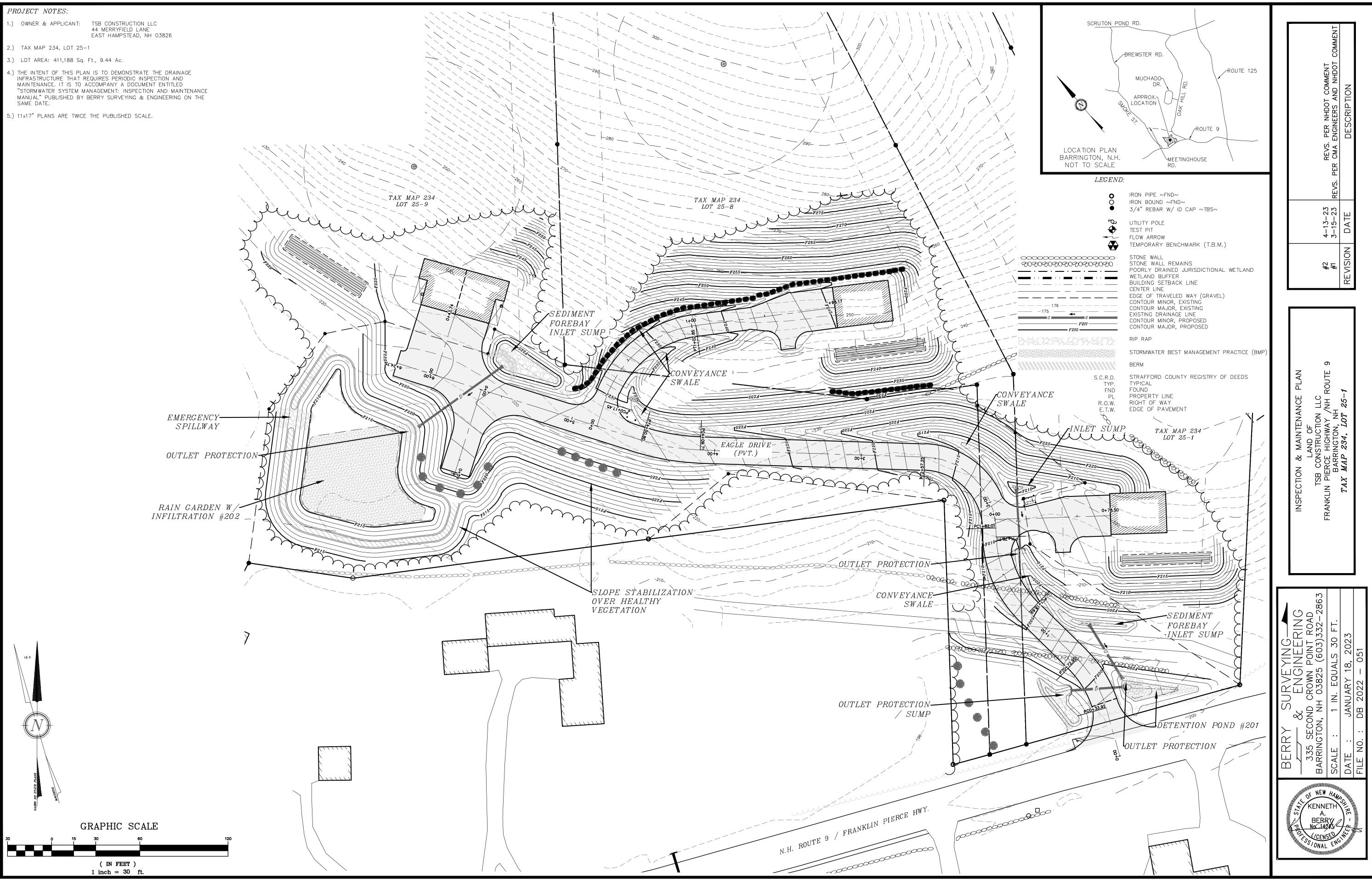
Printed Name:

Representing:





- INFRASTRUCTURE THAT REQUIRES PERIODIC INSPECTION AND MAINTENANCE. IT IS TO ACCOMPANY A DOCUMENT ENTITLED "STORMWATER SYSTEM MANAGEMENT: INSPECTION AND MAINTENANCE MANUAL" PUBLISHED BY BERRY SURVEYING & ENGINEERING ON THE SAME DATE.



Infiltration Feasibility Report

Tax Map 234, Lot 25-1 Franklin Pierce Highway Barrington, NH 03825

Prepared for:

TSB Construction, LLC 44 Merryfield Lane East Hampstead, NH 03826

Prepared By

Berry Surveying & Engineering 335 Second Crown Point Road Barrington, NH 03825 603-332-2863

File Number DB2022-051

January 18, 2023

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1.0 Location of Practices:

The project proposes locations of infiltration for ground water recharge as well as channel flow protection purposes via Rain Garden with Infiltration #202.

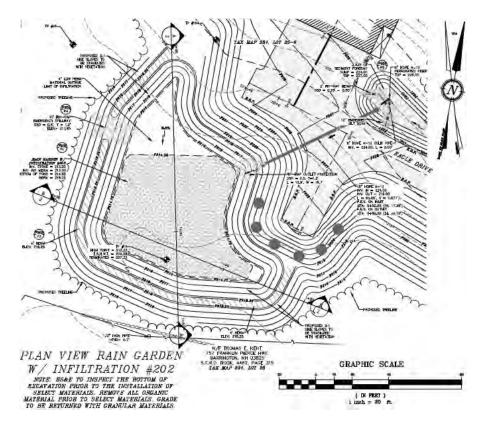
Rain Garden with Infiltration #202 (POND 202) – This Rain Garden is on the southerly side of the end of the proposed shared driveway. Runoff is collected in an inlet sump/sediment forebay where pre-treatment occurs, before being treated in the biomedia. This practice treats runoff from driveway, residential buildings and collects runoff from uphill of the area of construction.

2.0 Existing topography at the location of the practice

Rain Garden with Infiltration #202 (POND 202) – The existing topography within the area is at an 3-8% slope. The area was woodlands and is comprised of a natural state soils. This land has been used for forestry practices in the past.

3.0 Test Pit Locations

Rain Garden with Infiltration #202 (POND 202) – The practice has a surface area of 4,000 SF. The practice is located over test pits #15. See test pit profiles below. See test pit locations on Sheet P-202, Proposed Rain Garden with Infiltration #202 Detail Plan. The test holes were completed in Summer 2017, (See Site Specific Soil Map Report by John P Hayes III). The soil in the vicinity of this practice is Gloucester Sandy Loam, considered to be HSG A soil where the most restrictive published Ksat is 6.0 inches per hour. This practice was designed using 3.0 in. / hr.



Rain Garden with Infiltration #202 (POND 202) - (Reference Sheet P-202)

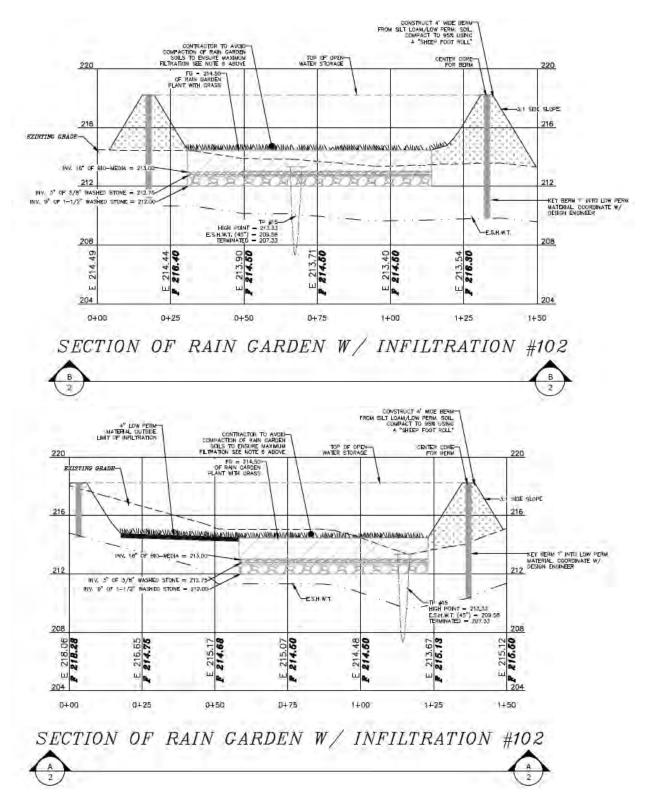
4.0 Seasonal high water table (SHWT) and bedrock elevations

TP#15:	Existing Surface Elevation of TP =	213.33′
	SHWT = 45 Inches	209.58'
	Bedrock = >72 Inches	207.33'
	Ground Water = N/A	
	Deepest Elevation of TP =	207.33'

Rain Garden w/ Infiltration #202 (Pond 202): Inv. Biomedia 213.00'

See cross section below.

2022-051 TSB Construction LLC, NH Route 9, Barrington Infiltration Feasibility Report



5.0 **Profile descriptions**

The following test pit data was collected, see profile below.

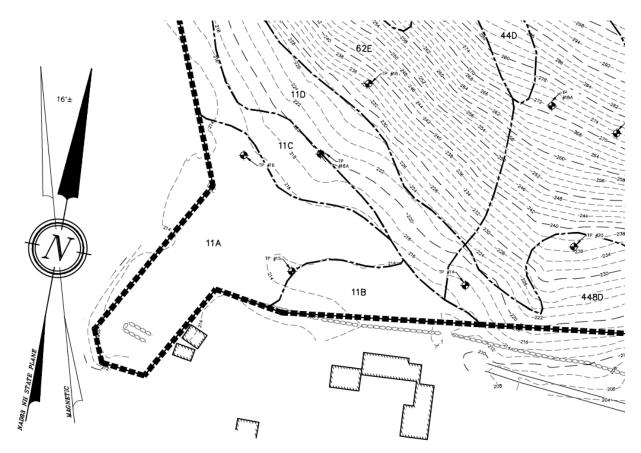
TEST PIT #15 Summer 2017

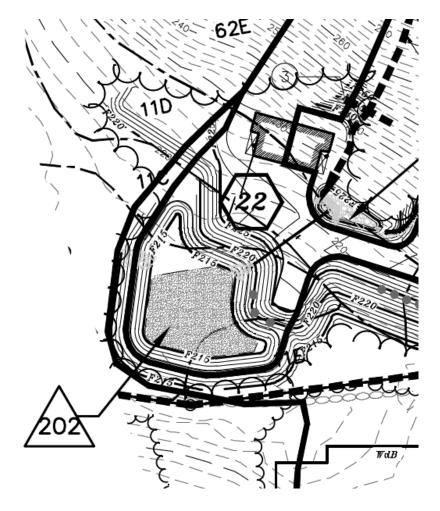
0-10"	10YR 3/3, FINE SANDY LOAM, GRANULAR, FRIABLE
10-12"	10YR 4/4, FINE SANDY LOAM, GRANULAR, FRIABLE
12-21"	2.5Y 5/4, FINE SANDY LOAM, GRANULAR, FRIABLE
21-39"	2.5Y 5/3, FINE SANDY LOAM, ANGULAR BLOCKY, FRIABLE
39-45"	10YR 4/6, COARSE SAND, SINGLE GRAIN, LOOSE
	30% GRAVEL/COBBLES
45-72"+	2.5Y 5/4, COARSE SAND, SINGLE GRAIN, LOOSE
	MOTTLES - 10YR 4/6 CONCENTRATIONS & DEPLETIONS
	30% GRAVEL/COBBLES

E.S.H.W.T. @ 45" ROOTS TO 40" RESTRICTIVE LAYER - N/A" WATER OBSERVED >72" LEDGE @ >72" TERMINATED @ 72" PERC. RATE = 4 MIN./IN.

6.0 Soil plan in the area of the constructed practice

Rain Garden with Infiltration is located over Gloucester Sandy Loam (Pond #202) See Test Pit #15.





Rain Garden with Infiltration #202 (Pond 202)

7.0 Summary of Infiltration Rate

Rain Garden with Infiltration #202 is located within a Gloucester Sandy Loam, HSG A soil area as mapped by Site Specific Soil Survey by John P. Hayes III, CSS, with a documented Ksat of 6.0 inches per hour. The design exfiltration rate for the rain garden is 3.0 inches per hour.

Amoozemeter testing was not conducted on site and the alternate method of using the USDA / NRCS published values was employed. Reference is made to K Sat Values for New Hampshire Soils (Including Hydrologic and DES Soil Lot Sizing Groups, sponsored by the Society of Soil Scientists of Norther New England, Publication #5 dated September 2009.

Respectfully submitted:

BERRY SURVEYING & ENGINEERING

Kevin R. Poulin, EIT Project Engineer

Kenneth A. Berry, PE, LLS CPSWQ, CPESC, CESSWI Principal, VP – Technical Operations

Control of Invasive Plants

New Hampshire Department of Agriculture, Markets & Food Douglas Cygan 603-271-3488 doug.cygan@agr.nh.gov

This guide lists garden plants and weeds which are already causing significant changes to natural areas in the Mid-Atlantic. Measures for controlling each species are indicated by number, e.g., (3), in the text with a full explanation at the end of this article. Click on the word <u>Control</u>: to jump to that section. Then click your "back" button to return to the text. Following each section suggested alternative plants are given. These alternatives are native plants, well adapted and needing little care, attractive to birds and butterflies, and an important part of the food web for our indigenous species.

INVASIVE TREES

NORWAY MAPLE (*Acer platanoides*) has large leaves similar to sugar maple. To easily confirm that the plant is Norway maple, break off a leaf and if it's truly Norway maple it will exude milky white sap. Fall foliage is yellow. (Exception: cultivars such as 'Crimson King,' which have red leaves in spring or summer, may have red autumn leaves.) The leaves turn color late, usually in late October after native trees have dropped their foliage. This tree suppresses growth of grass, garden plants, and forest understory beneath it, at least as far as the drip-line. Its wind-borne seeds can germinate and grow in deep shade. The presence of young Norway maples in our woodlands is increasing.

Control: (1); (7), (8), (9), or (10); (11) in mid-October to early November, before the leaves turn color.

TREE OF HEAVEN (*Ailanthus altissima*), is incredibly tough and can grow in the poorest conditions. It produces huge quantities of wind-borne seeds, grows rapidly, and secretes a toxin that kills other plants. Its long compound leaves, with 11-25 lance-shaped leaflets, smell like peanut butter or burnt coffee when crushed. Once established, this tree cannot be removed by mechanical means alone.

<u>Control</u>: (1) - seedlings only. Herbicide - use Garlon 3a (9) with no more than a 1[°] gap between cuts, or (10); plus (11) on re-growth. Or paint bottom 12[°] of bark with Garlon 4 Ultra (in February or March to protect surrounding plants). USE MAXIMUM STRENGTH SPECIFIED ON LABEL for all herbicide applications on Ailanthus. Glyphosate is not effective against Ailanthus.

INVASIVE SHRUBS

AUTUMN OLIVE (*Eleagnus umbellata*): Formerly recommended for erosion control and wildlife value, these have proved highly invasive and diminish the overall quality of wildlife habitat.

<u>*Control*</u>: (1) - up to 4⁺ diameter trunks; (7) or (10) or bury stump. Do not mow.

MULTIFLORA ROSE (*Rosa multiflora*), formerly recommended for erosion control, hedges, and wildlife habitat, becomes a huge shrub that chokes out all other vegetation and is too dense for many species of birds to nest in, though a few favor it. In shade, it grows up trees like a vine. It is covered with white flowers in June. (Our native roses have fewer flowers, mostly pink.) Distinguish multiflora by its size, and by the presence of very hard, curved thorns, and a fringed edge to the leaf stalk.

<u>Control</u>: (1) - pull seedlings, dig out larger plants at least 6" from the crown and 6" down; (4) on extensive infestations; (10) or (11). It may remain green in winter, so herbicide may applied when other plants are dormant. For foliar application, mix Rodeo with extra sticker-spreader, or use Roundup Sure Shot Foam on small plants.

