

## PLAN SET INDEX

**SHEET 1 - EXISTING CONDITIONS** SHEET 2 - PROPOSED CONDITIONS SHEET 3 - NOTES & DETAILS

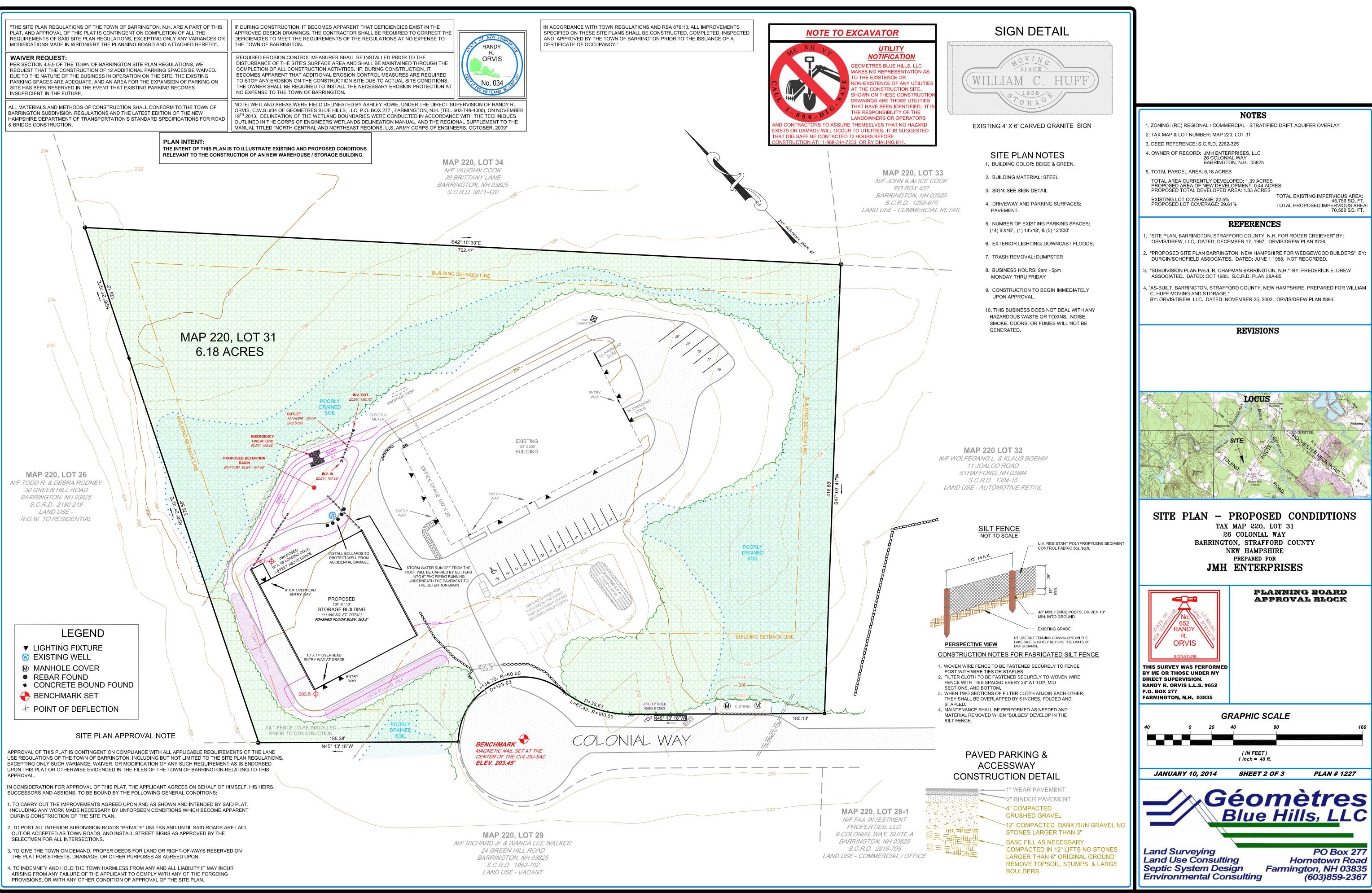
## 1. ZONING: (RC) REGIONAL / COMMERCIAL - STRATIFIED DRIFT AQUIFER OVERLAY TOTAL AREA CURRENTLY DEVELOPED: 1.39 ACRES PROPOSED AREA OF NEW DEVELOPMENT: 0.44 ACRES PROPOSED TOTAL DEVELOPED AREA: 1.83 ACRES TOTAL EXISTING IMPERVIOUS AREA: 45,756 SQ. FT. TOTAL PROPOSED IMPERVIOUS AREA 70,568 SQ. FT. "SITE PLAN, BARRINGTON, STRAFFORD COUNTY, N.H. FOR ROGER CREIEVER" BY: ORVIS/DREW, LLC. DATED: DECEMBER 17, 1997. ORVIS/DREW PLAN #726. "PROPOSED SITE PLAN BARRINGTON, NEW HAMPSHIRE FOR WEDGEWOOD BUILDERS" BY: DURGIN/SCHOFIELD ASSOCIATES. DATED: JUNE 1 1988. NOT RECORDED. "SUBDIVISION PLAN PAUL R. CHAPMAN BARRINGTON, N.H." BY: FREDERICK E. DREW ASSOCIATED. DATED: OCT 1985. S.C.R.D. PLAN 28A-85 . "AS-BUILT, BARRINGTON, STRAFFORD COUNTY, NEW HAMPSHIRE, PREPARED FOR WILLIAM BY: ORVIS/DREW, LLC. DATED: NOVEMBER 25, 2002. ORVIS/DREW PLAN #894. **REVISIONS** SITE PLAN - EXISTING CONDIDTIONS TAX MAP 220, LOT 31 26 COLONIAL WAY BARRINGTON, STRAFFORD COUNTY **NEW HAMPSHIRE** PREPARED FOR JMH ENTERPRISES PLANNING BOARD APPROVAL BLOCK GRAPHIC SCALE

