P.O. Box 1721 - Concord, NH 03302 tel: (603) 731-8500 • fax: (866) 929-6094 • sgp@ pernaw.com

Transportation: Engineering • Planning • Design

## MEMORANDUM

## Ref: 2124A

To: $\quad$ Scott R. Frankiewicz, LLS
New Hampshire Land Consultants, PLLC
From: Stephen G. Pernaw, P.E., PTOE
Subject: Proposed Mixed-Use Development - 2A Tactical, LLC
Barrington, New Hampshire
Date: September 13, 2021

As requested, Pernaw \& Company, Inc. has conducted this "Traffic Evaluation" for the proposed mixed-use development located on the northeast quadrant of the NH125 (Calef Highway) / Bumford Road intersection in Barrington, New Hampshire. This study meets or exceeds the requirements of a "Short Traffic Impact Analysis" analysis, as specified in the Barrington Site Plan Review Regulations. Although a "Full Traffic Impact Analysis" could be required given the gross floor area of the proposed building ( $>10,000 \mathrm{sf}$ ), the trip generation estimates contained herein fall well below the 1,000 (daily) and 100 (peak hour) vehicle-trip guidelines for a full analysis. The purpose of this memorandum is to summarize the results of our trip generation and trip distribution analysis, as well as our research of available traffic count data and sight distance evaluation. To summarize:

Proposed Development - According to the plan entitled "Site Layout Plan" prepared by N. H. Land Consultants (see Attachment 1), the proposed development involves the construction of one two-story commercial building ( $6,000 \mathrm{sf}$ ) that will be comprised of a $4,120-\mathrm{sf}$ office/classroom space, 3,376-sf of warehouse area, 2,624-sf of retail and 960 -sf of light industrial space for gunsmithing. Vehicular access to the site is proposed via one two-way driveway that will intersect the west side of Bumford Road approximately 125 -feet east of NH125, and a right-turn exit-only driveway located at the north end of the site on NH125.

Also proposed, at the rear of the site is a 3,572 -sf concrete pad with the capacity for eight storage trailers. Access to the concrete pad is proposed via a 10 -foot-wide paved access from Bumford Road and an internal gravel access from the rear area of the proposed building. The storage trailers will be used for the storage of pallets and accessories.

The proposed building will be open Monday through Thursday-(9:00 AM to 7:30 PM) and Friday and Saturday-(9:00 AM to 8:30 PM). Employees will be available for the sale of small firearms, ammunition and firearm repair services and accessories. Monthly gun safety courses will also be provided on Sunday from 10:00 AM to 4:00 PM with a maximum enrollment of ten people. The business will be closed during the gun safety courses.

Figure 1 shows the location of the subject site with respect to the area roadway system, and it also shows the location of the nearby NHDOT short-term automatic traffic recorder count stations on NH125.

Existing Conditions - NH125 is a state-maintained two-lane principal arterial highway with a north-south orientation in the study area. The pavement width is delineated with a four-inch double yellow centerline and four-inch single white edge lines and has paved shoulders on both sides of the highway. The speed limit is posted at 50 mph in each direction in this area. NH125 exhibits a slight downgrade of less than one-percent in the northbound direction.

Existing Traffic Volumes - Research at the NHDOT revealed that there are two short-term Automatic Traffic Recorder count stations located on NH125: 1) approximately 1.1 miles to the north (south of NH9) and, 2) approximately 2.1 miles to the south (north of Pinkham Road). According to the NHDOT reports, the section of NH125 north of the site carried an Annual Average Daily Traffic (AADT) volume of approximately 17,095 vehicles per day (vpd) in 2019, up slightly from $16,892 \mathrm{vpd}$ in 2018. The section south of the site carried AADT volumes of approximately 14,690 vpd in 2019, up slightly from 14,626 vpd in 2018 (see Attachments 2 -5). Interesting to note, the more recent traffic count data collected in July 2020 clearly shows the impact of the Covid-19 pandemic is waning.