BUSH HONEYSUCKLES (*Lonicera spp.*), including Belle, Amur, Morrow's, and Tatarian honeysuckle. (In our region, assume that any honeysuckle is exotic unless it is a scarlet-flowered vine). Bush honeysuckles create denser shade than native shrubs, reducing plant diversity and eliminating nest sites for many forest interior species.

<u>Control</u>: (2) on ornamentals; (1); on shady sites only, brush cut in early spring and again in early fall (3); (4) during the growing season; (7); or (10) late in the growing season.

BLUNT-LEAVED PRIVET (Ligustrum obtusifolium). <u>Control</u>: (1); (7) or (10); or trim off all flowers. Do not cut back or mow.

BURNING BUSH, WINGED EUONYMUS (*Euonymus alatus*), identified by wide, corky wings on the branches. <u>*Control:*</u> (1); (7) or (10); or trim off all flowers.

JAPANESE BARBERRY (*Berberis thunbergii*), and all cultivars and varieties. <u>*Control:*</u> (1); (7) or (10); or trim off all flowers.

INVASIVE WOODY VINES

All of these vines shade out the shrubs and young trees of the forest understory, eventually killing them, and changing the open structure of the forest into a dense tangle. DO NOT PLANT NEXT TO OPEN SPACE.

JAPANESE HONEYSUCKLE (*Lonicera japonica*), including Hall's honeysuckle, has gold-and-white flowers with a heavenly scent and sweet nectar in June. This is probably the familiar honeysuckle of your childhood. It is a rampant grower that spirals around trees, often strangling them. <u>Control:</u> (1); (3); (10); (11) in fall or early spring when native vegetation is dormant. Plan to re-treat repeatedly.

ORIENTAL BITTERSWEET (*Celastrus orbiculatus*) has almost completely displaced American bittersweet (*C. scandens*). The Asian plant has its flowers and bright orange seed capsules in clusters all along the stem, while the native species bears them only at the branch tips. <u>Control:</u> (1); keep ornamental plants cut back, remove all fruits as soon as they open, and bag or burn fruits; to eradicate use Garlon 3a (10).

JAPANESE KNOTWEED, MEXICAN BAMBOO (*Polygonum cuspidatum*) can grow in shade. The stems have knotty joints, reminiscent of bamboo. It grows 6-10' tall and has large pointed oval or triangular leaves.

Control: Cut at least 3 times each growing season and/or treat with Rodeo (10) or (11). In gardens, heavy mulch or dense shade may kill it.

INVASIVE HERBACEOUS PLANTS

GARLIC MUSTARD (*Alliaria petiolata*, *A. officinalis*), a white-flowered biennial with rough, scalloped leaves (kidney-, heart- or arrow-shaped), recognizable by the smell of garlic and taste of mustard when its leaves are crushed. (The odor fades by fall.)

<u>Control</u>: Pull before it flowers in spring (1), removing crown and roots. Tamp down soil afterwards. Once it has flowered, cut (2), being careful not to scatter seed, then bag and burn or send to the landfill. (11) may be appropriate in some settings.

JAPANESE STILT GRASS (*Microstegium vimineum*) can be identified by its lime-green color and a line of silvery hairs down the middle of the 2-3" long blade. It tolerates sun or dense shade and quickly invades areas left bare or disturbed by tilling or flooding. An annual grass, it builds up a large seed bank in the soil.

<u>Control</u>: Easily pulled in early to mid-summer (1) - be sure to pull before it goes to seed. If seeds have formed, bag and burn or send to landfill. Mowing weekly or when it has just begun to flower may prevent it from setting seed (3). Use glyphosate (11) or herbicidal soap (less effective) on large infestations. Follow up with (5) in spring.

MILE-A-MINUTE VINE, DEVIL'S TAIL TEARTHUMB (*Polygonum perfoliatum*), a rapidly growing annual vine with triangular leaves, barbed stems, and turquoise berries in August which are spread by birds. It quickly covers and shades out herbaceous plants. <u>Control</u>: same as for stilt grass.

SPOTTED KNAPWEED (Centaurea maculosa), a biennial with thistle-like flowers.

<u>Control</u>: Do NOT pull (1) unless the plant is young and the ground is very soft - the tap root will break off and produce several new plants. Wear sturdy gloves. (2); (6); (10) or (11).

CONTROL MEASURES

(1) PULL seedlings and small or shallow-rooted plants when soil is moist. Dig out larger plants, including the root systems. Use a forked spade or weed wrench for trees or shrubs.

(2) DEADHEAD to prevent spread of seeds of invasive plants. Cut off seeds or fruits before they ripen. Bag, and burn or send to a landfill.

(3) MOW or CUTTING at least 4 times a season to deplete plants' store of nutrients and carbohydrates, reduce seed formation, and kill or minimize spread of plants. If necessary, repeat each year.

(4) CONTROLLED BURNING during the spring, repeated over several years, allows native vegetation to compete more effectively with the invasive species. This requires a permit. Spot treatment with glyphosate in late fall can be used to make this method more effective.

(5) Use a CORN-BASED PRE-EMERGENCE HERBICIDE on annual weeds. This product is also an organic fertilizer, i.e., it can stimulate growth of existing plants, including weeds, so it is appropriate for lawns and gardens but may not be appropriate in woodlands.

(6) In lawns, SPOT TREAT with BROAD-LEAF WEEDKILLER. Good lawn-care practices (test soil; use lime and fertilizer only when soil test shows a need; mow high and frequently; leave clippings on lawn) reduce weed infestations.

(7) CUT DOWN the tree. Grind out the stump, or clip off re-growth.

(8) GIRDLE tree: cut through the bark and growing layer (cambium) all around the trunk, about 6" above the ground. Girdling is most effective in spring when the sap is rising, and from middle to late summer when the tree is sending down food to the roots. Clip off sucker sprouts.

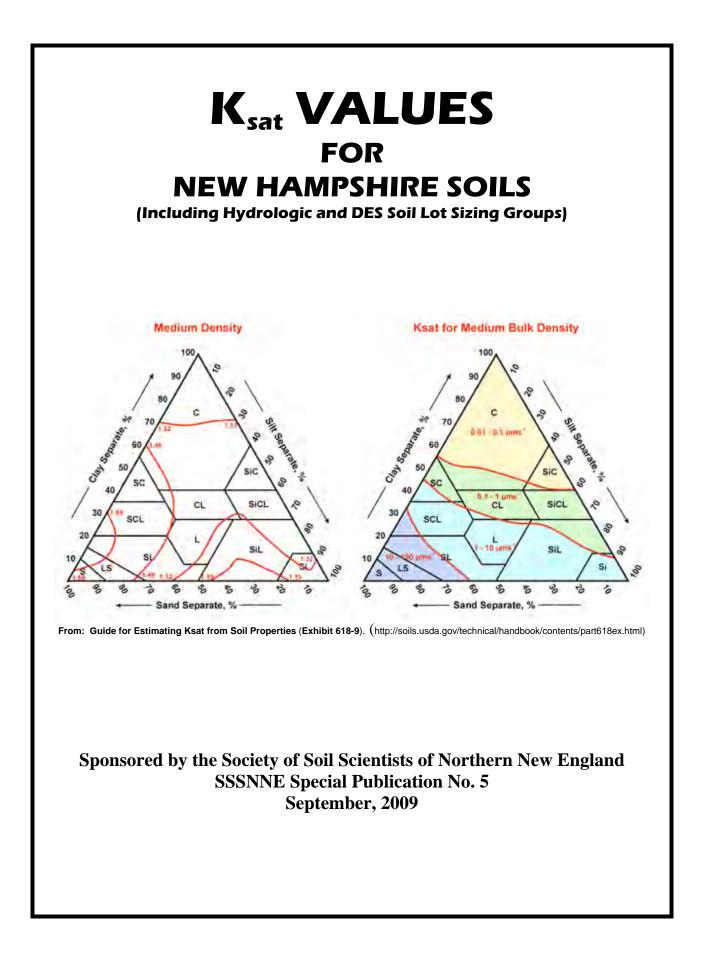
(9) FRILL: Using a machete, hatchet or similar device, hack scars (several holes in larger trees) downward into the cambium layer, and squirt in glyphosate (or triclopyr if recommended in text above). Follow label directions for Injection and Frill Applications. This is most effective from middle to late summer. Clip off any sucker sprouts or treat with glyphosate.

(10) CUT STEM / CUT STUMP WITH GLYPHOSATE (or triclopyr if specified above). Follow label directions for Cut Stump Application. Clip off sucker sprouts or paint with glyphosate. See Note on Herbicides.

(II) FOLIAR SPRAY WITH GLYPHOSATE herbicide (see Note on Herbicides). Use a backpack or garden sprayer or mist blower, following label directions. Avoid overspray and/or dripping onto non-target plants, because glyphosate kills most plants except moss. If it rolls off waxy or grass-like foliage, use additional sticker-spreader. Deciduous trees, shrubs, and perennials move nutrients down to the roots in late summer. Glyphosate is particularly effective at this time and when plants have just gone out of flowering. Several invasive species retain their foliage after native plants have lost theirs, and resume growth earlier in spring than most natives. This allows you to treat them without harming the natives. However, the plant must be actively growing for the herbicide to work. Retreatments may be necessary the following year if suckering occurs or the plant hasn't been entirely killed.

<u>NOTE ON HERBICIDES</u>: It is highly recommended that small populations try to be controlled using non-chemical methods wherever feasible. However, for large infestations, and for a few plants specified above, herbicide use is essential. Apply herbicides carefully to avoid non-target plants, glyphosate is the least environmentally damaging herbicide in most cases. Add food coloring for visibility, and a soap-based sticker such as Cide-Kick. Glyphosate is ineffective on some

plants; for these, triclopyr (Garlon) may be indicated. When using herbicides, read the entire label and observe all precautions listed, including proper disposal. If in doubt, call your local Cooperative Extension Service.



K_{sat} VALUES FOR NEW HAMPSHIRE SOILS

ABOUT THE SOCIETY OF SOIL SCIENTISTS OF NORTHERN NEW ENGLAND

The Society of Soil Scientists of Northern New England (SSSNNE) is a non-profit professional organization of soil scientists, both in the private and public sectors, which is dedicated to the advancement of soil science. The Society fosters the profession of soil classification, mapping and interpretation, and encourages the dissemination of information concerning soil science. With the intent of contributing to the general human welfare, the Society seeks to educate the public on the wise use of soils and the associated natural resources.

INTRODUCTION

The publication " K_{sat} Values for New Hampshire Soils" is designed to assist soil scientists, engineers, and other professionals by assembling tables of existing data for all soil series currently on the state soil legend with regard to K_{sat} values and hydrologic groupings (Hyd.Grp.). The need for this information has become more important since the adoption by the New Hampshire Department of Environmental Services of the revised Alteration of Terrain rules for stormwater management. Additional information has been provided for each soil series with regard to landform, temperature regime (Temp.), soil textures, NHDES Soil Lot Size Groupings (Group), whether the soil is a Spodosol (Spodosol?) and other information which will be valuable to a variety of soil information users.

The data for each soil series has been sorted 3 ways for ease of searching:

Table A-Sorted by Numerical Legend Table B-Sorted by Soil Series Name Table C-Sorted by NHDES Soil Group for Establishing Lot Size

The report represents cumulative efforts by private soil scientists and NHDES staff with assistance from the USDA Natural Resource Conservation Service.

Comments or inquires on the information in this publication may be directed to the Board of Directors at the following address:

Society of Soil Scientists of Northern New England PO Box 76 Durham, NH 03824

SATURATED HYDRAULIC CONDUCTIVITY (K_{SAT})

 K_{sat} refers to the ease with which pores in a saturated soil transmit water. The estimates presented here are expressed in terms of inches per hour (NRCS official data presents K_{sat} in both micrometers per second and inches per hour). K_{sat} values are based on soil characteristics observed in the field, particularly structure, consistence, porosity, and texture. (USDA NRCS, Web Soil Survey)

Saturated flow occurs when the soil water pressure is positive; that is, when the soil matric potential is zero (satiated wet condition). In most soils this situation takes place when about 95 percent of the total pore space is filled with water. The remaining 5 percent is filled with entrapped air. Saturated hydraulic conductivity cannot be used to describe water movement under unsaturated conditions. (Soil Survey Manual, 1993)

It is commonly known that soil features (and thus data) for a certain soil series name may be slightly different from one county soil survey to the next and the range in characteristics (via the Typical Pedon) may be slightly different. For example – a Marlow soil (series) in Carroll County may have a higher sand content in its B horizon as opposed to a Marlow soil (series) in Coos County; resulting in a slightly different Ksat range for the B horizon.

The K_{sat} data for this publication was obtained from the USDA-NRCS Soil Data Mart using the Typical Pedon from the county that best reflected the soil and/or had the most acres of that soil. This data is presented in B and C horizons only as it is assumed that the topsoil (A or A_p horizon) will be removed in typical construction practices.

References:

Web Soil Survey. Soil Survey Staff, Natural Resources Conservation Service, United States Department of Agriculture. Web Soil Survey. Available online at http://websoilsurvey.nrcs.usda.gov/.

Soil Data Mart. http://soildatamart.nrcs.usda.gov/.

Soil Survey Manual. Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18.

HYDROLOGIC SOIL GROUPS

Hydrologic group is a group of soils having the same runoff potential under similar storm and cover conditions.

Hydrologic groups are used in equations that estimate runoff from rainfall. These estimates are needed for solving hydrologic problems that arise in planning stormwater management, watershed protection, and flood-prevention projects and for planning or designing structures for the use, control, and disposal of water.

Classifications assigned to soils were based on the use of rainfall-runoff data from small watersheds and infiltrometer plots. From these data, relationships between soil properties and hydrologic groups were established. Assignment of soils to hydrologic groups is based on the relationship between soil properties and hydrologic groups. Wetness characteristics, permeability after prolonged wetting, and depth to very slowly permeable layers are properties that assist in estimating hydrologic groups. Minimum annual steady ponded infiltration rate for a bare ground surface determines the hydrologic soil groups.

Soil properties that influence runoff potential are those that influence the minimum rate of infiltration for a bare soil after prolonged wetting and when not frozen. These properties are depth to a seasonally high water table, intake rate and permeability after prolonged wetting, and depth to a very slowly permeable layer. (The influence of ground cover is treated independently, not in hydrologic soil groups.).

The soils in the United States are placed into four groups, A, B, C, and D, and three dual classes, A/D, B/D, and C/D. In the definitions of the classes, infiltration rate is the rate at which water enters the soil at the surface and is controlled by the surface conditions. Transmission rate is the rate at which water moves in the soil and is controlled by soil properties. Definitions of the classes are as follows:

Group A- Saturated hydraulic conductivity is very high or in the upper half of high and internal free water occurrence is very deep. Soils in this group have low runoff potential when thoroughly wet. Water is transmitted freely through the soil. Group A soils typically have less than 10 percent clay and more than 90 percent sand or gravel and have gravel or sand textures. Some soils having loamy sand, sandy loam, loam or silt loam textures may be placed in this group if they are well aggregated, of low bulk density, or contain greater than 35 percent rock fragments. The limits on the diagnostic physical characteristics of group A are as follows. The saturated hydraulic conductivity of all soil layers exceeds 40.0 micrometers per second (5.67 inches per hour). The depth to any water impermeable layer is greater than 50 centimeters [20 inches]. The depth to the water table is greater than 60 centimeters [24 inches]. Soils that are deeper than 100 centimeters [40 inches] to a water impermeable layer are in group A if the saturated hydraulic conductivity of all soil layers within 100 centimeters [40 inches] of the surface exceeds 10 micrometers per second (1.42 inches per hour).

