STORMWATER MANAGEMENT & SEDIMENT AND EROSION CONTROL PLAN

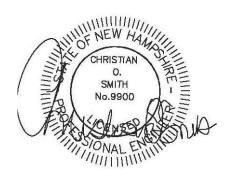
Prepared for:

THIBODEAU FAMILY REV. TRUST
76 YOUNG RD., BARRINGTON, NH 03885
(603) 767-3552
ROUTE 9 MIXED USE DEVELOPMENT

Prepared by:

BEALS ASSOCIATES, PLLC 70 PORTSMOUTH AVENUE STRATHAM, NH 03885

Project Number:
NH-1387
NH Route 9
Barrington, New Hampshire
August 24, 2022



DESIGN METHOD OBJECTIVES

The Thibodeau Family Rev. Trust proposes a 6-unit mixed use development on approximately 3.42-acres of land located off NH Route 9 in Barrington, NH. A drainage analysis of the proposed development was conducted for the purpose of estimating the peak rate of stormwater run-off and to subsequently design adequate drainage structures. Two models were compiled, one for the area in its existing (pre-construction) condition, and a second for its proposed (post-construction) condition. The analysis was conducted using data for the 2, 10, 25 and 50Yr – 24 Hr storm events using the USDA SCS TR-20 method within the HydroCAD Stormwater Modeling System environment. The purpose of this analysis is to estimate the peak rates of run-off from the site for swale adequacy purposes, and to compare the peak rate of run-off between the existing and proposed conditions. Rainfall data utilized is as provided in the Extreme Precipitation tables by Cornell University. Infiltration rates, for the ponds that are design to recharge stormwater, have been taken from the Ksat values provided in the Society of Soil Scientists of Northern New England SSSNNE Special Publication No. 5 September, 2009. The published values were then divided by 2 as a factor of safety as required by the NH Stormwater Manual.

<u>ANALYSIS</u>	<u>COMP</u>	COMPONENT PEAK RATE of DISCHARGE (CFS)						
		2 YR	10		25 \		50	YR
		Proposed	Existing	Proposed	Existing	Proposed	Existing	Proposed
Reach #100	0.52	0.52	2.01	1.75	3.28	2.83	4.08	3.67
Reach #200	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01
Reach #300	0.00	0.00	0.00	0.00	0.01	0.01	0.02	0.08

STORMWATER VOLUME COMPARISON (2-YR STORM IN AF) Existing Proposed

	Existing	rropose
Reach #100	0.148	0.134
Reach #200	0.00	0.00
Reach #300	0.00	0.00

The existing property is located on a parcel consisting of forest, a woods road, wetlands and an open area utilized as a landing for historic logging activity. The existing topography is such that the site analysis is divided into three subcatchments. The reaches flow offsite; westerly (to an abutting driveway culvert parallel to Route 9), Northerly to the abutting parcel and south easterly to an abutting parcel. Directions as can be seen on the existing conditions watershed plan (Sheet W1).

The proposed 6-unit mixed use development includes a private access drive that intersects NH Route 9 in a single location. This drive provides the required frontage for the residential and 4-proposed commercial lots. The proposed layout will divide the parcel into sixteen different subcatchments. The peak rate of run-off from the proposed development is equal to or decreased (though the 50-YR storm shows a 0.06 cfs increase for Reach 3, this is negligible) from that of the existing conditions under all design storms evaluated. The addition of swales, culverts, infiltration/filtration areas direct the treated run off to recharge it back into the ground water matrix. Driveway and parking area runoff receive treatment through filtration (bioretention) ponds prior to infiltration to the groundwater matrix. Some paved area and the unit roofs are infiltrated through stone drip edges and is also infiltrated. The potential for increased erosion and sedimentation is handled by way of stone check dams, siltfence and/or erosion control berms. The use of Best Management Practices per the New Hampshire Stormwater Manual has been applied to the design of these structures and will be observed during all stages of construction.

All land disturbed during construction will be permanently stabilized within 60 days of groundbreaking, and existing wetlands and abutters will suffer no adversity resulting from this development.

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Appendix I - Existing Conditions Analysis

Summary 2 YR - 24 HR rainfall = 3.08"

Complete 10 YR - 24 HR rainfall = 4.64"

Summary 25 YR - 24 HR rainfall = 5.86"

Summary 50 YR - 24 HR rainfall = 7.00"

Sheet W-1 Existing Conditions Watershed Plan

Appendix II - Proposed Conditions Analysis

Summary 2 YR - 24 HR rainfall = 3.08"

Complete 10 YR - 24 HR rainfall = 4.64"

Summary 25 YR - 24 HR rainfall = 5.86"

Summary 50 YR - 24 HR rainfall = 7.00"

Sheet W-2 Proposed Conditions Watershed Plan

Appendix III - Charts, Graphs, and Calculations

1.0 RAINFALL CHARACTERISTICS

This stormwater management plan includes an existing conditions analysis of the area involved in the proposed development, as well as proposed conditions, or post-construction analysis of the same location. These analyses were accomplished using the USDA SCS TR-20 Method within the HydroCAD Stormwater Modeling System. The curve numbers were developed using the SCS TR-55 Runoff Curve numbers for Urban Areas. A Type III SCS 24-hour rainfall distribution was utilized in analyzing the data for the 2, 10, 25 and 50Yr – 24 Hr storm events using the USDA SCS TR-20 method within the HydroCAD Stormwater Modeling System environment.

ANALYSIS COMPONENT PEAK I	RATE of	DISCHARGE (CFS)
---------------------------	---------	-------------	------

						- ~ /		
	2 YR		10 YR		25 YR		50 YR	
	Existing	Proposed	Existing	Proposed	Existing	Proposed	Existing	Proposed
Reach #100	0.52	0.52	2.01	1.75	3.28	2.83	4.08	3.67
Reach #200	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01
Reach #300	0.00	0.00	0.00	0.00	0.01	0.01	0.02	0.08

STORMWATER VOLUME COMPARISON (2-YR STORM IN AF)

	Existing	Proposed
Reach #100	0.148	0.134
Reach #200	0.00	0.00
Reach #300	0.00	0.00

2.0 EXISTING CONDITIONS

Reference:

Sheet W-1, Existing Conditions Watershed Plan (Enclosed)

Existing Conditions Plans

The existing property is located on a parcel consisting of forest, a woods road, wetlands and an open area utilized as a landing for historic logging activity. The existing topography is such that the site analysis is divided into three subcatchments. The reaches flow offsite; westerly (to an abutting driveway culvert parallel to Route 9), Northerly to the abutting parcel and south easterly to an abutting parcel. Directions as can be seen on the existing conditions watershed plan (Sheet W1). Classified by Site Specific Soil Mapping, the land within the drainage analysis is composed of slopes ranging from 3% to 25%, and soils categorized into the Hydrologic Soil Groups (HSG) A, B, and C. No flood hazard zone exists on the parcel.

3.0 PROPOSED CONDITIONS

Reference:

W-Sheets Proposed Conditions Watershed Plan (Enclosed)

C Sheets Proposed Conditions Plans

The addition of the impervious area from the 22' wide paved drive, parking areas and the 6 proposed units cause an increase in the curve number (Cn) and a decrease in the time of concentration (Tc), the net result being a potential increase in peak rates of run-off from the site. The proposed development divides the site into sixteen different post-construction subcatchments. Impervious

area take-offs were calculated digitally from the proposed road pavement and roof areas. Seasonal high-water tables for the treatment ponds and infiltration areas were modeled based on actual test pits logged on the parcel. The run-off is treated and infiltrated modeled as HydroCAD "reaches" and "ponds". These consist of constructed swales, existing flow paths through larger subcatchments, driveway culverts, infiltration/filtration basins and level spreaders. Required groundwater recharge will be exceeded by a single 1" storm (947 c.f. required and 3,920 c.f. provided by a 1-year storm).

In an effort to prevent the sedimentation of adjacent wetlands or abutting property, the driveway is equipped with roadside swales that terminate into culverts, or directly into sediment forebays or the mentioned BMP treatment ponds. Ksat values were utilized based on published data in SSSNNE Special Publication No. 5 (copy in appendices). The peak rate of run-off from the proposed development is equal to or decreased (though the 50-YR storm shows a 0.06 cfs increase for Reach 3, this is negligible) from that of the existing conditions under all design storms evaluated. The addition of swales, culverts, infiltration/filtration areas direct the treated run off to recharge it back into the ground water matrix. Driveway and parking area runoff receive treatment through filtration (bioretention) ponds prior to infiltration to the groundwater matrix. Some paved area and the unit roofs are infiltrated through stone drip edges and is also infiltrated. The potential for increased erosion and sedimentation is handled by way of stone check dams, silt fence and/or erosion control berms. The use of Best Management Practices per the New Hampshire Stormwater Manual has been applied to the design of these structures and will be observed during all stages of construction. All land disturbed during construction will be permanently stabilized within 60 days of groundbreaking, and existing wetlands and abutters will suffer no adversity resulting from this development. During construction, appropriate temporary and/or permanent BMP's will be applied so as to negate the potential for sediment-laden run-off to discharge into wetlands prior to the final stabilization of

the proposed grading. The structures outlined in this proposal provide for compliant treatment of

4.0 SEDIMENT & EROSION CONTROL PLANS BEST MANAGEMENT PRACTICES (BMP's)

stormwater run-off and for sediment control.

Reference:

C Sheets Proposed Conditions Plan

E Sheet Erosion & Sediment Control Details

The proposed site development is protected from erosion and the driveways and abutting properties are protected from sediment by the use of Best Management Practices as outlined in the New Hampshire Stormwater Manual. Any area disturbed by construction will be permanently restabilized within 60 days and abutting properties and wetlands will not be adversely affected by this development. All swales and drainage structures will be constructed and stabilized prior to having run-off directed to them.

4.1 Silt Fence / Erosion Control Berm and Construction Fence

The plan set demonstrates the location of silt fence or Erosion Control Berm for sediment control. In areas where the limits of construction need to be emphasized to operators, construction fence for added visibility will be installed. Sheet E-1, Erosion and Sediment Control Details, has the

specifications for installation and maintenance of the silt fence. Orange construction fence will be VISI Perimeter Fence by Conwed Plastic Fencing, or equal. The four-foot fencing to be installed using six-foot posts at least two feet in the ground with spacing of six to eight feet.

4.2 Drainage Swales / Stormwater Conveyance Channels

Drainage swales will be stabilized with vegetation for long term cover as outlined below, and on Sheet E-1 using seed mixture C. As a general rule, velocities in the swale should not exceed 3.0 feet per second for a vegetated swale although velocities as high as 4.5 FPS are allowed under certain soil conditions.

4.3 Vegetated Stabilization

All areas that are disturbed during construction will be stabilized with vegetated material within 60 days of breaking ground. Construction will be managed in such a manner that erosion is prevented and that no abutter's property will be subjected to any siltation, unless otherwise permitted. All areas to be planted with grass for long-term cover will follow the specification and on Sheet E-1 using seeding mixture C, as follows:

Mixture	Pounds	Pounds per
	per Acre	1,000 Sq. Ft.
Tall Fescue	20	0.45
Creeping Red Fescue	28	0.65
Total	48	1.10

4.4 Stabilized Construction Entrance

A temporary gravel construction entrance provides an area where mud can be dislodged from tires before the vehicle leaves the construction site to reduce the amount of mud and sediment transported onto paved municipal and state roads. The stone size for the pad should be between 1 and 2-inch coarse aggregate, and the pad itself constructed to a minimum length of 50' for the full width of the access road. The aggregate should be placed at least six inches thick. A plan view and profile are shown on Sheet E1 - Sediment and Erosion Control Detail Plan.

4.5 Level Spreaders

As mentioned above, the proposed site plan includes stone level spreaders above a vegetated buffer. Level spreaders must be more than six feet in width per the "New Hampshire Stormwater Maual." Stone Level spreaders enable any run-off directed towards them to be spread evenly into sheet flow prior to discharge into wetlands or treatment by a vegetated buffer, thus allowing for better buffer efficiency and a lesser potential for erosion.

4.6 Vegetated Buffers

Vegetated buffers are areas of land with natural or planted vegetation designed to receive sheet runoff from upgradient development. These natural areas, preferably wooded, are effective in removing sediment and sediment-laden pollutants from such run-off, although their effectiveness is severely diminished when forced to deal with concentrated flow and must therefore be equipped with a levelspreading device. Vegetated buffers should not have a slope exceeding fifteen percent and have a minimum length of seventy-five feet.

4.7 Environmental Dust Control

Dust will be controlled on the site by the use of multiple Best Management Practices. Mulching and temporary seeding will be the first line of protection to be utilized where problems occur. If dust problems are not solved by these applications, the use of water and calcium chloride can be applied. Calcium chloride will be applied at a rate that will keep the surface moist but not cause pollution.

4.8 Construction Sequence

- 1. Construct and/or install temporary and permanent sediment erosion and detention control facilities (silt fence/erosion control berm, vegetated swales, level spreaders, and constructed Vegetated buffers), as required. Erosion, sediment and facilities shall be installed and stabilized prior to any earth moving operation, and prior to directing run-off to them.
- 2. Clear, cut, grub, and dispose of debris in approved facilities.
- 3. Excavate and stockpile topsoil / loam. All disturbed areas shall be stabilized immediately after grading.
- 4. Construct the driveway and its associated drainage structures.
- 5. Begin permanent and temporary seeding and mulching. All cut and fill slopes and disturbed areas shall be seeded and mulched as required, or directed.
- 6. Daily, or as required, construct temporary berms, drainage ditches, sediment traps, etc. to prevent erosion on the site and prevent any siltation of abutting waters or property.
- 7. Inspect and maintain all erosion and sediment control measures during construction every two weeks and after every storm event with 0.5" or more rain.
- 9. Complete permanent seeding and landscaping.
- 9. Remove temporary erosion control measures after seeding areas have established themselves and site improvements are complete. Smooth and re-vegetate all disturbed areas.

- 10. All swales and all drainage ponds and structures will be constructed and fully stabilized prior to having run-off being directed to them.
- 11. Finish graveling all driveways/parking.

4.9 Temporary Erosion Control Measures

- 1. The smallest practical area of open soil shall be exposed at any one time.
- 2. Erosion, sediment control measures shall be installed as shown on the plans and at locations as required, or directed by the engineer.
- 3. All disturbed areas shall be returned to original grades and elevations. Disturbed areas shall be loamed with a minimum of 4" of loam and seeded with not less than 1.10 pound of seed per 1,000 square feet (48 pounds per acre) of area.
- 4. Silt fences and other barriers shall be inspected periodically and after every rainstorm during the life of the project. All damaged areas shall be repaired; sediment deposits shall periodically be removed and properly disposed of.
- 5. After all disturbed areas have been stabilized, the temporary erosion control measures are to be removed and the area disturbed by the removal smoothed and revegetated.
- 6. Areas must be seeded and mulched within 5 days of final grading, permanently stabilized within 15 days of final grading, or temporarily stabilized within 30 days of initial disturbance of soil.

4.10 Inspection and Maintenance Schedule

Fencing/Erosion Control Berm will be inspected during and after storm events to ensure that the fence still has integrity and is not allowing sediment to pass. Sediment build-up in swales and level spreaders will be removed if it is deeper than six inches.

5.0 CONCLUSION

This proposed development off of NH Route 9 in Barrington, NH will have no adverse effect on abutting property owners by way of storm water run-off or siltation. The post-construction peak rate of run-off for the site has been decreased from that of the existing conditions for the analyzed design storms and driveway run-off will treatment by either constructed or natural methods. Appropriate steps will be taken to eliminate erosion and sedimentation; these will be accomplished through the construction of a drainage system consisting of swales, driveway culverts, bioretention areas, and infiltration trenches/drip edges. The Best Management Practices developed by the State of New Hampshire have been utilized in the design of this system and these applications will be enforced throughout the construction process.

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A Terrain Alteration Permit (RSA 485: A-17) is not required for this project due to the area of disturbance being less than 100,000 square feet.

Respectfully Submitted,

BEALS ASSOCIATES, PLLC.