This data demonstrates that weekday traffic volumes in the area typically reach peak levels from 7:00 to 8:00 AM and from 3:00 to 4:00 or 4:00 to 5:00 PM on NH125, thus corresponding to the typical commuter periods. The diagrams on Page 3 summarize the daily and hourly variations in traffic demand along NH125, north and south of the subject site.

$\square=$ AUTOMATIC TRAFFIC RECORDER LOCATION (NHDOT)
= INTERSECTION TURNING MOVEMENT COUNT LOCATION

## Site Location

Figure 1

DAILY TRAFFIC VARIATIONS



Trip Generation - To estimate the quantity of vehicle-trips that will be generated by the proposed commercial building, Pernaw \& Company, Inc. considered several standard trip generation rates and equations published by the Institute of Transportation Engineers (ITE) ${ }^{1}$. Land Use Code 820 (Shopping Center), LUC 150 (Warehousing), LUC 715 (Single Tenant Office Building) and LUC 110 (General Light Industrial) are considered to be reasonable land use categories for each individual use in preparing Estimate A. The Estimate B trips were derived using the trip rates for LUC 861 (Sporting Goods Superstore). The gross floor area (square footage) was used as the independent variable in these cases.

The trip generation analysis is summarized on Table 1 and shows that the proposed mixed-use development is expected to generate approximately 11 vehicle-trips ( 9 arrivals, 2 departures) during the weekday AM peak hour period, and approximately 22 vehicle-trips ( 11 arrivals, 11 departures) during the PM peak hour period, on an average weekday basis (see Attachment 6 \& 7).

## Trip Generation Summary

|  | Estimate A |  |  |  |  | Estimate B |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \text { Retail }^{1} \\ (2,624 \mathrm{sf}) \\ \hline \end{gathered}$ | $\begin{gathered} \text { Warehouse }^{2} \\ (3,376 \mathrm{sf}) \\ \hline \end{gathered}$ | $\begin{gathered} \text { Office }{ }^{3} \\ (4,120 \mathrm{sf}) \\ \hline \end{gathered}$ | $\begin{gathered} \text { Gunsmith }^{4} \\ (960 \mathrm{sf}) \\ \hline \end{gathered}$ | Sub Total | $\begin{gathered} \text { Sports } \\ \text { Store }^{5} \\ (11,080 \mathrm{sf}) \\ \hline \end{gathered}$ |
| Weekday Total (24 hrs.) |  |  |  |  |  |  |
| Entering | 50 veh | 3 veh | 23 veh | 3 veh | 79 trips | 160 trips |
| Exiting | $\underline{50}$ veh | $\underline{3}$ veh | $\underline{23}$ veh | $\underline{3}$ veh | 79 trips | 160 trips |
| Total | 100 trips | 6 trips | 46 trips | 6 trips | 158 trips | 320 trips |
| AM Peak Hour |  |  |  |  |  |  |
| Entering | 1 veh | 1 veh | 6 veh | 1 veh | 9 trips | 3 trips |
| Exiting | $\frac{1}{1}$ veh | $\underline{0}$ veh | $\frac{1}{7} \frac{\mathrm{veh}}{}$ | $\underline{0}$ veh | $\underline{2}$ trips | 1 trips |
| Total | 2 trips | 1 trips | 7 trips | 1 trips | 11 trips | 4 trips |
| PM Peak Hour |  |  |  |  |  |  |
| Entering | 5 veh | 0 veh | 1 veh | 0 veh | 6 trips | 11 trips |
| Exiting | $\underline{5}$ veh | 1 veh | $\underline{6}$ veh | 1 veh | 13 trips | 11 trips |
| Total | 10 trips | 1 trips | 7 trips | 1 trips | 19 trips | 22 trips |