PLAN # 1227

PO Box 277

Hornetown Road

(603)859-2367



PLAN # 1227

PO Box 277

Hornetown Road

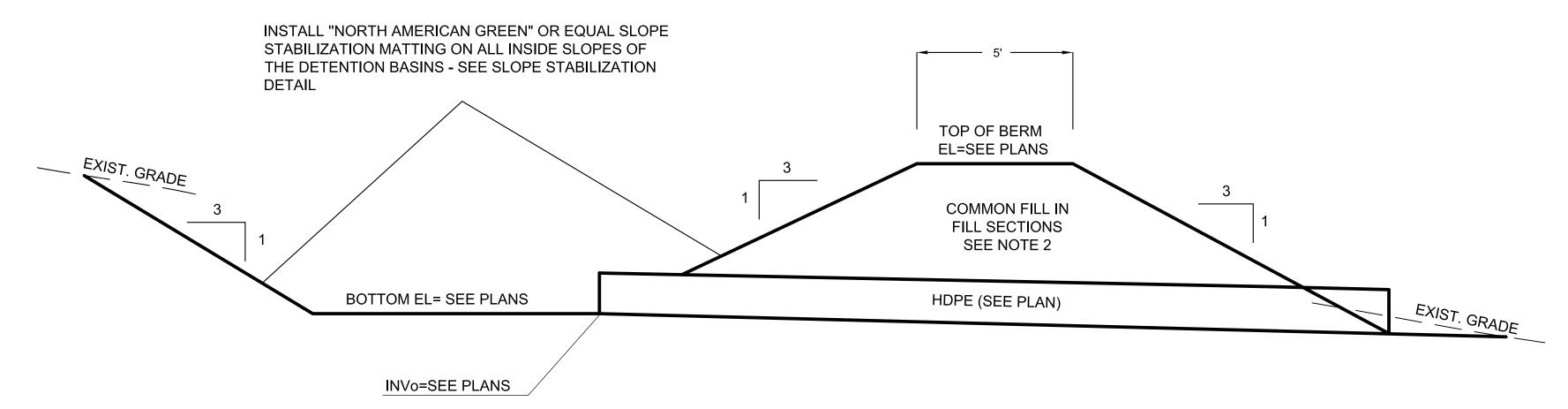
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TOTAL EXISTING IMPERVIOUS AREA:

TOTAL PROPOSED IMPERVIOUS AREA 70,568 SQ. FT.

PLAN - PROPOSED CONDIDTIONS
DETENTION BASIN DETAIL
TAX MAP 220, LOT 31
26 COLONIAL WAY
BARRINGTON, STRAFFORD COUNTY
NEW HAMPSHIRE

ENGINEERING, L JEFFREY BURD, P.E.