Christian O. Smith

Christian O. Smith, PE Principal

APPENDIX I

Existing Conditions Drainage Analysis

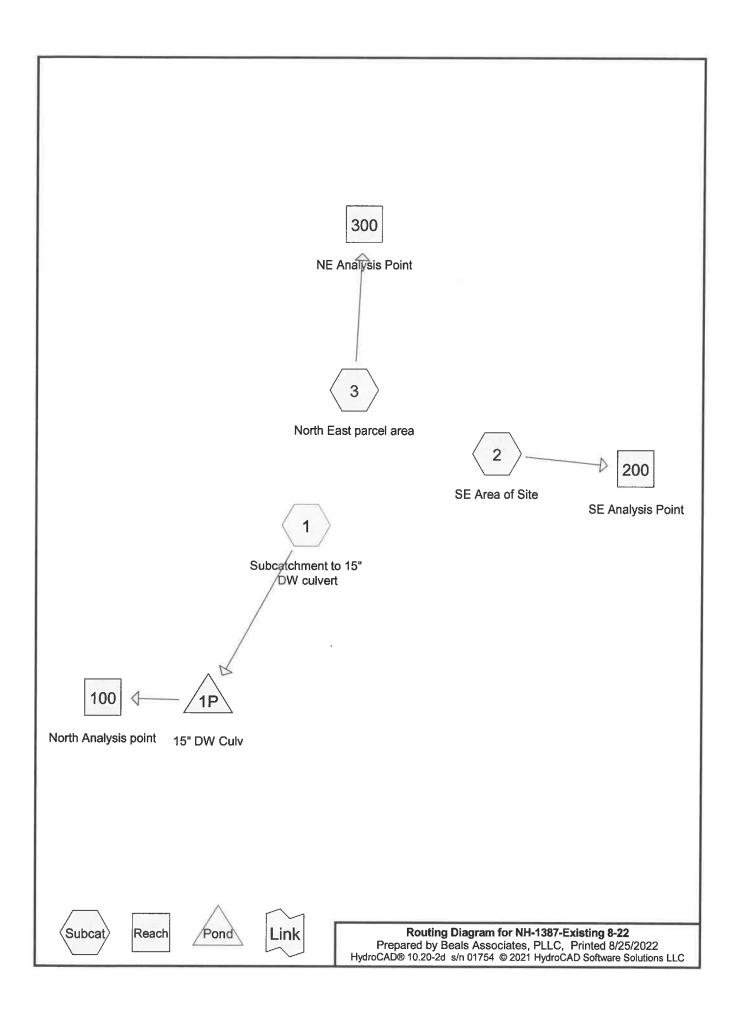
Summary 2 YR - 24 HR rainfall = 3.08"

Complete 10 YR - 24 HR rainfall = 4.64"

Summary 25 YR - 24 HR rainfall = 5.86"

Summary 50 YR - 24 HR rainfall = 7.00"

Sheet W-1 Existing Conditions Watershed Plan



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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.108	74	>75% Grass cover, Good, HSG C (1)
0.385	89	Paved roads w/open ditches, 50% imp, HSG B (1)
0.103	92	Paved roads w/open ditches, 50% imp, HSG C (1)
0.019	98	Roofs, HSG C (1)
0.082	98	Water Surface, HSG C (1)
1.830	30	Woods, Good, HSG A (1, 2, 3)
1.245	55	Woods, Good, HSG B (1)
1.478	70	Woods, Good, HSG C (1)
5.250	55	TOTAL AREA

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
1.830	HSG A	1, 2, 3
1.630	HSG B	1
1.790	HSG C	1
0.000	HSG D	
0.000	Other	
5.250		TOTAL AREA

Route 9 Barrington
Type III 24-hr 2 YR Rainfall=3.08"
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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1: Subcatchment to 15" DW Runoff Area=179,510 sf 8.38% Impervious Runoff Depth=0.43" Flow Length=594' Tc=20.6 min CN=62 Runoff=0.92 cfs 0.148 af

Subcatchment 2: SE Area of Site

Runoff Area=14,767 sf 0.00% Impervious Runoff Depth=0.00"

Flow Length=146' Tc=7.7 min CN=30 Runoff=0.00 cfs 0.000 af

Subcatchment 3: North East parcel area

Runoff Area=34,414 sf 0.00% Impervious Runoff Depth=0.00"
Flow Length=147' Tc=8.4 min CN=30 Runoff=0.00 cfs 0.000 af

Reach 100: North Analysis point Inflow=0.52 cfs 0.148 af
Outflow=0.52 cfs 0.148 af

Reach 200: SE Analysis Point Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af

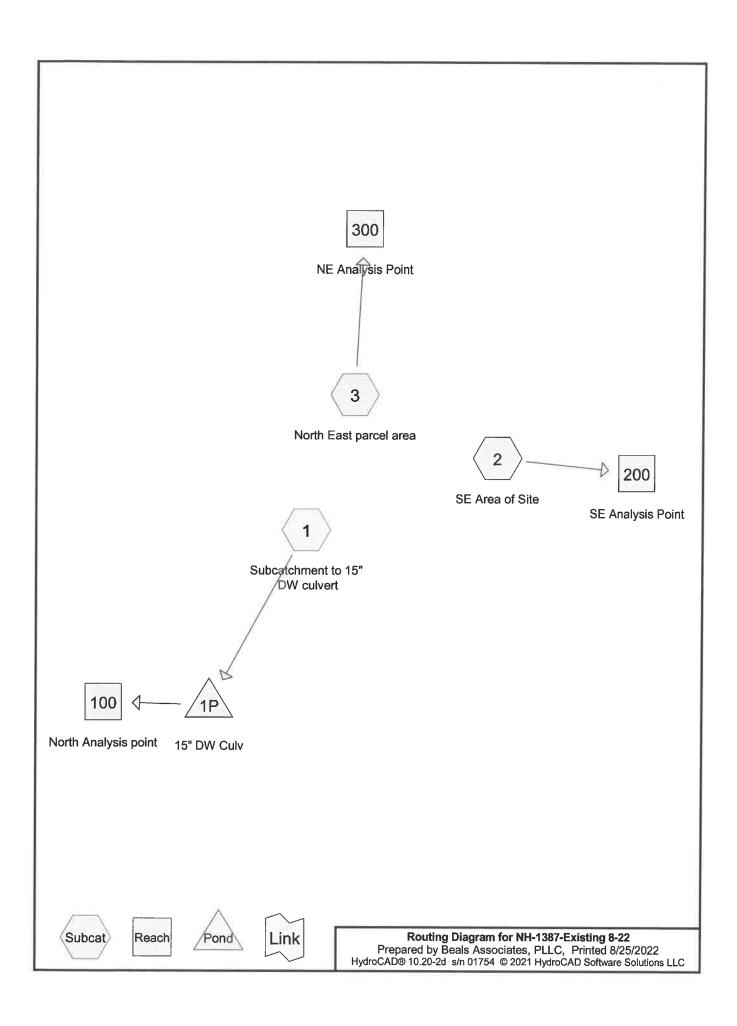
Reach 300: NE Analysis Point Inflow=0.00 cfs 0.000 af

Outflow=0.00 cfs 0.000 af

Pond 1P: 15" DW Culv

Peak Elev=204.50' Storage=1,051 cf Inflow=0.92 cfs 0.148 af
15.0" Round Culvert n=0.025 L=21.0' S=0.0052 '/' Outflow=0.52 cfs 0.148 af

Total Runoff Area = 5.250 ac Runoff Volume = 0.148 af Average Runoff Depth = 0.34" 93.42% Pervious = 4.905 ac 6.58% Impervious = 0.345 ac



Route 9 Barrington
Type III 24-hr 10 YR Rainfall=4.64"
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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1: Subcatchment to 15" DW Runoff Area=179,510 sf 8.38% Impervious Runoff Depth=1.22" Flow Length=594' Tc=20.6 min CN=62 Runoff=3.51 cfs 0.419 af

Subcatchment 2: SE Area of Site

Runoff Area=14,767 sf 0.00% Impervious Runoff Depth=0.00"

Flow Length=146' Tc=7.7 min CN=30 Runoff=0.00 cfs 0.000 af

Subcatchment 3: North East parcel area

Runoff Area=34,414 sf 0.00% Impervious Runoff Depth=0.00"

Flow Length=147' Tc=8.4 min CN=30 Runoff=0.00 cfs 0.000 af

Reach 100: North Analysis point Inflow=2.01 cfs 0.419 af
Outflow=2.01 cfs 0.419 af

Reach 200: SE Analysis Point Inflow=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af

Reach 300: NE Analysis Point Inflow=0.00 cfs 0.000 af

Outflow=0.00 cfs 0.000 af

Pond 1P: 15" DW Culv

Peak Elev=205.02' Storage=3,608 cf Inflow=3.51 cfs 0.419 af
15.0" Round Culvert n=0.025 L=21.0' S=0.0052'/' Outflow=2.01 cfs 0.419 af

Total Runoff Area = 5.250 ac Runoff Volume = 0.419 af Average Runoff Depth = 0.96" 93.42% Pervious = 4.905 ac 6.58% Impervious = 0.345 ac

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Summary for Subcatchment 1: Subcatchment to 15" DW culvert

Runoff =

3.51 cfs @ 12.32 hrs, Volume=

0.419 af, Depth= 1,22"

Routed to Pond 1P: 15" DW Culv

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

A	rea (sf)	CN I	Description		
	30,531	30	Woods, Go	od, HSG A	
	54,239			od, HSG B	
	64,384	70 \	Woods, Go	od, HSG C	
	4,691	74	>75% Gras	s cover, Go	ood, HSG C
	841	98 I	Roofs, HSG	G C	
	3,582	98 \	Nater Surfa	ace, HSG (
	16,759				litches, 50% imp, HSG B
	4,483	92 F	Paved road	s w/open d	itches, 50% imp, HSG C
1	79,510	62 Weighted Average			
1	64,466	91.62% Pervious Area			
	15,044	8	3.38% Impe	ervious Are	a
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.7	50	0.0400	0.09		Sheet Flow, Sheet
					Woods: Light underbrush n= 0.400 P2= 3.00"
7.5	327	0.0210	0.72		Shallow Concentrated Flow, Sheet to RS swale
					Woodland Kv= 5.0 fps
3.4	217	0.0050	1.06		Shallow Concentrated Flow, SC to Culv
					Grassed Waterway Kv= 15.0 fps
20.6	594	Total			

Summary for Subcatchment 2: SE Area of Site

Runoff

0.00 cfs @

1.00 hrs, Volume=

0.000 af, Depth= 0.00"

Routed to Reach 200 : SE Analysis Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

Area	(sf)	CN	Description	
14,	767	30	Woods, Good, HSG A	
14,	767		100.00% Pervious Area	

Route 9 Barrington
Type III 24-hr 10 YR Rainfall=4.64"
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- 12	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	6.7	50	0.1000	0.12		Sheet Flow, Sheet
	1.0	96	0.0938	1.53		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Sc to analysis point Woodland Kv= 5.0 fps
	7.7	146	Total			•

Summary for Subcatchment 3: North East parcel area

Runoff = 0.00 cfs @ 1.00 hrs, Volume=

0.000 af, Depth= 0.00"

Routed to Reach 300: NE Analysis Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

	A	rea (sf)	CN [Description		
		34,414	30 V	Voods, Go	od, HSG A	
34,414 100.00% Pervious Area						a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	7.4	50	0.0790	0.11		Sheet Flow, Sheet
	1.0	97	0.1130	1.68		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, SC to analysis point Woodland Kv= 5.0 fps
	8.4	147	Total			

Summary for Reach 100: North Analysis point

Inflow Area = 4.121 ac, 8.38% Impervious, Inflow Depth = 1.22" for 10 YR event

Inflow = 2.01 cfs @ 12.66 hrs, Volume= 0.419 af

Outflow = 2.01 cfs @ 12.66 hrs, Volume= 0.419 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Summary for Reach 200: SE Analysis Point

Inflow Area = 0.339 ac, 0.00% Impervious, Inflow Depth = 0.00" for 10 YR event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Route 9 Barrington
Type III 24-hr 10 YR Rainfall=4.64"
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Summary for Reach 300: NE Analysis Point

Inflow Area = 0.790 ac, 0.00% Impervious, Inflow Depth = 0.00" for 10 YR event

Inflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Summary for Pond 1P: 15" DW Culv

Inflow Area = 4.121 ac, 8.38% Impervious, Inflow Depth = 1.22" for 10 YR event

Inflow = 3.51 cfs @ 12.32 hrs, Volume= 0.419 af

Outflow = 2.01 cfs @ 12.66 hrs, Volume= 0.419 af, Atten= 43%, Lag= 20.2 min

Primary = 2.01 cfs @ 12.66 hrs, Volume= 0.419 af

Routed to Reach 100: North Analysis point

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 205.02' @ 12.66 hrs Surf.Area= 6,360 sf Storage= 3,608 cf

Plug-Flow detention time= 36.7 min calculated for 0.419 af (100% of inflow)

Center-of-Mass det. time= 36.4 min (924.6 - 888.2)

volume	invert	Avail.Storage	Storage Description	
#1	204.00'	12,535 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Flevation	Surf A	roa Inc	o Storo Cum Storo	

Cum.Store (cubic-feet)	Inc.Store (cubic-feet)	Surf.Area (sq-ft)	Elevation (feet)	
0	0	738	204.00	
12,535	12,535	11,797	206.00	

Device Routing Invert Outlet Devices

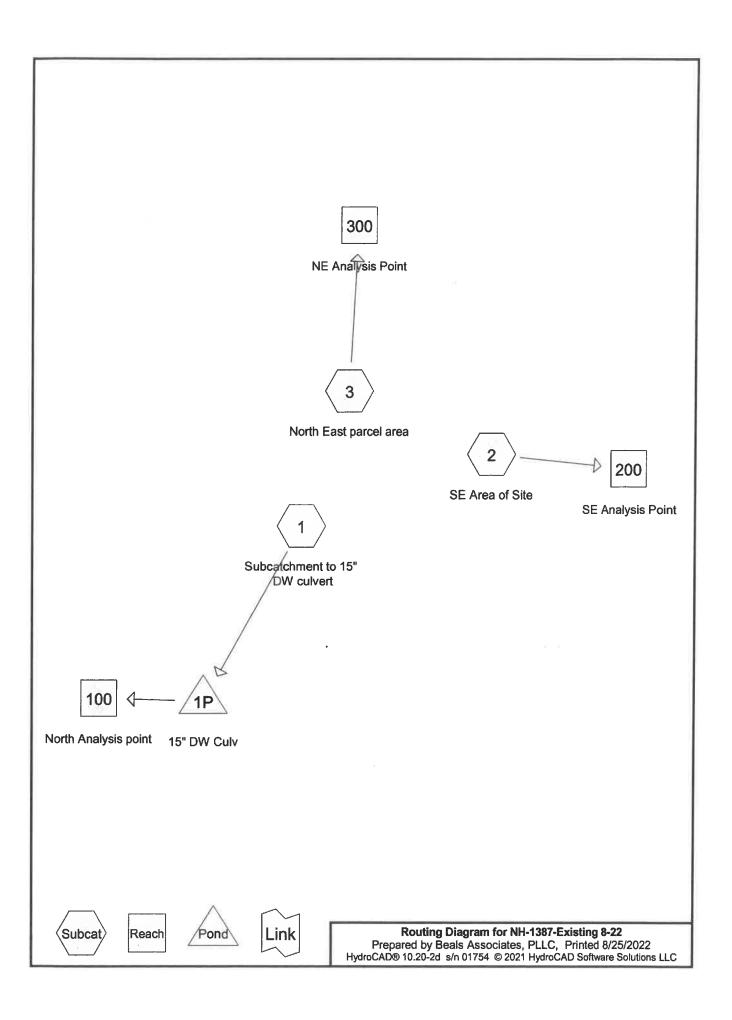
#1 Primary 204.01' 15.0" Round Culvert

L= 21.0' CMP, projecting, no headwall, Ke= 0.900

Inlet / Outlet Invert= 204.01' / 203.90' S= 0.0052 '/' Cc= 0.900

n= 0.025 Corrugated metal, Flow Area= 1.23 sf

Primary OutFlow Max=2.01 cfs @ 12.66 hrs HW=205.02' (Free Discharge)
1=Culvert (Barrel Controls 2.01 cfs @ 2.59 fps)



Route 9 Barrington
Type III 24-hr 25 YR Rainfall=5.86"
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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1: Subcatchment to 15" DW Runoff Area=179,510 sf 8.38% Impervious Runoff Depth=2.00" Flow Length=594' Tc=20.6 min CN=62 Runoff=6.10 cfs 0.685 af

Subcatchment 2: SE Area of Site

Runoff Area=14,767 sf 0.00% Impervious Runoff Depth=0.06"

Flow Length=146' Tc=7.7 min CN=30 Runoff=0.00 cfs 0.002 af

Subcatchment 3: North East parcel area

Runoff Area=34,414 sf 0.00% Impervious Runoff Depth=0.06"
Flow Length=147' Tc=8.4 min CN=30 Runoff=0.01 cfs 0.004 af

Reach 100: North Analysis point Inflow=3.28 cfs 0.685 af Outflow=3.28 cfs 0.685 af