Group B- Saturated hydraulic conductivity is in the lower half of high or in the upper half of moderately high and free water occurrence is deep or very deep. Soils in this group have moderately low runoff potential when thoroughly wet. Water transmission through the soil is unimpeded. Group B soils typically have between 10 percent and 20 percent clay and 50 percent to 90 percent sand and have loamy sand or sandy loam textures. Some soils having loam, silt loam, silt, or sandy clay loam textures may be placed in this group if they are well aggregated, of low bulk density, or contain greater than 35 percent rock fragments. The limits on the diagnostic physical characteristics of group B are as follows. The saturated hydraulic conductivity in the least transmissive layer between the surface and 50 centimeters [20 inches] ranges from 10.0 micrometers per second (1.42 inches per hour) to 40.0 micrometers per second (5.67 inches per hour). The depth to any water impermeable layer is greater than 50 centimeters [20 inches]. The depth to the water table is greater than 60 centimeters [24 inches]. Soils that are deeper than 100 centimeters [40 inches] to a water impermeable layer or water table are in group B if the saturated hydraulic conductivity of all soil layers within 100 centimeters [40 inches] of the surface exceeds 4.0 micrometers per second (0.57 inches per hour) but is less than 10.0 micrometers per second (1.42 inches per hour).

Group C- Saturated hydraulic conductivity is in the lower half of moderately high or in the upper half of moderately low and internal free water occurrence is deeper than shallow. Soils in this group have moderately high runoff potential when thoroughly wet. Water transmission through the soil is somewhat restricted. Group C soils typically have between 20 percent and 40 percent clay and less than 50 percent sand and have loam, silt loam, sandy clay loam, clay loam, and silty clay loam textures. Some soils having clay, silty clay, or sandy clay textures may be placed in this group if they are well aggregated, of low bulk density, or contain greater than 35 percent rock fragments. The limits on the diagnostic physical characteristics of group C are as follows. The saturated hydraulic conductivity in the least transmissive layer between the surface and 50 centimeters [20 inches] is between 1.0 micrometers per second (0.14 inches per hour) and 10.0 micrometers per second (1.42 inches per hour). The depth to any water impermeable layer is greater than 50 centimeters [20 inches]. The depth to the water table is greater than 60 centimeters [24 inches]. Soils that are deeper than 100 centimeters [40 inches] to a restriction or water table are in group C if the saturated hydraulic conductivity of all soil layers within 100 centimeters [40 inches] of the surface exceeds 0.40 micrometers per second (0.06 inches per hour) but is less than 4.0 micrometers per second (0.57 inches per hour).

Group D- Saturated hydraulic conductivity is below the upper half of moderately low, and/or internal free water occurrence is shallow or very shallow and transitory through permanent. Soils in this group have high runoff potential when thoroughly wet. Water movement through the soil is restricted or very restricted. Group D soils typically have greater than 40 percent clay, less than 50 percent sand, and have clayey textures. In some areas, they also have high shrink-swell potential. All soils with a depth to a water impermeable layer less than 50 centimeters [20 inches] and all soils with a water table within 60 centimeters [24 inches] of the surface are in this group, although some may have a dual classification, as described in the next section, if they can be adequately drained. The limits on the physical diagnostic characteristics of group D are as follows. For soils with a water impermeable layer at a depth between 50 centimeters and 100 centimeters [20 and 40 inches], the saturated hydraulic conductivity in the least transmissive soil layer is less than or equal to 1.0 micrometers per second (0.14 inches per hour). For soils that are deeper than 100 centimeters [40 inches] to a restriction or water table, the saturated hydraulic

conductivity of all soil layers within 100 centimeters [40 inches] of the surface is less than or equal to 0.40 micrometers per second (0.06 inches per hour).

Dual hydrologic soil groups-Certain wet soils are placed in group D based solely on the presence of a water table within 60 centimeters [24 inches] of the surface even though the saturated hydraulic conductivity may be favorable for water transmission. If these soils can be adequately drained, then they are assigned to dual hydrologic soil groups (A/D, B/D, and C/D) based on their saturated hydraulic conductivity and the water table depth when drained. The first letter applies to the drained condition and the second to the undrained condition. For the purpose of hydrologic soil group, adequately drained means that the seasonal high water table is kept at least 60 centimeters [24 inches] below the surface in a soil where it would be higher in a natural state.

References:

National Engineering Handbook, Natural Resource Conservation Service, U.S. Department of Agriculture.

Soil Data Mart. <u>http://soildatamart.nrcs.usda.gov/</u>.

Soil Survey Manual. Soil Survey Division Staff. 1993. Soil survey manual. Natural Resources Conservation Service. U.S. Department of Agriculture Handbook 18.

TABLE A NUMERICAL LEGEND

Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Occum	1	0.6	2.0	6.00	20.0	B	2	Flood Plain (Bottom Land)	mesic	loamy	no	loamy over loamy sand
Suncook	2	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	mesic	sandy	no	occasionally flooded
Lim	3	0.6	2.0	6.00	20.0	C	5	Flood Plain (Bottom Land)	mesic	loamy	no	
Pootatuck	4	0.6	6.0	6.00	20.0	B	3	Flood Plain (Bottom Land)	mesic	loamy	no	single grain in C
Rippowam	5	0.6	6.0	6.00	20.0	C	5	Flood Plain (Bottom Land)	mesic	loamy	no	onigio grant in o
Saco	6	0.6	2.0	6.00	20.0	D	6	Flood Plain (Bottom Land)	mesic	silty	no	strata
Hadley	8	0.6	2.0	0.60	6.0	В	2	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand
Winooski	9	0.6	6.0	0.60	6.0	В		Flood Plain (Bottom Land)	mesic	silty over loamy	no	
Merrimac	10	2.0	20.0	6.00	20.0	А	1	Outwash and Stream Terraces	mesic	gravelly sand	no	loamy cap
Gloucester	11	6.0	20.0	6.00	20.0	А	1	Sandy Till	mesic	sandy-skeletal	no	loamy cap
Hinckley	12	6.0	20.0	20.00	100.0	А	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	
Sheepscot	14	6.0	20.0	6.00	20.0	В	3	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravelly coarse sand
Searsport	15	6.0	20.0	6.00	20.0	D	6	Outwash and Stream Terraces	frigid	sandy	no	organic over sand
Saugatuck	16	0.06	0.2	6.00	20.0	С	5	Outwash and Stream Terraces	mesic	sandy	yes	ortstein
Colton, gravelly	21	6.0	20.0	20.00	100.0	Α	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravelly surface
Colton	22	6.0	20.0	20.00	100.0	А	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	* *
Masardis	23	6.0	20.0	6.00	20.0	А	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	slate, loamy cap
Agawam	24	6.0	20.0	20.00	100.0	В	2	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Windsor	26	6.0	20.0	6.00	20.0	А	1	Outwash and Stream Terraces	mesic	sandy	no	·
Groveton	27	0.6	2.0	0.60	6.0	В	2	Outwash and Stream Terraces	frigid	loamy	yes	loamy over sandy
Madawaska	28	0.6	2.0	6.00	20.0	В	3	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
Woodbridge	29	0.6	2.0	0.00	0.6	С	3	Firm, platy, loamy till	mesic	loamy	no	sandy loam in Cd
Unadilla	30	0.6	2.0	2.00	20.0	В	2	Terraces and glacial lake plains	mesic	silty	no	silty over gravelly
Hartland	31	0.6	2.0	0.20	2.0	В	2	Terraces and glacial lake plains	mesic	silty	no	very fine sandy loam
Boxford	32	0.1	0.2	0.00	0.2	С	3	Silt and Clay Deposits	mesic	fine	no	silty clay loam
Scitico	33	0.0	0.2	0.00	0.2	С	5	Silt and Clay Deposits	mesic	fine	no	
Wareham	34	6.0	20.0	6.00	20.0	С	5	Outwash and Stream Terraces	mesic	sandy	no	
Champlain	35	6.0	20.0	20.00	100.0	А	1	Outwash and Stream Terraces	frigid	gravelly sand	no	
Adams	36	6.0	20.0	20.00	99.0	А	1	Outwash and Stream Terraces	frigid	sandy	yes	
Melrose	37	2.0	6.0	0.00	0.2	С	3	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	silty clay loam in C
Eldridge	38	6.0	20.0	0.06	0.6	С	3	Sandy/loamy over silt/clay	mesic	sandy over loamy	no	
Millis	39					С	3	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Canton	42	2.0	6.0	6.00	20.0	В	2	Loose till, sandy textures	mesic	loamy over sandy	no	loamy over loamy sand
Montauk	44	0.6	6.0	0.06	0.6	С	3	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Henniker	46	0.6	2.0	0.06	0.6	С	3	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
Madawaska, aquentic	48	0.6	2.0	6.00	20.0	В	3	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
Whitman	49	0.0	0.2	0.00	0.2	D	6	Firm, platy, loamy till	mesic	loamy	no	mucky loam
Hermon	55	2.0	20.0	6.00	20.0	Α	1	Sandy Till	frigid	sandy-skeletal	yes	loamy cap
Becket	56	0.6	2.0	0.06	0.6	С	3	Firm, platy, sandy till	frigid	loamy	yes	gravelly sandy loam in Cd
Waumbeck	58	2.0	20.0	6.00	20.0	В	3	Loose till, sandy textures	frigid	sandy-skeletal	yes	very cobbly loamy sand
Charlton	62	0.6	6.0	0.60	6.0	В	2	Loose till, loamy textures	mesic	loamy	no	fine sandy loam
Paxton	66	0.6	2.0	0.00	0.2	С	3	Firm, platy, loamy till	mesic	loamy	no	
Sutton	68	0.6	6.0	0.60	6.0	В	3	Loose till, loamy textures	mesic	loamy	no	
Berkshire	72	0.6	6.0	0.60	6.0	В	2	Loose till, loamy textures	frigid	loamy	yes	fine sandy loam
Marlow	76	0.6	2.0	0.06	0.6	С	3	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Peru	78	0.6	2.0	0.06	0.6	С	3	Firm, platy, loamy till	frigid	loamy	yes	
Thorndike	84	0.6	2.0	0.60	2.0	C/D	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	less than 20 in. deep
Hollis	86	0.6	6.0	0.60	6.0	C/D	4	Loose till, bedrock	mesic	loamy	no	less than 20 in. deep
Winnecook	88	0.6	2.0	0.60	2.0	С	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Chatfield	89	0.6	6.0	0.60	6.0	В	4	Loose till, bedrock	mesic	loamy	no	20 to 40 in. deep
Hogback	91	2.0	6.0	2.00	6.0	С	4	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Lyman	92	2.0	6.0	2.00	6.0	A/D	4	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Woodstock	93	2.0	6.0	2.00	6.0	C/D	4	Loose till, bedrock	frigid	loamy	no	less than 20 in. deep
Rawsonville	98	0.6	6.0	0.60	6.0	С	4	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Tunbridge	99	0.6	6.0	0.60	6.0	С	4	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep

Soil Series	legend number	Ksat low - B in/hr	Ksat high - B in/hr	Ksat low - C in/hr	Ksat high - C in/hr	Hyd. Grp.	Group	Land Form	Temp.	Soil Textures	Spodosol ?	Other
Ondawa	101	0.6	6.0	6.00	20.0	B	2	Flood Plain (Bottom Land)	frigid	loamy	no .	loamy over loamy sand
Sundav	101	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	frigid	sandy	no	occasionally flooded
Winooski	103	0.6	6.0	0.60	6.0	В	3	Flood Plain (Bottom Land)	mesic	silty	no	very fine sandy loam
Podunk	104	0.6	6.0	6.00	20.0	B	3	Flood Plain (Bottom Land)	frigid	loamy	no	loamy to coarse sand in C
Rumney	105	0.6	6.0	6.00	20.0	С	5	Flood Plain (Bottom Land)	frigid	loamy	no	
Hadley	108	0.6	2.0	0.60	6.0	В	2	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand, occ flooded
Limerick	109	0.6	2.0	0.60	2.0	С	5	Flood Plain (Bottom Land)	mesic	silty	no	
Scarboro	115	6.0	20.0	6.00	20.0	D	6	Outwash and Stream Terraces	mesic	sandy	no	organic over sand, non stony
Finch	116					С	3	Outwash and Stream Terraces	frigid	sandy	yes	cemented (ortstein)
Sudbury	118	2.0	6.0	2.00	20.0	В	3	Outwash and Stream Terraces	mesic	sandy	no	loam over gravelly sand
Telos	123	0.6	2.0	0.02	0.2	С	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Chesuncook	126	0.6	2.0	0.02	0.2	С	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Allagash	127	0.6	2.0	6.00	20.0	В	2	Outwash and Stream Terraces	frigid	loamy over sandy	yes	loamy over sandy
Elliottsville	128	0.6	2.0	0.60	2.0	В	4	Friable till, silty, schist & phyllite	frigid	loamy	yes	20 to 40 in. deep
Hitchcock	130	0.6	2.0	0.06	0.6	В	3	Terraces and glacial lake plains	mesic	silty	no	silt loam to silt in C
Burnham	131	0.2	6.0	0.02	0.2	D	6	Firm, platy, silty till, schist & phylitte	frigid	loamy	no	organic over silt
Dartmouth	132	0.6	2.0	0.06	0.6	В	3	Terraces and glacial lake plains	mesic	silty	no	thin strata silty clay loam
Monson	133	0.6	2.0	0.60	2.0	D	4	Friable till, silty, schist & phyllite	frigid	loamy	yes	less than 20 in. deep
Maybid	134	0.0	0.2	0.00	0.2	D	6	Silt and Clay Deposits	mesic	fine	no	silt over clay
Shapleigh	136					C/D	4	Sandy Till	mesic	sandy	yes	less than 20 in. deep
Monadnock	142	0.6	2.0	2.00	6.0	В	2	Loose till, sandy textures	frigid	loamy over sandy, sandy-skeletal	yes	gravelly loamy sand in C
Acton	146	2.0	20.0	2.00	20.0	В	3	Loose till, sandy textures	mesic	sandy-skeletal	no	cobbly loamy sand
Vassalboro	150					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Success	154	2.0	6.0	6.00	20.0	Α	1	Sandy Till	frigid	sandy-skeletal	yes	cemented
Canterbury	166	0.6	2.0	0.06	0.6	С	3	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Sunapee	168	0.6	2.0	0.60	6.0	В	3	Loose till, loamy textures	frigid	loamy	yes	
Waskish	195					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Ondawa	201	0.6	6.0	6.00	20.0	В	2	Flood Plain (Bottom Land)	frigid	loamy	no	occ flood, loamy over I. sand
Sunday	202	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	frigid	sandy	no	frequently flooded
Fryeburg	208	0.6	2.0	2.00	6.0	В	2	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Charles	209	0.6	100.0	0.60	100.0	С	5	Flood Plain (Bottom Land)	frigid	silty	no	
Warwick	210	2.0	6.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	loamy-skeletal	no	loamy over slate gravel
Naumburg	214	6.0	20.0	6.00	20.0	С	5	Outwash and Stream Terraces	frigid	sandy	yes	
Boscawen	220	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	no	loamy cap
Bemis	224	0.6	0.2	0.00	0.2	С	5	Firm, platy, loamy till	cryic	loamy	no	
Bice	226	0.6	6.0	0.60	6.0	В	2	Loose till, loamy textures	frigid	loamy	no	sandy loam
Lanesboro	228	0.6	2.0	0.06	0.2	С	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	channery silt loam in Cd
Poocham	230	0.6	2.0	0.20	2.0	В	3	Terraces and glacial lake plains	mesic	silty	no	silt loam in C
Buxton	232	0.1	0.6	0.00	0.2	С	3	Silt and Clay Deposits	frigid	fine	no	silty clay
Scantic	233	0.0	0.2	0.00	0.2	D	5	Silt and Clay Deposits	frigid	fine	no	
Biddeford	234	0.0	0.2	0.00	0.2	D	6	Silt and Clay Deposits	frigid	fine	no	organic over clay
Buckland	237	0.6	2.0	0.06	0.2	С	3	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Elmridge	238	2.0	6.0	0.00	0.2	С	3	Sandy/loamy over silt/clay	mesic	loamy over clayey	no	
Brayton	240	0.6	2.0	0.06	0.6	С	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Lyme	246	0.6	6.0	0.60	6.0	С	5	Loose till, sandy textures	frigid	loamy	no	
Millsite	251	0.6	6.0	0.60	6.0	<u>C</u>	4	Loose till, bedrock	frigid	loamy	no	20 to 40 in. deep
Macomber	252	0.6	2.0	0.60	2.0	C	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Lombard	259	0.6	6.0	2.00	20.0	C/D	2	Weathered bedrock, phyllite	frigid	loamy	no	very channery
Sunapee var	269	0.6	2.0	0.60	6.0	B	3	Loose till, loamy textures	frigid	loamy	yes	frigid dystrudept
Chatfield Var.	289	0.6	6.0	0.60	6.0	B	3	Loose till, bedrock	mesic	loamy	no	mwd to swpd
Greenwood	295					A/D	6	Organic Materials - Freshwater	frigid	hemic	no	deep organic
Catden	296		2.0	0.00		A/D	6	Organic Materials - Freshwater	mesic	sapric	no	deep organic
Lovewell	307	0.6	2.0	0.60	2.0	B	3	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Quonset	310	2.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	shale
Deerfield	313	6.0	20.0	20.00	100.0	В	3	Outwash and Stream Terraces	mesic	sandy	no	single grain in C