## DETENTION BASIN CONSTRUCTION NOTES

- 1. THE FOUNDATION AREA SHALL BE CLEARED OF TREES. LOGS, STUMPS, ROOTS, BRUSH. BOULDERS, SOD AND RUBBISH. SCARIFY SURFACE BEFORE PLACING FILL. THE AREA SHALL BE MOIST FOR GOOD BONDING OF THE NEW FILL. KEEP STANDING WATER FROM FORMING ON OR NEAR THE FILL AREA.
- 2. THE FILL SHALL BE FREE OF DETRIMENTAL AMOUNTS OF SOD, ROOTS, FROZEN SOIL, STONES LARGER THAN 6 INCHES AND OTHER OBJECTIONABLE MATERIAL. CRUSHED GRAVEL (3/4") SHALL BE PLACED AROUND PIPES AND CONCRETE STRUCTURES.
- 3. THE PLACING AND SPREADING OF FILL SHALL BE STARTED AT THE LOWEST POINT IN THE BERM AREA AND BROUGHT UP IN HORIZONTAL LAYERS (LIFTS) OF ABOUT 12" SO THAT REQUIRED COMPACTION CAN BE OBTAINED. THE DISTRIBUTION AND GRADATION OF MATERIALS SHALL BE SUCH THAT NO LENSES, POCKETS, STREAKS OR LAYERS OF MATERIAL DIFFER SUBSTANTIALLY IN TEXTURE OR GRADATION FROM THE SURROUNDING MATERIAL.
- 4. THE MOISTURE CONTENT OF THE FILL MATERIAL SHALL BE ADEQUATE FOR OBTAINING THE REQUIRED COMPACTION.
- 5. CONSTRUCTION EQUIPMENT SHALL BE OPERATED OVER AREAS OR EACH LAYER OF FILL TO INSURE REQUIRED COMPACTION. USE SPECIAL EQUIPMENT IF NECESSARY. FILL ADJACENT TO PIPES AND STRUCTURES SHALL BE COMPACTED BY HAND TAMPING OR PLATE VIBRATOR. FILL ADJACENT TO CONCRETE STRUCTURES SHALL NOT BE COMPACTED UNTIL CONCRETE HAS CURED STRONG ENOUGH TO SUPPORT THE
- 6. FOR PROTECTION ALL EXPOSED AND DISTURBED SURFACES SHALL HAVE A COVER OF VEGETATION, PREFERABLY TOPSOIL AND SEED. FOLLOW SEEDING SPECIFICATIONS AND GENERAL NOTES IN THE EROSION CONTROL DETAILS SECTION IN THIS PLANSET.

### SAFETY

1. PONDS THAT ARE EASILY ACCESSIBLE IN POPULATED AREAS SHOULD INCORPORATE ALL POSSIBLE SAFETY PRECAUTIONS. DUE TO ONLY TEMPORARY WATER LEVELS IN THESE BASINS, FENCING IS NOT NECESSARY.

#### MAINTENANCE

- 1. MAINTENANCE IS NECESSARY IF THE BASIN IS TO CONTINUE TO FUNCTION AS DESIGNED. THE LANDOWNER MUST BE AWARE OF THE REQUIREMENTS FOR A PROPERLY OPERATIONAL BASIN AND A PLAN BE DEVELOPED FOR REGULAR SCHEDULED MAINTENANCE.
- 2. THE EMBANKMENT SHOULD BE INSPECTED TO DETERMINE IF RODENT BURROWS, WET AREAS OR EROSION OF THE FILL IS TAKING PLACE.
- 3. THE VEGETATION SHOULD BE PROTECTED FROM DAMAGE BY FIRE, GRAZING, TRAFFIC AND DENSE WEED GROWTH. LIME AND FERTILIZER SHOULD BE APPLIED AS NECESSARY AS DETERMINED BY SOIL TESTS. TREES AND SHRUBS SHOULD BE KEPT OFF THE EMBANKMENT AND EMERGENCY SPILLWAY AREAS.
- 4. PIPE INLETS AND SPILLWAY STRUCTURES SHOULD BE INSPECTED ANNUALLY AND AFTER EVERY MAJOR STORM. ACCUMULATED DEBRIS AND SEDIMENT SHOULD BE REMOVED. IF PIPES ARE COATED, THE COATING SHOULD BE CHECKED AND REPAIRED AS NECESSARY.
- 5. PIPE OUTLETS SHOULD BE INSPECTED ANNUALLY AND AFTER EVERY MAJOR STORM. THE CONDITION OF THE PIPES SHOULD BE NOTED AND REPAIRS MADE AS NECESSARY. IF EROSION IS TAKING PLACE, THEN MEASURES SHOULD BE TAKEN TO STABILIZE AND PROTECT THE AFFECTED AREA OF THE OUTLET.
- 6. SEDIMENT SHOULD BE CONTINUALLY CHECKED IN THE BASIN. WHEN SEDIMENT ACCUMULATIONS REACH THE PREDETERMINED DESIGN ELEVATION, THEN THE SEDIMENT SHOULD BE REMOVED AND PROPERLY DISPOSED