Reach 200: SE Analysis Point Inflow=0.00 cfs 0.002 af Outflow=0.00 cfs 0.002 af

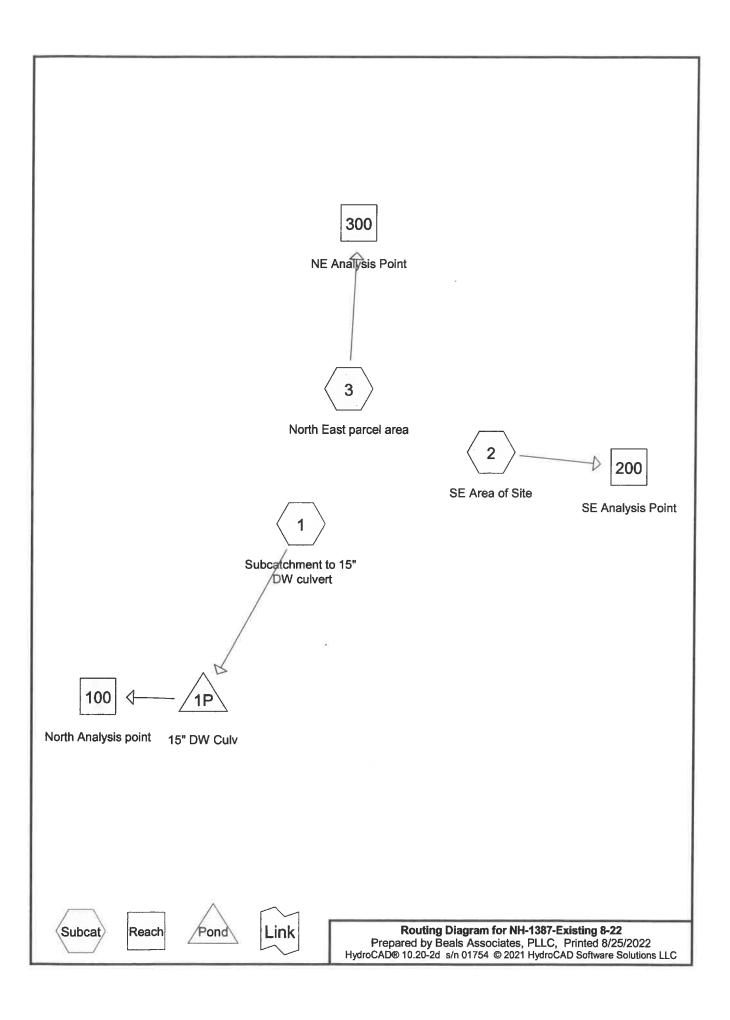
Reach 300: NE Analysis Point Inflow=0.01 cfs 0.004 af

Outflow=0.01 cfs 0.004 af

Pond 1P: 15" DW Culv

Peak Elev=205.40' Storage=6,479 cf Inflow=6.10 cfs 0.685 af 15.0" Round Culvert n=0.025 L=21.0' S=0.0052 '/' Outflow=3.28 cfs 0.685 af

Total Runoff Area = 5.250 ac Runoff Volume = 0.691 af Average Runoff Depth = 1.58" 93.42% Pervious = 4.905 ac 6.58% Impervious = 0.345 ac



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Route 9 Barrington
Type III 24-hr 50 YR Rainfall=7.00"

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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1: Subcatchment to 15" DW Runoff Area=179,510 sf 8.38% Impervious Runoff Depth=2.80" Flow Length=594' Tc=20.6 min CN=62 Runoff=8.80 cfs 0.962 af

Subcatchment 2: SE Area of Site

Runoff Area=14,767 sf 0.00% Impervious Runoff Depth=0.21"

Flow Length=146' Tc=7.7 min CN=30 Runoff=0.01 cfs 0.006 af

Subcatchment 3: North East parcel area

Runoff Area=34,414 sf 0.00% Impervious Runoff Depth=0.21"

Flow Length=147' Tc=8.4 min CN=30 Runoff=0.02 cfs 0.014 af

Reach 100: North Analysis point Inflow=4.08 cfs 0.962 af

Outflow=4.08 cfs 0.962 af

Reach 200: SE Analysis Point Inflow=0.01 cfs 0.006 af Outflow=0.01 cfs 0.006 af

Reach 300: NE Analysis Point Inflow=0.02 cfs 0.014 af

NE Analysis Point Inflow=0.02 cfs 0.014 af
Outflow=0.02 cfs 0.014 af

Pond 1P: 15" DW Culv

Peak Elev=205.79' Storage=10,148 cf Inflow=8.80 cfs 0.962 af
15.0" Round Culvert n=0.025 L=21.0' S=0.0052'/' Outflow=4.08 cfs 0.962 af

Total Runoff Area = 5.250 ac Runoff Volume = 0.982 af Average Runoff Depth = 2.24" 93.42% Pervious = 4.905 ac 6.58% Impervious = 0.345 ac

APPENDIX II

Proposed Conditions Drainage Analysis

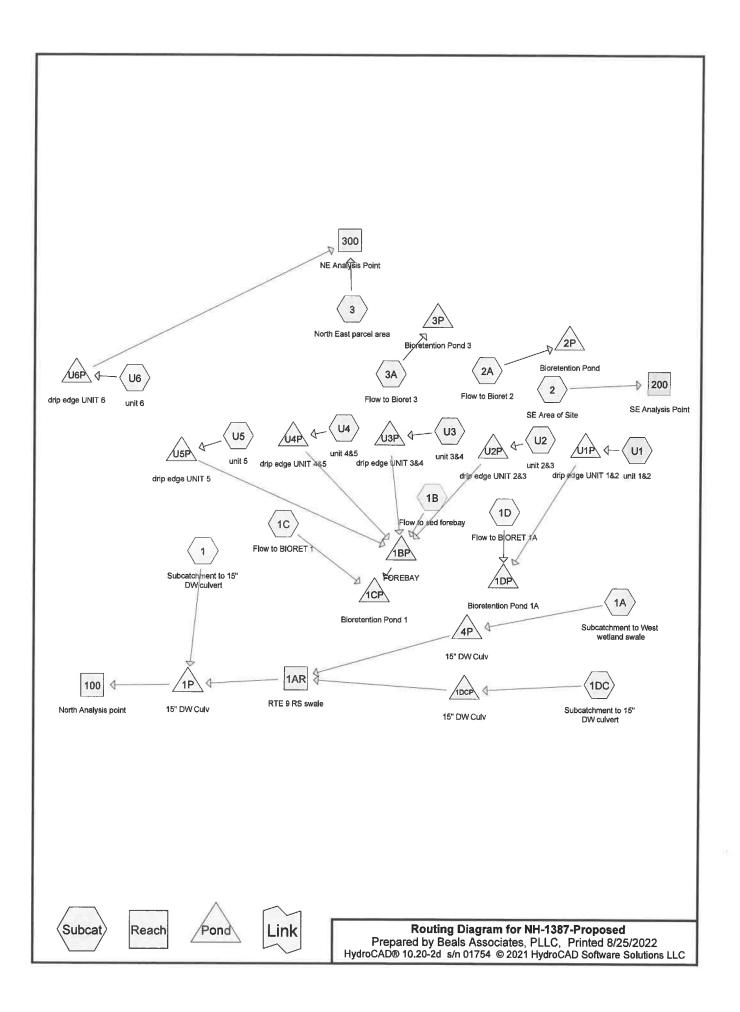
Summary 2 YR - 24 HR rainfall = 3.08"

Complete 10 YR - 24 HR rainfall = 4.64"

Summary 25 YR - 24 HR rainfall = 5.86"

Summary 50 YR - 24 HR rainfall = 7.00"

Sheet W-2 Proposed Conditions Watershed Plan



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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.555	0.555 39 >75% Grass cover, Good, HSG A (1, 1A, 1B, 1C, 1D, 2, 2A, 3, 3A, U1, U2, U3	
		U5, U6)
0.286	61	>75% Grass cover, Good, HSG B (1, 1A, 1B, 1C, 1D, 1DC)
0.290	74	>75% Grass cover, Good, HSG C (1A, 1C)
0.371	98	Paved parking, HSG A (1B, 1C, 1D, 2A, 3, 3A, U1, U2, U3, U4, U5, U6)
0.253	98	Paved parking, HSG B (1, 1B, 1C, 1D, 1DC)
0.086	98	Paved parking, HSG C (1, 1C, 1DC)
0.354	89	Paved roads w/open ditches, 50% imp, HSG B (1)
0.103	92	Paved roads w/open ditches, 50% imp, HSG C (1)
0.167	98	Roofs, HSG A (1, 1D, 2A, 3, U1, U2, U3, U4, U5, U6)
0.039	98	Roofs, HSG C (1)
0.082	98	Water Surface, HSG C (1A)
0.737	30	Woods, Good, HSG A (1, 1A, 1D, 2, 3)
0.736	55	Woods, Good, HSG B (1A, 1DC)
1.190	70	Woods, Good, HSG C (1, 1A)
5.250	63	TOTAL AREA

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2011	Listing	(aii	noaes)	}

Area	Soil	Subcatchment
(acres)	Group	Numbers
1.830	HSG A	1, 1A, 1B, 1C, 1D, 2, 2A, 3, 3A, U1, U2, U3, U4, U5, U6
1.630	HSG B	1, 1A, 1B, 1C, 1D, 1DC
1.790	HSG C	1, 1A, 1C, 1DC
0.000	HSG D	
0.000	Other	
5.250		TOTAL AREA

Subcatchment U6: unit 6

Runoff Area=5,298 sf 56.06% Impervious Runoff Depth=0.86"

Tc=6.0 min CN=72 Runoff=0.11 cfs 0.009 af

Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind+

Reach routing by Stor-Ind+T	rans method - Pond routing by Stor-Ind method
Subcatchment1: Subcatchmentto 15" D	DW Runoff Area=88,850 sf 10.46% Impervious Runoff Depth=0.47" Flow Length=594' Tc=20.6 min CN=68 Runoff=0.52 cfs 0.079 af
Subcatchment 1A: Subcatchment to Wes	st Runoff Area=36,139 sf 9.91% Impervious Runoff Depth=0.33" Flow Length=197' Tc=17.4 min CN=64 Runoff=0.12 cfs 0.023 af
Subcatchment1B: Flow to sed forebay	Runoff Area=10,220 sf 52.58% Impervious Runoff Depth=1.25" Tc=6.0 min CN=79 Runoff=0.34 cfs 0.024 af
Subcatchment 1C: Flow to BIORET 1	Runoff Area=12,144 sf 59.56% Impervious Runoff Depth=1.51" Tc=6.0 min CN=83 Runoff=0.49 cfs 0.035 af
Subcatchment1D: Flow to BIORET1A	Runoff Area=7,611 sf 58.88% Impervious Runoff Depth=1.38" Tc=6.0 min CN=81 Runoff=0.28 cfs 0.020 af
Subcatchment 1DC: Subcatchment to 15	Runoff Area=13,411 sf 49.44% Impervious Runoff Depth=1.25" Flow Length=248' Tc=15.2 min CN=86 Runoff=0.33 cfs 0.032 af
Subcatchment 2: SE Area of Site	Runoff Area=5,549 sf 0.00% Impervious Runoff Depth=0.00" Tc=6.0 min CN=33 Runoff=0.00 cfs 0.000 af
Subcatchment 2A: Flow to Bioret 2	Runoff Area=4,389 sf 29.64% Impervious Runoff Depth=0.24" Tc=6.0 min CN=56 Runoff=0.01 cfs 0.002 af
Subcatchment 3: North East parcel area	Runoff Area=24,131 sf 2.91% Impervious Runoff Depth=0.00" Flow Length=107' Tc=8.0 min CN=34 Runoff=0.00 cfs 0.000 af
Subcatchment 3A: Flow to Bioret 3	Runoff Area=3,667 sf 33.08% Impervious Runoff Depth=0.33" Tc=6.0 min CN=59 Runoff=0.02 cfs 0.002 af
Subcatchment U1: unit 1&2	Runoff Area=4,634 sf 71.41% Impervious Runoff Depth=1.38" Tc=6.0 min CN=81 Runoff=0.17 cfs 0.012 af
Subcatchment U2: unit 2&3	Runoff Area=3,165 sf 44.71% Impervious Runoff Depth=0.54" Tc=6.0 min CN=65 Runoff=0.03 cfs 0.003 af
Subcatchment U3: unit 3&4	Runoff Area=3,633 sf 54.78% Impervious Runoff Depth=0.81" Tc=6.0 min CN=71 Runoff=0.07 cfs 0.006 af
Subcatchment U4: unit 4&5	Runoff Area=4,112 sf 70.67% Impervious Runoff Depth=1.38" Tc=6.0 min CN=81 Runoff=0.15 cfs 0.011 af
Subcatchment U5: unit 5	Runoff Area=1,722 sf 60.34% Impervious Runoff Depth=1.01" Tc=6.0 min CN=75 Runoff=0.04 cfs 0.003 af
A	

NH-1387-Proposed Type III 24-hr 2 YR Rainfall=3.08"

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Reach 1AR: RTE 9 RS swale Avg. Flow Depth=0.35' Max Vel=1.04 fps Inflow=0.41 cfs 0.055 af n=0.022 L=304.0' S=0.0025 '/' Capacity=39.39 cfs Outflow=0.39 cfs 0.055 af

Reach 100: North Analysis point Inflow=0.52 cfs 0.134 af

Outflow=0.52 cfs 0.134 af

Reach 200: SE Analysis Point Inflow=0.00 cfs 0.000 af

Outflow=0.00 cfs 0.000 af

Reach 300: NE Analysis Point Inflow=0.00 cfs 0.000 af

Outflow=0.00 cfs 0.000 af

Pond 1BP: FOREBAY Peak Elev=209.70' Storage=15 cf Inflow=0.34 cfs 0.024 af

15.0" Round Culvert n=0.013 L=42.0' S=0.0100'/' Outflow=0.33 cfs 0.024 af

Pond 1CP: Bioretention Pond 1 Peak Elev=206.39' Storage=418 cf Inflow=0.82 cfs 0.059 af

Outflow=0.27 cfs 0.059 af

Pond 1DCP: 15" DW Culv Peak Elev=205.32' Storage=7 cf Inflow=0.33 cfs 0.032 af

15.0" Round Culvert n=0.013 L=43.0' S=0.0051'/' Outflow=0.33 cfs 0.032 af

Pond 1DP: Bioretention Pond 1A Peak Elev=205.51' Storage=102 cf Inflow=0.28 cfs 0.020 af

Outflow=0.12 cfs 0.020 af

Pond 1P: 15" DW Culv Peak Elev=204.50' Storage=1,054 cf Inflow=0.90 cfs 0.134 af

15.0" Round Culvert n=0.025 L=21.0' S=0.0052'/' Outflow=0.52 cfs 0.134 af

Pond 2P: Bioretention Pond Peak Elev=218.01' Storage=2 cf Inflow=0.01 cfs 0.002 af

Outflow=0.01 cfs 0.002 af

Pond 3P: Bioretention Pond 3 Peak Elev=223.01' Storage=2 cf Inflow=0.02 cfs 0.002 af

Outflow=0.01 cfs 0.002 af

Pond 4P: 15" DW Culv Peak Elev=205.19' Storage=3 cf Inflow=0.12 cfs 0.023 af

15.0" Round Culvert n=0.013 L=43.0' S=0.0051 '/' Outflow=0.12 cfs 0.023 af

Pond U1P: drip edge UNIT 1&2 Peak Elev=120.33' Storage=98 cf Inflow=0.17 cfs 0.012 af

Discarded=0.05 cfs 0.012 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.012 af

Pond U2P: drip edge UNIT 2&3

Peak Elev=120.52' Storage=4 cf Inflow=0.03 cfs 0.003 af

Discarded=0.03 cfs 0.003 af Primary=0.00 cfs 0.000 af Outflow=0.03 cfs 0.003 af

Pond U3P: drip edge UNIT 3&4 Peak Elev=120.12' Storage=24 cf Inflow=0.07 cfs 0.006 af

Discarded=0.03 cfs 0.006 af Primary=0.00 cfs 0.000 af Outflow=0.03 cfs 0.006 af

Pond U4P: drip edge UNIT 4&5 Peak Elev=120.37' Storage=91 cf Inflow=0.15 cfs 0.011 af

Discarded=0.04 cfs 0.011 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.011 af

Pond U5P: drip edge UNIT 5 Peak Elev=120.13' Storage=15 cf Inflow=0.04 cfs 0.003 af

Discarded=0.02 cfs 0.003 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.003 af

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Route 9 Barrington
Type III 24-hr 2 YR Rainfall=3.08"
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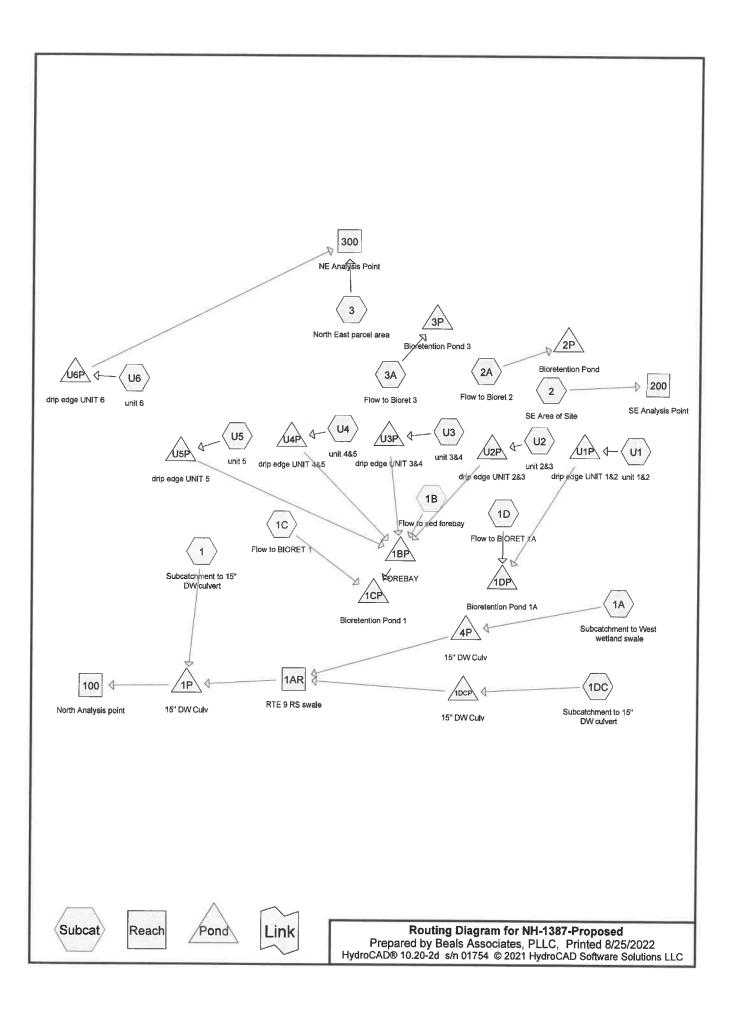
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Pond U6P: drip edge UNIT 6