Soil Series	legend		•		Ksat high - C	Hyd.	Group	Land Form	Temp.	Soil Textures	Spodosol	Other
	number	in/hr	in/hr	in/hr	in/hr	Grp.					?	
Pipestone	314					В	5	Outwash and Stream Terraces	mesic	sandy	yes	
Mashpee	315	6.0	20.0	6.00	20.0	В	5	Outwash and Stream Terraces	mesic	sandy	yes	
Bernardston	330	0.6	2.0	0.06	0.2	С	3	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Roundabout	333	0.2	2.0	0.06	0.6	С	5	Terraces and glacial lake plains	frigid	silty	no	silt loam in the C
Pittstown	334	0.6	2.0	0.06	0.2	С	3	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Elmwood	338	2.0	6.0	0.00	0.2	С	3	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	
Stissing	340	0.6	2.0	0.06	0.2	С	5	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	
Cardigan	357	0.6	2.0	0.60	2.0	В	4	Friable till, silty, schist & phyllite	mesic	loamy	no	20 to 40 in. deep
Kearsarge	359	0.6	2.0	0.60	2.0	В	4	Friable till, silty, schist & phyllite	mesic	loamy	no	less than 20 in. deep
Dutchess	366	0.6	2.0	0.60	2.0	В	2	Friable till, silty, schist & phyllite	mesic	loamy	no	very channery
Dixfield	378	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Timakwa	393			6.00	100.0	D	6	Organic Materials - Freshwater	mesic	sandy or sandy-skeletal	no	organic over sand
Chocorua	395			6.00	20.0	D	6	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Ipswich	397					D	6	Tidal Flat	mesic	hemic/sapric	no	deep organic
Suncook	402	6.0	20.0	6.00	20.0	A	1	Flood Plain (Bottomland)	mesic	sandy	no	frequent flooding
Metallak	404	6.0	100.0	6.00	100.0	В	3	Flood Plain (Bottom Land)	frigid	loamy over sandy	no	sandy or sandy-skeletal
Medomak	406	0.6	2.0	0.60	2.0	D	6	Flood Plain (Bottom Land)	frigid	silty	no	organic over silt
Haven	410	0.6	2.0	20.00	100.0	В	2	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Duane	413	6.0	20.0	6.00	20.0	В	3	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	cemented (ortstein)
Moosilauke	414	6.0	20.0	6.00	20.0	С	5	Loose till, sandy textures	frigid	sandy	no	
Grange	433	0.6	2.0	0.60	2.0	C	5	Outwash and Stream Terraces	frigid	co. loamy over sandy (skeletal)	no	
Swanton	438	2.0	6.0	0.00	0.2	С	5	Sandy/loamy over silt/clay	frigid	co. loamy over clayey	no	
Shaker	439	2.0	6.0	0.00	0.2	С	5	Sandy/loamy over silt/clay	mesic	co. loamy over clayey	no	
Chichester	442	0.6	2.0	2.00	6.0	В		Loose till, sandy textures	frigid	loamy over sandy	no	loamy over loamy sand
Newfields	444	0.6	2.0	0.60	2.0	В	3	Loose till, sandy textures	mesic	loamy over sandy	no	sandy or sandy-skeletal
Scituate	448	0.6	2.0	0.06	0.2	C	3	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Metacomet	458	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
Pennichuck	460	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	mesic	loamy-skeletal	no	20 to 40 in. deep
Gilmanton	478	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	no	fine sandy loam in Cd
Ossipee	495			0.20	2.0	D	6	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Natchaug	496			0.20 20.00	2.0 100.0	D	6	Organic Materials - Freshwater	mesic	loamy	no	organic over loam
Pawcatuck	497 501				99.0		6	Tidal Flat	mesic	sandy or sandy-skeletal	no	organic over sand
Abenaki		0.6	2.0	6.00 0.60	99.0	B C	2	Outwash and Stream Terraces	frigid	loamy over sandy-skeletal	no	loamy over gravelly
Cohas	505 510	0.6 2.0	2.0	20.00	100.0	-	5	Flood Plain (Bottom Land)	frigid	co. loamy over sandy (skeletal)	no	alata Jaamu aan
Hoosic Ninigret	510	2.0	6.0	6.00	20.0	A B	3	Outwash and Stream Terraces Outwash and Stream Terraces	mesic mesic	sandy-skeletal loamy over sandy	no no	slate, loamy cap sandy or sandy-skeletal
Leicester	513	0.6	6.0	0.60	20.0	C	5	Loose till, loamy textures	mesic	loamy	no	Sandy of Sandy-Skeletai
Au Gres	514	0.0	0.0	0.60	20.0	B	5	Outwash and Stream Terraces	frigid	sandy		single grain, loose
Machias	520	2.0	6.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	sandy or sandy-skeletal	yes	strata sand/gravel in C
Stetson	520	0.6	6.0	6.00	20.0	B	2	Outwash and Stream Terraces	frigid	sandy-skeletal	yes ves	loamy over gravelly
Caesar	525	20.0	100.0	20.00	100.0	A	2 1	Outwash and Stream Terraces	mesic	coarse sand	no	idaniy over gravelly
Scio	531	0.6	2.0	0.60	2.0	B	3	Terraces and glacial lake plains	mesic	silty	no	gravelly sand in 2C
Belgrade	532	0.6	2.0	0.06	2.0	B	3	Terraces and glacial lake plains	mesic	silty	no	strata of fine sand
Raynham	533	0.0	2.0	0.06	0.2	C	5	Terraces and glacial lake plains	mesic	silty	no	Strata of fine Sand
Binghamville	534	0.2	2.0	0.00	0.2	D	5	Terraces and glacial lake plains	mesic	silty	no	
Suffield	536	0.2	2.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	mesic	silty over clayey	no	deep to clay C
Squamscott	538	6.0	20.0	0.06	0.6	C	5	Sandy/loamy over silt/clay	mesic	sandy over loamy	yes	
Raypol	540	0.6	2.0	6.00	100.0	D	5	Outwash and Stream Terraces	mesic	co. loamy over sandy (skeletal)	no	
Walpole	546	2.0	6.0	6.00	20.0	C	5	Outwash and Stream Terraces	mesic	sandy	no	
Peacham	549	0.6	2.0	0.00	0.2	D	6	Firm, platy, silty till, schist & phylitte	frigid	loamy	no	organic over loam
Skerry	558	0.6	2.0	0.06	0.6	C	3	Firm, platy, sardy till	frigid	loamy	yes	loamy sand in Cd
Plaisted	563	0.6	2.0	0.06	0.6	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Howland	566	0.6	2.0	0.06	0.0	c	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	silt loam, platy in Cd
Monarda	569	0.2	2.0	0.02	0.2	D	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	Litt loani, platy in ou
	572	0.6	2.0	0.60	2.0	B	2	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam

Soil Series	legend	Ksat low - B	Ksat high - B	Ksat low - C	Ksat high - C	Hyd.	Group	Land Form	Temp.	Soil Textures	Spodosol	Other
	number	in/hr	in/hr	in/hr	in/hr	Grp.	-				. ?	
Dixmont	578	0.6	2.0	0.60	2.0	С	3	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam, platy in C
Cabot	589	0.6	2.0	0.06	0.2	D	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Westbrook	597			0.00	2.0	D	6	Tidal Flat	mesic	loamy	no	organic over loam
Mundal	610	0.6	2.0	0.06	0.6	С	3	Firm, platy, loamy till	frigid	loamy	yes	gravelly sandy loam in Cd
Croghan	613	20.0	100.0	20.00	100.0	В	3	Outwash and Stream Terraces	frigid	sandy	yes	single grain in C
Kinsman	614	6.0	20.0	6.00	20.0	С	5	Outwash and Stream Terraces	frigid	sandy	yes	
Salmon	630	0.6	2.0	0.60	2.0	В	2	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Nicholville	632	0.6	2.0	0.60	2.0	С	3	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Pemi	633	0.6	2.0	0.06	0.6	С	5	Terraces and glacial lake plains	frigid	silty	no	
Pillsbury	646	0.6	2.0	0.06	0.2	С	5	Firm, platy, loamy till	frigid	silty	no	
Ridgebury	656	0.6	6.0	0.00	0.2	С	5	Firm, platy, loamy till	mesic	loamy	no	
Canaan	663	2.0	20.0	2.00	20.0	С	4	Weathered Bedrock Till	frigid	loamy-skeletal	yes	less than 20 in. deep
Redstone	665	2.0	6.0	6.00	20.0	А	1	Weathered Bedrock Till	frigid	fragmental	yes	loamy cap
Sisk	667	0.6	2.0	0.00	0.6	С	3	Firm, platy, loamy till	cryic	loamy	yes	sandy loam in Cd
Surplus	669	0.6	2.0	0.00	0.6	С	3	Firm, platy, loamy till	cryic	loamy	yes	mwd, sandy loam in Cd
Glebe	671	2.0	6.0	2.00	6.0	С	4	Loose till, bedrock	cryic	loamy	yes	20 to 40 in. deep
Saddleback	673	0.6	2.0	0.60	2.0	C/D	4	Loose till, bedrock	cryic	loamy	yes	less than 20 in. deep
Ricker	674	2.0	6.0	2.00	6.0	А	4	Organic over bedrock (up to 4" of mineral)	cryic	fibric to hemic	no	well drained, less than 20 in. deep
Houghtonville	795	0.6	6.0	0.60	6.0	В	2	Loose till, loamy textures	frigid	loamy	yes	cobbly fine sandy loam
Matunuck	797			20.00	100.0	D	6	Tidal Flat	mesic	sandy	no	organic over sand
Meadowsedge	894					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Bucksport	895					D	6	Organic Materials - Freshwater	frigid	sapric	no	deep organic
Colonel	927	0.6	2.0	0.06	0.6	С	3	Firm, platy, loamy till	frigid	loamy	yes	loam in Cd
Pondicherry	992			6.00	20.0	D	6	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Wonsqueak	995			0.20	2.0	D	6	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Glover	NA	0.6	2.0	0.60	2	D	4	Friable till, silty, schist & phyllite	frigid	loamy	no	less than 20 in. deep

no longer recognized organic materials

> Sorted by Numerical Legend K_{sat} B and C horizons SSSNNE Special pub no. 5

TABLE B

SOIL SERIES

Soil Series	legend	Ksat low - B	Ksat high - B	Ksat low - C	Ksat high - C	Hyd.	Group	Land Form	Temp.	Soil Textures	Spodosol	Other
	number	in/hr	in/hr	in/hr	in/hr	Grp.					?	
Abenaki	501	0.6	2.0	6.00	99.0	В	2	Outwash and Stream Terraces	frigid	loamy over sandy-skeletal	no	loamy over gravelly
Acton	146	2.0	20.0	2.00	20.0	В	3	Loose till, sandy textures	mesic	sandy-skeletal	no	cobbly loamy sand
Adams	36	6.0	20.0	20.00	99.0	Α	1	Outwash and Stream Terraces	frigid	sandy	yes	
Agawam	24	6.0	20.0	20.00	100.0	В	2	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Allagash	127	0.6	2.0	6.00	20.0	В	2	Outwash and Stream Terraces	frigid	loamy over sandy	yes	loamy over sandy
Au Gres	516					В	5	Outwash and Stream Terraces	frigid	sandy	yes	single grain, loose
Bangor	572	0.6	2.0	0.60	2.0	В	2	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam
Becket	56	0.6	2.0	0.06	0.6	С	3	Firm, platy, sandy till	frigid	loamy	yes	gravelly sandy loam in Cd
Belgrade	532	0.6	2.0	0.06	2.0	В	3	Terraces and glacial lake plains	mesic	silty	no	strata of fine sand
Bemis	224	0.6	0.2	0.00	0.2	С	5	Firm, platy, loamy till	cryic	loamy	no	
Berkshire	72	0.6	6.0	0.60	6.0	В	2	Loose till, loamy textures	frigid	loamy	yes	fine sandy loam
Bernardston	330	0.6	2.0	0.06	0.2	С	3	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Bice	226	0.6	6.0	0.60	6.0	В	2	Loose till, loamy textures	frigid	loamy	no	sandy loam
Biddeford	234	0.0	0.2	0.00	0.2	D	6	Silt and Clay Deposits	frigid	fine	no	organic over clay
Binghamville	534	0.2	2.0	0.06	0.2	D	5	Terraces and glacial lake plains	mesic	silty	no	
Boscawen	220	6.0	20.0	20.00	100.0	Α	1	Outwash and Stream Terraces	frigid	sandy-skeletal	no	loamy cap
Boxford	32	0.1	0.2	0.00	0.2	С	3	Silt and Clay Deposits	mesic	fine	no	silty clay loam
Brayton	240	0.6	2.0	0.06	0.6	С	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Buckland	237	0.6	2.0	0.06	0.2	С	3	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Bucksport	895					D	6	Organic Materials - Freshwater	frigid	sapric	no	deep organic
Burnham	131	0.2	6.0	0.02	0.2	D	6	Firm, platy, silty till, schist & phylitte	frigid	loamv	no	organic over silt
Buxton	232	0.1	0.6	0.00	0.2	С	3	Silt and Clay Deposits	frigid	fine	no	silty clay
Cabot	589	0.6	2.0	0.06	0.2	D	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Caesar	526	20.0	100.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	coarse sand	no	
Canaan	663	2.0	20.0	2.00	20.0	C	4	Weathered Bedrock Till	frigid	loamy-skeletal	ves	less than 20 in. deep
Canterbury	166	0.6	2.0	0.06	0.6	Č	3	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Canton	42	2.0	6.0	6.00	20.0	В	2	Loose till, sandy textures	mesic	loamy over sandy	no	loamy over loamy sand
Cardigan	357	0.6	2.0	0.60	2.0	В	4	Friable till, silty, schist & phyllite	mesic	loamy	no	20 to 40 in. deep
Catden	296					A/D	6	Organic Materials - Freshwater	mesic	sapric	no	deep organic
Champlain	35	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	gravelly sand	no	
Charles	209	0.6	100.0	0.60	100.0	С	5	Flood Plain (Bottom Land)	frigid	silty	no	
Charlton	62	0.6	6.0	0.60	6.0	B	2	Loose till. loamy textures	mesic	loamy	no	fine sandy loam
Chatfield	89	0.6	6.0	0.60	6.0	B	4	Loose till, bedrock	mesic	loamy	no	20 to 40 in. deep
Chatfield Var.	289	0.6	6.0	0.60	6.0	B	3	Loose till, bedrock	mesic	loamy	no	mwd to swpd
Chesuncook	126	0.6	2.0	0.02	0.2	C	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	ves	channery silt loam in Cd
Chichester	442	0.6	2.0	2.00	6.0	В		Loose till, sandy textures	frigid	loamy over sandy	no	loamy over loamy sand
Chocorua	395			6.00	20.0	D	6	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Cohas	505	0.6	2.0	0.60	100.0	C	5	Flood Plain (Bottom Land)	frigid	co. loamy over sandy (skeletal)	no	
Colonel	927	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	loam in Cd
Colton	22	6.0	20.0	20.00	100.0	Ă	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	
Colton, gravelly	21	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravelly surface
Croghan	613	20.0	100.0	20.00	100.0	B	3	Outwash and Stream Terraces	frigid	sandy	yes	single grain in C
Dartmouth	132	0.6	2.0	0.06	0.6	B	3	Terraces and glacial lake plains	mesic	silty	no	thin strata silty clay loam
Deerfield	313	6.0	20.0	20.00	100.0	B	3	Outwash and Stream Terraces	mesic	sandy	no	single grain in C
Dixfield	378	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Dixmont	578	0.6	2.0	0.60	2.0	C	3	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam, platy in C
Duane	413	6.0	20.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	cemented (ortstein)
Dutchess	366	0.6	2.0	0.60	2.0	B	2	Friable till, silty, schist & phyllite	mesic	loamy	no	very channery
Eldridge	38	6.0	20.0	0.06	0.6	C	3	Sandy/loamy over silt/clay	mesic	sandy over loamy	no	very enamery
Elliottsville	128	0.6	2.0	0.60	2.0	B	4	Friable till, silty, schist & phyllite	frigid	loamy	ves	20 to 40 in. deep
Elmridge	238	2.0	6.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	mesic	loamy over clayey	no	20 10 40 III. deep
Elmwood	338	2.0	6.0	0.00	0.2	c	3	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	
	550	2.0	0.0	0.00	0.2	C	3	Outwash and Stream Terraces	frigid	sandy	ves	cemented (ortstein)