${ }^{1}$ ITE Land Use Code 820 - Shopping Center (2,624 sf) (Trip Rate M ethod)
${ }^{2}$ ITE Land Use Code 150 - Warehousing (3,376 sf) (Trip Rate M ethod)
${ }^{3}$ ITE Land Use Code 715 - Single Tenant Office Building (4, 20 sf) (Trip Rate M ethod)
${ }^{4}$ ITE Land Use Code 110 - General Light Industrial ( 0.960 sf) (Trip Rate M ethod)
${ }^{5}$ ITE Land Use Code 861-Sporting Goods Superstore (11,080 sf) (Trip Rate M ethod)
Trip Distribution - This analysis was based on commuting pattern data from the latest census and our local knowledge of the study area. The results indicate that the majority (approximately $70 \%$ ) of site traffic will travel to/from points north on NH125 (see Attachment 8). Figure 2 shows the distribution of site traffic during the worst-case PM peak hour period.

[^0]

PM PEAK HOUR

Sight Distance - Sight distance at intersections is an important safety consideration. The operator of a vehicle approaching an intersection should have an unobstructed view of the intersection and sufficient length of roadway to enable a full stop, should it be required to avoid a collision. Similarly, exiting vehicles from a minor approach (proposed site driveway and from Bumford Road) should have sufficient visibility of approaching traffic in order to safely enter the traffic flow on to the major street (NH125).

With routine trimming/maintenance of roadside vegetation (and snowbanks) within the highway right-of-way, the required stopping sight distances for the post speed limit ( $50 \mathrm{mph}=425$ feet) and a reasonable design speed ( $60 \mathrm{mph}=570$ feet) are achievable given the favorable horizontal and vertical alignment features of the highway. Attachments $9 \& 10$ include photographs showing the driver's view from the Bumford Road and Proposed Site Driveway approaches to NH125.

## Findings \& Conclusions:

1. Access to the subject site is proposed via one two-way driveway that will intersect the west side of Bumford Road (approximately 125 -feet east of NH125) and an exit-only driveway on the east side of NH125 (north of Bumford Road).
2. According to data collected at the NHDOT count station that is located approximately 1.1 miles north of the proposed site, this section of NH125 carried an AADT volume of approximately 17,095 vehicles per day in 2019. Another count station on NH125, located 2.1-miles to the south of the site, carried an AADT volume of 14,690 vpd in 2019. The highest hourly traffic volumes occurred from 3:00 to 4:00 or 4:00 to 5:00 PM on weekdays.
3. The trip generation analyses indicates that the proposed commercial building will generate approximately 11 vehicle-trips during the AM peak hour ( 9 arrivals, 2 departures) and 19 vehicle-trips during the PM peak hour ( 6 arrivals, 13 departures) when fully operational. These levels are well below the 100 hourly trip level for a "Full" analysis.
4. Site traffic is expected to be distributed approximately $70 \%$ to/from the north and $20 \%$ to/from the south; based on the analysis of the commuting pattern data and our local knowledge of the study area.
5. Based on the trip generation estimates contained herein, the subject driveways will function adequately with one shared departure lane on its approach to Bumford Road and NH125, and these approaches should operate under STOP sign control (MUTCD \#R1-1).
6. Development sites that generate fewer than 500 vehicle-trips per day are generally considered to be "low" traffic generators. Based on the daily estimate of $156-320$ vehicletrips per day (see Table 1), the proposed mixed-use development is not considered to be a major traffic generator.
7. This section of NH125 exhibits a straight horizontal alignment and the vertical profile is essentially flat. This means that with the appropriate clearing of roadside vegetation on both sides of the proposed site driveway, the stopping sight distances requirements for the posted speed limit ( 50 mph ), a reasonable design speed ( 60 mph ), and the NHDOT 400-foot guideline are easily achievable.
8. This section of NH125 is under the jurisdiction of NHDOT District 6, and their approval to construct the site driveway on NH125 is required through the Driveway Permit system. The proposed site driveway on Bumford Road requires updating any Driveway Permit that was previously issued to the Town of Barrington.

Attachments


Stephen G. Pernaw \& Company, Inc.