6 Peak Elev=121.63' Storage=39 cf Inflow=0.11 cfs 0.009 af Discarded=0.05 cfs 0.009 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.009 af

Total Runoff Area = 5.250 ac Runoff Volume = 0.262 af Average Runoff Depth = 0.60" 76.63% Pervious = 4.023 ac 23.37% Impervious = 1.227 ac



Route 9 Barrington
Type III 24-hr 10 YR Rainfall=4.64"
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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1: Subcatchmentto 15" DW Runoff Area=88,850 sf 10.46% Impervious Runoff Depth=1,29" Flow Length=594' Tc=20.6 min CN=68 Runoff=1.85 cfs 0.219 af Subcatchment 1A: Subcatchment to West Runoff Area=36,139 sf 9.91% Impervious Runoff Depth=1.04" Flow Length=197' Tc=17.4 min CN=64 Runoff=0.60 cfs 0.072 af Subcatchment 1B: Flow to sed forebay Runoff Area=10,220 sf 52.58% Impervious Runoff Depth=2.49" Tc=6.0 min CN=79 Runoff=0.69 cfs 0.049 af Subcatchment 1C: Flow to BIORET 1 Runoff Area=12,144 sf 59.56% Impervious Runoff Depth=2.85" Tc=6.0 min CN=83 Runoff=0.93 cfs 0.066 af Subcatchment 1D: Flow to BIORET 1A Runoff Area=7,611 sf 58.88% Impervious Runoff Depth=2.67" Tc=6.0 min CN=81 Runoff=0.55 cfs 0.039 af Subcatchment1DC: Subcatchmentto 15" Runoff Area=13,411 sf 49.44% Impervious Runoff Depth=2.49" Flow Length=248' Tc=15.2 min CN=86 Runoff=0.68 cfs 0.064 af Subcatchment 2: SE Area of Site Runoff Area=5,549 sf 0.00% Impervious Runoff Depth=0.02" Tc=6.0 min CN=33 Runoff=0.00 cfs 0.000 af Subcatchment 2A: Flow to Bioret 2 Runoff Area=4,389 sf 29.64% Impervious Runoff Depth=0.86" Tc=6.0 min CN=56 Runoff=0.08 cfs 0.007 af Runoff Area=24,131 sf 2.91% Impervious Runoff Depth=0.03" Subcatchment 3: North East parcel area Flow Length=107' Tc=8.0 min CN=34 Runoff=0.00 cfs 0.001 af Subcatchment 3A: Flow to Bioret 3 Runoff Area=3,667 sf 33.08% Impervious Runoff Depth=1.04" Tc=6.0 min CN=59 Runoff=0.09 cfs 0.007 af Subcatchment U1: unit 1&2 Runoff Area=4,634 sf 71.41% Impervious Runoff Depth=2.67" Tc=6.0 min CN=81 Runoff=0.33 cfs 0.024 af

Subcatchment U2: unit 2&3

Runoff Area=3,165 sf 44.71% Impervious Runoff Depth=1.42"

Tc=6.0 min CN=65 Runoff=0.11 cfs 0.009 af

Subcatchment U3: unit 3&4 Runoff Area=3,633 sf 54.78% Impervious Runoff Depth=1.85"
Tc=6.0 min CN=71 Runoff=0.18 cfs 0.013 af

Subcatchment U4: unit 4&5 Runoff Area=4,112 sf 70.67% Impervious Runoff Depth=2.67"

Tc=6.0 min CN=81 Runoff=0.30 cfs 0.021 af

Subcatchment U5: unit 5 Runoff Area=1,722 sf 60.34% Impervious Runoff Depth=2.16"
Tc=6.0 min CN=75 Runoff=0.10 cfs 0.007 af

Subcatchment U6: unit 6 Runoff Area=5,298 sf 56.06% Impervious Runoff Depth=1.92"

Tc=6.0 min CN=72 Runoff=0.27 cfs 0.020 af

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Reach 1AR: RTE 9 RS swale Avg. Flow Depth=0.54' Max Vel=1.37 fps Inflow=1.25 cfs 0.136 af

n=0.022 L=304.0' S=0.0025 '/' Capacity=39.39 cfs Outflow=1.20 cfs 0.136 af

Reach 100: North Analysis point Inflow=1.75 cfs 0.354 af

Outflow=1.75 cfs 0.354 af

Reach 200: SE Analysis Point Inflow=0.00 cfs 0.000 af

Outflow=0.00 cfs 0.000 af

Reach 300: NE Analysis Point Inflow=0.00 cfs 0.001 af

Outflow=0.00 cfs 0.001 af

Pond 1BP: FOREBAY Peak Elev=209.83' Storage=31 cf Inflow=0.69 cfs 0.049 af

15.0" Round Culvert n=0.013 L=42.0' S=0.0100 '/' Outflow=0.68 cfs 0.049 af

Pond 1CP: Bioretention Pond 1 Peak Elev=208.32' Storage=1,389 cf Inflow=1.60 cfs 0.115 af

Outflow=0.30 cfs 0.115 af

Pond 1DCP: 15" DW Culv Peak Elev=205.47' Storage=13 cf Inflow=0.68 cfs 0.064 af

15.0" Round Culvert n=0.013 L=43.0' S=0.0051 '/' Outflow=0.68 cfs 0.064 af

Pond 1DP: Bioretention Pond 1A Peak Elev=207.39' Storage=410 cf Inflow=0.55 cfs 0.039 af

Outflow=0.12 cfs 0.039 af

Pond 1P: 15" DW Culv Peak Elev=204.94' Storage=3,132 cf Inflow=3.04 cfs 0.354 af

15.0" Round Culvert n=0.025 L=21.0' S=0.0052 '/' Outflow=1.75 cfs 0.354 af

Pond 2P: Bioretention Pond Peak Elev=218.10' Storage=25 cf Inflow=0.08 cfs 0.007 af

Outflow=0.04 cfs 0.007 af

Pond 3P: Bioretention Pond 3 Peak Elev=223.04' Storage=15 cf Inflow=0.09 cfs 0.007 af

Outflow=0.07 cfs 0.007 af

Pond 4P: 15" DW Culv Peak Elev=205.44' Storage=12 cf Inflow=0.60 cfs 0.072 af

15.0" Round Culvert n=0.013 L=43.0' S=0.0051 '/' Outflow=0.60 cfs 0.072 af

Pond U1P: drip edge UNIT 1&2 Peak Elev=121.06' Storage=311 cf Inflow=0.33 cfs 0.024 af

Discarded=0.05 cfs 0.024 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.024 af

Pond U2P: drip edge UNIT 2&3 Peak Elev=120.76' Storage=58 cf Inflow=0.11 cfs 0.009 af

Discarded=0.04 cfs 0.009 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.009 af

Pond U3P: drip edge UNIT 3&4 Peak Elev=120.74' Storage=145 cf Inflow=0.18 cfs 0.013 af

Discarded=0.03 cfs 0.013 af Primary=0.00 cfs 0.000 af Outflow=0.03 cfs 0.013 af

Pond U4P: drip edge UNIT 4&5 Peak Elev=121.14' Storage=283 cf Inflow=0.30 cfs 0.021 af

Discarded=0.04 cfs 0.021 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.021 af

Pond U5P: drip edge UNIT 5 Peak Elev=120.65' Storage=78 cf Inflow=0.10 cfs 0.007 af

Discarded=0.02 cfs 0.007 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.007 af

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Route 9 Barrington
Type III 24-hr 10 YR Rainfall=4.64"

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Pond U6P: drip edge UNIT 6

6 Peak Elev=122.23' Storage=220 cf Inflow=0.27 cfs 0.020 af Discarded=0.05 cfs 0.020 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.020 af

Total Runoff Area = 5.250 ac Runoff Volume = 0.617 af Average Runoff Depth = 1.41" 76.63% Pervious = 4.023 ac 23.37% Impervious = 1.227 ac

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Summary for Subcatchment 1: Subcatchment to 15" DW culvert

Runoff 1.85 cfs @ 12.32 hrs, Volume=

0.219 af, Depth= 1.29"

Routed to Pond 1P: 15" DW Culv

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

	Area (sf)	CN	Description	1			
	4,665	30	Woods, Good, HSG A				
	22,429		Woods, Go				
	34,210		Woods, Go				
	1,411				ood, HSG A		
	718				ood, HSG B		
	11,355				ood, HSG C		
	339		Roofs, HS0		,		
	1,682		Roofs, HSC				
	1,481	98	Paved park	ing, HSG E	3		
	1,019		Paved park				
	5,058				litches, 50% imp, HSG B		
	4,483		Paved roads w/open ditches, 50% imp, HSG C				
	88,850						
	79,559		89.54% Pervious Area				
	9,292		10.46% Impervious Area				
			•				
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
9.7	50	0.0400	0.09		Sheet Flow, Sheet		
					Woods: Light underbrush n= 0.400 P2= 3.00"		
7.5	327	0.0210	0.72		Shallow Concentrated Flow, Flow to RS swale		
					Woodland Kv= 5.0 fps		
3.4	217	0.0050	1.06		Shallow Concentrated Flow, SC to Culv		
					Grassed Waterway Kv= 15.0 fps		
20.6	594	Total					

Summary for Subcatchment 1A: Subcatchment to West wetland swale

Runoff 0.60 cfs @ 12.28 hrs, Volume=

0.072 af, Depth= 1.04"

Routed to Pond 4P: 15" DW Culv

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

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- A	Area (sf)	CN	Description					
	4,824	30	Noods, Go	od, HSG A				
	8,718	55 Y	Noods, Go	od, HSG B				
	17,663	70	Woods, Good, HSG C					
	129	39	>75% Grass cover, Good, HSG A					
	585	61 :	>75% Gras	s cover, Go	ood, HSG B			
	638	74 :	>75% Gras	s cover, Go	ood, HSG C			
	3,582	98	Nater Surfa	ace, HSG C				
	36,139	64 \	Neighted A	verage				
	32,557		90.09% Pei					
	3,582	9	9.91% Impe	ervious Are	a			
	,							
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)		(cfs)	·			
12.7	50	0.0200	0.07		Sheet Flow, Sheet			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
4.7	147	0.0110	0.52		Shallow Concentrated Flow, SC to CULV			
					Woodland Kv= 5.0 fps			
17.4	197	Total						

Summary for Subcatchment 1B: Flow to sed forebay

Runoff = 0.69 cfs @ 12.09 hrs, Volume=

0.049 af. Depth= 2.49"

Routed to Pond 1BP: FOREBAY

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

	Area (sf)	CN	Description								
	645	39	39 >75% Grass cover, Good, HSG A								
	4,201	61	>75% Grass cover, Good, HSG B								
	725	98	Paved parking, HSG A								
	4,649	98	Paved park	ing, HSG B							
	10,220	79	79 Weighted Average								
	4,846		47.42% Pervious Area								
	5,374		52.58% lmp	pervious Ar	ea						
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description						
6.0					Direct Entry, Direct						

Summary for Subcatchment 1C: Flow to BIORET 1

Runoff = 0.93 cfs @ 12.09 hrs, Volume=

0.066 af, Depth= 2.85"

Routed to Pond 1CP: Bioretention Pond 1

Type III 24-hr 10 YR Rainfall=4.64"

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A	rea (sf)	CN	Description									
	626	39	>75% Grass	5% Grass cover, Good, HSG A								
	3,661	61		75% Grass cover, Good, HSG B								
	624	74	>75% Grass	75% Grass cover, Good, HSG C								
	2,376	98	Paved parking	ved parking, HSG A								
	3,026	98	Paved parkii	ng, HSG B	3							
	1,831	98	Paved parkii	ng, HSG C								
	12,144	83	Weighted Average									
	4,911			40.44% Pervious Area								
	7,233		59.56% Impe	rea								
Tc	Length	Slop		Capacity	Description							
(min)_	(feet)	(ft/f	t) (ft/sec)	(cfs)								
6.0					Direct Entry, Direct							

Summary for Subcatchment 1D: Flow to BIORET 1A

Runoff = 0.55 cfs @ 12.09 hrs, Volume=

0.039 af, Depth= 2.67"

Routed to Pond 1DP: Bioretention Pond 1A

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

A	rea (sf)	CN	Description									
	38	30	Woods, Good, HSG A									
	464	39	>75% Gras	>75% Grass cover, Good, HSG A								
	2,628	61	>75% Gras	s cover, Go	ood, HSG B							
	244	98	Roofs, HSC	Roofs, HSG A								
	2,915	98	Paved park	aved parking, HSG A								
	1,322	98	Paved park	aved parking, HSG B								
	7,611	81 Weighted Average										
	3,130		41.12% Pervious Area									
	4,481		58.88% Imp	pervious Are	rea							
			£ .									
Тс	Length	Slope	 Velocity 	Capacity	Description							
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)								
6.0					Direct Entry, Direct							

Summary for Subcatchment 1DC: Subcatchment to 15" DW culvert

Runoff = 0.68 cfs @ 12.21 hrs, Volume=

0.064 af, Depth= 2.49"

Routed to Pond 1DCP: 15" DW Culv

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A	Area (sf)	CN [Description							
	907	55 V	55 Woods, Good, HSG B							
	684	61 >	>75% Grass cover, Good, HSG B							
	10,379	89 F	Paved roads w/open ditches, 50% imp, HSG B							
	561	98 F	Paved park	ing, HSG B						
	880	98 F	Paved park	ing, HSG C	}					
	13,411	86 V	Veighted A	verage						
	6,781			vious Area						
	6,631	4	9.44% Imp	ervious Ar	ea					
			_							
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
12.4	50	0.0030	0.07		Sheet Flow, Sheet					
					Grass: Short n= 0.150 P2= 3.00"					
2.8	198	0.0060	1.16		Shallow Concentrated Flow, SC to Culv					
					Grassed Waterway Kv= 15.0 fps					
15.2	248	Total								

Summary for Subcatchment 2: SE Area of Site

Runoff = 0.00 cfs @ 21.90 hrs, Volume=

0.000 af, Depth= 0.02"

Routed to Reach 200 : SE Analysis Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

A	rea (sf)	CN	Description								
	3,897	30	Woods, Go	oods, Good, HSG A							
	1,652	39	>75% Gras	5% Grass cover, Good, HSG A							
	5,549	33	Weighted A	ighted Average							
	5,549		100.00% P		a						
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description						
6.0					Direct Entry, Direct						

Summary for Subcatchment 2A: Flow to Bioret 2

Runoff = 0.08 cfs @ 12.11 hrs, Volume=

0.007 af, Depth= 0.86"

Routed to Pond 2P: Bioretention Pond

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A	rea (sf)	CN	Description	Description							
	3,088	39	>75% Gras	75% Grass cover, Good, HSG A							
	218	98	Roofs, HSG	oofs, HSG A							
	1,083	98	Paved park	ing, HSG A							
	4,389	56	Weighted A	verage							
	3,088		70.36% Pei	70.36% Pervious Area							
	1,301		29.64% Imp	ervious Ar	ea						
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description						
6.0					Direct Entry, Direct						

Summary for Subcatchment 3: North East parcel area

Runoff = 0.00 cfs @ 20.72 hrs, Volume=

0.001 af, Depth= 0.03"

Routed to Reach 300: NE Analysis Point

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

	A	rea (sf)	CN I	Description							
		18,675	30 \	30 Woods, Good, HSG A							
		4,753	39 :	>75% Gras	s cover, Go	ood, HSG A					
		96	98 I	Roofs, HSG	Α						
-		607	98	Paved park	ing, HSG A						
		24,131	34 \	Neighted A	verage						
		23,428	ç	97.09% Pei	vious Area	ı					
		703	2	2.91% Impe	ervious Are	a					
	_										
	Тс	Length	Slope		Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	7.4	50	0.0785	0.11		Sheet Flow, Sheet					
						Woods: Light underbrush n= 0.400 P2= 3.00"					
	0.6	57	0.1050	1.62		Shallow Concentrated Flow, SC to analysis point					
_						Woodland Kv= 5.0 fps					
	8.0	107	Total								

Summary for Subcatchment 3A: Flow to Bioret 3

Runoff = 0.09 cfs @ 12.10 hrs, Volume=

0.007 af, Depth= 1.04"

Routed to Pond 3P: Bioretention Pond 3

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A	rea (sf)	CN	Description								
	2,454	39	>75% Gras	5% Grass cover, Good, HSG A							
	1,213			aved parking, HSG A							
	3,667	59	Weighted Average								
	2,454			6.92% Pervious Area							
	1,213		33.08% Imp								
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description						
6.0					Direct Entry, Direct						