Fryeburg 20 Gilmanton 47 Glebe 67 Gloucester 1 Glover N Grange 43 Greenwood 22 Groveton 2 Hadley 10 Hadley 11 Hartland 3 Haven 44 Henniker 4	Imber 208 478 671 11 NA 433 295 27 8 108 31 440 46	in/hr 0.6 0.6 2.0 6.0 0.6 0.6 0.6 0.6 0.6 0.6 0	in/hr 2.0 2.0 6.0 20.0 2.0 2.0 2.0 2.0 2.0 2.0	in/hr 2.00 0.06 2.00 6.00 0.60 0.60 0.60	in/hr 6.0 0.6 6.0 20.0 2 2.0	Grp. B C C A D C	2 3 4 1 4	Flood Plain (Bottom Land) Firm, platy, loamy till Loose till, bedrock Sandy Till	frigid frigid cryic	silty loamy	? no no	very fine sandy loam fine sandy loam in Cd
Gilmanton 47 Glebe 67 Gloucester 1 Glover N Grange 43 Greenwood 22 Groveton 2 Hadley 8 Hadley 10 Hartland 3 Haven 44 Henniker 4 Hermon 55	478 671 11 NA 433 295 27 8 108 31 410	0.6 2.0 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6	2.0 6.0 20.0 2.0 2.0 2.0 2.0 2.0	0.06 2.00 6.00 0.60 0.60 0.60	0.6 6.0 20.0 2	C C A D	3 4 1	Firm, platy, loamy till Loose till, bedrock	frigid	loamy	-	, ,
Glebe 67 Gloucester 1 Glover N Grange 43 Greenwood 29 Groveton 29 Hadley 10 Hartland 3 Haven 47 Henniker 4 Hermon 5	671 11 NA 433 295 27 8 108 31 410	2.0 6.0 0.6 0.6 0.6 0.6 0.6 0.6 0.6	6.0 20.0 2.0 2.0 2.0 2.0 2.0	2.00 6.00 0.60 0.60 0.60	6.0 20.0 2	C A D	4	Loose till, bedrock	0	,	no	fine sandy loam in Cd
Gloucester 1 Glover N Grange 43 Greenwood 29 Groveton 29 Hadley 28 Hadley 10 Hartland 3 Haven 47 Henniker 44 Hermon 55	11 NA 433 295 27 8 108 31 410	6.0 0.6 0.6 0.6 0.6 0.6 0.6 0.6	20.0 2.0 2.0 2.0 2.0 2.0	6.00 0.60 0.60 0.60	20.0 2	A	1		cryic			
Glover N Grange 42 Greenwood 22 Groveton 22 Hadley 8 Hadley 10 Hartland 3 Haven 42 Henniker 4 Herrmon 55	NA 433 295 27 8 108 31 410	0.6 0.6 0.6 0.6 0.6 0.6 0.6	2.0 2.0 2.0 2.0 2.0	0.60 0.60 0.60	2	D		Sandy Till		loamy	yes	20 to 40 in. deep
Grange 43 Greenwood 29 Groveton 2 Hadley 8 Hadley 10 Harland 3 Haven 44 Henniker 44 Hermon 5	433 295 27 8 108 31 410	0.6 0.6 0.6 0.6 0.6	2.0 2.0 2.0	0.60			4		mesic	sandy-skeletal	no	loamy cap
Greenwood 29 Groveton 2 Hadley 2 Hadley 10 Hartland 3 Haven 41 Henniker 4 Hermon 5	295 27 8 108 31 410	0.6 0.6 0.6 0.6	2.0 2.0	0.60	2.0	С		Friable till, silty, schist & phyllite	frigid	loamy	no	less than 20 in. deep
Groveton2Hadley8Hadley10Hartland3Haven4'Henniker4Hermon5	27 8 108 31 410	0.6 0.6 0.6	2.0				5	Outwash and Stream Terraces	frigid	co. loamy over sandy (skeletal)	no	
Hadley8Hadley10Hartland3Haven4'Henniker4Hermon5	8 108 31 410	0.6 0.6 0.6	2.0			A/D	6	Organic Materials - Freshwater	frigid	hemic	no	deep organic
Hadley10Hartland3Haven4'Henniker4Hermon5	108 31 410	0.6 0.6			6.0	В	2	Outwash and Stream Terraces	frigid	loamy	yes	loamy over sandy
Hartland3Haven4'Henniker4Hermon5	31 410	0.6	20	0.60	6.0	В	2	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand
Haven4'Henniker4Hermon5	410		-	0.60	6.0	В	2	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand, occ flooded
Henniker 4 Hermon 5	-		2.0	0.20	2.0	В	2	Terraces and glacial lake plains	mesic	silty	no	very fine sandy loam
Hermon 5	46	0.6	2.0	20.00	100.0	В	2	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
		0.6	2.0	0.06	0.6	С	3	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
	55	2.0	20.0	6.00	20.0	A	1	Sandy Till	frigid	sandy-skeletal	yes	loamy cap
	12	6.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	
	130	0.6	2.0	0.06	0.6	В	3	Terraces and glacial lake plains	mesic	silty	no	silt loam to silt in C
	91	2.0	6.0	2.00	6.0	С	4	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
	86	0.6	6.0	0.60	6.0	C/D	4	Loose till, bedrock	mesic	loamy	no	less than 20 in. deep
	510	2.0	20.0	20.00	100.0	A	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	slate, loamy cap
	795	0.6	6.0	0.60	6.0	В	2	Loose till, loamy textures	frigid	loamy	yes	cobbly fine sandy loam
	566	0.6	2.0	0.06	0.2	С	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	silt loam, platy in Cd
	397					D	6	Tidal Flat	mesic	hemic/sapric	no	deep organic
<u> </u>	359	0.6	2.0	0.60	2.0	В	4	Friable till, silty, schist & phyllite	mesic	loamy	no	less than 20 in. deep
	614	6.0	20.0	6.00	20.0	С	5	Outwash and Stream Terraces	frigid	sandy	yes	
	228	0.6	2.0	0.06	0.2	С	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	channery silt loam in Cd
	514	0.6	6.0	0.60	20.0	С	5	Loose till, loamy textures	mesic	loamy	no	
	3	0.6	2.0	6.00	20.0	С	5	Flood Plain (Bottom Land)	mesic	loamy	no	
	109	0.6	2.0	0.60	2.0	C	5	Flood Plain (Bottom Land)	mesic	silty	no	
	259	0.6	6.0	2.00	20.0	C/D B	2	Weathered bedrock, phyllite	frigid	loamy	no	very channery
	307	0.6	2.0	0.60	2.0		3	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
	92 246	2.0 0.6	6.0 6.0	2.00 0.60	6.0 6.0	A/D C	4	Loose till, bedrock Loose till, sandy textures	frigid frigid	loamy	yes	less than 20 in. deep
	246 520	2.0	6.0	6.00	20.0	B	3		0	loamy	no	strata sand/gravel in C
	252	2.0	2.0	0.60	20.0	C	4	Outwash and Stream Terraces	frigid	sandy or sandy-skeletal loamy-skeletal	yes ves	20 to 40 in. deep
	232	0.6	2.0	6.00	20.0	B	3	Friable till, silty, schist & phyllite Outwash and Stream Terraces	frigid frigid	loamy over sandy	yes	sandy or sandy-skeletal
	48	0.6	2.0	6.00	20.0	B	3	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
	76	0.6	2.0	0.06	0.6	C	3	Firm, platy, loamy till	frigid	loamy	ves	fine sandy loam in Cd
	23	6.0	20.0	6.00	20.0	A	1	Outwash and Stream Terraces	frigid	sandy-skeletal	ves	slate, loamy cap
	315	6.0	20.0	6.00	20.0	В	5	Outwash and Stream Terraces	mesic	sandy	ves	blate, iourny oup
	797	0.0	20.0	20.00	100.0	D	6	Tidal Flat	mesic	sandy	no	organic over sand
	134	0.0	0.2	0.00	0.2	D	6	Silt and Clay Deposits	mesic	fine	no	silt over clay
	894	0.0		0.00	0.2	D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
	406	0.6	2.0	0.60	2.0	D	6	Flood Plain (Bottom Land)	frigid	silty	no	organic over silt
	37	2.0	6.0	0.00	0.2	C	3	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	silty clay loam in C
	10	2.0	20.0	6.00	20.0	A	1	Outwash and Stream Terraces	mesic	gravelly sand	no	loamy cap
	458	0.6	2.0	0.06	0.6	C	3	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
	404	6.0	100.0	6.00	100.0	B	3	Flood Plain (Bottom Land)	frigid	loamy over sandy	no	sandy or sandy-skeletal
	39					C	3	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
	251	0.6	6.0	0.60	6.0	C	4	Loose till, bedrock	frigid	loamy	no	20 to 40 in. deep
	142	0.6	2.0	2.00	6.0	B	2	Loose till, sandy textures	frigid	bamy over sandy, sandy-skeleta	ves	gravelly loamy sand in C
	569	0.2	2.0	0.02	0.2	D	5	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	g. s. siy ioaniy cana in o
	133	0.6	2.0	0.60	2.0	D	4	Friable till, silty, schist & phyllite	frigid	loamy	ves	less than 20 in. deep
	44	0.6	6.0	0.06	0.6	C	3	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
	414	6.0	20.0	6.00	20.0	c	5	Loose till, sandy textures	frigid	sandy	no	isaniy cana in cu

Soil Series	legend	Ksat low - B	Ksat high - B	Ksat low - C	Ksat high - C	Hyd.	Group	Land Form	Temp.	Soil Textures	Spodosol	Other
	number	in/hr	in/hr	in/hr	in/hr	Grp.					?	
Mundal	610	0.6	2.0	0.06	0.6	С	3	Firm, platy, loamy till	frigid	loamy	yes	gravelly sandy loam in Cd
Natchaug	496			0.20	2.0	D	6	Organic Materials - Freshwater	mesic	loamy	no	organic over loam
Naumburg	214	6.0	20.0	6.00	20.0	С	5	Outwash and Stream Terraces	frigid	sandy	yes	
Newfields	444	0.6	2.0	0.60	2.0	В	3	Loose till, sandy textures	mesic	loamy over sandy	no	sandy or sandy-skeletal
Nicholville	632	0.6	2.0	0.60	2.0	С	3	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Ninigret	513	0.6	6.0	6.00	20.0	В	3	Outwash and Stream Terraces	mesic	loamy over sandy	no	sandy or sandy-skeletal
Occum	1	0.6	2.0	6.00	20.0	В	2	Flood Plain (Bottom Land)	mesic	loamy	no	loamy over loamy sand
Ondawa	101	0.6	6.0	6.00	20.0	В	2	Flood Plain (Bottom Land)	frigid	loamy	no	loamy over loamy sand
Ondawa	201	0.6	6.0	6.00	20.0	В	2	Flood Plain (Bottom Land)	frigid	loamy	no	occ flood, loamy over I. sand
Ossipee	495			0.20	2.0	D	6	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Pawcatuck	497			20.00	100.0	D	6	Tidal Flat	mesic	sandy or sandy-skeletal	no	organic over sand
Paxton	66	0.6	2.0	0.00	0.2	С	3	Firm, platy, loamy till	mesic	loamy	no	
Peacham	549	0.6	2.0	0.00	0.2	D	6	Firm, platy, silty till, schist & phylitte	frigid	loamy	no	organic over loam
Pemi	633	0.6	2.0	0.06	0.6	С	5	Terraces and glacial lake plains	frigid	silty	no	
Pennichuck	460	0.6	2.0	0.60	2.0	В	4	Friable till, silty, schist & phyllite	mesic	loamy-skeletal	no	20 to 40 in. deep
Peru	78	0.6	2.0	0.06	0.6	С	3	Firm, platy, loamy till	frigid	loamy	yes	
Pillsbury	646	0.6	2.0	0.06	0.2	С	5	Firm, platy, loamy till	frigid	silty	no	
Pipestone	314					В	5	Outwash and Stream Terraces	mesic	sandy	yes	
Pittstown	334	0.6	2.0	0.06	0.2	С	3	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Plaisted	563	0.6	2.0	0.06	0.6	С	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Podunk	104	0.6	6.0	6.00	20.0	В	3	Flood Plain (Bottom Land)	frigid	loamy	no	loamy to coarse sand in C
Pondicherry	992			6.00	20.0	D	6	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Poocham	230	0.6	2.0	0.20	2.0	В	3	Terraces and glacial lake plains	mesic	silty	no	silt loam in C
Pootatuck	4	0.6	6.0	6.00	20.0	В	3	Flood Plain (Bottom Land)	mesic	loamy	no	single grain in C
Quonset	310	2.0	20.0	20.00	100.0	Α	1	Outwash and Stream Terraces	mesic	sandy-skeletal	no	shale
Rawsonville	98	0.6	6.0	0.60	6.0	С	4	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Raynham	533	0.2	2.0	0.06	0.2	С	5	Terraces and glacial lake plains	mesic	silty	no	
Raypol	540	0.6	2.0	6.00	100.0	D	5	Outwash and Stream Terraces	mesic	co. loamy over sandy (skeletal)	no	
Redstone	665	2.0	6.0	6.00	20.0	Α	1	Weathered Bedrock Till	frigid	fragmental	yes	loamy cap
Ricker	674	2.0	6.0	2.00	6.0	Α	4	rganic over bedrock (up to 4" of miner	cryic	fibric to hemic	no	well drained, less than 20 in. deep
Ridgebury	656	0.6	6.0	0.00	0.2	С	5	Firm, platy, loamy till	mesic	loamy	no	
Rippowam	5	0.6	6.0	6.00	20.0	С	5	Flood Plain (Bottom Land)	mesic	loamy	no	
Roundabout	333	0.2	2.0	0.06	0.6	С	5	Terraces and glacial lake plains	frigid	silty	no	silt loam in the C
Rumney	105	0.6	6.0	6.00	20.0	С	5	Flood Plain (Bottom Land)	frigid	loamy	no	
Saco	6	0.6	2.0	6.00	20.0	D	6	Flood Plain (Bottom Land)	mesic	silty	no	strata
Saddleback	673	0.6	2.0	0.60	2.0	C/D	4	Loose till, bedrock	cryic	loamy	yes	less than 20 in. deep
Salmon	630	0.6	2.0	0.60	2.0	В	2	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Saugatuck	16	0.06	0.2	6.00	20.0	С	5	Outwash and Stream Terraces	mesic	sandy	yes	ortstein
Scantic	233	0.0	0.2	0.00	0.2	D	5	Silt and Clay Deposits	frigid	fine	no	
Scarboro	115	6.0	20.0	6.00	20.0	D	6	Outwash and Stream Terraces	mesic	sandy	no	organic over sand, non stony
Scio	531	0.6	2.0	0.60	2.0	В	3	Terraces and glacial lake plains	mesic	silty	no	gravelly sand in 2C
Scitico	33	0.0	0.2	0.00	0.2	С	5	Silt and Clay Deposits	mesic	fine	no	
Scituate	448	0.6	2.0	0.06	0.2	С	3	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Searsport	15	6.0	20.0	6.00	20.0	D	6	Outwash and Stream Terraces	frigid	sandy	no	organic over sand
Shaker	439	2.0	6.0	0.00	0.2	С	5	Sandy/loamy over silt/clay	mesic	co. loamy over clayey	no	
Shapleigh	136					C/D	4	Sandy Till	mesic	sandy	yes	less than 20 in. deep
Sheepscot	14	6.0	20.0	6.00	20.0	В	3	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravelly coarse sand
Sisk	667	0.6	2.0	0.00	0.6	С	3	Firm, platy, loamy till	cryic	loamy	yes	sandy loam in Cd
Skerry	558	0.6	2.0	0.06	0.6	С	3	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Squamscott	538	6.0	20.0	0.06	0.6	С	5	Sandy/loamy over silt/clay	mesic	sandy over loamy	yes	
Stetson	523	0.6	6.0	6.00	20.0	В	2	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	loamy over gravelly
Stissing	340	0.6	2.0	0.06	0.2	С	5	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	
Success	154	2.0	6.0	6.00	20.0	Α	1	Sandy Till	frigid	sandy-skeletal	yes	cemented
Sudbury	118	2.0	6.0	2.00	20.0	В	3	Outwash and Stream Terraces	mesic	sandy	no	loam over gravelly sand