## PLACED CLASS "C" STONE FILL - BOTTOM OF BASIN

## TYPICAL EMERGENCY SPILLWAY DETAIL

NOT TO SCALE

### DETENTION BASIN DETAIL

SECTION NOT TO SCALE SHEET

3 OF 3

#### **DRAINAGE REPORT**

#### William C. Huff Moving and Storage

Tax Map 220, Lot 31
Non-Residential Site Development
Barrington, NH

January 22, 2014

Prepared for: **Geometres Blue Hills, LLC** PO BOX 277 Farmington, NH 03835

Prepared By: **RJB Engineering, LLC**15 Pleasant Street, Suite 5

Concord, NH 03301

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**USGS Map** 

#### **Appendix:**

Predevelopment computations

10-year pre-development computations – node listing 50-year pre-development computations - summaries

Post-development computations

10-year post-development computations – node listing 50-year post-development computations - summaries

Pre-development Drainage Area Plan

Post-development Drainage Area Plan

#### Introduction

This project is the expansion of an existing moving and storage business on a 6.18 acre property located on Colonial way in Barrington. The property is currently developed with an existing 20,000 s.f. building, access roads, parking, and related site improvements. The property is zoned for commercial purposes.

There is an existing seasonal stream that bisects the property and carries stormwater runoff from the upstream drainage basin and the subject property easterly to NH Route 125 and ultimately into the Isinglass River. The drainage on the existing property is open drainage, basically sheet drainage over the developed area draining over a vegetated buffer and into the adjacent wetland and seasonal stream.

The proposed project consists of constructing a new 11,770 s.f. building and an access driveway surrounding the new building. Drainage runoff from the new improvements will be routed to a new detention basin designed to mitigate peak storm events such that there is no increase is stormwater runoff from the proposed impervious surfaces.

#### **Existing Conditions**

The property is identified on tax map 220 as lot 31, is 6.18 acres in size, and has 382 feet of frontage on Colonial Way. The property is in the Regional / Commercial (RC) zoning district and the Stratified Drift Aquifer Overlay District. The middle portion of the site is developed with an existing 20,000 s.f. building, access roads, parking for 20 vehicles, and related site improvements. The current improvements occupy approximately 2.0 acres of the subject property. The remaining area is unimproved and wooded.

As noted above there is an existing seasonal stream that bisects the property and carries stormwater runoff from the upstream drainage basin and the subject property easterly to NH Route 125 and ultimately into the Isinglass River. The upstream drainage basin is over 35 acres of wooded undeveloped land. There are poorly drained soils surrounding the existing development. The soils in the development area are mapped as Saugatuck loamy sand as taken from the NRCS Web Soil Survey. The northwesterly portion of the property is unimproved and wooded. Slopes on the subject property are fairly flat ranging from 1 to 4%.

The existing site drainage basically sheets off of the proposed development area into the surrounding poorly drained soils. There is a depression at the northeasterly corner of the property that acts naturally to detain runoff from peak storm events and thereby reduce peak flows in the seasonal stream.

#### **Proposed development**

This project is an expansion to the existing moving and storage business. Proposed site improvements include constructing a new 11,770 s.f. building and an access driveway surrounding the new building. The total area of impervious surfaces added to the site as a result of the improvements is approximately 22,850 s.f. The total area to be disturbed during construction is approximately 33,000 s.f. There are no wetland impacts associated with this project.

The proposed drainage system for the new expansion will be an open system with stormwater flows directed to a new detention basin designed to mitigate peak storm events such that there is no increase is stormwater runoff from the proposed impervious surfaces. The calculations demonstrate that there are <u>no</u> increases in peak flows as a result from the development.

#### Design methodology

The drainage analysis in this study was completed using HydroCad Version 10.0, a stormwater modeling program utilizing TR-20 and TR-55 methodology. This program performs both the hydrologic computations for determination of runoff flows, and the hydraulic calculations for pipe, ditch, and pond design. Calculations were performed for 10, and 50-year return frequency storms in accordance with Municipal regulations. The following design parameters were used:

Rainfall distribution: Type III 2-year storm rainfall: 3.00 inches 10-year storm rainfall: 4.30 inches 50-year storm rainfall: 5.65 inches

#### **Design analysis**

Peak runoff flows have been evaluated in this study to insure that postdevelopment flows do not exceed pre-development flows. Mitigation for increased flows has been provided by using a proposed detention basin as previously described. The stormwater runoff has been modeled to a single point in the adjacent seasonal stream on the subject property (see drainage area plans). Pre and post development flows were analyzed at that point of reference. The peak flows are shown in the accompanying table:

Storm event	Pre-development (cfs)	Post-development (cfs)
10-year	24.8	24.7
50-year	42.7	42.5

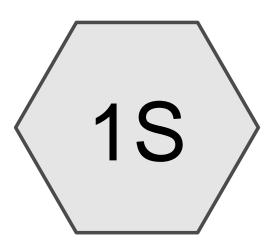
#### **Stormwater Treatment**

Stormwater treatment is provided to reduce pollutants and sediment from discharging into downstream public waters. The design of this site largely relies on the proposed vegetation surrounding the proposed improvements to act as a vegetated buffer strip, consistent with the design of the original site. The detention basin will further provide treatment by capturing sediment and pollutants.

#### **Erosion Control Measures**

This site is very flat and the soils are sandy. There is therefore not a high level of concern for erosion during construction. It is recommended that silt fence be placed downslope of the construction limits adjacent to the poorly drained soils to minimize any possible sediment migration. As the disturbance of this site is less than 50,000 s.f. a Stormwater Pollution Prevention Plan (SWPPP) is not required. The contractor would however be expected to follow reasonable means and methods to minimize any erosion concerns.

Pre-development computations



sub 1









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#### **Area Listing (selected nodes)**

Area	CN	Description
(acres)		(subcatchment-numbers)
0.400	91	Urban industrial, 72% imp, HSG C (1S)
0.500	30	Woods, Good, HSG A (1S)
1.700	55	Woods, Good, HSG B (1S)
35.500	70	Woods, Good, HSG C (1S)
38.100	69	TOTAL AREA

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#### **Soil Listing (selected nodes)**

Area (acres)	Soil Group	Subcatchment Numbers
0.500	HSG A	1S
1.700	HSG B	1S
35.900	HSG C	1S
0.000	HSG D	
0.000	Other	
38.100		TOTAL
		AREA

#### 01-21-14 Huff Storage-PRE

*Type III 24-hr 10-yr Rainfall=4.30"*Printed 1/25/2014

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Time span=5.00-24.00 hrs, dt=0.05 hrs, 381 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment 1S: sub 1** 

Runoff Area=38.100 ac 0.76% Impervious Runoff Depth>1.44" Flow Length=1,500' Tc=58.7 min CN=69 Runoff=24.75 cfs 4.579 af

Total Runoff Area = 38.100 ac Runoff Volume = 4.579 af Average Runoff Depth = 1.44" 99.24% Pervious = 37.812 ac 0.76% Impervious = 0.288 ac

#### 01-21-14 Huff Storage-PRE

*Type III 24-hr 50-yr Rainfall=5.65"*Printed 1/25/2014

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Time span=5.00-24.00 hrs, dt=0.05 hrs, 381 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment 1S: sub 1** 

Runoff Area=38.100 ac 0.76% Impervious Runoff Depth>2.41" Flow Length=1,500' Tc=58.7 min CN=69 Runoff=42.69 cfs 7.644 af

Total Runoff Area = 38.100 ac Runoff Volume = 7.644 af Average Runoff Depth = 2.41" 99.24% Pervious = 37.812 ac 0.76% Impervious = 0.288 ac Prepared by RJB Engineering
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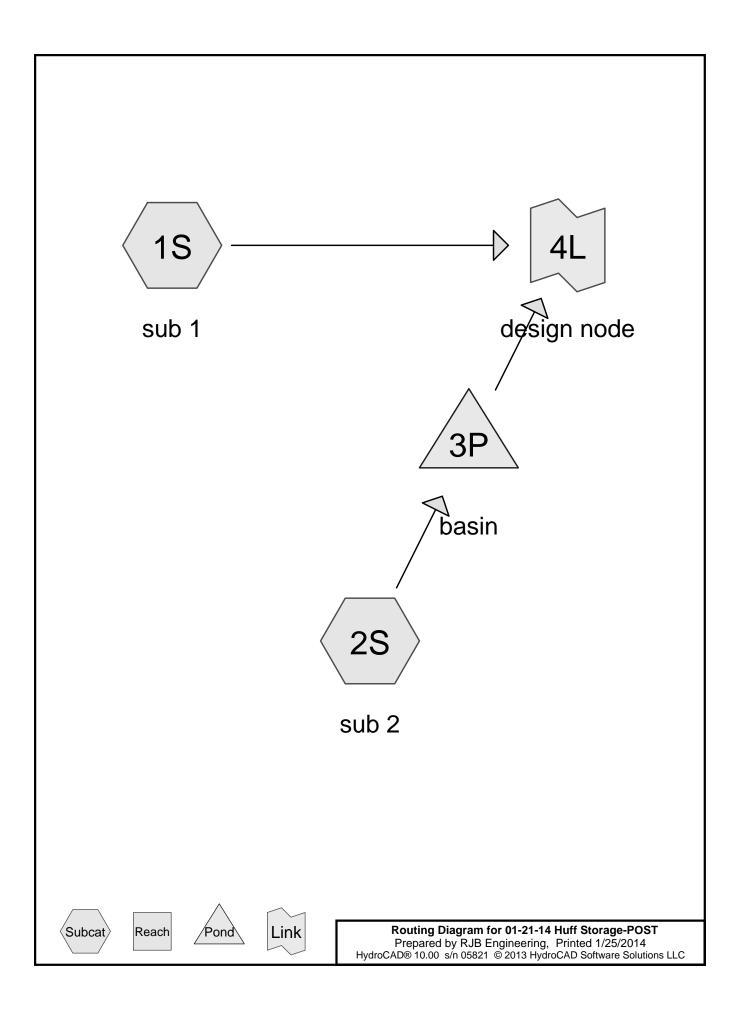
#### **Summary for Subcatchment 1S: sub 1**