Summary for Subcatchment U1: unit 1&2

Runoff = 0.33 cfs @ 12.09 hrs, Volume=

0.024 af, Depth= 2.67"

Routed to Pond U1P: drip edge UNIT 1&2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

A	rea (sf)	CN	Description								
	1,325	39	>75% Gras	75% Grass cover, Good, HSG A							
	1,410			Roofs, HSG A							
	1,899	98	Paved park	ing, HSG A	A						
	4,634	81	Weighted A	verage							
	1,325			8.59% Pervious Area							
	3,309		71.41% lmp	pervious Ar	rea						
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description						
6.0					Direct Entry, direct	_					

Summary for Subcatchment U2: unit 2&3

Runoff = 0.11 cfs @ 12.10 hrs, Volume=

0.009 af, Depth= 1.42"

Routed to Pond U2P: drip edge UNIT 2&3

Area (sf)	CN_	Description
1,750	39	>75% Grass cover, Good, HSG A
1,128	98	Roofs, HSG A
287	98	Paved parking, HSG A
3,165	65	Weighted Average
1,750		55.29% Pervious Area
1,415		44.71% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
6.0					Direct Entry, direct	

Summary for Subcatchment U3: unit 3&4

Runoff =

0.18 cfs @ 12.09 hrs, Volume=

0.013 af, Depth= 1.85"

Routed to Pond U3P: drip edge UNIT 3&4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

A	rea (sf)	CN	Description						
	1,643	39	>75% Gras	s cover, Go	ood, HSG A				
	993	98	Roofs, HSC	A	,				
	997	98	Paved park	ing, HSG A					
	3,633 1,643 1,990	71	45.22% Pe	Veighted Average 45.22% Pervious Area 54.78% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft							
6.0			Direct Entry, direct						

Summary for Subcatchment U4: unit 4&5

Runoff

0.30 cfs @ 12.09 hrs, Volume=

0.021 af, Depth= 2.67"

Routed to Pond U4P: drip edge UNIT 4&5

A	rea (sf)	CN	Description							
	1,206	39	>75% Gras	s cover, Go	ood, HSG A					
	1,444	98	Roofs, HSC	βA						
	1,462	98	Paved park	ing, HSG A	4					
	4,112	81	Weighted A	Veighted Average						
	1,206		29.33% Pei	29.33% Pervious Area						
	2,906		70.67% lmp	pervious Ar	rea					
_										
Tc	Length	Slope		Capacity	Description					
(min)	(feet)	(ft/ft	(ft/sec)	(ft/sec) (cfs)						
6.0					Direct Entry, direct					

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Summary for Subcatchment U5: unit 5

Runoff 0.10 cfs @ 12.09 hrs, Volume=

0.007 af, Depth= 2.16"

Routed to Pond U5P: drip edge UNIT 5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

A	rea (sf)	CN	Description							
	683	39	>75% Gras	s cover, Go	ood, HSG A					
	587		Roofs, HSC		,					
	452	98	Paved park	ing, HSG A	\					
	1,722		Weighted Average							
	683		39.66% Pei							
	1,039		60.34% lmp	pervious Ar	ea					
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description					
6.0					Direct Entry, direct					

Summary for Subcatchment U6: unit 6

Runoff 0.27 cfs @ 12.09 hrs, Volume= 0.020 af, Depth= 1.92"

Routed to Pond U6P: drip edge UNIT 6

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10 YR Rainfall=4.64"

A	rea (sf)	CN	Description							
	2,328	39	>75% Gras	s cover, Go	ood, HSG A					
	821	98	Roofs, HSG	A ·	,					
	2,149	98	Paved park	ing, HSG A						
	5,298 2,328 2,970		43.94% Pei	Veighted Average I3.94% Pervious Area I6.06% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft	, 1 . A =							
6.0					Direct Entry, direct					

Summary for Reach 1AR: RTE 9 RS swale

Inflow Area = 1.138 ac, 20.61% Impervious, Inflow Depth = 1.43" for 10 YR event

Inflow 0.136 af

1.25 cfs @ 12.25 hrs, Volume= 1.20 cfs @ 12.36 hrs, Volume= Outflow = 0.136 af, Atten= 4%, Lag= 6.9 min

Routed to Pond 1P: 15" DW Culv

Route 9 Barrington Type III 24-hr 10 YR Rainfall=4.64"

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Routing by Stor-Ind+Trans method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Max. Velocity= 1.37 fps, Min. Travel Time= 3.7 min

Avg. Velocity = 0.58 fps, Avg. Travel Time= 8.7 min

Peak Storage= 266 cf @ 12.30 hrs Average Depth at Peak Storage= 0.54', Surface Width= 3.24' Bank-Full Depth= 2.00' Flow Area= 12.0 sf, Capacity= 39.39 cfs

0.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight Side Slope Z-value= 3.0 '/' Top Width= 12.00' Length= 304.0' Slope= 0.0025 '/' Inlet Invert= 204.78', Outlet Invert= 204.01'



Summary for Reach 100: North Analysis point

Inflow Area = 3.177 ac, 14.09% Impervious, Inflow Depth = 1.34" for 10 YR event

Inflow = 1.75 cfs @ 12.66 hrs, Volume= 0.354 af

Outflow = 1.75 cfs @ 12.66 hrs, Volume= 0.354 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Summary for Reach 200: SE Analysis Point

Inflow Area = 0.127 ac, 0.00% Impervious, Inflow Depth = 0.02" for 10 YR event

Inflow = 0.00 cfs @ 21.90 hrs, Volume= 0.000 af

Outflow = 0.00 cfs @ 21.90 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

Summary for Reach 300: NE Analysis Point

Inflow Area = 0.676 ac, 12.48% Impervious, Inflow Depth = 0.02" for 10 YR event

Inflow = 0.00 cfs @ 20.72 hrs, Volume= 0.001 af

Outflow = 0.00 cfs @ 20.72 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs

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Summary for Pond 1BP: FOREBAY

Inflow Area = 0.525 ac, 55.68% Impervious, Inflow Depth = 1.12" for 10 YR event

Inflow = 0.69 cfs @ 12.09 hrs. Volume= 0.049 af

Outflow = 0.68 cfs @ 12.10 hrs, Volume= 0.049 af, Atten= 1%, Lag= 0.8 min

Primary = 0.68 cfs @ 12.10 hrs, Volume= 0.049 af

Routed to Pond 1CP: Bioretention Pond 1

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 209.83' @ 12.10 hrs Surf.Area= 144 sf Storage= 31 cf

Plug-Flow detention time= 0.9 min calculated for 0.049 af (100% of inflow)

Center-of-Mass det. time= 0.9 min (828.7 - 827.8)

Volume	Invert	Avail.Sto	rage	Storage I	Description	
#1	209.42'	5	05 cf	Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevation (feet)	Sui	rf.Area (sq-ft)		.Store c-feet)	Cum.Store (cubic-feet)	
209.42 210.00 211.00		6 201	*,	0 60	0 60	
	outina	689 Invert	Outle	445 et Devices	505	

#1 Primary 209.42' 15.0" Round Culvert

L= 42.0' CPP, mitered to conform to fill, Ke= 0.700

Inlet / Outlet Invert= 209.42' / 209.00' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.68 cfs @ 12.10 hrs HW=209.83' (Free Discharge)

1=Culvert (Inlet Controls 0.68 cfs @ 1.93 fps)

Summary for Pond 1CP: Bioretention Pond 1

Inflow Area = 0.803 ac, 57.03% Impervious, Inflow Depth = 1.72" for 10 YR event

Inflow = 1.60 cfs @ 12.09 hrs, Volume= 0.115 af

Outflow = 0.30 cfs @ 12.55 hrs, Volume= 0.115 af, Atten= 81%, Lag= 27.6 min

Discarded = 0.30 cfs @ 12.55 hrs, Volume= 0.115 af

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 208.32' @ 12.55 hrs Surf.Area= 1,312 sf Storage= 1,389 cf

Flood Elev= 209.50' Surf.Area= 1,898 sf Storage= 3,280 cf

Plug-Flow detention time= 30.8 min calculated for 0.115 af (100% of inflow)

Center-of-Mass det. time= 30.8 min (852.5 - 821.7)

Volume	Invert	Avail.Storage	Storage Description
#1	205.50'	3,280 cf	Custom Stage Data (Conic)Listed below (Recalc)

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Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
205.50	1,172	0.0	0	0	1,172
206.50	1,172	40.0	469	469	1,293
208.00	1,172	30.0	527	996	1,475
209.00	1,641	100.0	1,400	2,396	1,963
209.50	1,898	100.0	884	3,280	2,230

Device Routing Invert Outlet Devices

#1 Discarded 205.50' 10.000 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.30 cfs @ 12.55 hrs HW=208.32' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.30 cfs)

Summary for Pond 1DCP: 15" DW Culv

Inflow Area = 0.308 ac, 49.44% Impervious, Inflow Depth = 2.49" for 10 YR event

Inflow = 0.68 cfs @ 12.21 hrs, Volume= 0.064 af

Outflow = 0.68 cfs @ 12.22 hrs, Volume= 0.064 af, Atten= 0%, Lag= 0.3 min

Primary = 0.68 cfs @ 12.22 hrs, Volume= 0.064 af

Routed to Reach 1AR: RTE 9 RS swale

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 205.47' @ 12.22 hrs Surf.Area= 50 sf Storage= 13 cf

Plug-Flow detention time= 0.5 min calculated for 0.064 af (100% of inflow)

Center-of-Mass det. time= 0.5 min (836.8 - 836.3)

Volume	Inv	ert Avail.Sto	rage S	Storage D	escription		
#1	205.0	00' 3	17 cf (Custom S	tage Data (P	rismatic)Listed below (Recalc)	
Elevatio (fee		Surf.Area (sq-ft)	Inc.S (cubic-	Store feet)	Cum.Store (cubic-feet)		
205.0	-	6		0	0		
206.0	0	101		54	54		
207.0	0	425		263	317		
Device	Routing	Invert	Outlet	Devices			
#1	Primary	205.00'	L= 43.		projecting, no	headwall, Ke= 0.900 204.78' S= 0.0051 '/' Cc= 0.900	

n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.68 cfs @ 12.22 hrs HW=205.47' (Free Discharge)
1=Culvert (Barrel Controls 0.68 cfs @ 2.42 fps)

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Summary for Pond 1DP: Bioretention Pond 1A

Inflow Area = 0.281 ac, 63.62% Impervious, Inflow Depth = 1.66" for 10 YR event

Inflow = 0.55 cfs @ 12.09 hrs, Volume= 0.039 af

Outflow = 0.12 cfs @ 11.78 hrs, Volume= 0.039 af, Atten= 79%, Lag= 0.0 min

Discarded = 0.12 cfs @ 11.78 hrs, Volume= 0.039 af

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 207.39' @ 12.52 hrs Surf.Area= 502 sf Storage= 410 cf

Plug-Flow detention time= 20.1 min calculated for 0.039 af (100% of inflow)

Center-of-Mass det. time= 20.1 min (842.4 - 822.3)

Volume	Invert	Ava	il.Storage	Storage Descript	tion		
#1	205.00'		1,116 cf	Custom Stage I	Data (Conic)Listed	below (Recalc)	
Elevation (feet)		f.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
205.00		502	0.0	0	0	502	
206.00		502	40.0	201	201	581	
207.50		502	30.0	226	427	701	
208.00		687	100.0	296	723	891	
208.50		890	100.0	393	1,116	1,100	
	louting	In	vert Outle	et Devices			
#1 D	iscarded	205	.00' 10.0	00 in/hr Exfiltrati	on over Surface a	rea Phase-In= 0.01'	

Discarded OutFlow Max=0.12 cfs @ 11.78 hrs HW=205.04' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.12 cfs)

Summary for Pond 1P: 15" DW Culv

Inflow Area = 3.177 ac, 14.09% Impervious, Inflow Depth = 1.34" for 10 YR event

Inflow = 3.04 cfs @ 12.34 hrs, Volume= 0.354 af

Outflow = 1.75 cfs @ 12.66 hrs, Volume= 0.354 af, Atten= 42%, Lag= 19.6 min

Primary = 1.75 cfs @ 12.66 hrs, Volume= 0.354 af

Routed to Reach 100: North Analysis point

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 204.94' @ 12.66 hrs Surf.Area= 5,931 sf Storage= 3,132 cf

Plug-Flow detention time= 38.1 min calculated for 0.354 af (100% of inflow)

Center-of-Mass det. time= 38.0 min (920.2 - 882.2)

Volume	Invert	Avail.Storage	Storage Description
#1	204.00'	12,535 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Type III 24-hr 10 YR Rainfall=4.64" Printed 8/25/2022

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
204.00	738	0	0
206.00	11,797	12,535	12,535

Device Routing Invert Outlet Devices #1 Primary 204.01' 15.0" Round Culvert

> L= 21.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 204.01' / 203.90' S= 0.0052 '/' Cc= 0.900 n= 0.025 Corrugated metal, Flow Area= 1.23 sf

Primary OutFlow Max=1.75 cfs @ 12.66 hrs HW=204.94' (Free Discharge) 1=Culvert (Barrel Controls 1.75 cfs @ 2.49 fps)

Summary for Pond 2P: Bioretention Pond

Inflow Area = 0.101 ac, 29.64% Impervious, Inflow Depth = 0.86" for 10 YR event

Inflow 0.08 cfs @ 12.11 hrs, Volume= 0.007 af

Outflow 0.04 cfs @ 12.07 hrs, Volume= 0.007 af, Atten= 46%, Lag= 0.0 min

Discarded = 0.04 cfs @ 12.07 hrs, Volume= 0.007 af

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 218.10' @ 12.37 hrs Surf.Area= 618 sf Storage= 25 cf

Flood Elev= 221.00' Surf.Area= 827 sf Storage= 885 cf

Plug-Flow detention time= 4.3 min calculated for 0.007 af (100% of inflow)

Center-of-Mass det. time= 4.3 min (900.3 - 896.0)

Volume	Invert	Ava	il.Storage	Storage Descrip	tion		
#1	218.00'		885 cf	Custom Stage	Data (Conic)Listed be	low (Recalc)	
Elevation (feet)		rf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
218.00		618	0.0	0	0	618	
219.00		618	40.0	247	247	706	
220.50		618	30.0	278	525	838	
221.00		827	100.0	360	885	1,053	
Device F	Routing	In	vert Outle	et Devices			
#1 [Discarded	218	3.00' 3.00	0 in/hr Exfiltratio	n over Surface area	Phase-In= 0.01'	

Discarded OutFlow Max=0.04 cfs @ 12.07 hrs HW=218.03' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

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Summary for Pond 3P: Bioretention Pond 3

Inflow Area = 0.084 ac, 33.08% Impervious, Inflow Depth = 1.04" for 10 YR event

Inflow = 0.09 cfs @ 12.10 hrs, Volume= 0.007 af

Outflow = 0.07 cfs @ 12.10 hrs, Volume= 0.007 af, Atten= 24%, Lag= 0.0 min

Discarded = 0.07 cfs @ 12.10 hrs, Volume= 0.007 af

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 223.04' @ 12.18 hrs Surf.Area= 947 sf Storage= 15 cf

Flood Elev= 226.00' Surf.Area= 1,184 sf Storage= 1,337 cf

Plug-Flow detention time= 3.0 min calculated for 0.007 af (100% of inflow)

Center-of-Mass det. time= 3.0 min (887.7 - 884.7)

Volume	Inver	t Ava	il.Storage	Storage Descrip	otion		
#1	223.00)'	1,337 cf	Custom Stage	Data (Conic)Listed	below (Recalc)	
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
223.0	00	947	0.0	0	0	947	
224.0		947	40.0	379	379	1,056	
225.5		947	30.0	426	805	1,220	
226.0	00	1,184	100.0	532	1,337	1,464	
Device	Routing	In	vert Outl	et Devices			
#1	Discarded	223	3.00' 3.00	0 in/hr Exfiltration	on over Surface are	a Phase-In= 0.01'	

Discarded OutFlow Max=0.07 cfs @ 12.10 hrs HW=223.03' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.07 cfs)

Summary for Pond 4P: 15" DW Culv

Inflow Area = 0.830 ac, 9.91% Impervious, Inflow Depth = 1.04" for 10 YR event

Inflow = 0.60 cfs @ 12.28 hrs, Volume= 0.072 af

Outflow = 0.60 cfs @ 12.28 hrs, Volume= 0.072 af, Atten= 0%, Lag= 0.3 min

Primary = 0.60 cfs @ 12.28 hrs, Volume= 0.072 af

Routed to Reach 1AR: RTE 9 RS swale

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 205.44' @ 12.28 hrs Surf.Area= 48 sf Storage= 12 cf

Plug-Flow detention time= 0.4 min calculated for 0.072 af (100% of inflow)

Center-of-Mass det. time= 0.4 min (895.7 - 895.2)

Volume	Invert	Avail.Storage	Storage Description
#1	205.00'	317 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
205.00	6	0	0
206.00	101	54	54
207.00	425	263	317

Device Routing Invert Outlet Devices

#1 Primary 205.00' 15.0" Round Culvert

L= 43.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 205.00' / 204.78' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.60 cfs @ 12.28 hrs HW=205.44' (Free Discharge)
1=Culvert (Barrel Controls 0.60 cfs @ 2.35 fps)