ATTACHMENTS


## 2MS2

Transportation Data Management System


Directions: 2-WAY NB SB
AADT 3

| Year | AADT | DHV-30 | K \% | D \% | PA | BC | Src |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2020 | 18,845 | 2,314 | 12 | 58 | $17,149(91 \%)$ | $1,696(9 \%)$ |  |  |
| 2019 | $17,095^{3}$ |  | 11 | 58 | $15,658(92 \%)$ | $1,437(8 \%)$ | Grown <br> from 2018 <br> 2018 | $16,892^{3}$ |
| 2017 | 16,561 | 1,740 | 11 | 58 | $15,367(93 \%)$ | $1,194(7 \%)$ |  |  |
| 2016 | $14,708^{3}$ |  |  |  |  |  |  |  |
| Grown |  |  |  |  |  |  |  |  |
| from 2017 |  |  |  |  |  |  |  |  |



| Travel Demand Model |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Model <br> Year | Model <br> AADT | AM PHV | AM PPV | MD PHV | MD PPV | PM PHV | PM PPV | NT PHV | NT PPV |


| VOLUME COUNT |  |  |  |
| :---: | :---: | :---: | :---: |
|  | Date | Int | Total |
| 4 | Thu 7/9/2020 | 60 | 23,454 |
| th | Wed 7/8/2020 | 60 | 19,969 |
| 4 | Tue 7/7/2020 | 60 | 21,595 |
| 04 | Thu 5/11/2017 | 60 | 19,169 |
| T | Wed 5/10/2017 | 60 | 19,175 |
| 48 | Tue 5/9/2017 | 60 | 18,398 |
| \% | Thu 7/10/2014 | 60 | 17,953 |
| +6 | Wed 7/9/2014 | 60 | 15,959 |
| +6 | Tue 7/8/2014 | 60 | 16,136 |
| 4 | Thu 6/16/2011 | 60 | 17,358 |
|  | 『1「.1 |  | $\underline{\square}$ |


| VOLUME TREND |  |
| :---: | :---: |
| Year |  |
| 2020 | $10 \%$ |
| 2019 | $1 \%$ |
| 2018 | $2 \%$ |
| 2017 | $13 \%$ |
| 2016 | $2 \%$ |
| 2015 | $3 \%$ |
| 2014 | $0 \%$ |
| 2011 | $0 \%$ |
| 2008 | $-2 \%$ |
| 2005 | $-6 \%$ |

Excel Version

| Weekly Volume Report |  |  |  |
| ---: | :--- | ---: | :--- |
| Location ID: | 82027055 | Type: | SPOT |
| Located On: | Calef Hwy | $:$ |  |
| Direction: | 2-WAY |  |  |
| Community: | BARRINGTON | Period: | Mon $7 / 6 / 2020-$ Sun $7 / 12 / 2020$ |
| AADT: | 18845 |  |  |