Soil Series	legend	Ksat low - B	Ksat high - B	Ksat low - C	Ksat high - C	Hyd.	Group	Land Form	Temp.	Soil Textures	Spodosol	Other
	number	in/hr	in/hr	in/hr	in/hr	Grp.	-				?	
Suffield	536	0.6	2.0	0.00	0.2	С	3	Sandy/loamy over silt/clay	mesic	silty over clayey	no	deep to clay C
Sunapee	168	0.6	2.0	0.60	6.0	В	3	Loose till, loamy textures	frigid	loamy	yes	
Sunapee var	269	0.6	2.0	0.60	6.0	В	3	Loose till, loamy textures	frigid	loamy	yes	frigid dystrudept
Suncook	2	6.0	20.0	6.00	20.0	Α	1	Flood Plain (Bottomland)	mesic	sandy	no	occasionally flooded
Suncook	402	6.0	20.0	6.00	20.0	Α	1	Flood Plain (Bottomland)	mesic	sandy	no	frequent flooding
Sunday	102	6.0	20.0	6.00	20.0	Α	1	Flood Plain (Bottomland)	frigid	sandy	no	occasionally flooded
Sunday	202	6.0	20.0	6.00	20.0	Α	1	Flood Plain (Bottomland)	frigid	sandy	no	frequently flooded
Surplus	669	0.6	2.0	0.00	0.6	С	3	Firm, platy, loamy till	cryic	loamy	yes	mwd, sandy loam in Cd
Sutton	68	0.6	6.0	0.60	6.0	В	3	Loose till, loamy textures	mesic	loamy	no	
Swanton	438	2.0	6.0	0.00	0.2	С	5	Sandy/loamy over silt/clay	frigid	co. loamy over clayey	no	
Telos	123	0.6	2.0	0.02	0.2	С	3	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Thorndike	84	0.6	2.0	0.60	2.0	C/D	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	less than 20 in. deep
Timakwa	393			6.00	100.0	D	6	Organic Materials - Freshwater	mesic	sandy or sandy-skeletal	no	organic over sand
Tunbridge	99	0.6	6.0	0.60	6.0	С	4	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Unadilla	30	0.6	2.0	2.00	20.0	В	2	Terraces and glacial lake plains	mesic	silty	no	silty over gravelly
Vassalboro	150					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Walpole	546	2.0	6.0	6.00	20.0	С	5	Outwash and Stream Terraces	mesic	sandy	no	• •
Wareham	34	6.0	20.0	6.00	20.0	С	5	Outwash and Stream Terraces	mesic	sandy	no	
Warwick	210	2.0	6.0	20.00	100.0	Α	1	Outwash and Stream Terraces	mesic	loamy-skeletal	no	loamy over slate gravel
Waskish	195					D	6	Organic Materials - Freshwater	frigid	peat	no	deep organic
Waumbeck	58	2.0	20.0	6.00	20.0	В	3	Loose till, sandy textures	frigid	sandy-skeletal	yes	very cobbly loamy sand
Westbrook	597			0.00	2.0	D	6	Tidal Flat	mesic	loamy	no	organic over loam
Whitman	49	0.0	0.2	0.00	0.2	D	6	Firm, platy, loamy till	mesic	loamy	no	mucky loam
Windsor	26	6.0	20.0	6.00	20.0	Α	1	Outwash and Stream Terraces	mesic	sandy	no	
Winnecook	88	0.6	2.0	0.60	2.0	С	4	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Winooski	9	0.6	6.0	0.60	6.0	В		Flood Plain (Bottom Land)	mesic	silty over loamy	no	· · · · ·
Winooski	103	0.6	6.0	0.60	6.0	В	3	Flood Plain (Bottom Land)	mesic	silty	no	very fine sandy loam
Wonsqueak	995			0.20	2.0	D	6	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Woodbridge	29	0.6	2.0	0.00	0.6	С	3	Firm, platy, loamy till	mesic	loamy	no	sandy loam in Cd
Woodstock	93	2.0	6.0	2.00	6.0	C/D	4	Loose till, bedrock	frigid	loamy	no	less than 20 in. deep



no longer recognized organic materials

TABLE C

NHDES SOIL GROUPINGS

Soil Series	number	NHDES	Ksat low - B	Ksat high - B	Ksat low - C	Ksat high - C	Hyd.	Land Form	Temp.	Soil Textures	Spodosol	Other
		Soil Group	in/hr	in/hr	in/hr	in/hr	Grp.				?	
Adams	36	1	6.0	20.0	20.00	99.0	Α	Outwash and Stream Terraces	frigid	sandy	yes	
Boscawen	220	1	6.0	20.0	20.00	100.0	Α	Outwash and Stream Terraces	frigid	sandy-skeletal	no	loamy cap
Caesar	526	1	20.0	100.0	20.00	100.0	Α	Outwash and Stream Terraces	mesic	coarse sand	no	
Champlain	35	1	6.0	20.0	20.00	100.0	Α	Outwash and Stream Terraces	frigid	gravelly sand	no	
Colton	22	1	6.0	20.0	20.00	100.0	Α	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	
Colton, gravelly	21	1	6.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravelly surface
Gloucester	11	1	6.0	20.0	6.00	20.0	A	Sandy Till	mesic	sandy-skeletal	no	loamy cap
Hermon	55	1	2.0	20.0	6.00	20.0	A	Sandy Till	frigid	sandy-skeletal	yes	loamy cap
Hinckley	12	1	6.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	mesic	sandy-skeletal	no	
Hoosic	510	1	2.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	mesic	sandy-skeletal	no	slate, loamy cap
Masardis	23	1	6.0	20.0	6.00	20.0	A	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	slate, loamy cap
Merrimac	10	1	2.0	20.0	6.00	20.0	A	Outwash and Stream Terraces	mesic	gravelly sand	no	loamy cap
Quonset	310	1	2.0	20.0	20.00	100.0	A	Outwash and Stream Terraces	mesic	sandy-skeletal	no	shale
Redstone	665	1	2.0	6.0	6.00	20.0	A	Weathered Bedrock Till	frigid	fragmental	yes	loamy cap
Success	154	1	2.0	6.0	6.00	20.0	A	Sandy Till	frigid	sandy-skeletal	yes	cemented
Suncook	2	1	6.0	20.0	6.00	20.0	A	Flood Plain (Bottomland)	mesic	sandy	no	occasionally flooded
Suncook	402	1	6.0	20.0	6.00	20.0	A	Flood Plain (Bottomland)	mesic	sandy	no	frequent flooding
Sunday	102	1	6.0	20.0 20.0	6.00	20.0 20.0	A	Flood Plain (Bottomland)	frigid	sandy	no	occasionally flooded
Sunday Warwick	202 210	1	6.0 2.0	6.0	6.00 20.00	100.0	A	Flood Plain (Bottomland) Outwash and Stream Terraces	frigid	sandy	no	frequently flooded
Windsor	210	1	6.0	20.0	6.00	20.0	A	Outwash and Stream Terraces	mesic mesic	loamy-skeletal sandy	no no	loamy over slate gravel
WINGSON	20	I	0.0	20.0	6.00	20.0	A	Outwash and Stream remaces	mesic	sandy	no	
Abenaki	501	2	0.6	2.0	6.00	99.0	В	Outwash and Stream Terraces	frigid	loamy over sandy-skeletal	no	loamy over gravelly
Agawam	24	2	6.0	20.0	20.00	100.0	B	Outwash and Stream Terraces	mesic	loamy over sandy-skeletar	no	loamy over sand/gravel
Allagash	127	2	0.6	2.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	loamy over sandy	ves	loamy over sandy
Bangor	572	2	0.6	2.0	0.60	2.0	B	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam
Berkshire	72	2	0.6	6.0	0.60	6.0	B	Loose till, loamy textures	frigid	loamy	yes	fine sandy loam
Bice	226	2	0.6	6.0	0.60	6.0	B	Loose till, loamy textures	frigid	loamy	no	sandy loam
Canton	42	2	2.0	6.0	6.00	20.0	B	Loose till, sandy textures	mesic	loamy over sandy	no	loamy over loamy sand
Charlton	62	2	0.6	6.0	0.60	6.0	В	Loose till, loamy textures	mesic	loamy	no	fine sandy loam
Dutchess	366	2	0.6	2.0	0.60	2.0	В	Friable till, silty, schist & phyllite	mesic	loamy	no	very channery
Fryeburg	208	2	0.6	2.0	2.00	6.0	В	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Groveton	27	2	0.6	2.0	0.60	6.0	В	Outwash and Stream Terraces	frigid	loamy	yes	loamy over sandy
Hadley	8	2	0.6	2.0	0.60	6.0	В	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand
Hadley	108	2	0.6	2.0	0.60	6.0	В	Flood Plain (Bottom Land)	mesic	silty	no	strata of fine sand, occ flooded
Hartland	31	2	0.6	2.0	0.20	2.0	В	Terraces and glacial lake plains	mesic	silty	no	very fine sandy loam
Haven	410	2	0.6	2.0	20.00	100.0	В	Outwash and Stream Terraces	mesic	loamy over sandy	no	loamy over sand/gravel
Houghtonville	795	2	0.6	6.0	0.60	6.0	В	Loose till, loamy textures	frigid	loamy	yes	cobbly fine sandy loam
Lombard	259	2	0.6	6.0	2.00	20.0	C/D	Weathered bedrock, phyllite	frigid	loamy	no	very channery
Monadnock	142	2	0.6	2.0	2.00	6.0	В	Loose till, sandy textures	frigid	oamy over sandy, sandy-skelet	yes	gravelly loamy sand in C
Occum	1	2	0.6	2.0	6.00	20.0	В	Flood Plain (Bottom Land)	mesic	loamy	no	loamy over loamy sand
Ondawa	101	2	0.6	6.0	6.00	20.0	В	Flood Plain (Bottom Land)	frigid	loamy	no	loamy over loamy sand
Ondawa	201	2	0.6	6.0	6.00	20.0	В	Flood Plain (Bottom Land)	frigid	loamy	no	occ flood, loamy over I. sand
Salmon	630	2	0.6	2.0	0.60	2.0	В	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Stetson	523	2	0.6	6.0	6.00	20.0	В	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	loamy over gravelly
Unadilla	30	2	0.6	2.0	2.00	20.0	В	Terraces and glacial lake plains	mesic	silty	no	silty over gravelly
Chichester	442	2	0.6	2.0	2.00	6.0	В	Loose till, sandy textures	frigid	loamy over sandy	no	loamy over loamy sand
							_					
Acton	146	3	2.0	20.0	2.00	20.0	В	Loose till, sandy textures	mesic	sandy-skeletal	no	cobbly loamy sand
Becket	56	3	0.6	2.0	0.06	0.6	С	Firm, platy, sandy till	frigid	loamy	yes	gravelly sandy loam in Cd
Belgrade	532	3	0.6	2.0	0.06	2.0	В	Terraces and glacial lake plains	mesic	silty	no	strata of fine sand
Bernardston	330	3	0.6	2.0	0.06	0.2	C	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Boxford	32	3	0.1	0.2	0.00	0.2	С	Silt and Clay Deposits	mesic	fine	no	silty clay loam

Sorted by DES Soil Group for Establishing Lot Size K_{sat} B and C horizons SSSNNE pub no. 5