Summary for Pond U1P: drip edge UNIT 1&2

Inflow Area = 0.106 ac, 71.41% Impervious, Inflow Depth = 2.67" for 10 YR event Inflow 0.33 cfs @ 12.09 hrs, Volume= 0.024 af Outflow 0.05 cfs @ 11.73 hrs, Volume= 0.024 af, Atten= 85%, Lag= 0.0 min Discarded = 0.05 cfs @ 11.73 hrs, Volume= 0.024 af Primary 0.00 cfs @ 1.00 hrs. Volume= 0.000 af Routed to Pond 1DP: Bioretention Pond 1A

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 121.06' @ 12.60 hrs Surf.Area= 735 sf Storage= 311 cf

Plug-Flow detention time= 41.5 min calculated for 0.024 af (100% of inflow) Center-of-Mass det. time= 41.5 min (863.8 - 822.3)

Volume	Inve	rt Ava	il.Storage	Storage Descri	ption	
#1	120.00	0'	735 cf	Custom Stage	Data (Prismatic)Liste	ed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
120.0 122.5		735 735	0.0 40.0	0 735	0 735	
Device	Routing	In	vert Out	let Devices		
#1 #2	Discarded Primary		2.45' Cus Hea			l

Discarded OutFlow Max=0.05 cfs @ 11.73 hrs HW=120.03' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=120.00' (Free Discharge) 2=Custom Weir/Orifice (Controls 0.00 cfs)

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Summary for Pond U2P: drip edge UNIT 2&3

Inflow Area = 0.073 ac, 44.71% Impervious, Inflow Depth = 1.42" for 10 YR event
Inflow = 0.11 cfs @ 12.10 hrs, Volume= 0.009 af
Outflow = 0.04 cfs @ 11.99 hrs, Volume= 0.009 af, Atten= 66%, Lag= 0.0 min

Discarded = 0.04 cfs @ 11.99 hrs, Volume= 0.009 af Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routed to Pond 1BP: FOREBAY

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 120.76' @ 12.45 hrs Surf.Area= 560 sf Storage= 58 cf

Plug-Flow detention time= 8.3 min calculated for 0.009 af (100% of inflow)

Center-of-Mass det. time= 8.3 min (873.9 - 865.7)

Volume	Inver	rt Ava	il.Storage	Storage Descri	ption	
#1	120.50)'	448 cf	Custom Stage	Data (Prismatic)Liste	ed below (Recalc)
Elevatio	-	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
120.5	50	560	0.0	0	0	
122.5	50	560	40.0	448	448	
Device	Routing	In	vert Out	let Devices		
#1	Discarded	120	.50' 3.0 0	00 in/hr Exfiltrati	on over Surface area	
#2	Primary	122			e, Cv= 2.62 (C= 3.28)	
				nd (feet) 0.00 0.0		
			VVIC	th (feet) 8.00 8.0	JU	

Discarded OutFlow Max=0.04 cfs @ 11.99 hrs HW=120.52' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=120.50' (Free Discharge) —2=Custom Weir/Orifice (Controls 0.00 cfs)

Summary for Pond U3P: drip edge UNIT 3&4

Inflow Area = 0.083 ac, 54.78% Impervious, Inflow Depth = 1.85" for 10 YR event Inflow 0.18 cfs @ 12.09 hrs, Volume= 0.013 af 0.03 cfs @ 11.84 hrs, Volume= 0.03 cfs @ 11.84 hrs, Volume= Outflow = 0.013 af, Atten= 81%, Lag= 0.0 min Discarded = 0.013 af Primary 0.00 cfs @ 1.00 hrs, Volume= 0.000 af Routed to Pond 1BP: FOREBAY

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 120.74' @ 12.57 hrs Surf, Area= 493 sf Storage= 145 cf

Plug-Flow detention time= 27.5 min calculated for 0.013 af (100% of inflow) Center-of-Mass det. time= 27.5 min (876.6 - 849.1)

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Volume	Inve	ert Ava	il.Stor	age Storage Descr	ription	
#1	120.0	0'	49	3 cf Custom Stage	e Data (Prismatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Void: (%	1110101010	Cum.Store (cubic-feet)	
120.0 122.5	_	493 493	0.0 40.0	-	0 493	
Device	Routing	In	vert	Outlet Devices		
#1 #2	Discarde Primary		2.45'	3.000 in/hr Exfiltrat Custom Weir/Orific Head (feet) 0.00 0. Width (feet) 8.00 8	ce, Cv= 2.62 (C= 3 05	

Discarded OutFlow Max=0.03 cfs @ 11.84 hrs HW=120.03' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=120.00' (Free Discharge) 2=Custom Weir/Orifice (Controls 0.00 cfs)

Summary for Pond U4P: drip edge UNIT 4&5

Inflow Area =	0.094 ac, 70.6	37% Impervious,	Inflow Depth = 2.67	" for 10 YR event		
Inflow =	0.30 cfs @ 12	2.09 hrs, Volume=	= 0.021 af			
Outflow =	0.04 cfs @ 11	1.72 hrs, Volume=	= 0.021 af, A	tten= 85%, Lag= 0.0 min		
Discarded =	0.04 cfs @ 11	1.72 hrs, Volume=	= 0.021 af			
Primary =	0.00 cfs @ 1	1.00 hrs, Volume=	= 0.000 af			
Routed to Pond 1BP : FOREBAY						

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 121.14' @ 12.62 hrs Surf.Area= 621 sf Storage= 283 cf

Plug-Flow detention time= 45.7 min calculated for 0.021 af (100% of inflow) Center-of-Mass det. time= 45.7 min (868.0 - 822.3)

Volume	Inve	<u>ert Ava</u>	il.Stora	age Stor	age Descrip	tion	
#1	120.0	0'	62	1 cf Cus	tom Stage	Data (Prismatic)Listed	below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Voids (%	65	Inc.Store ubic-feet)	Cum.Store (cubic-feet)	
120.0 122.5		621 621	0.0 40.0)	0 621	0 621	
Device	Routing	In	vert	Outlet De	vices		
#1	Discarde	d 120	.00'	3.000 in/h	nr Exfiltratio	n over Surface area	
#2	Primary	122				Cv= 2.62 (C= 3.28)	
				`	et) 0.00 0.00 et) 8.00 8.0		

Type III 24-hr 10 YR Rainfall=4.64"

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Discarded OutFlow Max=0.04 cfs @ 11.72 hrs HW=120.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=120.00' (Free Discharge) 2=Custom Weir/Orifice (Controls 0.00 cfs)

Summary for Pond U5P: drip edge UNIT 5

Inflow Area = 0.040 ac, 60.34% Impervious, Inflow Depth = 2.16" for 10 YR event

Inflow = 0.10 cfs @ 12.09 hrs, Volume= 0.007 af

Outflow = 0.02 cfs @ 11.82 hrs, Volume= 0.007 af, Atten= 79%, Lag= 0.0 min

Discarded = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Primary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routed to Pond 1BP: FOREBAY

Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 120.65' @ 12.54 hrs Surf.Area= 296 sf Storage= 78 cf

Plug-Flow detention time= 23.0 min calculated for 0.007 af (100% of inflow) Center-of-Mass det. time= 23.0 min (861.5 - 838.5)

Volume	Inve	ert Ava	il.Storage	 Storage Descri 	iption	
#1	120.0	00'	237 ct	Custom Stage	Data (Prismatic)Liste	d below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
120.0 122.0	_	296 296	0.0 40.0	0 237	0 237	
Device	Routing	In	vert Ou	tlet Devices		
#1 #2	#1 Discarded 120.00'		1.95' Cu He			

Discarded OutFlow Max=0.02 cfs @ 11.82 hrs HW=120.02' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=120.00' (Free Discharge)

—2=Custom Weir/Orifice (Controls 0.00 cfs)

Summary for Pond U6P: drip edge UNIT 6

Inflow Area =	0.122 ac, 56.06% Impervious, Inflow	Depth = 1.92" for 10 YR event
Inflow =	0.27 cfs @ 12.09 hrs, Volume=	0.020 af
Outflow =	0.05 cfs @ 11.84 hrs, Volume=	0.020 af, Atten= 80%, Lag= 0.0 min
Discarded =	0.05 cfs @ 11.84 hrs, Volume=	0.020 af
Primary =	0.00 cfs @ 1.00 hrs, Volume=	0.000 af
Routed to Read	ch 300 · NE Analysis Point	

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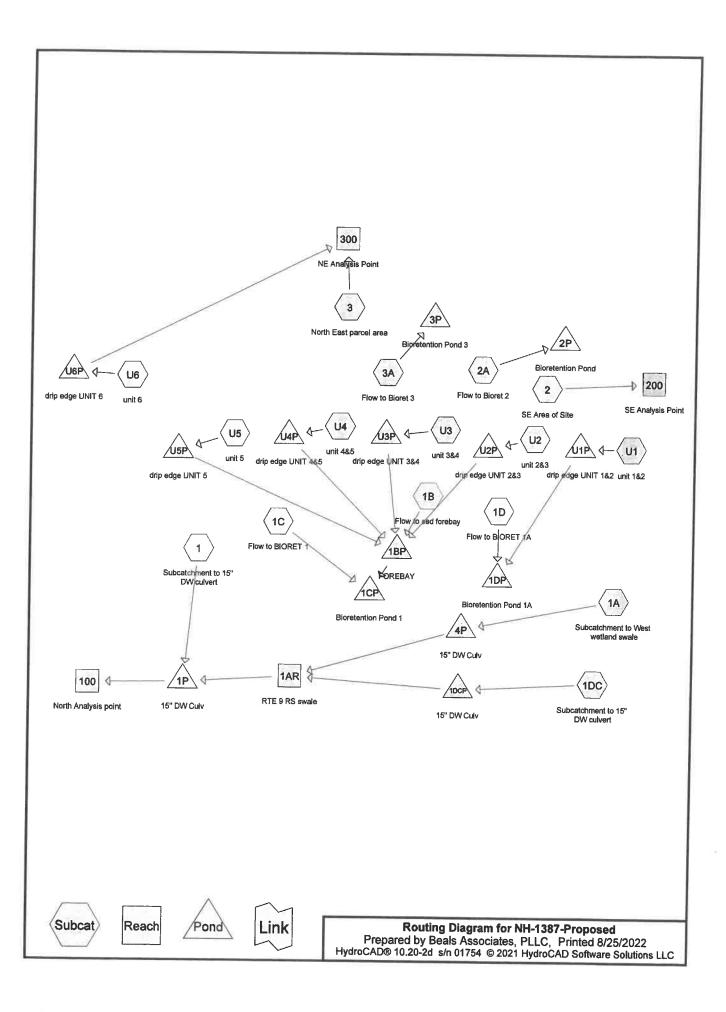
Routing by Stor-Ind method, Time Span= 1.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 122.23' @ 12.56 hrs Surf.Area= 759 sf Storage= 220 cf

Plug-Flow detention time= 26.9 min calculated for 0.020 af (100% of inflow) Center-of-Mass det. time= 26.9 min (873.3 - 846.4)

Volume	Inve	rt Ava	il.Stora	ige Storage Descr	iption	
#1	121.50)'	759	of Custom Stage	e Data (Prismatio	c)Listed below (Recalc)
Elevation (fee	- 10	Surf.Area (sq-ft)	Voids (%)		Cum.Store (cubic-feet)	
121.		759	0.0	-	0	
122.0		759	40.0	152	152	
124.0	00	759	40.0	607	759	
Device	Routing	In	vert	Outlet Devices		
#1	Discarded	121	.50'	3.000 in/hr Exfiltrat	ion over Surface	area
#2	Primary	124		Custom Weir/Orific		
	·		İ	Head (feet) 0.05 0.	10	,
				Width (feet) 8.00 8.	.00	

Discarded OutFlow Max=0.05 cfs @ 11.84 hrs HW=121.53' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.00 cfs @ 1.00 hrs HW=121.50' (Free Discharge) 2=Custom Weir/Orifice (Controls 0.00 cfs)



Route 9 Barrington Type III 24-hr 25 YR Rainfall=5.86" Printed 8/25/2022

Tc=6.0 min CN=72 Runoff=0.41 cfs 0.029 af

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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Tr	ans method - Pond routing by Stor-Ind method
	N Runoff Area=88,850 sf 10.46% Impervious Runoff Depth=2.08" Flow Length=594' Tc=20.6 min CN=68 Runoff=3.17 cfs 0.353 af
Subcatchment1A: Subcatchment to Wes	t Runoff Area=36,139 sf 9.91% Impervious Runoff Depth=1.75" Flow Length=197' Tc=17.4 min CN=64 Runoff=1.12 cfs 0.121 af
Subcatchment1B: Flow to sed forebay	Runoff Area=10,220 sf 52.58% Impervious Runoff Depth=3.55" Tc=6.0 min CN=79 Runoff=0.98 cfs 0.070 af
Subcatchment 1C: Flow to BIORET 1	Runoff Area=12,144 sf 59.56% Impervious Runoff Depth=3.96" Tc=6.0 min CN=83 Runoff=1.28 cfs 0.092 af
Subcatchment1D: Flow to BIORET1A	Runoff Area=7,611 sf 58.88% Impervious Runoff Depth=3.76" Tc=6.0 min CN=81 Runoff=0.77 cfs 0.055 af
	Runoff Area=13,411 sf 49.44% Impervious Runoff Depth=3.55" Flow Length=248' Tc=15.2 min CN=86 Runoff=0.97 cfs 0.091 af
Subcatchment 2: SE Area of Site	Runoff Area=5,549 sf 0.00% Impervious Runoff Depth=0.15" Tc=6.0 min CN=33 Runoff=0.00 cfs 0.002 af
Subcatchment 2A: Flow to Bioret 2	Runoff Area=4,389 sf 29.64% Impervious Runoff Depth=1.51" Tc=6.0 min CN=56 Runoff=0.16 cfs 0.013 af
Subcatchment3: North East parcel area	Runoff Area=24,131 sf 2.91% Impervious Runoff Depth=0.18" Flow Length=107' Tc=8.0 min CN=34 Runoff=0.01 cfs 0.008 af
Subcatchment 3A: Flow to Bioret 3	Runoff Area=3,667 sf 33.08% Impervious Runoff Depth=1.75" Tc=6.0 min CN=59 Runoff=0.16 cfs 0.012 af
Subcatchment U1: unit 1&2	Runoff Area=4,634 sf 71.41% Impervious Runoff Depth=3.76" Tc=6.0 min CN=81 Runoff=0.47 cfs 0.033 af
Subcatchment U2: unit 2&3	Runoff Area=3,165 sf 44.71% Impervious Runoff Depth=2.25" Tc=6.0 min CN=65 Runoff=0.19 cfs 0.014 af
Subcatchment U3: unit 3&4	Runoff Area=3,633 sf 54.78% Impervious Runoff Depth=2.79" Tc=6.0 min CN=71 Runoff=0.27 cfs 0.019 af
Subcatchment U4: unit 4&5	Runoff Area=4,112 sf 70.67% Impervious Runoff Depth=3.76" Tc=6.0 min CN=81 Runoff=0.41 cfs 0.030 af
Subcatchment U5: unit 5	Runoff Area=1,722 sf 60.34% Impervious Runoff Depth=3.16" Tc=6.0 min CN=75 Runoff=0.15 cfs 0.010 af
SubcatchmentU6: unit 6	Runoff Area=5,298 sf 56.06% Impervious Runoff Depth=2.88"

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Reach 1AR: RTE 9 RS swale Avg. Flow Depth=0.65' Max Vel=1.56 fps Inflow=2.05 cfs 0.212 af n=0.022 L=304.0' S=0.0025 '/' Capacity=39.39 cfs Outflow=1.99 cfs 0.212 af

Reach 100: North Analysis point Inflow=2,83 cfs 0,565 af

Outflow=2.83 cfs 0.565 af

Reach 200: SE Analysis Point Inflow=0.00 cfs 0.002 af

Outflow=0.00 cfs 0.002 af

Reach 300: NE Analysis Point Inflow=0.01 cfs 0.008 af

Outflow=0.01 cfs 0.008 af

Pond 1BP: FOREBAY Peak Elev=209.92' Storage=45 cf Inflow=0.98 cfs 0.070 af

15.0" Round Culvert n=0.013 L=42.0' S=0.0100 '/' Outflow=0.96 cfs 0.070 af

Pond 1CP: Bioretention Pond 1 Peak Elev=208.89' Storage=2,212 cf Inflow=2.24 cfs 0.162 af

Outflow=0.37 cfs 0.162 af

Pond 1DCP: 15" DW Culv Peak Elev=205.57' Storage=19 cf Inflow=0.97 cfs 0.091 af

15.0" Round Culvert n=0.013 L=43.0' S=0.0051 '/' Outflow=0.97 cfs 0.091 af

Pond 1DP: Bioretention Pond 1A Peak Elev=207.93' Storage=673 cf Inflow=0.77 cfs 0.055 af

Outflow=0.15 cfs 0.055 af

Pond 1P: 15" DW Culv Peak Elev=205.26' Storage=5,329 cf Inflow=5.13 cfs 0.566 af

15.0" Round Culvert n=0.025 L=21.0' S=0.0052'/' Outflow=2.83 cfs 0.565 af

Pond 2P: Bioretention Pond Peak Elev=218.45' Storage=110 cf Inflow=0.16 cfs 0.013 af