Runoff 42.69 cfs @ 12.83 hrs, Volume= 7.644 af, Depth> 2.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=5.65"

Area	(ac) C	N Des	cription			
0.:	500	30 Wo	ods, Good,	HSG A		
1.	700	55 Wo	ods, Good,	HSG B		
35.:	35.500 70 Woods, Good, HSG C					
0.4	400	91 Urb	an industria	ıl, 72% imp	, HSG C	
38.	100	59 Wei	ghted Aver	age		
37.	812	99.2	4% Pervio	us Area		
0.2	288	0.76	5% Impervi	ous Area		
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
29.3	100	0.0100	0.06		Sheet Flow,	
					Woods: Light underbrush $n=0.400 P2=3.00$ "	
26.7	800	0.0100	0.50		Shallow Concentrated Flow,	
					Woodland $Kv = 5.0 \text{ fps}$	
2.7	600	0.0150	3.64	14.56	Channel Flow,	
					Area= $4.0 \text{ sf Perim} = 4.0' \text{ r} = 1.00'$	
					n= 0.050 Mountain streams w/large boulders	
58.7	1,500	Total				

Post-development computations



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#### **Area Listing (all nodes)**

Area	CN	Description
(acres)		(subcatchment-numbers)
0.114	74	>75% Grass cover, Good, HSG C (2S)
0.079	96	Gravel surface, HSG C (2S)
0.210	98	Paved parking, HSG C (2S)
0.275	98	Roofs, HSG C (2S)
0.500	30	Woods, Good, HSG A (1S)
1.700	55	Woods, Good, HSG B (1S)
35.222	70	Woods, Good, HSG C (1S)
38.100	69	TOTAL AREA

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#### Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.500	HSG A	1S
1.700	HSG B	1S
35.900	HSG C	1S, 2S
0.000	HSG D	
0.000	Other	
38.100		TOTAL
		AREA

#### 01-21-14 Huff Storage-POST

Type III 24-hr 10-yr Rainfall=4.30" Printed 1/25/2014

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Time span=5.00-24.00 hrs, dt=0.05 hrs, 381 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: sub 1 Runoff Area=37.422 ac 0.00% Impervious Runoff Depth>1.44"

Flow Length=1,500' Tc=58.7 min CN=69 Runoff=24.31 cfs 4.498 af

Subcatchment 2S: sub 2 Runoff Area=0.678 ac 71.53% Impervious Runoff Depth>3.60"

Tc=6.0 min CN=94 Runoff=2.64 cfs 0.203 af

Pond 3P: basin Peak Elev=197.79' Storage=1,432 cf Inflow=2.64 cfs 0.203 af

Outflow=1.77 cfs 0.201 af

Link 4L: design node Inflow=24.74 cfs 4.698 af

Primary=24.74 cfs 4.698 af

Total Runoff Area = 38.100 ac Runoff Volume = 4.701 af Average Runoff Depth = 1.48" 98.73% Pervious = 37.615 ac 1.27% Impervious = 0.485 ac

#### 01-21-14 Huff Storage-POST

Type III 24-hr 50-yr Rainfall=5.65" Printed 1/25/2014

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Time span=5.00-24.00 hrs, dt=0.05 hrs, 381 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: sub 1 Runoff Area=37.422 ac 0.00% Impervious Runoff Depth>2.41"