| Start Time | Mon | Tue | Wed | Thu | Fri | Sat | Sun | Avg | Graph |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM |  | 53 | 60 | 71 |  |  |  | 61 |  | 0.3\% |
| 1:00 AM |  | 30 | 51 | 41 |  |  |  | 41 |  | 0.2\% |
| 2:00 AM |  | 51 | 41 | 51 |  |  |  | 48 |  | 0.2\% |
| 3:00 AM |  | 87 | 71 | 72 |  |  |  | 77 |  | 0.4\% |
| 4:00 AM |  | 188 | 180 | 209 |  |  |  | 192 | - | 0.9\% |
| 5:00 AM |  | 522 | 532 | 538 |  |  |  | 531 | $\square$ | 2.4\% |
| 6:00 AM |  | 1027 | 948 | 1038 |  |  |  | 1,004 | $\square$ | 4.6\% |
| 7:00 AM |  | 1223 | (1241) | 1284 |  |  |  | 1,249 | $\square$ | 5.8\% |
| 8:00 AM |  | 1072 | 1120 | 1124 |  |  |  | 1,105 | $\square$ | 5.1\% |
| 9:00 AM |  | 1005 | 1063 | 1122 |  |  |  | 1,063 | $\square$ | 4.9\% |
| 10:00 AM |  | 1194 | 1180 | 1279 |  |  |  | 1,218 | $\square$ | 5.6\% |
| 11:00 AM |  | 1312 | 1244 | 1548 |  |  |  | 1,368 | $\square$ | 6.3\% |
| 12:00 PM |  | 1439 | 1390 | 1682 |  |  |  | 1,504 | $\square$ | 6.9\% |
| 1:00 PM |  | 1642 | 1525 | 1721 |  |  |  | 1,629 | $\square$ | 7.5\% |
| 2:00 PM |  | 1840 | 1661 | 1910 |  |  |  | 1,804 | $\square$ | 8.3\% |
| 3:00 PM |  | 2078 | 1896 | 2165 |  |  |  | 2,046 | $\square$ | 9.4\% |
| 4:00 PM |  | 2314 | 1835 | 2203 |  |  |  | 2,117 | $\square$ | 9.8\% |
| 5:00 PM |  | 1756 | 1359 | 1872 |  |  |  | 1,662 | $\square$ | 7.7\% |
| 6:00 PM |  | 993 | 858 | 1196 |  |  |  | 1,016 | $\square$ | 4.7\% |
| 7:00 PM |  | 639 | 597 | 880 |  |  |  | 705 | $\square$ | 3.3\% |
| 8:00 PM |  | 508 | 494 | 639 |  |  |  | 547 | $\square$ | 2.5\% |
| 9:00 PM |  | 301 | 306 | 411 |  |  |  | 339 | - | 1.6\% |
| 10:00 PM |  | 182 | 201 | 255 |  |  |  | 213 | - | 1.0\% |
| 11:00 PM |  | 139 | 116 | 143 |  |  |  | 133 | - | 0.6\% |
| Total | 0 | 21,595 | 19,969 | 23,454 | 0 | 0 | 0 |  |  |  |
| 24hr Total |  | 21595 | 19969 | 23454 |  |  |  | 21,673 |  |  |
| AM Pk Hr |  | 11:00 | 11:00 | 11:00 |  |  |  |  |  |  |
| AM Peak |  | 1312 | 1244 | 1548 |  |  |  | 1,368 |  |  |
| PM Pk Hr |  | 4:00 | 3:00 | 4:00 |  |  |  |  |  |  |
| PM Peak |  | 2314 | 1896 | 2203 |  |  |  | 2,138 |  |  |
| \% Pk Hr |  | 10.72\% | 9.49\% | 9.39\% |  |  |  | 9.87\% |  |  |



AADT $\boldsymbol{*}$

| Year | AADT | DHV-30 | K \% | D \% | PA | BC | Src |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2020 | 13,252 | 1,414 | 11 | 59 | $12,057(91 \%)$ | $1,195(9 \%)$ |  |
| 2019 | 14,690 | 1,491 | 10 | 60 | $13,457(92 \%)$ | $1,233(8 \%)$ |  |
| 2018 | 14,626 | 1,507 | 10 | 57 | $13,485(92 \%)$ | $1,141(8 \%)$ |  |
| 2017 | 14,551 |  |  |  |  |  |  |
| 2016 | 14,456 |  |  |  |  |  |  |
| $\ggg \gg 1$ | $1-5$ of 66 |  |  |  |  |  |  |


| Travel Demand Model |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | ModeI <br> Year | Model <br> AADT | AM PHV | AM PPV | MD PHV | MD PPV | PM PHV | PM PPV | NT PHV | NT PPV |


| VOLUME COUNT |  |  |  |
| :---: | :---: | :---: | :---: |
|  | Date | Int | Total |
| ** | Sun 6/20/2021 | 60 | 15,524 |
| * | Sat 6/19/2021 | 60 | 16,205 |
| * | Fri 6/18/2021 | 60 | 19,003 |
| * 6 | Thu 6/17/2021 | 60 | 17,312 |
| * | Wed 6/16/2021 | 60 | 16,893 |
| * | Tue 6/15/2021 | 60 | 16,018 |
| * | Mon 6/14/2021 | 60 | 15,210 |
| 也 | Sun 6/13/2021 | 60 | 15,110 |
| * | Sat 6/12/2021 | 60 | 16,422 |
| * | Fri 6/11/2021 | 60 | 17,887 |
|  | $\rightarrow \gg 1$ | $\begin{array}{r} 1-10 \text { of } 9669 \\ \text { Tn nata } \end{array}$ |  |