Outflow=0.04 cfs 0.013 af

Pond 3P: Bioretention Pond 3 Peak Elev=223.19' Storage=70 cf Inflow=0.16 cfs 0.012 af

Outflow=0.07 cfs 0.012 af

Pond 4P: 15" DW Culv Peak Elev=205.61' Storage=22 cf Inflow=1.12 cfs 0.121 af

15.0" Round Culvert n=0.013 L=43.0' S=0.0051 '/' Outflow=1.12 cfs 0.121 af

Pond U1P: drip edge UNIT 1&2 Peak Elev=121.74' Storage=511 cf Inflow=0.47 cfs 0.033 af

Discarded=0.05 cfs 0.033 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.033 af

Pond U2P: drip edge UNIT 2&3 Peak Elev=121.15' Storage=146 cf Inflow=0.19 cfs 0.014 af

Discarded=0.04 cfs 0.014 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.014 af

Pond U3P: drip edge UNIT 3&4 Peak Elev=121.42' Storage=279 cf Inflow=0.27 cfs 0.019 af

Discarded=0.03 cfs 0.019 af Primary=0.00 cfs 0.000 af Outflow=0.03 cfs 0.019 af

Pond U4P: drip edge UNIT 4&5 Peak Elev=121.86' Storage=463 cf Inflow=0.41 cfs 0.030 af

Discarded=0.04 cfs 0.030 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.030 af

Pond U5P: drip edge UNIT 5 Peak Elev=121.21' Storage=143 cf Inflow=0.15 cfs 0.010 af

Discarded=0.02 cfs 0.010 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.010 af

Route 9 Barrington
Type III 24-hr 25 YR Rainfall=5.86"

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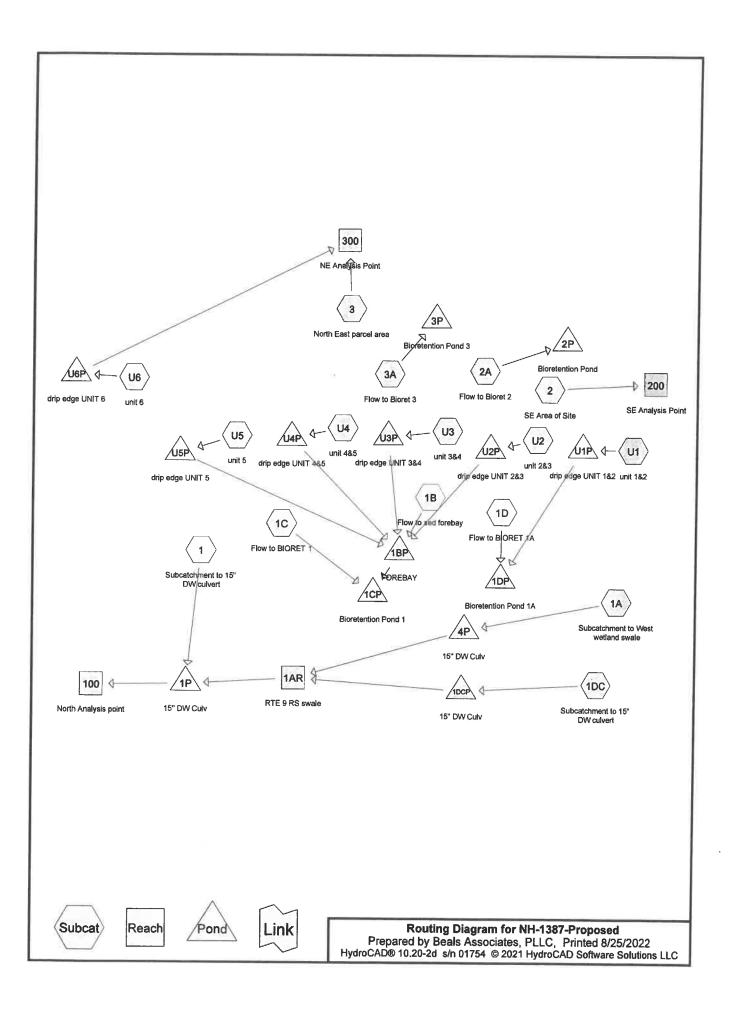
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Pond U6P: drip edge UNIT 6 Peak Elev=122.87' Storage=417 cf Inflow=0.41 cfs 0.029 af Discarded=0.05 cfs 0.029 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.029 af

Total Runoff Area = 5.250 ac Runoff Volume = 0.952 af Average Runoff Depth = 2.18" 76.63% Pervious = 4.023 ac 23.37% Impervious = 1.227 ac



Route 9 Barrington Type III 24-hr 50 YR Rainfall=7.00" Printed 8/25/2022

Tc=6.0 min CN=72 Runoff=0.55 cfs 0.039 af

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Time span=1.00-72.00 hrs, dt=0.01 hrs, 7101 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method					
	N Runoff Area=88,850 sf 10.46% Impervious Runoff Depth=2.90" Flow Length=594' Tc=20.6 min CN=68 Runoff=4.53 cfs 0.493 af				
Subcatchment 1A: Subcatchment to Wes	t Runoff Area=36,139 sf 9.91% Impervious Runoff Depth=2.51" Flow Length=197' Tc=17.4 min CN=64 Runoff=1.67 cfs 0.173 af				
Subcatchment1B: Flow to sed forebay	Runoff Area=10,220 sf 52.58% Impervious Runoff Depth=4.58" Tc=6.0 min CN=79 Runoff=1.25 cfs 0.090 af				
Subcatchment 1C: Flow to BIORET 1	Runoff Area=12,144 sf 59.56% Impervious Runoff Depth=5.03" Tc=6.0 min CN=83 Runoff=1.61 cfs 0.117 af				
Subcatchment 1D: Flow to BIORET 1A	Runoff Area=7,611 sf 58.88% Impervious Runoff Depth=4.81" Tc=6.0 min CN=81 Runoff=0.97 cfs 0.070 af				
Subcatchment 1DC: Subcatchment to 15"	Runoff Area=13,411 sf 49.44% Impervious Runoff Depth=4.58" Flow Length=248' Tc=15.2 min CN=86 Runoff=1.24 cfs 0.118 af				
Subcatchment 2: SE Area of Site	Runoff Area=5,549 sf 0.00% Impervious Runoff Depth=0.37" Tc=6.0 min CN=33 Runoff=0.01 cfs 0.004 af				
Subcatchment 2A: Flow to Bioret 2	Runoff Area=4,389 sf 29.64% Impervious Runoff Depth=2.22" Tc=6.0 min CN=56 Runoff=0.25 cfs 0.019 af				
Subcatchment3: North East parcel area	Runoff Area=24,131 sf 2.91% Impervious Runoff Depth=0.43" Flow Length=107' Tc=8.0 min CN=34 Runoff=0.08 cfs 0.020 af				
Subcatchment 3A: Flow to Bioret 3	Runoff Area=3,667 sf 33.08% Impervious Runoff Depth=2.51" Tc=6.0 min CN=59 Runoff=0.24 cfs 0.018 af				
Subcatchment U1: unit 1&2	Runoff Area=4,634 sf 71.41% Impervious Runoff Depth=4.81" Tc=6.0 min CN=81 Runoff=0.59 cfs 0.043 af				
Subcatchment U2: unit 2&3	Runoff Area=3,165 sf 44.71% Impervious Runoff Depth=3.10" Tc=6.0 min CN=65 Runoff=0.26 cfs 0.019 af				
Subcatchment U3: unit 3&4	Runoff Area=3,633 sf 54.78% Impervious Runoff Depth=3.72" Tc=6.0 min CN=71 Runoff=0.36 cfs 0.026 af				
Subcatchment U4: unit 4&5	Runoff Area=4,112 sf 70.67% Impervious Runoff Depth=4.81" Tc=6.0 min CN=81 Runoff=0.53 cfs 0.038 af				
Subcatchment U5: unit 5	Runoff Area=1,722 sf 60.34% Impervious Runoff Depth=4.15" Tc=6.0 min CN=75 Runoff=0.19 cfs 0.014 af				
Subcatchment U6: unit 6	Runoff Area=5,298 sf 56.06% Impervious Runoff Depth=3.83"				

NH-1387-Proposed
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Type III 24-hr 50 YR Rainfall=7.00"

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Reach 1AR: RTE 9 RS swale

Avg. Flow Depth=0.74' Max Vel=1.69 fps Inflow=2.87 cfs 0.291 af n=0.022 L=304.0' S=0.0025 '/' Capacity=39.39 cfs Outflow=2.79 cfs 0.291 af

Reach 100: North Analysis point Inflow=3.67 cfs 0.784 af

Outflow=3.67 cfs 0.784 af

Reach 200: SE Analysis Point Inflow=0.01 cfs 0.004 af

Outflow=0.01 cfs 0.004 af

Reach 300: NE Analysis Point Inflow=0.08 cfs 0.020 af

Outflow=0.08 cfs 0.020 af

Pond 1BP: FOREBAY Peak Elev=209.99' Storage=58 cf Inflow=1.25 cfs 0.091 af

15.0" Round Culvert n=0.013 L=42.0' S=0.0100 '/' Outflow=1.23 cfs 0.091 af

Pond 1CP: Bioretention Pond 1 Peak Elev=209.36' Storage=3,028 cf Inflow=2.84 cfs 0.207 af

Outflow=0.42 cfs 0.207 af

Pond 1DCP: 15" DW Culv Peak Elev=205.65' Storage=24 cf Inflow=1.24 cfs 0.118 af

15.0" Round Culvert n=0.013 L=43.0' S=0.0051'/' Outflow=1.24 cfs 0.118 af

Pond 1DP: Bioretention Pond 1A Peak Elev=208.27' Storage=921 cf Inflow=0.97 cfs 0.070 af

Outflow=0.18 cfs 0.070 af

Pond 1P: 15" DW Culv Peak Elev=205.56' Storage=7,894 cf Inflow=7.29 cfs 0.784 af

15.0" Round Culvert n=0.025 L=21.0' S=0.0052 '/' Outflow=3.67 cfs 0.784 af

Pond 2P: Bioretention Pond Peak Elev=218.89' Storage=220 cf Inflow=0.25 cfs 0.019 af

Outflow=0.04 cfs 0.019 af

Pond 3P: Bioretention Pond 3 Peak Elev=223.41' Storage=154 cf Inflow=0.24 cfs 0.018 af

Outflow=0.07 cfs 0.018 af

Pond 4P: 15" DW Culv Peak Elev=205.77' Storage=33 cf Inflow=1.67 cfs 0.173 af

15.0" Round Culvert n=0.013 L=43.0' S=0.0051 '/' Outflow=1.66 cfs 0.173 af

Pond U1P: drip edge UNIT 1&2 Peak Elev=122.44' Storage=717 cf Inflow=0.59 cfs 0.043 af

Discarded=0.05 cfs 0.043 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.043 af

Pond U2P: drip edge UNIT 2&3 Peak Elev=121.61' Storage=248 cf Inflow=0.26 cfs 0.019 af

Discarded=0.04 cfs 0.019 af Primary=0.00 cfs 0.000 af Outflow=0.04 cfs 0.019 af

Pond U3P: drip edge UNIT 3&4 Peak Elev=122.16' Storage=427 cf Inflow=0.36 cfs 0.026 af

Discarded=0.03 cfs 0.026 af Primary=0.00 cfs 0.000 af Outflow=0.03 cfs 0.026 af

Pond U4P: drip edge UNIT 4&5 Peak Elev=122.46' Storage=612 cf Inflow=0.53 cfs 0.038 af

Discarded=0.04 cfs 0.037 af Primary=0.05 cfs 0.001 af Outflow=0.09 cfs 0.038 af

Pond U5P: drip edge UNIT 5 Peak Elev=121.79' Storage=212 cf Inflow=0.19 cfs 0.014 af

Discarded=0.02 cfs 0.014 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.014 af

Route 9 Barrington
Type III 24-hr 50 YR Rainfall=7.00"

Prepared by Beals Associates, PLLC

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Pond U6P: drip edge UNIT 6 Peak Elev=123.58' Storage=633 cf Inflow=0.55 cfs 0.039 af Discarded=0.05 cfs 0.039 af Primary=0.00 cfs 0.000 af Outflow=0.05 cfs 0.039 af

Total Runoff Area = 5.250 ac Runoff Volume = 1.298 af Average Runoff Depth = 2.97" 76.63% Pervious = 4.023 ac 23.37% Impervious = 1.227 ac

APPENDIX III

Charts, Graphs, and Calculations

Groundwater Recharge Volume (GRV) Calculation

0.54	ac	Area of HSG A soil that was replaced by impervious cover	0.40"
0.16	_	Area of HSG B soil that was replaced by impervious cover	0.25"
	_	1 1	
0.07	_ ac	Area of HSG C soil that was replaced by impervious cover	0.10"
	ac	Area of HSG D soil or impervious cover that was replaced by impervious cover	0.0"
0.3-	4 inches	Rd = weighted groundwater recharge depth	
0.2609	ac-in	GRV = AI * Rd	
947	cf	GRV conversion (ac-in x 43,560 sf/ac x 1ft/12")	

Provide calculations below showing that the project meets the groundwater recharge requirements (Env. Wq 1507.04):					
Groundwater recharge is met under a 1-YR storm event (total recharge = 3,920 Cu. Ft.)					
g					

NHDES Alteration of Terrain Last Revised: August 2013

		RIP	RAP C	CALCULA	TIONS			
				se Developm				
Thibodeau Family Rev. trust								
Barrington, NH								
		В		sociates, Pl				
				tsmouth Av	ve			
			Stra	tham, NH				
Rip Rap equations were ob	tained from	the NH S	tormwa	ter Manual				
Rip Rap was sized for the 1	0 year storm	event (4	.64").					
TAILWATER < HALF T	HE Do							
$La = (1.8 \times Q) / Do 3/2 + (7.6)$	x Do) Q		Q = 1	Peak Flow	& Do is Di	ameter of Pip	e	
W = La + 3Do or defined cl		1				Ī		
$d50 = (0.02 \times Q4/3) / (Tw \times Q4/3)$	(Do)	Tw = Ta	ailwater	Depth				
T = Largest stone size of d5		T= Thic		-				
Ü				tone Size (0).25' Min.)			
Culvert or	Tail Water	Dischg.	Dia.	Length of	Width of	Calculated	Actual	Thickness
Catch Basin	(Feet)			Rip Rap	Rip Rap	Rip Rap	Rip Rap	of Apron
(Sta. No.)	Tw	Q	Do	La (feet)	W (feet)	(0.25 Min)	(Feet)	(Feet)
15" HDPE (Pond #1DCP)	0.52	0.84	1.25	9.8	13.6	0.02	0.25	0.56
15" HDPE (Pond #1BP)	0.34	0.48	1.25	9.4	13.1	0.02	0.25	0.56
End of curb (STA 3+00)	0.40	0.49	2.00	14.3	20.3	0.01	0.25	0.56
End of ball (STITS : 00)	0.10	0.12	2.00	1110	2010	5,01	0,20	0.00
Table 7-24 Recommended Rip			2	To all a	0.5	E4		Torahaa
d50 Size =	0.25	Feet	f Stone (1	Inches	0.5	Feet	6 e of Stone (Inc	Inches
% of Weight Smaller Than the Given d50 Size			i Sione (1			From	on Stolle (Inc	To
100%		From 5		То 6		9		12
85%		4		5		8		11
50%		3		5		6		9
15%		1		2		2		3
13%		1		2		2		3

John P. Hayes III CSS, CWS, 7 Limestone Way North Hampton, NH 03862 603-205-4396 johnphayes@comcast.net

2/15/22

Paul Tibodeau 76 Young Road Barrington, NH Job # 22-002

2/9/22 Site Specific Soil Survey Map 234 Lot 77 Route 9 Barrington, NH

Dear Paul,

This letter report presents the findings of a Site Specific Soil Survey conducted on the referenced properties by John P. Hayes III on February 9, 2022. The soil survey was conducted in accordance with the New Hampshire Supplement of the Site-Specific Soil Mapping Standard For New Hampshire and Vermont, Version 5.0, December 2017, Special Publication #3, published by the Society of Soil Scientist of Northern New England.

The properties that are subject of the soil survey is located northeast side of Route 9, and southeast of Oak Hill Road in Barrington, NH. The mapped area of the pacel is 3.42 acres in size. The plans used for these soil maps are a 40 scale plan, where 1 inch equals 40 feet, with two foot contours.

The purpose of the soil survey is to provide the client with soils information for urban and suburban or rural land planning. Soil characteristics on the property were evaluated through observation of numerous hand auger probes conducted throughout the property. Slope phases were determined with the use of the topography provided on the plan. The Site-specific Soil Map Units identified are taken from the New Hampshire State-Wide Numerical Soils Legend, Issue #10 January 2011, and are briefly described below. Official Series Descriptions (OSD) for each of these soil series are enclosed with this report. The soil map units comply with the Range In Characteristics described in the OSD. Any limiting enclusions on the site, do not exceed 15 percent of any of the soil map units. Dissimilar inclusions, if any, will be noted in the report. Limits of the Site Specific mapping units are highlighted on the plan. The Hydrological Soil Groups for each of the soil series was determined using SSSNNE Publication No. 5 Ksat Values for New Hampshire Soils September 2009. Limits of the Site Specific mapping units are highlighted on the plan.