Flow Length=1,500' Tc=58.7 min CN=69 Runoff=41.93 cfs 7.508 af

Subcatchment 2S: sub 2 Runoff Area=0.678 ac 71.53% Impervious Runoff Depth>4.91"

Tc=6.0 min CN=94 Runoff=3.55 cfs 0.277 af

Pond 3P: basin

Peak Elev=197.97' Storage=1,842 cf Inflow=3.55 cfs 0.277 af

Outflow=2.31 cfs 0.274 af

Link 4L: design node

Inflow=42.52 cfs 7.782 af
Primary=42.52 cfs 7.782 af

Total Runoff Area = 38.100 ac Runoff Volume = 7.785 af Average Runoff Depth = 2.45" 98.73% Pervious = 37.615 ac 1.27% Impervious = 0.485 ac

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#### **Summary for Subcatchment 1S: sub 1**

Runoff = 41.93 cfs @ 12.83 hrs, Volume= 7.508 af, Depth> 2.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=5.65"

Area (	(ac) C	N Des	cription		
0.3	500	30 Woo	ods, Good,	HSG A	
1.	700	55 Woo	ods, Good,	HSG B	
35.2	222 ′	70 Wo	ods, Good,	HSG C	
37.4	422	59 Wei	ghted Aver	age	
37.4	422	100	.00% Pervi	ous Area	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
29.3	100	0.0100	0.06		Sheet Flow,
					Woods: Light underbrush $n=0.400 P2=3.00$ "
26.7	800	0.0100	0.50		Shallow Concentrated Flow,
					Woodland $Kv = 5.0 \text{ fps}$
2.7	600	0.0150	3.64	14.56	Channel Flow,
					Area= $4.0 \text{ sf Perim} = 4.0' \text{ r} = 1.00'$
					n= 0.050 Mountain streams w/large boulders
58.7	1,500	Total			

#### **Summary for Subcatchment 2S: sub 2**

Runoff = 3.55 cfs @ 12.09 hrs, Volume= 0.277 af, Depth> 4.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 50-yr Rainfall=5.65"

Area (ac	c) CN	Descripti	ion		
0.27	5 98	Roofs, H	ISG C		
0.21	0 98	Paved pa	arking, I	HSG C	
0.07	9 96	Gravel su	urface, l	HSG C	
0.11	4 74	>75% Gr	rass cov	er, Good,	, HSG C
0.67	8 94	Weighted	d Avera	ige	
0.19	3	28.47% I	Perviou	s Area	
0.48	5	71.53% I	Impervi	ous Area	
		G1 T7 1		<b>a</b>	
	ength		•	Capacity	Description
(min)	(feet)	(ft/ft) (ft	t/sec)	(cfs)	
6.0					Direct Entry,

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#### **Summary for Pond 3P: basin**

[82] Warning: Early inflow requires earlier time span

Inflow Area = 0.678 ac, 71.53% Impervious, Inflow Depth > 4.91" for 50-yr event

Inflow = 3.55 cfs @ 12.09 hrs, Volume= 0.277 af

Outflow = 2.31 cfs @ 12.19 hrs, Volume= 0.274 af, Atten= 35%, Lag= 5.9 min

Primary = 2.31 cfs @ 12.19 hrs, Volume= 0.274 af

Routing by Stor-Ind method, Time Span= 5.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 197.97' @ 12.19 hrs Surf.Area= 2,288 sf Storage= 1,842 cf

Plug-Flow detention time= 27.5 min calculated for 0.274 af (99% of inflow)

Center-of-Mass det. time= 19.6 min (793.6 - 774.0)

Volume	Inve	rt Avail.Sto	orage Storage	ge Description
#1	197.0	0' 4,6	20 cf Custor	m Stage Data (Prismatic)Listed below (Recalc)
Elevation	on S	Surf.Area	Inc.Store	Cum.Store
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)
197.0	00	1,500	0	0
199.0	00	3,120	4,620	4,620
Device	Routing	Invert	Outlet Device	ces
#1	Primary	197.00'	12.0" Roun	<b>nd Culvert</b> L= 30.0' CPP, mitered to conform to fill, Ke= 0.700
			Inlet / Outlet	et Invert= 197.00' / 196.70' S= 0.0100 '/' Cc= 0.900
			n=0.011 Co	oncrete pipe, straight & clean, Flow Area= 0.79 sf
#2	Primary	198.00'	4.0' long x	6.0' breadth Broad-Crested Rectangular Weir
			Head (feet)	0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00
			3.50 4.00 4	4.50 5.00 5.50
			Coef. (Engli	ish) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66
			2.66 2.67 2	2.69 2.72 2.76 2.83