VOLUME TREND

| Year | Annual Growth |
| :---: | :---: |
| 2020 | $-10 \%$ |
| 2019 | $0 \%$ |
| 2018 | $1 \%$ |
| 2017 | $1 \%$ |
| 2016 | $3 \%$ |
| 2015 | $1 \%$ |
| 2014 | $1 \%$ |
| 2013 | $2 \%$ |
| 2012 | $1 \%$ |
| 2011 | $-2 \%$ |
| $\ll>1$ | $1-10$ of 65 |

Transportation Data Management System

Excel Version

| Weekly Volume Report |  |  |  |
| ---: | :--- | ---: | :--- |
| Location ID: | 02255001 | Type: | SPOT |
| Located On: | Calef Hwy | $:$ |  |
| Direction: | 2-WAY |  |  |
| Community: | BARRINGTON | Period: | Mon 6/14/2021 - Sun 6/20/2021 |
| AADT: |  |  |  |


| Start Time | Mon | Tue | Wed | Thu | Fri | Sat | Sun | Avg | Graph |
| ---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | ---: | ---: |
| 12:00 AM | 42 | 53 | 52 | 53 | 58 | 89 | 95 | 63 | $0.4 \%$ |
| 1:00 AM | 34 | 40 | 39 | 35 | 44 | 49 | 52 | 42 | $0.3 \%$ |
| 2:00 AM | 40 | 54 | 34 | 55 | 44 | 50 | 37 | 45 | $0.3 \%$ |
| 3:00 AM | 65 | 57 | 70 | 61 | 76 | 51 | 29 | 58 | $0.4 \%$ |
| 4:00 AM | 153 | 156 | 147 | 170 | 148 | 84 | 63 | 132 | $0.8 \%$ |
| 5:00 AM | 475 | 448 | 480 | 473 | 419 | 173 | 121 | 370 | $2.2 \%$ |
| 6:00 AM | 987 | 984 | 1024 | 991 | 951 | 351 | 242 | 790 | $4.8 \%$ |
| 7:00 AM | 1231 | 1263 | 1242 | 1162 | 1109 | 612 | 420 | 1,006 | $6.1 \%$ |
| 8:00 AM | 1045 | 1060 | 1061 | 1044 | 1010 | 812 | 657 | 956 | $5.8 \%$ |
| 9:00 AM | 857 | 800 | 931 | 917 | 993 | 1007 | 999 | 929 | $5.6 \%$ |
| 10:00 AM | 834 | 843 | 831 | 931 | 1022 | 1152 | 1235 | 978 |  |
| 11:00 AM | 871 | 907 | 957 | 923 | 1112 | 1358 | 1342 | 1,067 | $5.9 \%$ |
| 12:00 PM | 850 | 935 | 952 | 1049 | 1201 | 1302 | 1334 | 1,089 | $6.4 \%$ |
| 1:00 PM | 899 | 952 | 976 | 1028 | 1258 | 1297 | 1202 | 1,087 | $6.6 \%$ |
| 2:00 PM | 1046 | 1097 | 1192 | 1111 | 1378 | 1160 | 1156 | 1,163 |  |
| 3:00 PM | 1243 | 1260 | 1340 | 1416 | 1457 | 1161 | 1118 | 1,285 |  |
| 4:00 PM | 1340 | 1495 | 1500 | 1548 | 1553 | 1130 | 1130 | 1,385 |  |
| 5:00 PM | 1176 | 1288 | 1358 | 1424 | 1438 | 1114 | 1192 | 1,284 |  |
| 6:00 PM | 770 | 806 | 893 | 917 | 1129 | 891 | 998 | 915 |  |
| 7:00 PM | 410 | 521 | 665 | 710 | 883 | 791 | 814 | 685 | $7.0 \%$ |
| 8:00 PM | 327 | 422 | 466 | 555 | 629 | 641 | 573 | 516 |  |
| 9:00 PM | 231 | 288 | 357 | 360 | 501 | 459 | 373 | 367 |  |
| 10:00 PM | 171 | 173 | 187 | 244 | 409 | 288 | 222 | 242 |  |
| 11:00 PM | 113 | 116 | 139 | 135 | 181 | 183 | 120 | 141 |  |
| Total | 15,210 | 16,018 | 16,893 | 17,312 | 19,003 | 16,205 | 15,524 |  |  |
| 24hr Total | 15210 | 16018 | 16893 | 17312 | 19003 | 16205 | 15524 | 16,595 |  |
| AM Pk Hr | $7: 00$ | $7: 00$ | $7: 00$ | $7: 00$ | $11: 00$ | $11: 00$ | $11: 00$ |  |  |
| AM Peak | 1231 | 1263 | 1242 | 1162 | 1112 | 1358 | 1342 | 1,244 |  |
| PM Pk Hr | $4: 00$ | $4: 00$ | $4: 00$ | $4: 00$ | $4: 00$ | $12: 00$ | $12: 00$ |  |  |
| PM Peak | 1340 | 1495 | 1500 | 1548 | 1553 | 1302 | 1334 | 1,439 | $5.5 \%$ |
| \% Pk Hr | $8.81 \%$ | $9.33 \%$ | $8.88 \%$ | $8.94 \%$ | $8.17 \%$ | $8.38 \%$ | $8.64 \%$ | $8.74 \%$ | $4.1 \%$ |