The Strafford County Soil Survey recognises the soil series Saugatuck as both somewhat poorly drained and poorly drained. The soil map units of the Saugatuck soil with the map unit denominator P, contain the poorly drained component of the soil series.

MAP UNIT#	SOIL TAXANOMIC NAME	SLOPES	HYDRO LOGIC SOIL GROUP	DESCRIPTION
De (State No. 313)	Deerfield	ABC	В	The Deerfield series consists of very deep, moderately well drained soils formed in glaciofluvial deposits. These soils are found on the southeast portion of the lot, at the toe of the slope. These soils are deep to bedrock. Saturated hydraulic conductivity is high or very high. Some inclusions of somewhat poorly drained Saugatuck, and somewhat poorly drained Deerfield Variant soils may be present, but are less than 10 percent of the mapped areas. Estimated seasonal high water tables in these soils range from 16 to 36 inches.
Gs (State No. 111)	Gloucester (Very Stony)	BCDE	A	The Gloucester series consists of very deep, somewhat excessively drained soils formed in sandy till. These soils are found on the hill on the northeastern portion of the lot These soils are deep to bedrock. Saturated hydraulic conductivity is high or very high. Some inclusions of well drained Charlton soils, and moderately well drained Deerfield soils may be present, but are less than 10 percent of the mapped areas. Estimated seasonal high water tables in these soils range from 38 to 60 inches.
Sb (State No. 16)	Saugatuck (Somewhat Poorly Drained)	A	C	The Saugatuck series consists of very deep, somewhat poorly drained soils with cemented subsoil. These soils formed in sandy glaciofluvial deposits on lake plains, till plains, and outwash plains. These soils are found in the southeast and northwest portion of the property, and the front portion of the property, near the road. These soils are deep to bedrock. Estimated seasonal high water tables is 10 to 14 inches.
Sb P (State No. 16)	Saugatuck (Poorly Drained)	A	8	The Saugatuck series consists of very deep, poorly drained soils with cemented subsoil. These soils formed in sandy glaciofluvial deposits on lake plains, till plains, and outwash plains. These soils are found in the southwest side of the lot and the front portion of the property, near the road These soils are deep to bedrock. Estimated seasonal high water tables is less than 10 inches.

Slope Phases

Alpha Slope Symbol	Range
A	0-3%
В	3-8%
C	8-15%
D	15-25%
E	25 - 50%
F	> 50%

I trust that this Soil Survey and report meet your current planning needs. Please do not hesitate to contact me if you have any questions.

Sincerely:

John P. Hagn III

JOHN P.
HAYES MI

John P. Hayes III CSS, CWS

cemented (ortstein)

sandy

gravelly sandy loam in Cd single grain in C fine sandy loam in Cd silt loam, platy in C cemented (ortstein) channery sift loam in Cd channery silt loam in Cd thin strata silty clay loam loamy over sand/grave loamy over loamy sand 20 to 40 in. deep loamy over loamy sand loamy over gravetly cobbly loamy sand less than 20 in. deep single grain, loose strata of fine sand deep organic organic over silt silty clay fine sandy loam 20 to 40 in. deep loamy over sandy organic over sand 20 to 40 in. deep organic over clay fine sandy loam loamy cap silty day loam gravelly surface single grain in C deep organic very channery loam in Cd mwd to swpo sandy loam loam in Cd loam in Cd silt loam Other Spodosol **왕** 은 은 Ses 욛 frigid co. loamy over sandy (skeletal) frigid loamy frigid sandy-skeletal loamy over sandy-skeletal sandy or sandy-skeletal loamy over clayey barry over clayey loamy over sandy loamy over sandy oamy over sandy loamy over sandy sandy over loamy silty
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sandy-skeletal Soil Textures sandy-skeletal loamy-skeletal sandy-skeletal coarse sand sapric loamy fine sapric sandy loamy loamy loamy loamy loamy oamy loamy loamy sandy loamy loamy frigid frigid frigid frigid frigid mesic cryic frigid mesic frigid frigid frigid frigid mesic mesic mesic mesic Temp. mesic frigid frigid frigid mesic mesic frigid frigid frigid frigid mesic mesic frigid frigid frigid frigid frigid Friable till, slity, schist & philite
Firm, platy, sandy till
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Firm, platy, loany till
Loose till, loamy textures
Firm, platy, silty till, schist & phyllite
Loose till, loamy textures Firm, platy, silly till, schist & phylitte
Sitt and Clay Deposits
Firm, platy, silty till, schist & phyllite
Outwash and Stream Terraces
Weathered Bedrock Till
Firm, platy, loamy till
Loose till, sandy textures
Friable till, sity, schist & phyllite
Organic Materials - Freshwater Firm, platy, sifty till, schist & phyllite Firm, platy, sifty till, schist & phyllite Firm, plary, loamy till
Outwash and Stream Terraces
Outwash and Stream Terraces
Outwash and Stream Terraces
Terraces and glacial lake plains Friable till, sitry, schist & phyllite Sandy/loamy over sill/clay Sandy/loamy over silt/clay Outwash and Stream Terraces Terraces and glacial lake plains Outwash and Stream Terraces Organic Materials - Freshwater Organic Materials - Freshwater Flood Plain (Bottom Land) Outwash and Stream Terraces Friable till, sifty, schist & phyllite Outwash and Stream Terraces Friable till, sitty, schist & phyllite Outwash and Stream Terraces Sandy/loamy over silt/clay Flood Plain (Bottom Land) Loose till, loamy textures Loose till, sandy textures Silt and Clay Deposits Silt and Clay Deposits Firm, platy, loamy till Firm, platy, loamy till Loose till, bedrock Loose till, bedrock and Form Group Hyd. Grb ΑĐ m m m m ω Ksat high - C 1 100.0 1 1 20.00 20.00 20.00 20.00 20.00 힏 100.0 20.0 20.0 20.0 20.0 20.0 000 0.2 0.6 0.0 0.2 0.5 0.2 Ksat low - C 20.00 20.06 0.00 0.06 0.06 20.00 20.00 20.00 0. 0.60 0.00 Ksat high - B 20.0 20.0 20.0 20.0 20.0 2.0 602 200200 Ksat low - B ij 0.00 0. 0 0 0 0 0 0 number legend 501 36 36 24 516 516 132 313 378 378 366 366 38 128 128 338 613 Colton, gravelly Bernardston Boscawen Boxford Brayton Buckland Binghamville Chesuncook Chaffield Var Soil Series Bucksport
Burnham
Buxton
Cabot Berkshire Canterbury Cardigan Catden Charles Charlton Chatfield Chichester Chocorua Dixmont
Duane
Dutchess
Eldridge Elmwood Belgrade Biddeford Champlain Dartmouth Elliottsville Agawam Allagash Au Gres Bangor Becket Elmridge Canaan Croghan Acton Adams Deerfield Dixfield Bernis Canton Cohas Colonel Colton Bice

SSSNNE Special Publication No. 5

September, 2009

SSSNNE special pub no. 5 Sorted by Soil Series K set B and C horizons

sandy or sandy-skeletal loamy sand in Cd 20 to 40 in. deep gravelly loamy sand in C strata of fine sand, occ flooded 20 to 40 in. deep sandy or sandy-skeletal sandy or sandy-skeletal very fine sandy loam fine sandy loam in Cd 20 to 40 in. deep loamy cap less than 20 in. deep loamy over sand/gravel loamy sand in Cd channery silt loam in Cd silt loam to silt in C less than 20 in. deep less than 20 in. deep cobbly fine sandy loam strata sand/gravel in C very fine sandy loam less than 20 in. deep very fine sandy loam less than 20 in. deep fine sandy loam in Co less than 20 in. deep loamy sand in Cd silt loam, platy in Cd siffy clay loam in C loamy cap strata of fine sand organic over sand loamy over sandy loamy sand in Cd slate, loamy cap slate, loamy cap organic over silt very channery deep organic deep organic deep organic silt over clay loamy cap Spodosol Se Se yes 5 8 일일 일 yes yes 22222222 yes S 5 5 co. loamy over sandy (skeletal oamy over sandy, sandy-sketet loamy sandy or sandy-skeletal loamy over sandy loamy sandy-skeletal sandy-skeletal loamy over sandy loamy over clayey gravelly sand loamy over sandy loamy sandy-skeletal loamy over sandy Soil Textures sandy-skeletal loamv-skeletal sandy-skeletal hemic/sapric loamy loamy silty soamy silty silty loamy silty loamy hemic loamy loamy loamy loamy loamy sandy loamy sandy loamy loamy loamy Іоашу fine fine peat silty sitty silty silty silty frigid frigid frigid frigid mesic mesic mesic frigid frigid mesic mesic mesic mesic frigid mesic mesic frigid frigid frigid frigid mesic frigid frigid Temp. frigid mesic mesic mesic mesic frigid frigid frigid frigid mesic mesic mesic frigid frigid frigid frigid frigid frigid Outwash and Stream Terraces
Firm, platy, silty till, schist & phyllite
Loose till, loamy textures
Flood Plain (Bottom Land)
Flood Plain (Bottom Land)
Weathered bedrock, phyllite
Flood Plain (Bottom Land) Flood Plain (Bottom Land)
Flood Plain (Bottom Land)
Terraces and glacial lake plains
Outwash and Stream Terraces
Firm, platy, sandy till
Sandy TIII Loose till, loamy textures Firm, platy, sifty till, schist & phyllite Firm, plety, loamy till
Outwash and Stream Terraces
Outwash and Stream Terraces
Tidal Flat Firm, platy, silty till, schist & phylitie
Friable till, silty, schist & phyllite
Firm, platy, sandy till
Loose till, sandy textures Organic Materials - Freshwater Outwash and Stream Terraces Friable till, sitty, schist & phyllite Outwash and Stream Terraces Outwash and Stream Terraces Terraces and glacial lake plains Friable till, sifty, schist & phyllite Outwash and Stream Terraces Outwash and Stream Terraces Outwash and Stream Terraces Outwash and Stream Terraces Friable till, sifty, schist & phyllite Silt and Clay Deposits Organic Materials - Freshwater Flood Plain (Bottom Land) Outwash and Stream Terraces Flood Plain (Bottom Land) Flood Plain (Bottom Land) Loose till, sandy textures Sandy/loamy over silt/clay Loose till, sandy textures Firm, platy, loamy till Loose till, bedrock Firm, platy, sandy till Firm, platy, sandy till Loose till, bedrock Loose till, bedrock Loose till, bedrock Loose till, bedrock and Form Group Hyd. Q/Ω 8 0 Ş **⊲**|മ| Ksat high - C 200 200 200 200 200 200 200 200 200 00 8.0 20.0 20.0 20.0 20.0 0.0000 ö Ksat low 20.00 20.00 20.00 20.00 20.00 0.0 0.06 0.00 0.00 Ksat high - B 6.0 2.0 0.0 0.0 20.0 20.0 20.0 6.0 0.2 Ksat low - B 0.6 0.0 0.6 0.0000 9.0 0.0 0.000 0.6 0.6 0.6 208 671 8 8 8 8 8 8 3108 410 46 55 130 130 130 91 91 91 510 510 339 359 359 514 514 88 Hitchcock
Hogback
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Houghtonville adawaska, aque Grange Greenwood Groveton Meadowsedge Kearsarge Macchias Metacomet Metallak Millis Moosilauke Soil Series Glimanton Glebe Gloucester Glover Lanesboro Monadnock Hadley Hartland Marlow Masardis Haven Hinckley Leicester Matunuck Medomak Mashpee Henniker Ipswich Kinsman Lombard Lovewell Lyman Lyme Monarda Monson Hadley Hermon Limerick Melrose Millsite Montauk Maybid 트

SSSNNE Special Publication No. 5 September, 2009



John P. Hayes III CSS, CWS 7 Limestone Way North Hampton, NH 03862 Phone:603-205-4396 johnphayes@comcast.net

Job# 22-002

Test Pit Logs Map 234 Lot 77 Route 9 Barrington NH

Test Pit 1

		Test T tr T		
Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-5	10YR 3/3 Dark Brown	Fine Sandy Loam	Granular	Friable
5-14	7.5YR 5/6 Strong Brown	Fine Sandy Loam	Granular	Friable
14-22	10YR 5/6 Yellowish Brown	Fine Sandy Loam	Granular	Friable
22-38	2.5Y 5/4 Light Olive Brown	Gravelly Sandy Loam	Massive	Friable
38-68	2.5Y 5/2 Grayish Brown	Gravelly Sandy Loam with Redoximorphic features present	Massive	Friable

ESHWT: 38 in. Restrictive Layer: None Observed H2O: None Refusal: None

Test Pit 2

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-5	10YR 3/3 Dark Brown	Fine Sandy Loam	Granular	Friable
5-16	7.5YR 5/6 Strong Brown	Fine Sandy Loam	Granular	Friable
16-30	10YR 5/6 Yellowish Brown	Fine Sandy Loam	Granular	Friable
30-40	2.5Y 5/4 Light Olive Brown	Gravelly Sandy Loam	Massive	Friable
40-68	2.5Y 5/2 Grayish Brown	Gravelly Sandy Loam with Redoximorphic features present	Massive	Friable

ESHWT: 40 in. Restrictive Layer: None Observed H2O: None Refusal: None



John P. Hayes III CSS, CWS 7 Limestone Way North Hampton, NH 03862 Phone: 603-205-4396 johnphayes@comcast.net

Job# 22-002

Test Pit Logs Map 234 Lot 77 Route 9 Barrington NH

Test Pit 3

		A D D T T T T T		
Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-5	10YR 3/3 Dark Brown	Fine Sandy Loam	Granular	Friable
5-16	7.5YR 5/6 Strong Brown	Fine Sandy Loam	Granular	Friable
16-32	10YR 5/6 Yellowish Brown	Fine Sandy Loam	Granular	Friable
32-42	2.5Y 5/4 Light Olive Brown	Gravelly Sandy Loam	Massive	Friable
42-70	2.5Y 5/2 Grayish Brown	Gravelly Sandy Loam with Redoximorphic features present	Massive	Friable

ESHWT: 42 in. Restrictive Layer: None Observed H2O: None Refusal: None

Test Pit 4

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Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-5	10YR 3/3 Dark Brown	Fine Sandy Loam	Granular	Friable
5-16	7.5YR 5/6 Strong Brown	Fine Sandy Loam	Granular	Friable
16-30	10YR 5/6 Yellowish Brown	Fine Sandy Loam	Granular	Friable
30-42	2.5Y 5/4 Light Olive Brown	Gravelly Sandy Loam	Massive	Friable
42-68	2.5Y 5/2 Grayish Brown	Gravelly Sandy Loam with Redoximorphic features present	Massive	Friable

ESHWT: 42 in. Restrictive Layer: None Observed H2O: None Refusal: None



John P. Hayes HI CSS, CWS 7 Limestone Way North Hampton, NH 03862 Phone: 603-205-4396 johnphayes@comcast.net

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Test Pit Logs Map 234 Lot 77 Route 9 Barrington NH

Test Pit 5

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-5	10YR 3/3 Dark Brown	Fine Sandy Loam	Granular	Friable
5-16	7.5YR 5/6 Strong Brown	Fine Sandy Loam	Granular	Friable
16-32	10YR 5/6 Yellowish Brown	Fine Sandy Loam	Granular	Friable
32-42	2.5Y 5/4 Light Olive Brown	Gravelly Sandy Loam	Massive	Friable
42-68	2.5Y 5/2 Grayish Brown	Gravelly Sandy Loam with Redoximorphic features present	Massive	Friable

ESHWT: 42 in. Restrictive Layer: None Observed H2O: None Refusal: None

Test Pit 6

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-5	10YR 3/3 Dark Brown	Fine Sandy Loam	Granular	Friable
.5-16	10YR 5/6 Yellowish Brown	Fine Sandy Loam	Granular *	Friable
16-30	2.5Y 5/4 Light Olive Brown	Gravelly Fine Sandy Loam	Massive	Friable
30-64	2.5Y 5/2 Grayish Brown	Gravelly Fine Sandy Loam with Redoximorphic features present	Massive	Firm

ESHWT: 30 in. Restrictive Layer: 30 in. Observed H2O: None Refusal: None

John P. Hayes III CSS, CWS
7 Limestone Way
North Hampton, NH 03862
Phone:603-205-4396
johnphayes@comcast.net



Job# 22-002

Test Pit Logs Map 234 Lot 77 Route 9 Barrington NH

Test Pit 7

Depth (inches)	Color	Textural Classification	Soil Structure	Soil Consistance
0-5	10YR 3/3 Dark Brown	Fine Sandy Loam	Granular	Friable
5-14	10YR 5/6 Yellowish Brown	Fine Sandy Loam	Granular	Friable
14-26	2.5Y 5/4 Light Olive Brown	Gravelly Fine Sandy Loam	Massive	Friable
26-66	2.5Y 5/2 Grayish Brown	Gravelly Fine Sandy Loam with Redoximorphic features present	Massive	Firm

ESHWT: 26 in. Restrictive Layer: 26 in. Observed H2O: None Refusal: None