**Primary OutFlow** Max=2.30 cfs @ 12.19 hrs HW=197.97' (Free Discharge)

**1=Culvert** (Inlet Controls 2.30 cfs @ 2.96 fps)

**—2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

#### Summary for Link 4L: design node

Inflow Area = 38.100 ac, 1.27% Impervious, Inflow Depth > 2.45" for 50-yr event

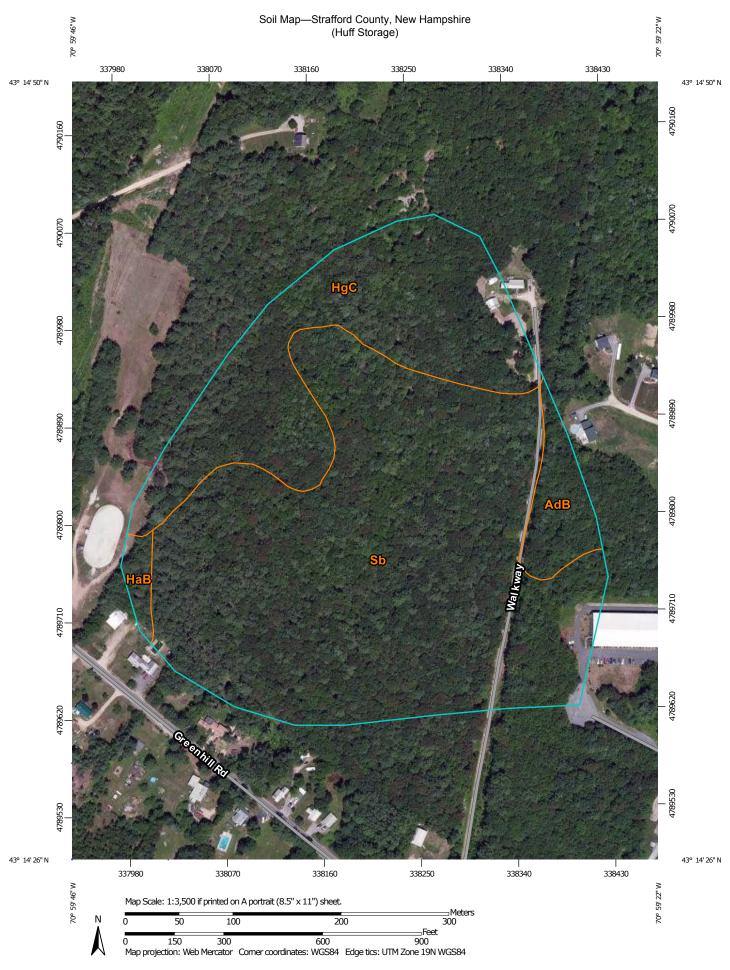
Inflow = 42.52 cfs @ 12.83 hrs, Volume= 7.782 af

Primary = 42.52 cfs @ 12.83 hrs, Volume= 7.782 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-24.00 hrs, dt= 0.05 hrs

Off-site Drainage Area Map





## MAP LEGEND

#### Special Line Features Very Stony Spot Stony Spot Spoil Area Wet Spot Other W 8 Soil Map Unit Polygons Area of Interest (AOI) Soil Map Unit Points Soil Map Unit Lines Area of Interest (AOI) Soils

## Water Features Special Point Features



Streams and Canals



Interstate Highways

Rails

ŧ

**Transportation** 





Landfill

Major Roads Local Roads

US Routes





Topographic Map

Background









Perennial Water Rock Outcrop

Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

# MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting Enlargement of maps beyond the scale of mapping can cause soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements

http://websoilsurvey.nrcs.usda.gov Source of Map: Natural Resources Conservation Service Coordinate System: Web Mercator (EPSG:3857) Web Soil Survey URL:

Albers equal-area conic projection, should be used if more accurate distance and area. A projection that preserves area, such as the Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Strafford County, New Hampshire Version 13, Dec 31, 2013 Survey Area Data: Soil Survey Area:

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Jun 20, 2010—May 1,

imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background

#### **Map Unit Legend**

Strafford County, New Hampshire (NH017)							
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI				
AdB	Acton very stony fine sandy loam, 0 to 8 percent slopes	1.7	4.6%				
НаВ	Hinckley loamy sand, 3 to 8 percent slopes	0.5	1.3%				
HgC	Hollis-Gloucester very rocky fine sandy loams, 8 to 15 percent slopes	10.3	27.1%				
Sb	Saugatuck loamy sand	25.6	67.0%				
Totals for Area of Interest		38.1	100.0%				

Drainage Area Plans