Alternative: Alternative 1
Phase:
Project:

| Phase: <br> Project: 2124A |  |  |  |  |  |  |  | Open Date: $9 / 10 / 2021$ <br> Analysis Date: $9 / 10 / 2021$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Weekday Average Daily Trips |  |  | Weekday AM Peak Hour of Adjacent Street Traffic |  |  |  | Weekday PM Peak Hour of Adjacent Street Traffic |  |  |  |
| ITE Land Use | * Enter | Exit | Total | * | Enter | Exit | Total | * | Enter | Exit | Total |
| $\begin{array}{cc}110 & \text { GINDUSTRIAL } 1 \\ & 0.96 \quad 1000 \text { Sq. Ft. GFA }\end{array}$ | 3 | 2 | 5 |  | 1 | 0 | 1 |  | 0 | 1 | 1 |
| 150 WAREHOUSE 1 $3.38 \quad 1000 \text { Sq. Ft. GFA }$ | 3 | 3 | 6 |  | 1 | 0 | 1 |  | 0 | 1 | 1 |
| 715 OFFICESINGLE 1 <br> 4.12 1000 Sq. Ft. GFA | 23 | 23 | 46 |  |  |  | 0 |  |  |  | 0 |
| 820 CENTERSHOPPING 1 $2.62 \quad 1000$ Sq. Ft. GLA | 50 | 49 | 99 |  | 1 | 1 | 2 |  | 5 | 5 | 10 |
| 861 SUPERSTORESPORTS 1 <br> 11.081000 Sq. Ft. GFA | 160 | 159 | 319 |  | 3 | 1 | 4 |  | 11 | 11 | 22 |
| Unadjusted Volume | 239 | 236 | 475 |  | 6 | 2 | 8 |  | 16 | 18 | 34 |
| Internal Capture Trips | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Pass-By Trips | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 2 | 2 | 4 |
| Volume Added to Adjacent Streets | 239 | 236 | 475 |  | 6 | 2 | 8 |  | 14 | 16 | 30 |