Soil Series	number	NHDES	Ksat low - B	Ksat high - B	Ksat low - C	Ksat high - C	Hyd.	Land Form	Temp.	Soil Textures	Spodosol	Other
		Soil Group	in/hr	in/hr	in/hr	in/hr	Grp.				?	
Buckland	237	3	0.6	2.0	0.06	0.2	С	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Buxton	232	3	0.1	0.6	0.00	0.2	С	Silt and Clay Deposits	frigid	fine	no	silty clay
Canterbury	166	3	0.6	2.0	0.06	0.6	С	Firm, platy, loamy till	frigid	loamy	no	loam in Cd
Chatfield Var.	289	3	0.6	6.0	0.60	6.0	В	Loose till, bedrock	mesic	loamy	no	mwd to swpd
Chesuncook	126	3	0.6	2.0	0.02	0.2	С	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd
Colonel	927	3	0.6	2.0	0.06	0.6	С	Firm, platy, loamy till	frigid	loamy	yes	loam in Cd
Croghan	613	3	20.0	100.0	20.00	100.0	В	Outwash and Stream Terraces	frigid	sandy	yes	single grain in C
Dartmouth	132	3	0.6	2.0	0.06	0.6	В	Terraces and glacial lake plains	mesic	silty	no	thin strata silty clay loam
Deerfield	313	3	6.0	20.0	20.00	100.0	В	Outwash and Stream Terraces	mesic	sandy	no	single grain in C
Dixfield	378	3	0.6	2.0	0.06	0.6	С	Firm, platy, loamy till	frigid	loamy	yes	fine sandy loam in Cd
Dixmont	578	3	0.6	2.0	0.60	2.0	С	Friable till, silty, schist & phyllite	frigid	loamy	yes	silt loam, platy in C
Duane	413	3	6.0	20.0	6.00	20.0	В	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	cemented (ortstein)
Eldridge	38	3	6.0	20.0	0.06	0.6	С	Sandy/loamy over silt/clay	mesic	sandy over loamy	no	
Elmridge	238	3	2.0	6.0	0.00	0.2	С	Sandy/loamy over silt/clay	mesic	loamy over clayey	no	
Elmwood	338	3	2.0	6.0	0.00	0.2	С	Sandy/loamy over silt/clay	frigid	loamy over clayey	no	
Finch	116	3					C	Outwash and Stream Terraces	frigid	sandy	yes	cemented (ortstein)
Gilmanton	478	3	0.6	2.0	0.06	0.6	C	Firm, platy, loamy till	frigid	loamy	no	fine sandy loam in Cd
Henniker	46	3	0.6	2.0	0.06	0.6	С	Firm, platy, sandy till	frigid	loamy	no	loamy sand in Cd
Hitchcock	130	3	0.6	2.0	0.06	0.6	В	Terraces and glacial lake plains	mesic	silty	no	silt loam to silt in C
Howland	566	3	0.6	2.0	0.06	0.2	С	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	silt loam, platy in Cd
Lanesboro	228	3	0.6	2.0	0.06	0.2	С	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	channery silt loam in Cd
Lovewell	307	3	0.6	2.0	0.60	2.0	B	Flood Plain (Bottom Land)	frigid	silty	no	very fine sandy loam
Machias	520	3	2.0	6.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	sandy or sandy-skeletal	yes	strata sand/gravel in C
Madawaska	28	3	0.6	2.0	6.00	20.0	B	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
ladawaska, aquer	48	3	0.6	2.0 2.0	6.00 0.06	20.0 0.6	B C	Outwash and Stream Terraces	frigid	loamy over sandy	yes	sandy or sandy-skeletal
Marlow Melrose	37	3	2.0	6.0	0.00	0.8	C C	Firm, platy, loamy till Sandy/loamy over silt/clay	frigid frigid	loamy	yes	fine sandy loam in Cd silty clay loam in C
Metacomet	458	3	0.6	2.0	0.00	0.2	C C	Firm, platy, sandy till	frigid	loamy over clayey loamy	no	loamy sand in Cd
Metallak	404	3	6.0	100.0	6.00	100.0	B	Flood Plain (Bottom Land)	frigid	loamy over sandy	no	sandy or sandy-skeletal
Millis	39	3	0.0	100.0	0.00	100.0	C	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Montauk	44	3	0.6	6.0	0.06	0.6	c	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Mundal	610	3	0.6	2.0	0.06	0.6	c	Firm, platy, loamy till	frigid	loamy	ves	gravelly sandy loam in Cd
Newfields	444	3	0.6	2.0	0.60	2.0	В	Loose till, sandy textures	mesic	loamy over sandy	no	sandy or sandy-skeletal
Nicholville	632	3	0.6	2.0	0.60	2.0	C	Terraces and glacial lake plains	frigid	silty	yes	very fine sandy loam
Ninigret	513	3	0.6	6.0	6.00	20.0	B	Outwash and Stream Terraces	mesic	loamy over sandy	no	sandy or sandy-skeletal
Paxton	66	3	0.6	2.0	0.00	0.2	C	Firm, platy, loamy till	mesic	loamy	no	
Peru	78	3	0.6	2.0	0.06	0.6	Č	Firm, platy, loamy till	frigid	loamy	ves	
Pittstown	334	3	0.6	2.0	0.06	0.2	Č	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	channery silt loam in Cd
Plaisted	563	3	0.6	2.0	0.06	0.6	C	Firm, platy, silty till, schist & phyllite	frigid	loamy	ves	channery silt loam in Cd
Podunk	104	3	0.6	6.0	6.00	20.0	В	Flood Plain (Bottom Land)	frigid	loamy	no	loamy to coarse sand in C
Poocham	230	3	0.6	2.0	0.20	2.0	В	Terraces and glacial lake plains	mesic	silty	no	silt loam in C
Pootatuck	4	3	0.6	6.0	6.00	20.0	В	Flood Plain (Bottom Land)	mesic	loamy	no	single grain in C
Scio	531	3	0.6	2.0	0.60	2.0	В	Terraces and glacial lake plains	mesic	silty	no	gravelly sand in 2C
Scituate	448	3	0.6	2.0	0.06	0.2	С	Firm, platy, sandy till	mesic	loamy	no	loamy sand in Cd
Sheepscot	14	3	6.0	20.0	6.00	20.0	В	Outwash and Stream Terraces	frigid	sandy-skeletal	yes	gravelly coarse sand
Sisk	667	3	0.6	2.0	0.00	0.6	С	Firm, platy, loamy till	cryic	loamy	yes	sandy loam in Cd
Skerry	558	3	0.6	2.0	0.06	0.6	С	Firm, platy, sandy till	frigid	loamy	yes	loamy sand in Cd
Sudbury	118	3	2.0	6.0	2.00	20.0	В	Outwash and Stream Terraces	mesic	sandy	no	loam over gravelly sand
Suffield	536	3	0.6	2.0	0.00	0.2	С	Sandy/loamy over silt/clay	mesic	silty over clayey	no	deep to clay C
Sunapee	168	3	0.6	2.0	0.60	6.0	В	Loose till, loamy textures	frigid	loamy	yes	
Sunapee var	269	3	0.6	2.0	0.60	6.0	В	Loose till, loamy textures	frigid	loamy	yes	frigid dystrudept
Surplus	669	3	0.6	2.0	0.00	0.6	С	Firm, platy, loamy till	cryic	loamy	yes	mwd, sandy loam in Cd
Sutton	68	3	0.6	6.0	0.60	6.0	В	Loose till, loamy textures	mesic	loamy	no	
Telos	123	3	0.6	2.0	0.02	0.2	С	Firm, platy, silty till, schist & phyllite	frigid	loamy	yes	channery silt loam in Cd

Soil Series	number	NHDES	Ksat low - B	Ksat high - B	Ksat low - C	Ksat high - C	Hyd.	Land Form	Temp.	Soil Textures	Spodosol	Other
		Soil Group	in/hr	in/hr	in/hr	in/hr	Grp.				?	
Waumbeck	58	3	2.0	20.0	6.00	20.0	В	Loose till, sandy textures	frigid	sandy-skeletal	yes	very cobbly loamy sand
Winooski	103	3	0.6	6.0	0.60	6.0	В	Flood Plain (Bottom Land)	mesic	silty	no	very fine sandy loam
Woodbridge	29	3	0.6	2.0	0.00	0.6	С	Firm, platy, loamy till	mesic	loamy	no	sandy loam in Cd
Winooski	9	3	0.6	6.0	0.60	6.0	В	Flood Plain (Bottom Land)	mesic	silty over loamy	no	
Canaan	663	4	2.0	20.0	2.00	20.0	С	Weathered Bedrock Till	frigid	loamy-skeletal	yes	less than 20 in. deep
Cardigan	357	4	0.6	2.0	0.60	2.0	В	Friable till, silty, schist & phyllite	mesic	loamy	no	20 to 40 in. deep
Chatfield	89	4	0.6	6.0	0.60	6.0	В	Loose till, bedrock	mesic	loamy	no	20 to 40 in. deep
Elliottsville	128	4	0.6	2.0	0.60	2.0	В	Friable till, silty, schist & phyllite	frigid	loamy	yes	20 to 40 in. deep
Glebe	671	4	2.0	6.0	2.00	6.0	С	Loose till, bedrock	cryic	loamy	yes	20 to 40 in. deep
Glover	NA	4	0.6	2.0	0.60	2	D	Friable till, silty, schist & phyllite	frigid	loamy	no	less than 20 in. deep
Hogback	91	4	2.0	6.0	2.00	6.0	С	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Hollis	86	4	0.6	6.0	0.60	6.0	C/D	Loose till, bedrock	mesic	loamy	no	less than 20 in. deep
Kearsarge	359	4	0.6	2.0	0.60	2.0	В	Friable till, silty, schist & phyllite	mesic	loamy	no	less than 20 in. deep
Lyman	92	4	2.0	6.0	2.00	6.0	A/D	Loose till, bedrock	frigid	loamy	yes	less than 20 in. deep
Macomber	252	4	0.6	2.0	0.60	2.0	C	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Millsite	251	4	0.6	6.0	0.60	6.0	С	Loose till, bedrock	frigid	loamy	no	20 to 40 in. deep
Monson	133	4	0.6	2.0	0.60	2.0	D	Friable till, silty, schist & phyllite	frigid	loamy	yes	less than 20 in. deep
Pennichuck	460	4	0.6	2.0	0.60	2.0	В	Friable till, silty, schist & phyllite	mesic	loamy-skeletal	no	20 to 40 in. deep
Rawsonville	98	4	0.6	6.0	0.60	6.0	C	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Ricker	674	4	2.0	6.0	2.00	6.0	A	rganic over bedrock (up to 4" of minera	cryic	fibric to hemic	no	well drained, less than 20 in. deep
Saddleback	673	4	0.6	2.0	0.60	2.0	C/D	Loose till, bedrock	cryic	loamy	yes	less than 20 in. deep
Shapleigh	136	4					C/D	Sandy Till	mesic	sandy	yes	less than 20 in. deep
Thorndike	84	4	0.6	2.0	0.60	2.0	C/D	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	less than 20 in. deep
Tunbridge	99	4	0.6	6.0	0.60	6.0	C	Loose till, bedrock	frigid	loamy	yes	20 to 40 in. deep
Winnecook	88	4	0.6	2.0	0.60	2.0	C	Friable till, silty, schist & phyllite	frigid	loamy-skeletal	yes	20 to 40 in. deep
Woodstock	93	4	2.0	6.0	2.00	6.0	C/D	Loose till, bedrock	frigid	loamy	no	less than 20 in. deep
Au Gres	516	5					В	Outwash and Stream Terraces	frigid	sandy	yes	single grain, loose
Bemis	224	5	0.6	0.2	0.00	0.2	C	Firm, platy, loamy till	cryic	loamy	no	·····g··· g······, ·····
Binghamville	534	5	0.2	2.0	0.06	0.2	D	Terraces and glacial lake plains	mesic	silty	no	
Brayton	240	5	0.6	2.0	0.06	0.6	С	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Cabot	589	5	0.6	2.0	0.06	0.2	D	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Charles	209	5	0.6	100.0	0.60	100.0	С	Flood Plain (Bottom Land)	frigid	silty	no	
Cohas	505	5	0.6	2.0	0.60	100.0	С	Flood Plain (Bottom Land)	frigid	co. loamy over sandy (skeletal)	no	
Grange	433	5	0.6	2.0	0.60	2.0	С	Outwash and Stream Terraces	frigid	co. loamy over sandy (skeletal)	no	
Kinsman	614	5	6.0	20.0	6.00	20.0	С	Outwash and Stream Terraces	frigid	sandy	yes	
Leicester	514	5	0.6	6.0	0.60	20.0	С	Loose till, loamy textures	mesic	loamy	no	
Lim	3	5	0.6	2.0	6.00	20.0	С	Flood Plain (Bottom Land)	mesic	loamy	no	
Limerick	109	5	0.6	2.0	0.60	2.0	С	Flood Plain (Bottom Land)	mesic	silty	no	
Lyme	246	5	0.6	6.0	0.60	6.0	С	Loose till, sandy textures	frigid	loamy	no	
Mashpee	315	5	6.0	20.0	6.00	20.0	В	Outwash and Stream Terraces	mesic	sandy	yes	
Monarda	569	5	0.2	2.0	0.02	0.2	D	Firm, platy, silty till, schist & phyllite	frigid	loamy	no	
Moosilauke	414	5	6.0	20.0	6.00	20.0	С	Loose till, sandy textures	frigid	sandy	no	
Naumburg	214	5	6.0	20.0	6.00	20.0	С	Outwash and Stream Terraces	frigid	sandy	yes	
Pemi	633	5	0.6	2.0	0.06	0.6	С	Terraces and glacial lake plains	frigid	silty	no	
Pillsbury	646	5	0.6	2.0	0.06	0.2	С	Firm, platy, loamy till	frigid	silty	no	
Pipestone	314	5					В	Outwash and Stream Terraces	mesic	sandy	yes	
Raynham	533	5	0.2	2.0	0.06	0.2	С	Terraces and glacial lake plains	mesic	silty	no	
Raypol	540	5	0.6	2.0	6.00	100.0	D	Outwash and Stream Terraces	mesic	co. loamy over sandy (skeletal)	no	
Ridgebury	656	5	0.6	6.0	0.00	0.2	С	Firm, platy, loamy till	mesic	loamy	no	
Rippowam	5	5	0.6	6.0	6.00	20.0	С	Flood Plain (Bottom Land)	mesic	loamy	no	
Roundabout	333	5	0.2	2.0	0.06	0.6	С	Terraces and glacial lake plains	frigid	silty	no	silt loam in the C
Rumney	105	5	0.6	6.0	6.00	20.0	С	Flood Plain (Bottom Land)	frigid	loamy	no	

Sorted by DES Soil Group for Establishing Lot Size K_{sat} B and C horizons SSSNNE pub no. 5

Soil Series	number	NHDES	Ksat low - B	Ksat high - B	Ksat low - C	Ksat high - C	Hyd.	Land Form	Temp.	Soil Textures	Spodosol	Other
		Soil Group	in/hr	in/hr	in/hr	in/hr	Grp.				?	
Saugatuck	16	5	0.06	0.2	6.00	20.0	С	Outwash and Stream Terraces	mesic	sandy	yes	ortstein
Scantic	233	5	0.0	0.2	0.00	0.2	D	Silt and Clay Deposits	frigid	fine	no	
Scitico	33	5	0.0	0.2	0.00	0.2	С	Silt and Clay Deposits	mesic	fine	no	
Shaker	439	5	2.0	6.0	0.00	0.2	С	Sandy/loamy over silt/clay	mesic	co. loamy over clayey	no	
Squamscott	538	5	6.0	20.0	0.06	0.6	С	Sandy/loamy over silt/clay	mesic	sandy over loamy	yes	
Stissing	340	5	0.6	2.0	0.06	0.2	С	Firm, platy, silty till, schist & phyllite	mesic	loamy	no	
Swanton	438	5	2.0	6.0	0.00	0.2	С	Sandy/loamy over silt/clay	frigid	co. loamy over clayey	no	
Walpole	546	5	2.0	6.0	6.00	20.0	С	Outwash and Stream Terraces	mesic	sandy	no	
Wareham	34	5	6.0	20.0	6.00	20.0	С	Outwash and Stream Terraces	mesic	sandy	no	
										· · · · ·		
Biddeford	234	6	0.0	0.2	0.00	0.2	D	Silt and Clay Deposits	frigid	fine	no	organic over clay
Bucksport	895	6					D	Organic Materials - Freshwater	frigid	sapric	no	deep organic
Burnham	131	6	0.2	6.0	0.02	0.2	D	Firm, platy, silty till, schist & phylitte	frigid	loamy	no	organic over silt
Catden	296	6					A/D	Organic Materials - Freshwater	mesic	sapric	no	deep organic
Chocorua	395	6			6.00	20.0	D	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Greenwood	295	6					A/D	Organic Materials - Freshwater	frigid	hemic	no	deep organic
Ipswich	397	6					D	Tidal Flat	mesic	hemic/sapric	no	deep organic
Matunuck	797	6			20.00	100.0	D	Tidal Flat	mesic	sandy	no	organic over sand
Maybid	134	6	0.0	0.2	0.00	0.2	D	Silt and Clay Deposits	mesic	fine	no	silt over clay
Meadowsedge	894	6					D	Organic Materials - Freshwater	frigid	peat	no	deep organic
Medomak	406	6	0.6	2.0	0.60	2.0	D	Flood Plain (Bottom Land)	frigid	silty	no	organic over silt
Natchaug	496	6			0.20	2.0	D	Organic Materials - Freshwater	mesic	loamy	no	organic over loam
Ossipee	495	6			0.20	2.0	D	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
Pawcatuck	497	6			20.00	100.0	D	Tidal Flat	mesic	sandy or sandy-skeletal	no	organic over sand
Peacham	549	6	0.6	2.0	0.00	0.2	D	Firm, platy, silty till, schist & phylitte	frigid	loamy	no	organic over loam
Pondicherry	992	6			6.00	20.0	D	Organic Materials - Freshwater	frigid	sandy or sandy-skeletal	no	organic over sand
Saco	6	6	0.6	2.0	6.00	20.0	D	Flood Plain (Bottom Land)	mesic	silty	no	strata
Scarboro	115	6	6.0	20.0	6.00	20.0	D	Outwash and Stream Terraces	mesic	sandy	no	organic over sand, non stony
Searsport	15	6	6.0	20.0	6.00	20.0	D	Outwash and Stream Terraces	frigid	sandy	no	organic over sand
Timakwa	393	6			6.00	100.0	D	Organic Materials - Freshwater	mesic	sandy or sandy-skeletal	no	organic over sand
Vassalboro	150	6					D	Organic Materials - Freshwater	frigid	peat	no	deep organic
Waskish	195	6					D	Organic Materials - Freshwater	frigid	peat	no	deep organic
Westbrook	597	6			0.00	2.0	D	Tidal Flat	mesic	loamy	no	organic over loam
Whitman	49	6	0.0	0.2	0.00	0.2	D	Firm, platy, loamy till	mesic	loamy	no	mucky loam
Wonsqueak	995	6			0.20	2.0	D	Organic Materials - Freshwater	frigid	loamy	no	organic over loam
		ÿ			0.20					iouity		siguine erer iedin

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Section 1:

filtrexx[®] LAND IMPROVEMENT SYSTE

Erosion & Sediment Control – Construction Activities

SWPPP Cut Sheet:

Filtrexx® Inlet Protection

Sediment & Perimeter Control Technology

PURPOSE & DESCRIPTION

Filtrexx[®] Inlet protection is a three-dimensional tubular sediment control and storm water runoff filtration device typically used for storm drain inlet protection of sediment and soluble pollutants (such as phosphorus and petroleum hydrocarbons) on and around construction activities.