[^1]
## Trip Generation Summary

| Alternative: Alternative 1 |  |  |
| :--- | ---: | :--- |
| Phase: |  | Open Date: $9 / 10 / 2021$ |
| Project: | $2124 A$ | Analysis Date: |


|  | Land Use | Weekday AM Peak Hour of Generator |  |  |  | Weekday PM Peak Hour of Generator |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | * | Enter | Exit | Total | * | Enter | Exit | Total |
| 110 | GINDUSTRIAL 1 |  |  |  | 0 |  |  |  | 0 |
|  | $0.96 \quad 1000$ Sq. Ft. GFA |  |  |  |  |  |  |  |  |
| 150 | WAREHOUSE 1 |  |  |  | 0 |  |  |  | 0 |
|  | 3.38 1000 Sq. Ft. GFA |  |  |  |  |  |  |  |  |
| 715 | OFFICESINGLE 1 |  | 6 | 1 | 7 |  | 1 | 6 | 7 |
|  | 4.12 1000 Sq. Ft. GFA |  |  |  |  |  |  |  |  |
| 820 | CENTERSHOPPING 1 |  |  |  | 0 |  |  |  | 0 |
|  | 2.62 1000 Sq. Ft. GLA |  |  |  |  |  |  |  |  |
| 861 | SUPERSTORESPORTS 1 |  |  |  | 0 |  |  |  | 0 |
|  | 11.081000 Sq. Ft. GFA |  |  |  |  |  |  |  |  |
| Unadjusted Volume |  |  | 6 | 1 | 7 |  | 1 | 6 | 7 |
| Internal Capture Trips |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Pass-By Trips |  |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Volume Added to Adjacent Streets |  |  | 6 | 1 | 7 |  | 1 | 6 | 7 |

Total Weekday AM Peak Hour of Generator Internal Capture $=0$ Percent
Total Weekday PM Peak Hour of Generator Internal Capture $=0$ Percent

Stephen G. Pernaw \& Company, Inc.

## TRIP DISTRIBUTION ANALYSIS

Home Destination Report - Where Workers Live Who are Employed in the Selection Area - by County Subdivisions

|  |  | Gateway \% |  | Gateway Allocation |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | NH125 N | NH125 S | NH125 N | NH125 S |  |
| OUTBOUND | Count |  |  |  |  |  |
| Barrington town (Strafford, NH) | 377 | 0.55 | 0.45 | 207 | 170 | 377 |
| Rochester city (Strafford, NH) | 364 | 1.00 |  | 364 | 0 | 364 |
| Dover city (Strafford, NH) | 169 | 0.50 | 0.50 | 85 | 85 | 170 |
| Farmington town (Strafford, NH) | 94 | 1.00 |  | 94 | 0 | 94 |
| Somersworth city (Strafford, NH) | 86 | 1.00 |  | 86 | 0 | 86 |
| Portsmouth city (Rockingham, NH) | 65 | 0.50 | 0.50 | 33 | 33 | 66 |
| Strafford town (Strafford, NH) | 62 | 1.00 |  | 62 | 0 | 62 |
| Lee town (Strafford, NH) | 48 |  | 1.00 | 0 | 48 | 48 |
| Northwood town (Rockingham, NH ) | 46 |  | 1.00 | 0 | 46 | 46 |
| Milton town (Strafford, NH) | 39 | 1.00 |  | 39 | 0 | 39 |
|  | 1350 |  |  | 970 | 382 | 1352 |
|  |  |  |  | 71.7\% | 28.3\% | 100\% |
|  |  |  |  | 70\% | 30\% | 100\% |

$\xrightarrow[\text { Pernaw \& Company, Inc }]{2}$

## Looking Left



## Looking Right




## Looking Left



## Looking Right




[^0]:    ${ }^{1}$ Institute of Transportation Engineers, Trip Generation, tenth edition (Washington, D.C., 2017).

[^1]:    Total Weekday Average Daily Trips Internal Capture $=0$ Percent
    Total Weekday AM Peak Hour of Adjacent Street Traffic Internal Capture $=0$ Percent Total Weekday PM Peak Hour of Adjacent Street Traffic Internal Capture $=0$ Percent