APPLICATION

Drain inlets are located in areas that receive runoff from surrounding lands, often exposed and disturbed soils, and are located at a low point, or in a sump. Inlet protection used around drain inlets (or Drain Inlet protection) should completely enclose the circumference of the drain and where possible should not be placed on a grade or slope. Inlet protection used around drain inlets should never be the only form of site sediment control and should be accompanied by erosion control/slope stabilization practices, such as Slope protection or rolled erosion control blankets (RECB). Inlet protection should never be placed where they divert runoff flow from the drain inlet, or on top of the inlet, which can cause flooding. Under high runoff and sediment loading conditions placement of 1-2 in (25-50 mm) diameter rock (AASHTO #2) may be placed around the outer circumference of the Inlet protection up to 1/2 the height of the Inlet protection. This will slow runoff velocity as it contacts the Inlet protection and will reduce sediment build-up and clogging of the Inlet protection.

Curb inlets are generally located on paved surfaces and are designed to rapidly drain storm runoff from roadways to prevent flooding that poses a hazard to vehicular traffic. Inlet protection devices should be placed in a manner which intercepts runoff prior to entering the inlet, but does not block or divert runoff from the inlet. To prevent diversion of runoff, Inlet protection used around curbs (or Curb

Inlet protection) should be used in low points, or sumps, and minor slopes or grades. Inlet protection should never be placed in or on the curb inlet drain, or placed in a manner than obstructs vehicular traffic. Inlet protection height should be at least 1 in (25 mm) lower than top of curb inlet to allow for overflow into the drain and not over the curb. Maximum sediment removal efficiency occurs when minor ponding exists behind Inlet protection but should never lead to flooding.

Curb sediment containment systems are used to reduce the sediment and pollutant load flowing to a curb inlet. They are generally placed on paved surfaces perpendicular to runoff flow and should be lower than the height of the curb. Curb sediment containment systems should never cause flooding or placed where they are a hazard to vehicular traffic. Inlet protection used for curb sediment containment (or Curb Sediment Containment Inlet protection) can be placed on a grade but should never be placed directly upslope from curb inlet where it may inadvertently divert runoff from entering curb inlet.

INSTALLATION

- 1. Inlet protection used for inlet protection to reduce sediment and soluble pollutants entering storm drains shall meet Filtrexx® FilterSoxxTM Material Specifications and use Certified Filtrexx[®] FilterMedia[™].
- 2. Contractor is required to be a Filtrexx[®] Certified[™] Installer as determined by Filtrexx[®] International, LLC (440-926-2607 or visit web site at Filtrexx.com). Certification shall be considered current if appropriate identification is shown during time of bid or at time of application (current list of installers can be found at www.filtrexx.com). Look for the Filtrexx® Certified[™] Installer Seal.

- **3.** Filtrexx[®] Inlet protection shall be placed at locations indicated on plans as directed by the Engineer. Inlet protection should be installed in a pattern that allows complete protection of the inlet area.
- 4. Installation of curb Inlet protection will ensure a minimal overlap of at least 1 ft (300mm) on either side of the opening being protected. The Inlet protection will be anchored to the soil behind the curb using staples, stakes or other devices capable of holding the Inlet protection in place.
- 5. Standard Inlet protection for curb inlet protection and curb sediment containment will use 8 in (200mm) diameter Inlet protection, and drain inlets on soil will use 12 in (300mm) or 18 in (450mm) diameter Inlet protection. In severe flow situations, larger Inlet protection may be specified by the Engineer. During curb installation, Inlet protection shall be compacted to be slightly shorter than curb height.
- 6. If Inlet protection becomes clogged with debris and sediment, they shall be maintained so as to assure proper drainage and water flow into the storm drain. In severe storm events, overflow of the Inlet protection may be acceptable in order to keep the area from flooding.
- 7. Curb and drain Inlet protection shall be positioned so as to provide a permeable physical barrier to the drain itself, allowing sediment to collect on the outside of the Inlet protection.
- 8. For drains and inlets that have only curb cuts, without street grates, a spacer is required in order to keep the Inlet protection away from the drain opening. This spacer should be a hog wire screen bent to overlap the grate opening and keep the sock from falling into the opening. Use at least one spacer for every 4 ft (1.2m) of curb drain opening. The wire grid also prevents other floatable waste from passing over the Inlet protection.
- 9. Stakes shall be installed through the middle of the drain Inlet protection on 5 ft (1.5m) centers, using 2 in (50mm) x 2 in (50mm) x 3 ft (1m) wood stakes.
- **10.** Staking depth for sand and silt loam soils shall be 12 in (300mm), and 8 in (200mm) for clay soils.

INSPECTION AND MAINTENANCE

Routine inspection should be conducted within 24 hrs of a runoff event or as designated by the regulating authority. Inlet protection should be regularly inspected to make sure they maintain their

shape and are producing adequate hydraulic flowthrough. If ponding becomes excessive, additional Inlet protection may be required or sediment removal may be necessary. Inlet protection shall be inspected until contributing drainage area has been permanently stabilized and construction activity has ceased

- 1. The Contractor shall maintain the Inlet protection in a functional condition at all times and it shall be routinely inspected.
- 2. If the Inlet protection has been damaged, it shall be repaired, or replaced if beyond repair.
- 3. The Contractor shall remove sediment at the base of the upslope side of the Inlet protection when accumulation has reached 1/2 of the effective height of the Inlet protection, or as directed by the Engineer. Alternatively, for drain Inlet protection a new Soxx™ may be placed on top of the original increasing the sediment storage capacity without soil disturbance.
- **4.** Inlet protection shall be maintained until disturbed area above or around the device has been permanently stabilized and construction activity has ceased.
- 5. Regular maintenance includes lifting the Inlet protection and cleaning around and under them as sediment collects.
- 6. The FilterMedia[™] will be removed from paved areas or dispersed on site soil or behind curb once disturbed area has been permanently stabilized, construction activity has ceased, or as determined by the Engineer.

Grade (%)	Spacing (ft)	Spacing (mm)
0.5	100	30
1.0	50	15
2.0	25	8
3.0	16	5
4.0	13	4
5.0	10	3

 Table 2.4 Spacing for Curb Sediment

 Containment Systems.

Source: Fifield, 2001.

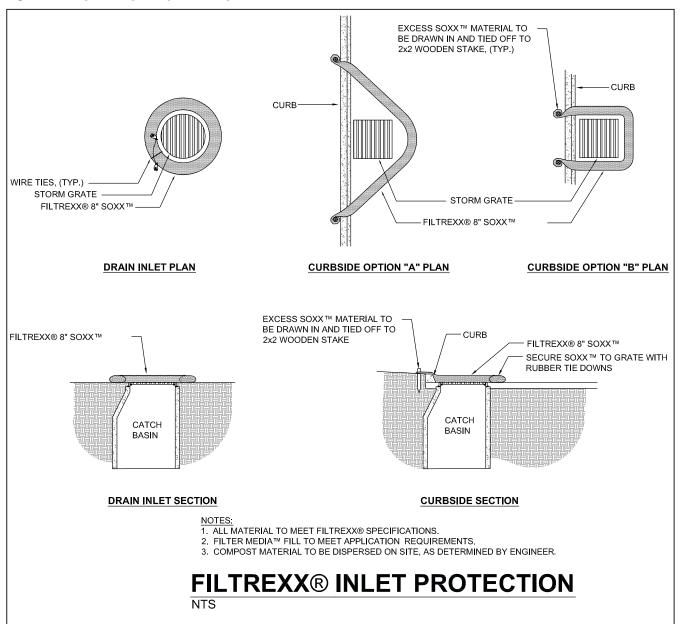
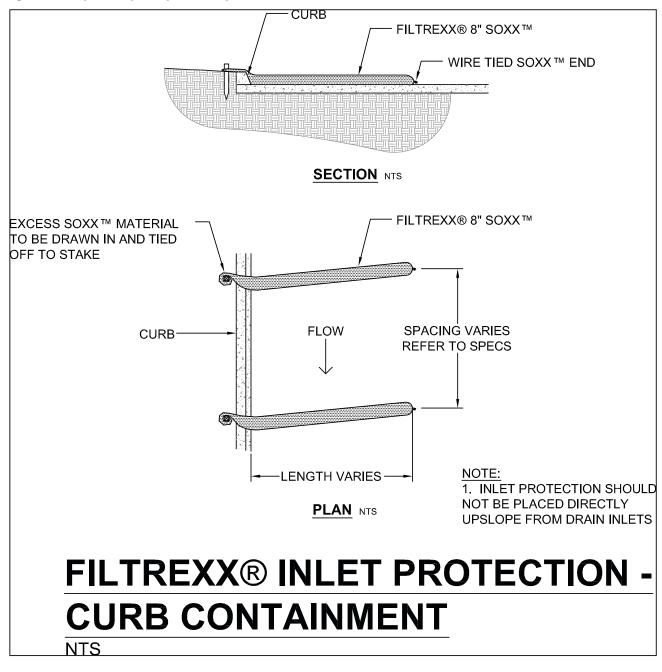


Figure 2.1. Engineering Design Drawing for Curb and Drain Inlet Protection





Section 1:

filtrexx[®] LAND IMPROVEMENT SYSTEMS

Erosion & Sediment Control – Construction Activities

SWPPP Cut Sheet: Filtrexx[®] Sediment Control

Sediment & Perimeter Control Technology

PURPOSE & DESCRIPTION

Filtrexx[®] Sediment control is a three-dimensional tubular sediment control and storm water runoff filtration device typically used for perimeter control of sediment and other soluble pollutants (such as phosphorus and petroleum hydrocarbons), on and around construction activities.

APPLICATION

Filtrexx® Sediment control is to be installed down slope of any disturbed area requiring erosion and sediment control and filtration of soluble pollutants from runoff. Sediment control is effective when installed perpendicular to sheet or low concentrated flow. Acceptable applications include:

- Site perimeters
- Above and below disturbed areas subject to sheet • runoff, interrill and rill erosion
- Above and below exposed and erodable slopes
- Around area drains or inlets located in a 'sump'
- On compacted soils where trenching of silt fence is difficult or impossible
- Around sensitive trees where trenching of silt fence is not beneficial for tree survival or may unnecessarily disturb established vegetation.
- On frozen ground where trenching of silt fence is impossible.
- On paved surfaces where trenching of silt fence is impossible.

INSTALLATION

- 1. Sediment control used for perimeter control of sediment and soluble pollutants in storm runoff shall meet Filtrexx[®] Soxx[™] Material Specifications and use Certified Filtrexx® FilterMediaTM.
- 2. Contractor is required to be Filtrexx[®] Certified[™], or use pre-filled Filtrexx® Sediment control

products manufactured by a Filtrexx® Certified Manufacturer[™] as determined by Filtrexx[®] International, LLC (440-926-2607 or visit www.filtrexx.com). Certification shall be considered current if appropriate identification is shown during time of bid or at time of application. Look for the Filtrexx[®] Certified[™] Seal.

- 3. Sediment control will be placed at locations indicated on plans as directed by the Engineer.
- 4. Sediment control should be installed parallel to the base of the slope or other disturbed area. In extreme conditions (i.e., 2:1 slopes), a second Sediment control shall be constructed at the top of the slope.
- 5. Effective Soxx[™] height in the field should be as follows: 8" Diameter Sediment control = 6.5" high, 12" Diameter Sediment control = 9.5" high, 18" Diameter SiltSoxx[™] = 14.5" high, 24" Diameter Sediment control = 19" high.
- 6. Stakes shall be installed through the middle of the Sediment control on 10 ft (3m) centers, using 2 in (50mm) by 2 in (50mm) by 3 ft (1m) hard wood stakes. In the event staking is not possible, i.e., when Sediment control is used on pavement, heavy concrete blocks shall be used behind the Sediment control to help stabilize during rainfall/runoff events.
- 7. Staking depth for sand and silt loam soils shall be 12 in (300mm), and 8 in (200mm) for clay soils.
- 8. Loose compost may be backfilled along the upslope side of the Sediment control, filling the seam between the soil surface and the device, improving filtration and sediment retention.
- 9. If the Sediment control is to be left as a permanent filter or part of the natural landscape, it may be seeded at time of installation for

establishment of permanent vegetation. The Engineer will specify seed requirements.

 Filtrexx[®] Sediment control is not to be used in perennial, ephemeral, or intermittent streams.

See design drawing schematic for correct Filtrexx[®] Sediment control installation (Figure 1.1).

INSPECTION AND MAINTENANCE

Routine inspection should be conducted within 24 hrs of a runoff event or as designated by the regulating authority. Sediment control should be regularly inspected to make sure they maintain their shape and are producing adequate hydraulic flow-through. If ponding becomes excessive, additional Sediment control may be required to reduce effective slope length or sediment removal may be necessary. Sediment control shall be inspected until area above has been permanently stabilized and construction activity has ceased

- 1. The Contractor shall maintain the Sediment control in a functional condition at all times and it shall be routinely inspected.
- 2. If the Sediment control has been damaged, it shall be repaired, or replaced if beyond repair.

- **3.** The Contractor shall remove sediment at the base of the upslope side of the Sediment control when accumulation has reached 1/2 of the effective height of the Sediment control, or as directed by the Engineer. Alternatively, a new Sediment control can be placed on top of and slightly behind the original one creating more sediment storage capacity without soil disturbance.
- 4. Sediment control shall be maintained until disturbed area above the device has been permanently stabilized and construction activity has ceased.
- The FilterMedia[™] will be dispersed on site once disturbed area has been permanently stabilized, construction activity has ceased, or as determined by the Engineer.
- **6.** For long-term sediment and pollution control applications, Sediment control can be seeded at the time of installation to create a vegetative filtering system for prolonged and increased filtration of sediment and soluble pollutants (contained vegetative filter strip). The appropriate seed mix shall be determined by the Engineer.

	Maximum Slope Length Above Sediment Control in Feet (meters)*									
Slope Percent	8 in (200 mm) Sediment control	12 in (300 mm) Sediment control	18 in (450 mm) Sediment control	24 in (600mm) Sediment control	32 in (800mm) Sediment control 26 in (650 mm) **					
	6.5 in (160 mm)**	9.5 in (240 mm) **	14.5 in (360 mm) **	19 in (480 mm) **						
2 (or less)	600 (180)	750 (225)	1000 (300)	1300 (400)	1650 (500)					
5	400 (120)	500 (150)	550 (165)	650 (200)	750 (225)					
10	200 (60)	250 (75)	300 (90)	400 (120)	500 (150)					
15	140 (40)	170 (50)	200 (60)	325 (100)	450 (140)					
20	100 (30)	125 (38)	140 (42)	260 (80)	400 (120)					
25	80 (24)	100 (30)	110 (33)	200 (60)	275 (85)					
30	60 (18)	75 (23)	90 (27)	130 (40)	200 (60)					
35	60 (18)	75 (23)	80 (24)	115 (35)	150 (45)					
40	60 (18)	75 (23)	80 (24)	100 (30)	125 (38)					
45	40 (12)	50 (15)	60 (18)	80 (24)	100 (30)					
50	40 (12)	50 (15)	55 (17)	65 (20)	75 (23)					

* Based on a failure point of 36 in (0.9 m) super silt fence (wire reinforced) at 1000 ft (303 m) of slope, watershed width equivalent to receiving length of sediment control device, 1 in/ 24 hr (25 mm/24 hr) rain event.

** Effective height of Sediment control after installation and with constant head from runoff as determined by Ohio State University.

